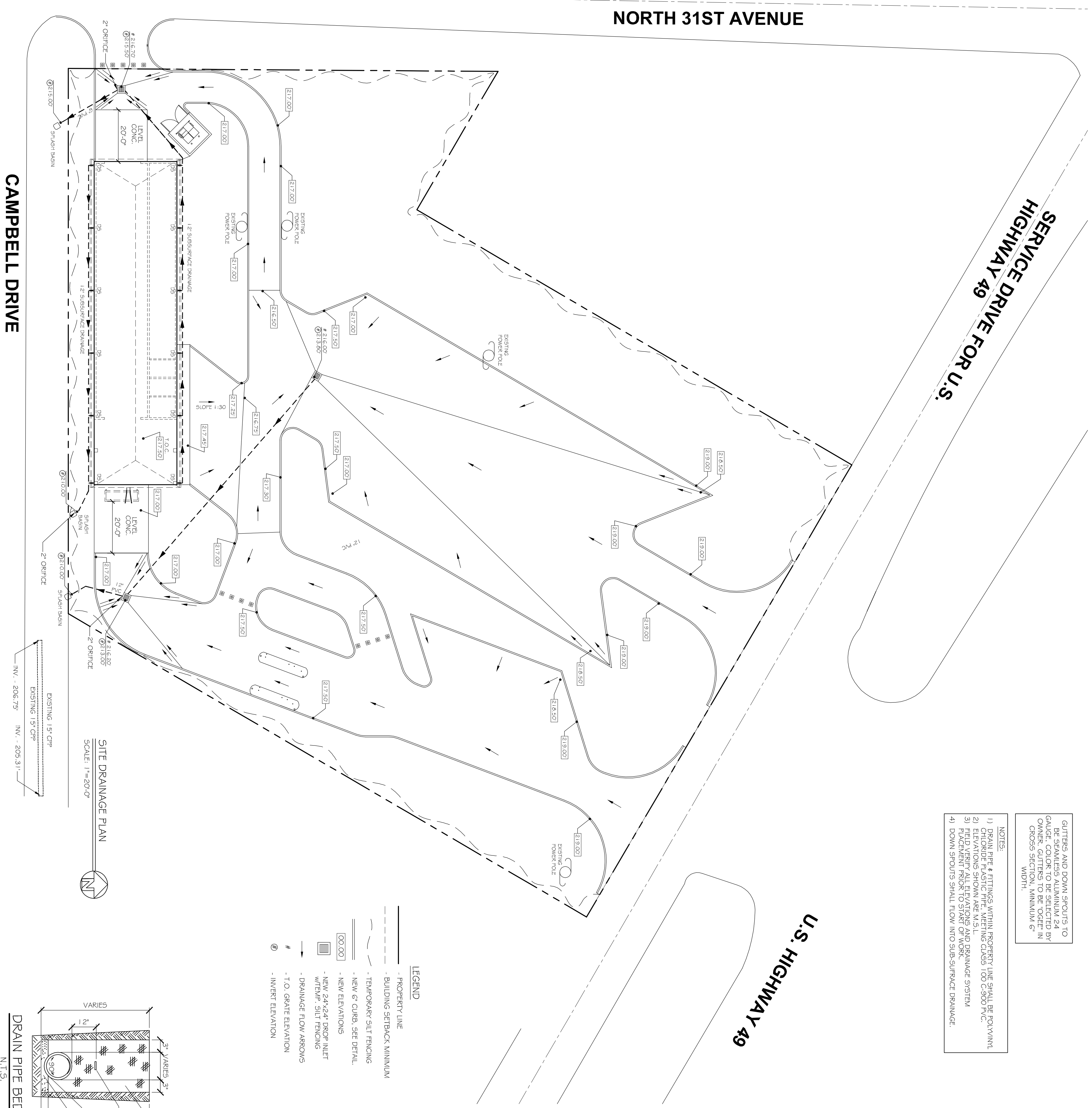


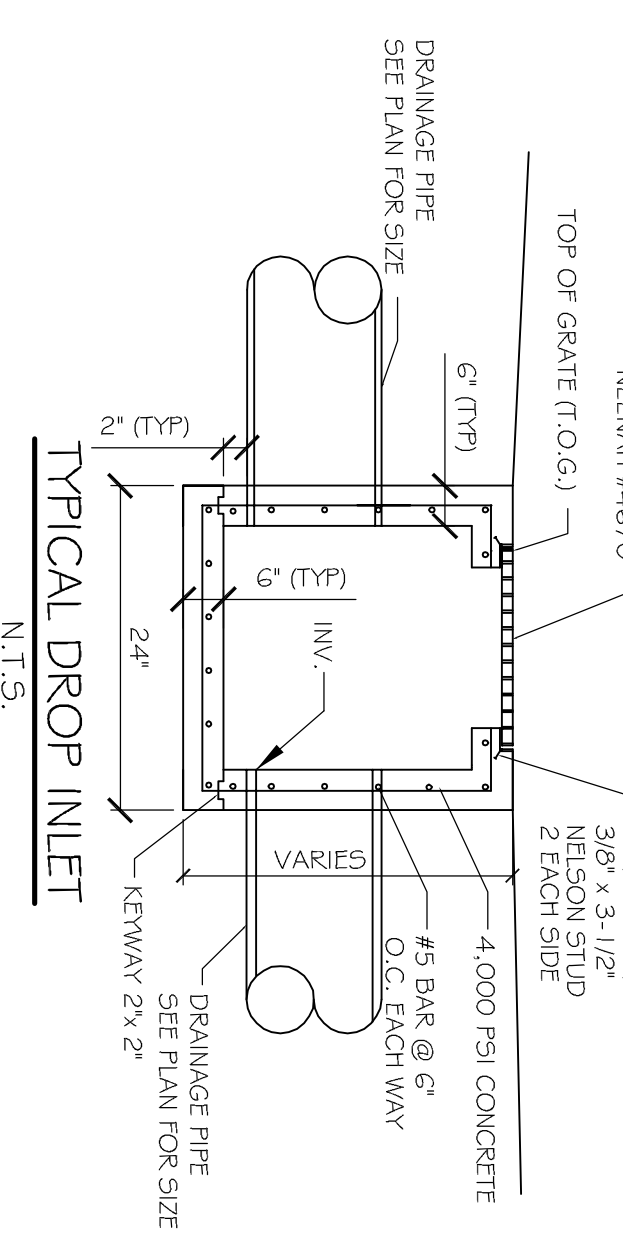
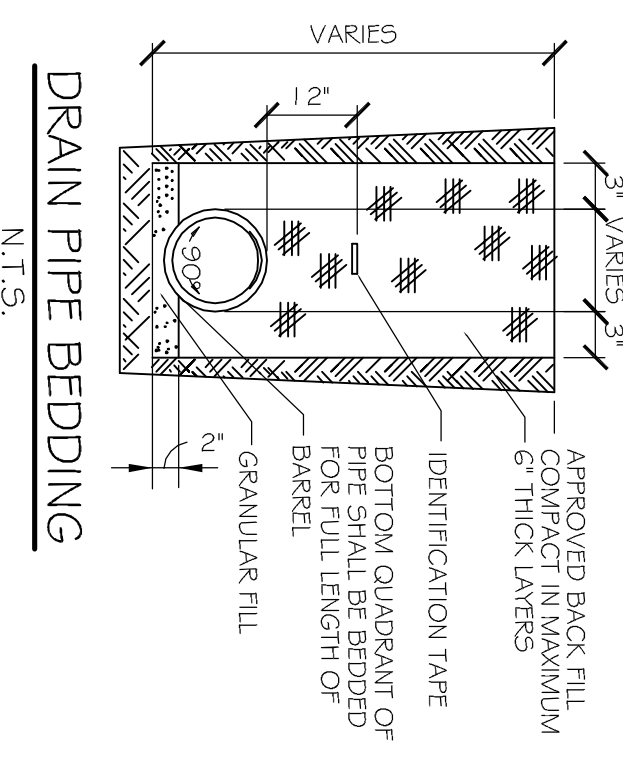
GUTTERS AND DOWN SPOUTS TO BE SPANIES. MINIMUM 24 GAUGE. COLOR TO BE SELECTED BY OWNER. GUTTERS TO BE OCCUR IN CROSS SECTION, MINIMUM 6" WIDTH.

- NOTES:
- 1) DRAIN PIPE & FITTINGS WITHIN PROPERTY LINE SHALL BE POLYVINYL CHLORIDE PLASTIC PIPE, MEETING CLASS 100 C-900 PVC.
 - 2) ELEVATIONS SHOWN ARE M.S.L. AND DRAINAGE SYSTEM.
 - 3) FLOW DIRECTION TO BE SHOWN ON WORK.
 - 4) DOWN SPOUTS SHALL FLOW INTO SUB-SURFACE DRAINAGE.



SITE DRAINAGE PLAN
SCALE: 1"=20'-0"
N

- LEGEND
- PROPERTY LINE
 - BUILDING SETBACK MINIMUM
 - TEMPORARY SILT FENCING
 - NEW 6" CURB, SEE DETAIL
 - NEW ELEVATIONS
 - NEW 24"x24" DROP INLET W/TEMP. SILT FENCING
 - DRAINAGE FLOW ARROWS
 - T.O. GRATE ELEVATION
 - INVERT ELEVATION



PROJECT:		Rainforest Carwash	
Formula used:		STORMWATER RUNOFF CALCULATIONS	
[1] RATIONAL METHOD - Q=Ai			
where:	Q= Peak discharge of watershed in cubic feet per second (cfs) due to maximum storm assumed.		
A= Area of watershed in acres.			
c= Coefficient of runoff [2]			
i= Intensity of rainfall in inches per hour based on concentration time [3]			
[4] TC= $(1.48 \times 10^{-5} \times L^{0.77})^{0.77}$			
TC= Time of concentration - time required for rain falling at most remote point to reach outlet.			
c= Site runoff coefficient based on conditions shown.			
s= Percent slope of watershed flow.			
PRIOR DEVELOPMENT			
25 Year Frequency			
Q _p = Aci	Watersight Surfaces	1779	sqft = 0.027 Acres
	General Surface c(1) = 0.9		
	Green Space c(2) = 0.25	69570	sqft = 1.393 Acres
	Summary c = 0.15	61948	sqft = 1.420 Acres
Duration (D) = Time of concentration (TC)			
where:	L = 245	run-off length ft	Elev diff = 5
	c = 0.16	run-off coef	
	S = 2.0408	percent slope	
	TC = D = 43.49	minutes	
where:	Intensity	1 = 7.66	In/hr
POST DEVELOPMENT			
25 Year Frequency			
Q _p = Aci	Watersight Surfaces	31948	sqft = 0.733 Acres
	General Surface c(1) = 0.9		
	Green Space c(2) = 0.25	29001	sqft = 0.686 Acres
	Summary c = 0.54	61948	sqft = 1.420 Acres
Duration (D) = Time of concentration (TC)			
where:	L = 160	run-off length ft	Elev diff = 3
	c = 0.54	run-off coef	
	S = 2.0000	percent slope	
	TC = D = 13.24	minutes	
where:	Intensity	1 = 7.66	In/hr
DETENTION REQUIREMENTS			
Detention required Q _{2-Q}	4.05 cfs		
ONE HOUR DETENTION	14608.9 cuft	195	feet
DETENTION DIMENSIONS	DEPTH	15	feet
	DEPTH	0.87	feet
DISCHARGE END AREA REQUIREMENTS			
10 Year Frequency			
[9] A= $\frac{Q}{1.486 \times S^{0.584}}$			
where:	A= Discharge Area required		
g= Acceleration of gravity			
c= Discharge coefficient			
h= Hydraulic head			
Q= Flow volume from run-off			
Pipe Sizing Site Drainage	0.179 cfs	1 = 2.50	feet
	c = 0.62	coefficient	
	g = 32.16	ft/sec/sec	
REQUIRED CONDUIT = 2.00 inch inside diameter			
References: The Civil Engineering Handbook, 1995, Ed. # 31, pp. 1036			
1. Chen, W. F. The Civil Engineering Handbook, 1995, Ed. # 31, pp. 1802			
2. Chow, V. T. Open Channel Hydraulics, 1959, Ed. # 1, pp. 18-21			
3. Savitsky, E. V. Data Book for Civil Engineers, Vol. 1, 1960, pp. B, pp. 18-21			
4. Chen, W. F. The Civil Engineering Handbook, 1995, Ed. # 31, 2 Region, Edition (4-0-013)			
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RAINFOREST CARWASH #2

6485 U.S. HWY. 49

HATTIESBURG, MISSISSIPPI

JOB No: 2212 DATE: 08-12-2014

DRAWN BY: CKD CHECKED BY: CKD



DAMMON

ENGINEERING, INC.

Architects & Engineers

CHIEF ENGINEER: BRIAN MISTICH, P.E.
554 OLD SPANISH TRAIL
SLIDELL, LA 70456

dammoneengineering.com
dammoneng@bellsouth.net
PHONE: 985-649-5832
FAX: 985-641-5990

SITE DRAINAGE PLAN

SHEET No: 00 OF 00

C4