UNITED STRUCTURES OF AMERICA

U.S.A.

DRAWING PACKAGE

GENERAL NOTES

1.1 Fabrication shall be in accordance with U.S.A.'s standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1". U.S.A.'s manufacturing procedures have been certified by:

Reference	Certification numbers
S.B.C.C.I.	#9011
I.C.B.O.	FA-327
MIAMI-DADE COUNTY	#1058
Houston	U.S.A. #135
Indiana (For Beam and Column)	299512
Indiana (For Rigid Frame)	299513
Indiana (For Straight Column)	299510
A.I.S.C. Certification For Metal Buildings	
Member MBMA	

1.2 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification TT-P-664 and is U.S.A.'s Red Oxide color. This paint is not intended for long term exposure to the elements. U.S.A. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or storage. U.S.A. shall not be responsible for any field applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 9th Edition)

1.3 GALVANIZED OR SPECIAL COATINGS:
See Contract Documents and ref. note on the top right hand of this page.

1.4 ALL BOLTS ARE 1/2"Ø x Ø'-1" A307 EXCEPT:
a) Eave strut connection - 1/2"Ø x Ø'-1 1/4" A307
b) Endwall rafter splice - 5/8"Ø x Ø'-1 3/4" A325-N
c) Endwall column to rafter connection - 1/2"Ø x Ø'-1 1/4" A325-N
d) Main frame connections - SEE CROSS SECTION

NOTE: Washers are not supplied unless noted otherwise on drawing

1.5 MATERIALS

MATERIALS	ASTM DESIGNATION	MINIMUM YIELD (U.N.)
Wide Flange Shapes	A572 or A992	FY = 50 KSI
Other Rolled Mill Shapes	A36 or A572	FY = 36 KSI or 50 KSI
Structural Steel Plate (Webs/Connections)	A1011 or A572	FY = 55 KSI
Structural Steel Plate (Flanges)	A529 or A572	FY = 55 KSI
Cold Formed Light Gage Shapes	A1011SS / HSLAS CLASS 1	FY = 55 KSI
Cable Bracing	A475-78	Extra High Strength
Rod Bracing	A36	FY = 36 KSI
Roof and Wall Sheeting	A792	FY = 50 , 80 KSI
Machine Bolts	A307	FY = 36 KSI
High Strength Bolts (1/2"Ø to 1"Ø)	A325-N	FU = 120 KSI
High Strength Bolts (1 1/8"Ø to 1 1/2"Ø)	A325-N	FU = 105 KSI
Anchor Bolts (if supplied)	F1554	FY = 36 KSI
Pipe	A53, GRADE B	FY = 35 KSI
H.S.S. TUBE	A500, GRADE C	FY = 46 KSI

1.6 HIGH-STRENGTH BOLT TIGHTENING REQUIREMENTS

High Strength Bolts are ASTM A325-N, unless otherwise noted.

All high strength bolts must be fully pre-tensioned unless otherwise specifically noted for particular connection details.

The recommended method for pre-tensioning high strength bolts without washers is the turn-of-the-nut method.

Procedures for the turn-of-the-nut method, including pre-installation verification, shall be per AISC/RCSC Specifications. See AISC 13th Ed., Chapter 16.2, Sections 7 and 8.2.1

Note: The Pre-installation verification procedure requires the use of a "tension calibrator", which is a hydraulic tension indicating device, such as a Skidmore Testing Device.

All ASTM A325 bolts are provided with lubricated nuts. The erector should employ proper job site handling and storage procedures to protect fastener assemblies against excessive weathering and exposure to dust, dirt, moisture, heat, etc. Depending on job site handling and the severity of job site conditions, it may be necessary to re-lubricate the nuts. The necessity for reapplying lubricant maybe determined by pre-installation verification as referenced above. For additional information on job site storage and handling, see AISC 13th Ed., Chapter 16.2, Section 2.2

1.7 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION:
INSIDE- Under roof panels at eave
OUTSIDE - Between endwall panels and rake trim
- Under continuous ridge vent skirts

1.8 ERECTION & TEMPORARY SUPPORTS:
The erector is responsible to determine, furnish and install all required temporary supports and bracing. These temporary supports and bracing shall be sufficient to secure the building and / or bare structural steel against loads likely to be encountered during erection including those due to wind load and erection operations. (Section 7.10.3 AISC Code of Standard Practice)

1.9 ERECTION AND UNLOADING NOT BY U.S.A.

1.10 SHORTAGES
Any claims or shortages by buyer must be made to U.S.A. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless U.S.A. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of U.S.A.

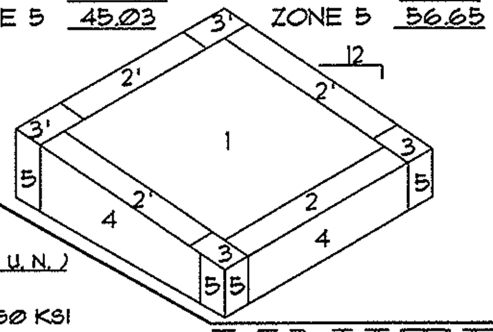
1.12 ERECTOR NOTE
Erect steel with piece marks corresponding to location of piece marks on erection drawings. Erect and plumb bays progressively to insure overall width and length is maintained.

1.13 ACCESSORIES
Accessories are not wind rated.

1.14 ROOF SYSTEMS
The performance of roof systems supplied by USA is dependent on correct and proper erection and roof system installation by qualified roofing installers. USA is not responsible for non-performance due to improper or defective installation. USA does not certify erectors or roofing installers.

(BLDG.B)
BASIC WIND PRESSURE: 29.05 psf
INTERNAL PRESSURE COEFFICIENT: 0.55
EDGE STRIP DIMENSION: 3 FT.
COMPONENT AND CLADDING PRESSURE:

ZONE	PRESSURE (psf)	SUCTION (psf)
ZONE 1	24.69	47.94
ZONE 2	24.69	53.75
ZONE 2'	24.69	62.46
ZONE 3	24.69	68.27
ZONE 3'	24.69	91.51
ZONE 4	45.03	47.94
ZONE 5	45.03	56.65



QUALIFICATION OF HOT-DIP GALVANIZING PROCESS USED ON PRIMARY AND SECONDARY STEEL

Zinc coatings produced by the hot-dip galvanizing process are excellent corrosion-protection systems. When the coating becomes very thick or dull grey, the coating may not be applicable for architectural purposes.

Differences in the luster and color of galvanized coatings and the presence or absence of spangle has no affect on the coating performance. The well-known spangle effect found on galvanizing is simply a factor of primary crystallization.

Additionally, handling techniques for galvanizing may require the use of chain slings, wire or other holding devices to immerse material into the galvanizing kettle. These handling devices may leave a mark on the galvanized item.

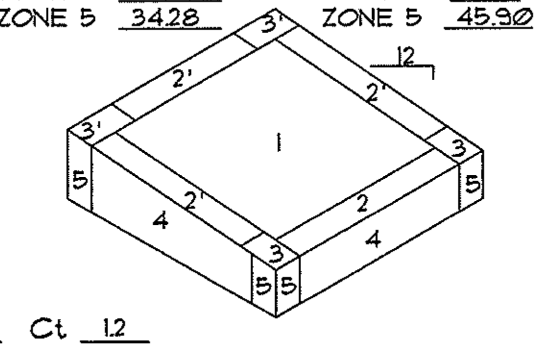
The galvanizing process is for corrosion protection. Inconsistencies in finish and handling marks are not cause for rejection of the product.

ALUMINUM-ZINC COATING

The Aluminum-Zinc coating is subject to variances in spangle from coil to coil which may result in noticeable shade variation in installed panels. The Aluminum-Zinc coating is also subject to differential weathering after panel installation. Panels may appear to be different shades due to the weathering characteristic. If a consistent appearance is required, USA recommends that pre-painted panels be used in lieu of Aluminum-Zinc. Shade variation in panels manufactured from Aluminum-Zinc coated material do not diminish the structural integrity of the product. These shade variations should be anticipated and are not a cause for rejection.

(BLDG. A)
BASIC WIND PRESSURE: 29.05 psf
INTERNAL PRESSURE COEFFICIENT: 0.18
EDGE STRIP DIMENSION: 7.367
COMPONENT AND CLADDING PRESSURE:

ZONE	PRESSURE (psf)	SUCTION (psf)
ZONE 1	13.94	34.28
ZONE 2	13.94	57.52
ZONE 2'	-	-
ZONE 3	13.94	86.57
ZONE 3'	-	-
ZONE 4	34.28	37.19
ZONE 5	34.28	45.90



JOB # 36506A
BUYER : CORSO FABRICATORS, INC.
END USE CUSTOMER : COVINGTON PICK UP STATION
BUYER ORDER # :
JOBSITE : COVINGTON, LA COUNTY : ST. TAMMANY
BASIC BUILDING : SSRF 73'-8 X 83'-4 X VARIES ½:12 (BLDG "A")
SSRF 15'-Ø X 38'-Ø X 24'-Ø X 2:12 (LS) (BLDG "B")

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

CODE

(Code of Latest Issue to be Used Unless Otherwise Noted.)

☐ USA Standard (MBMA) — Code Edition: 2006
☐ SBC ■ IBC
☐ UBC ☐ BOCA ☐ ASCE
EXPOSURE ■ B ☐ C ☐ D
☐ OTHER ________

LOADS

■ LL 20 PSF ☐ Tributary Area Reduction
■ Collateral Load: 2.0 psf
■ Snow Load: Pg 5.0 psf Pf 5.0 psf Ce 9.0 Is 1.0 Ct 1.2
■ Wind Load 130 mph Iw 1.0 Exposure B
Internal Pressure Coefficient ±0.18(BLDG.A) ±0.55(BLDG.B) Design Wind Pressure 29.05 psf
■ Seismic Data: Use Group I Sds 12.7 Sd1 8.5 Site Class D
Ie 1.0 Ss 11.9 S1 5.3
Seismic Design Category B
Seismic-force-resisting System: Transverse Load: R= 3½ Cs= 0.036
Longitudinal Load: R= 3¼ Cs= 0.039
Design Base Shear V= 2.5 kips Equivalent Lateral Force Procedure
☐ Crane Load: Capacity ____ tons Type ____ Class ____
(See Cross Sections and/or calculations for loading)
■ Mezzanine Load: DL 20.0 psf LL 40.0 psf CL ____ psf
■ Other Loads: (1) 1,050 # WALL LOUVER FAN, CAT WALK @ LN.1
FROM LINES C - A IS NOT SUPPORTED BY USA'S STEEL.
MEZZANINE PLATFORM IS NOT SUPPORTED BY USA'S STEEL.

ROOF	■ 26GA ■ USA "PBR" ☐ 24GA ☐ ☐ 24GA USA "Guardian I" SSR ☐ 24GA USA "Guardian II" SSR ☐ ____ Special Requirements N/A	☐ White ■ Aluminum Zinc ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000 Color ____	☐ UL90-Const* ____ ☐ Northern Roof ☐ Wide Mastic Clip: ☐ Fixed ☐ Floating ☐ Articulating ☐ Low ☐ High	Fasteners: ☐ #14 x 1" STS Member & Stitch ☐ #12 x 1¼" SDS Member Only W/ STS Stitch ■ #12 x 1¼" SDS Member with Laptek Stitch ☐ Long Life Fasteners ☐ Stainless Steel Fasteners Thermal Blocks: ☐ By USA ☐ By Others
WALL	■ 26GA ☐ USA "PBA" ☐ ____ ☐ 24GA ■ USA "PBR" Color ** Special Requirements ____	☐ Kynar Unicote 3000 ■ Silicone Poly Unicote 2000 ☐ Long Life Fasteners ☐ Stainless Steel Fasteners		Fasteners: ☐ #14 x 1" STS Member & Stitch ☐ #12 x 1¼" SDS Member Only W/ STS Stitch ■ #12 x 1¼" SDS Member with Laptek Stitch
TRIM	■ Standard W/Gutters & Downspout ☐ Simple Eave Trim ■ Sculptured Eave Trim ☐ Base Trim ☐ MF01 ☐ # 201 ☐ DP01 ☐ Base Closure Gutter ** Downspout ** Eave ** Rake ** Corner ** Jamb & Head ** Base Trim ____ Special Requirements 24ga. MATERIAL ☐ Silicone Poly Unicote 2000 ■ Kynar Unicote 3000			
FASCIA PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ ☐ 24GA Special Requirements ____	Trim Color ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000		
SOFFIT PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ ☐ 24GA Special Requirements ____	Trim Color ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000		
BACK PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ ☐ 24GA Special Requirements ____	Trim Color ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000		
PARTITION PANEL	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ ☐ 24GA Special Requirements ____	Trim Color ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000		
LINER	☐ 26GA USA ☐ "PBA" ☐ "PBR" ☐ ____ Color ____ ☐ 24GA Special Requirements ____	Trim Color ____ ☐ Silicone Poly Unicote 2000 ☐ Kynar Unicote 3000		

ALLOWABLE DEFLECTIONS:
U.S.A./MBMA STANDARDS

WIND DEFLECTION BASED ON:
50 YEAR MEAN RECURRENCE WIND SPEED

** NOTE! Customer To Provide Colors Denoted With **(If Any) On Returned Dwg's.

ALL FLAT FACED PANELS HAVE A TENDENCY TO OIL CAN. USING HEAVIER GUAGES, NARROWER WIDTHS, EMBOSSING OR STIFFENER BEADS (WHEN AVAILABLE) CAN REDUCE OIL CANNING.
UNDER NO CIRCUMSTANCES IS OIL CANNING CAUSE FOR REJECTION

34	6/18/07
33	4/11/07
32	8/2/06
31	12/12/05
30	03/18/05

BUYER/END USE CUSTOMER RESPONSIBILITES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release U.S.A. to fabricate upon receiving such.

2.2 United Structures of America's (hereafter referred to as U.S.A.) standard specifications apply unless stipulated otherwise in the Contract Documents. U.S.A.'s design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between U.S.A.'s structural steel plans and plans for other trades, U.S.A.'s plans shall govern. (Section. 3 AISC Code of Standard Practice)

2.4 Approval of U.S.A. drawings and calculations indicates that U.S.A. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the U.S.A.'s design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed U.S.A.'s Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by U.S.A. and U.S.A.'s steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or U.S.A.'s assumptions will govern (Section 4 and Commentary, AISC Code of Standard Practice)

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that U.S.A.'s plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that U.S.A. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by U.S.A.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with U.S.A.'s "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practice.)

2.9 United Structures of America is responsible for the design of anchor bolts to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, United Structures of America does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Section IV MBMA Low Rise Building Systems Manual)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to U.S.A. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (Section 7 AISC Code of Standard Practice, 9th Edition)

2.11 FIELD MODIFICATIONS - WARNING Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 9th Edition)

The strength, stability, and safety of this building may be affected by cutting, drilling or removing any components of the building. No such modifications may be made without the written approval of United Structures of America, Inc.

Collateral loads or other components provided by others may be supported on this building in the designated locations if specifically included in the design loads. Unless the connections for such components have been designed and detailed by U.S.A. the customer is responsible to ensure appropriate connection details are utilized.

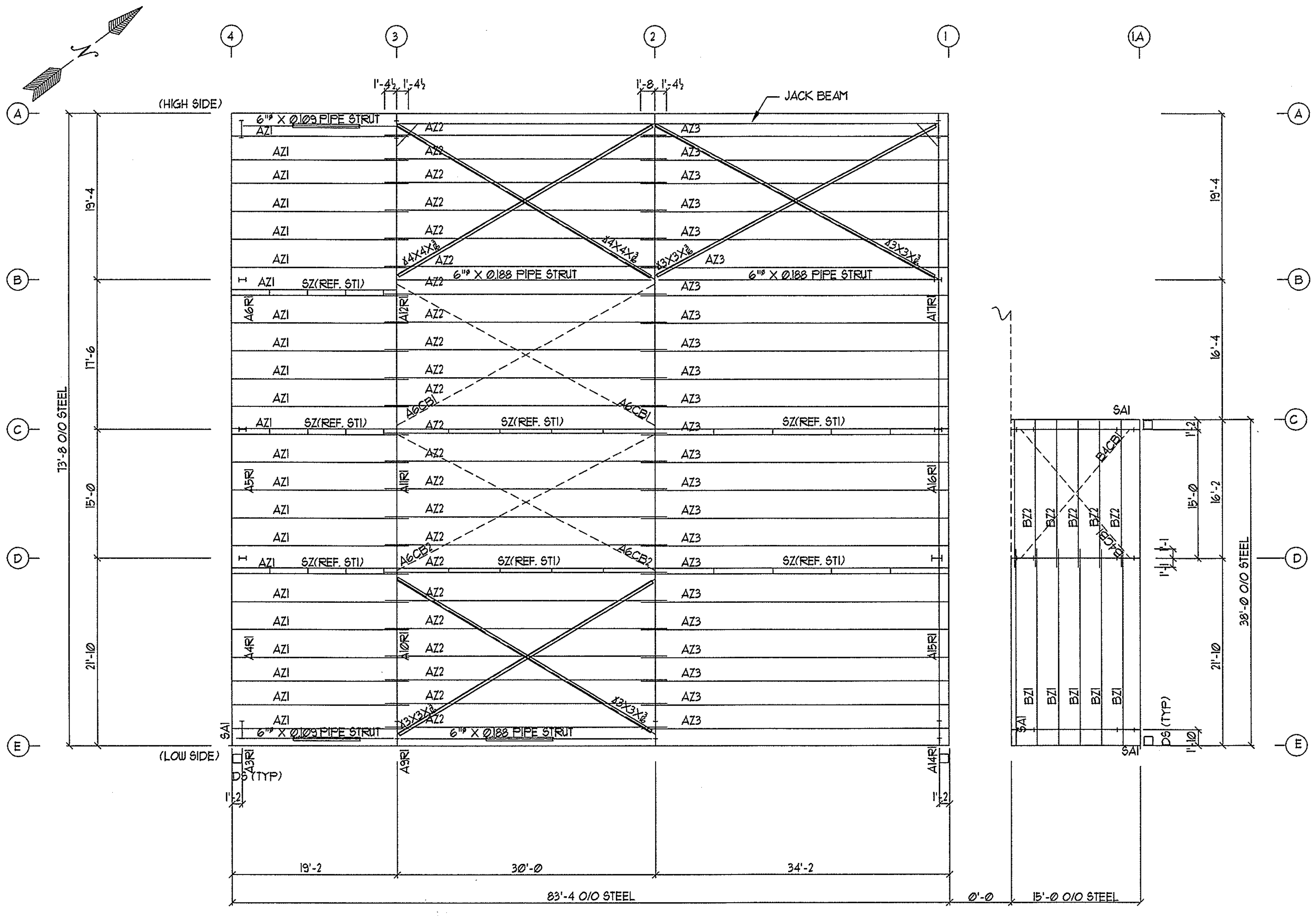
2.12 WARNING In no case should Aluminized Zinc steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Aluminized Zinc alloy coating when they are in contact with Aluminized Zinc steel panels. Even run-off from copper flashing, wiring, or tubing onto Aluminized Zinc should be avoided.

2.13 SAFETY COMMITMENT UNITED STRUCTURES of AMERICA has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of U.S.A.
It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help ensure worker safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended. Contractors/Erectors should be aware of OSHA regulations for Steel Erection. The following OSHA Regulations (Standards 29 CFR) are applicable to Steel Erection: OSHA Steel Erection Regulations, Part Number 1926, Subpart "R"

2.14 Manufacturer is not responsible for bodily injuries or material damages during unloading, handling, storage, or erection.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

P1	PERMIT	5/21/08
Job Number : 36506A		
SHEET L1 of 1		



ROOF FRAMING PLAN

**NOTE!
CUSTOMER TO PROVIDE COLORS ON RETURNED DRAWINGS

ERECTOR NOTE
Erect steel with piece marks corresponding to location of piece marks on construction drawings. Erect and plumb bays progressively to insure overall width and length is maintained. Construction drawings are not intended to specify any particular method or sequence of erection to be followed. Erector is and shall remain solely responsible for the safety and appropriateness of all techniques, methods and procedures utilized in the erection of the building. Piece marks are randomly selected to identify individual parts and in no way indicate any sequence to erection.

PANELS	**TRIM**	MK.	QTY.	DESCRIPTION	MK.	QTY.	DESCRIPTION
ROOF-26GA/PBR/A. ZINC	GUTTER-26GA/ **						
WALL-26GA/PBR/ **	DOWNSPOUT-26GA/ **	A	ONE	7'-0 3/4x7'-0 3/4 FRAMED OPNG LOUVER/FAN BY OTHERS			
	RAKE-26GA/ **			W/SILL. (FOR FAN)			
	CORNER-26GA/ **	B	ONE	3'-4x8'-2 FRAMED OPNG WALK DOOR BY OTHERS W/SILL.			
	EAVE-26GA/ **			AT ELEVATION 15'-4 3/4			
	JAMB/HEAD-26GA/ **						

CUSTOMER NOTE:
REFERENCE ST DRAWING(S) FOR STANDARD SECTIONS.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

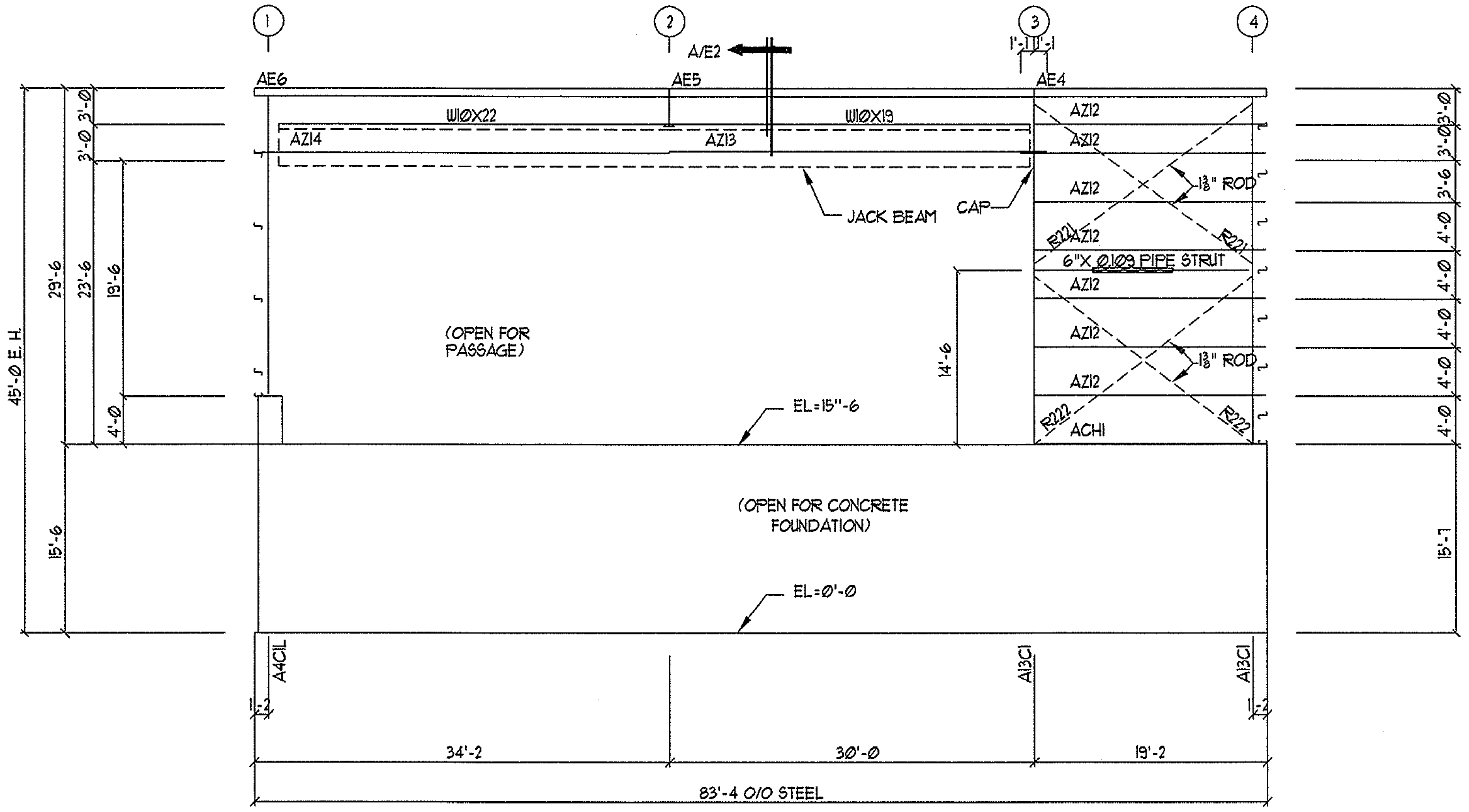
MAY 23 2008

ISSUE	REV. DESCRIPTION	DATE	BY DSN
P1	PERMIT	5/21	KJL

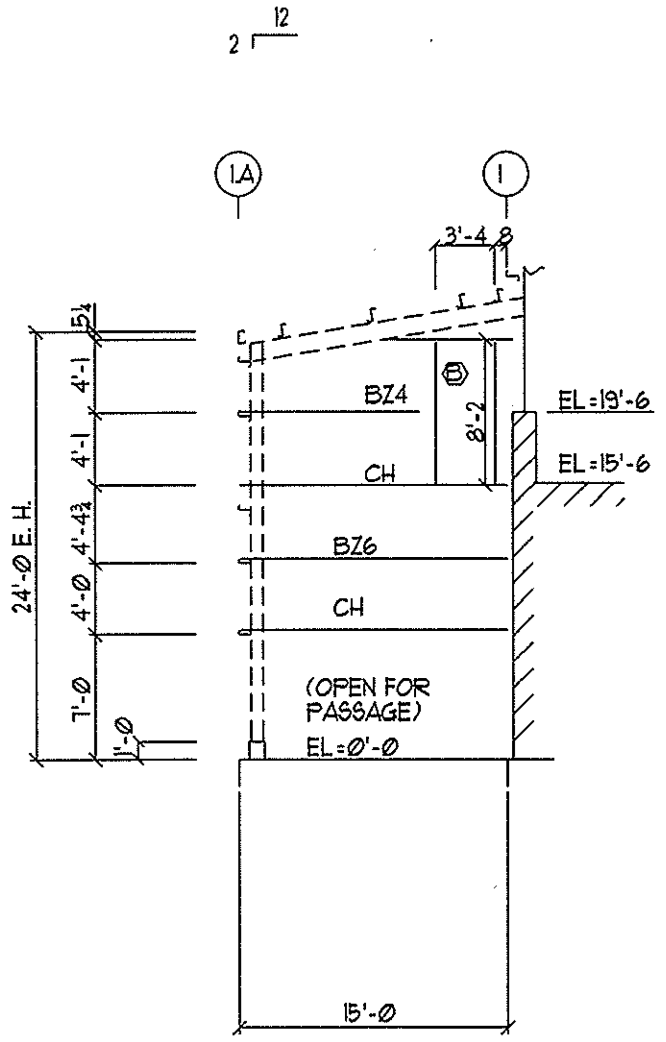
U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
HOUSTON, TEXAS

DESCRIPTION: ROOF FRAMING PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

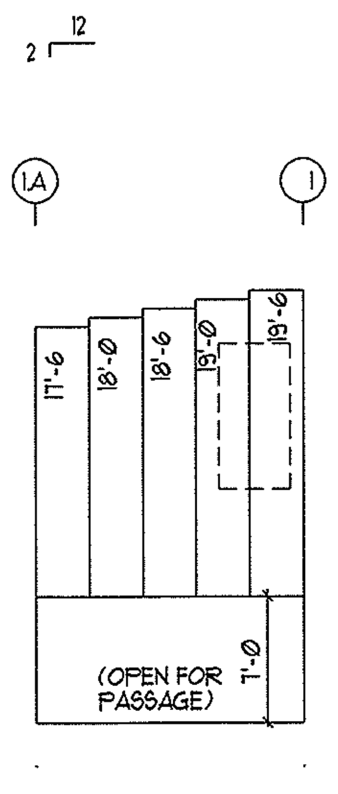
DRAWING BY	DATE	CHECK BY	DATE
CJ	5/14/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E1 of 10	ISSUE P1



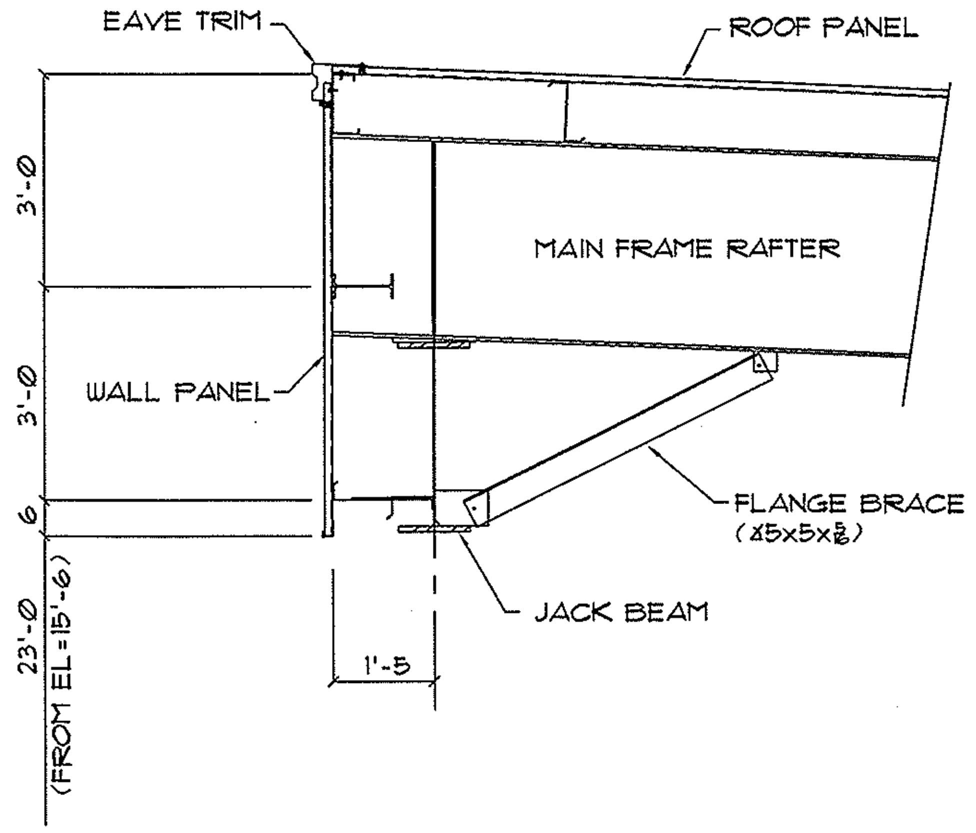
BACK SIDEWALL FRAMING PLAN AT COLUMN LINE "A"



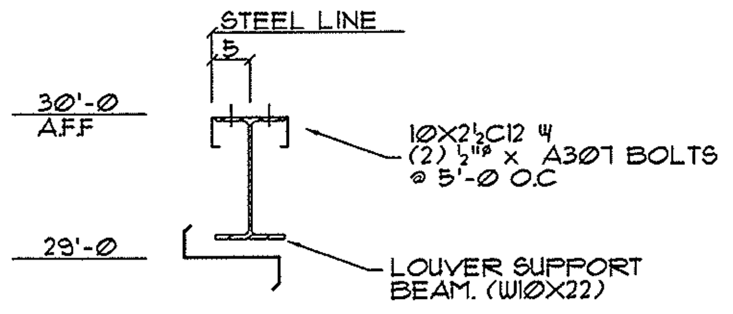
RIGHT ENDWALL FRAMING PLAN AT COLUMN LINE "C"



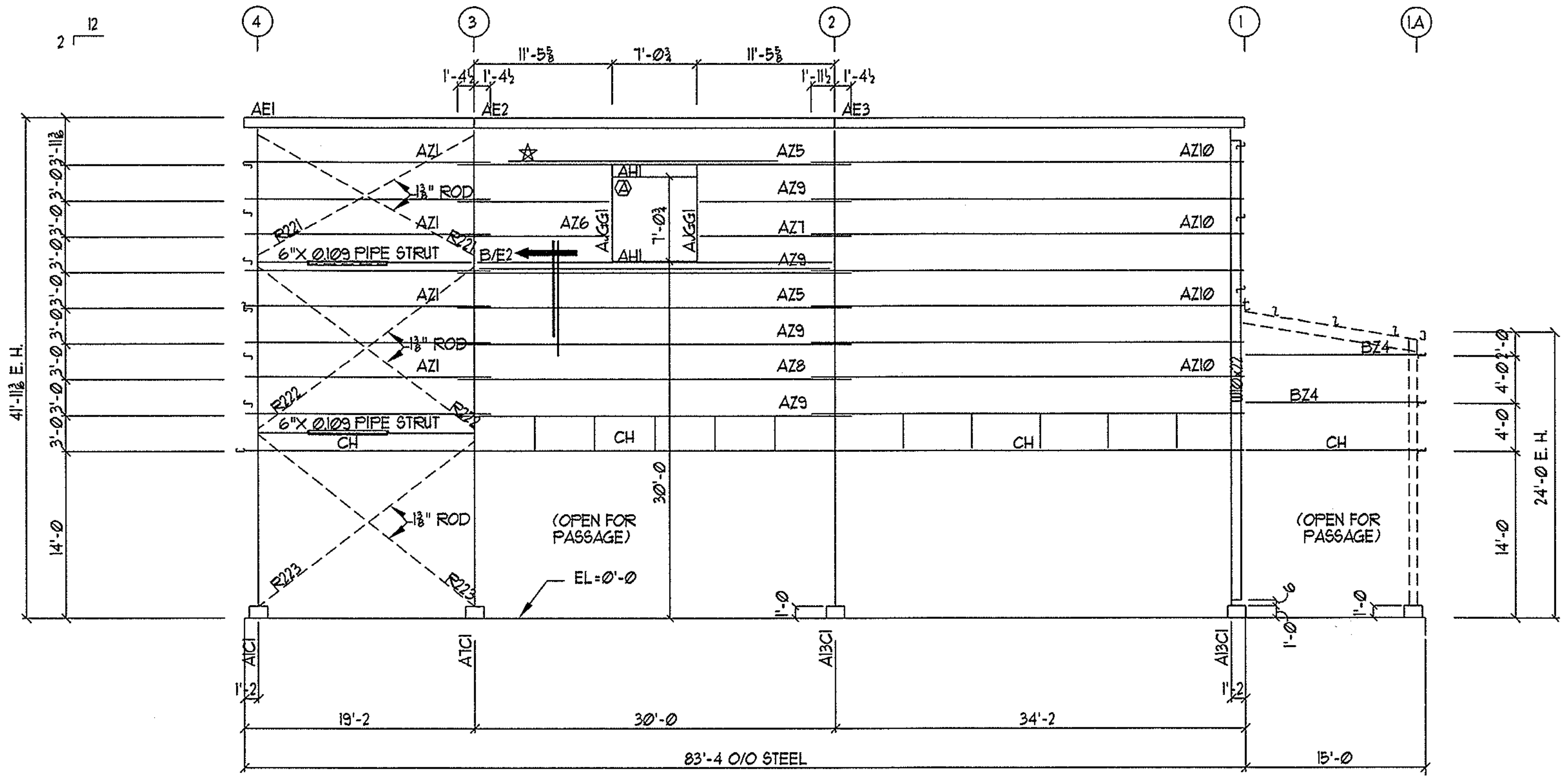
RIGHT ENDWALL SHEETING PLAN AT COLUMN LINE "C"



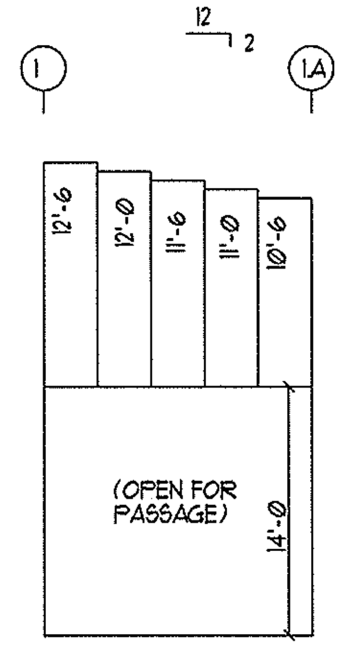
SECTION "A"/E2



SECTION B/E2

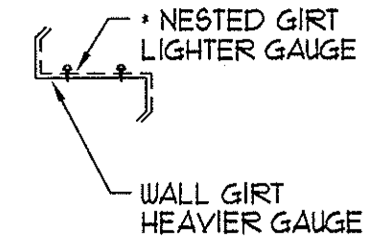


FRONT SIDEWALL FRAMING PLAN AT COLUMN LINE "E"



LEFT ENDWALL SHEETING PLAN AT COLUMN LINE "E"

☆ CENTER IN BAY AND FIELD ATTACH WITH (2) MEMBER SCREWS AT 5'-0 ON CENTER.



SECTION "A"

(BZ22)= 2 12GA GIRTS
(BZ23)= 1 12GA GIRT & 1 13GA GIRT
(BZ24)= 1 12GA GIRT & 1 14GA GIRT
(BZ25)= 1 12GA GIRT & 1 15GA GIRT
(BZ26)= 1 12GA GIRT & 1 16GA GIRT

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PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

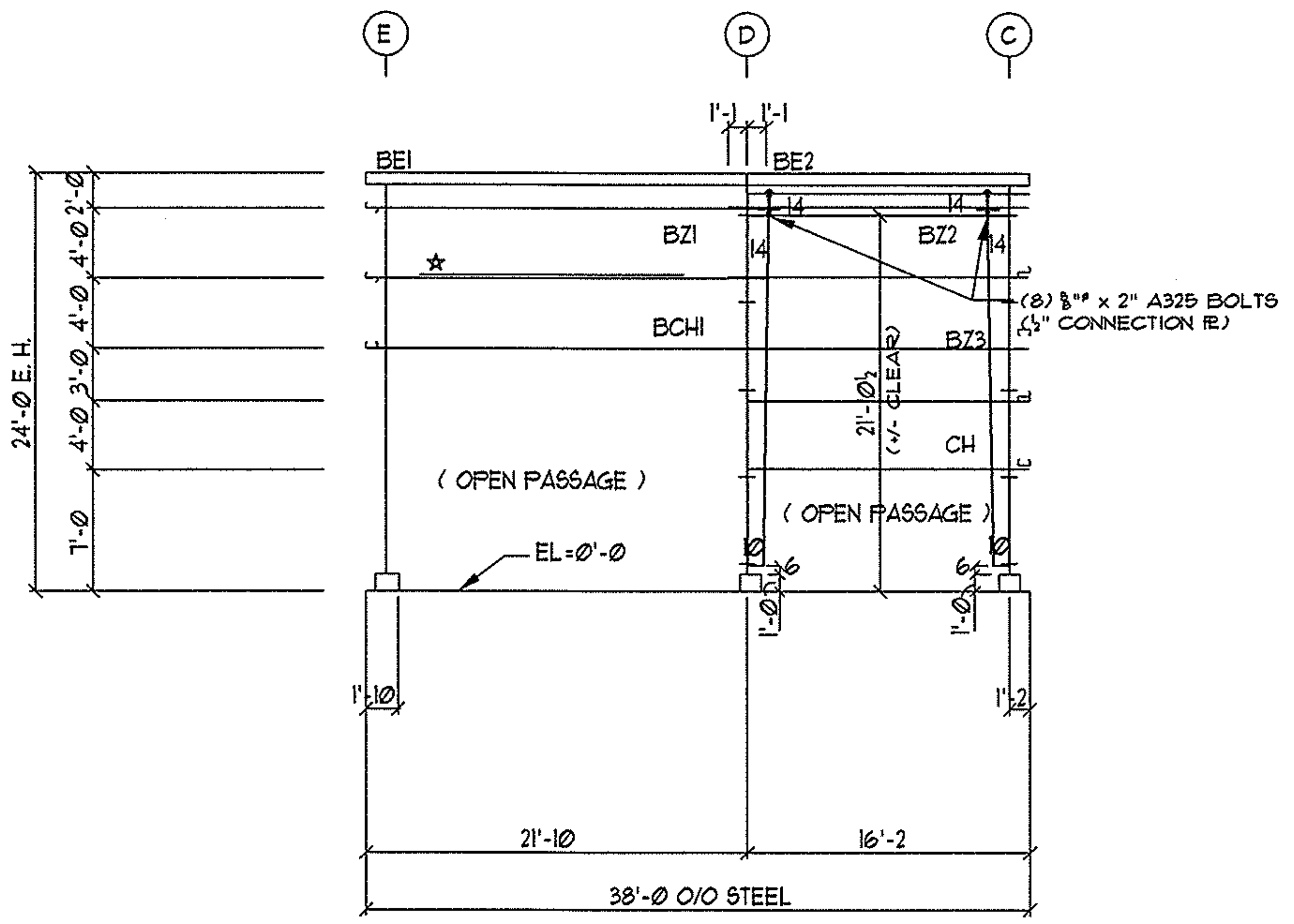
MAY 23 2008

ISSUE	REV. DESCRIPTION	DATE	BY
P1	PERMIT	5/21	KTL

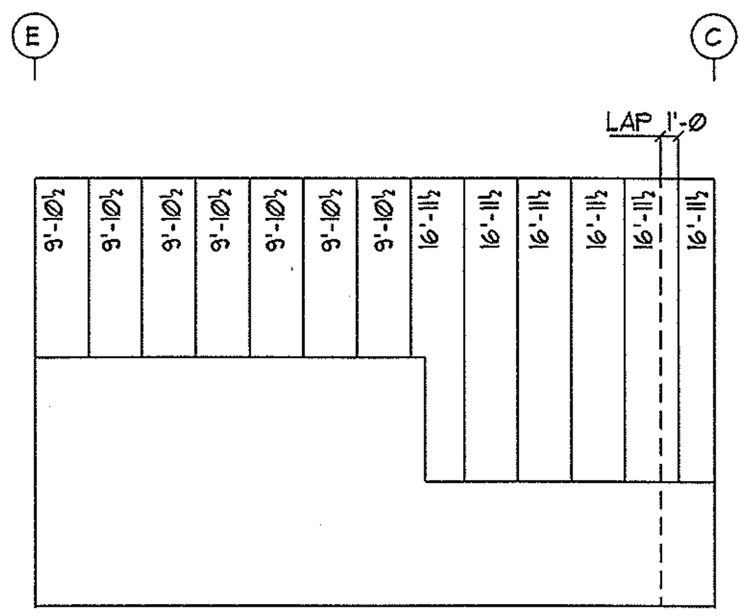
U.S.A.
UNITED STRUCTURES OF AMERICA, INC.

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E2 of 10	ISSUE P1

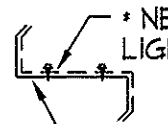


FRONT SIDEWALL FRAMING PLAN AT COLUMN LINE "1A"



FRONT SIDEWALL SHEETING PLAN AT COLUMN LINE "1A"

☆ CENTER IN BAY AND FIELD ATTACH WITH (2) MEMBER SCREWS AT 5'-0 ON CENTER.



SECTION "A"

(8Z22)= 2 12GA GIRTS
(8Z23)= 1 12GA GIRT & 1 13GA GIRT
(8Z24)= 1 12GA GIRT & 1 14GA GIRT
(8Z25)= 1 12GA GIRT & 1 15GA GIRT
(8Z26)= 1 12GA GIRT & 1 16GA GIRT

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

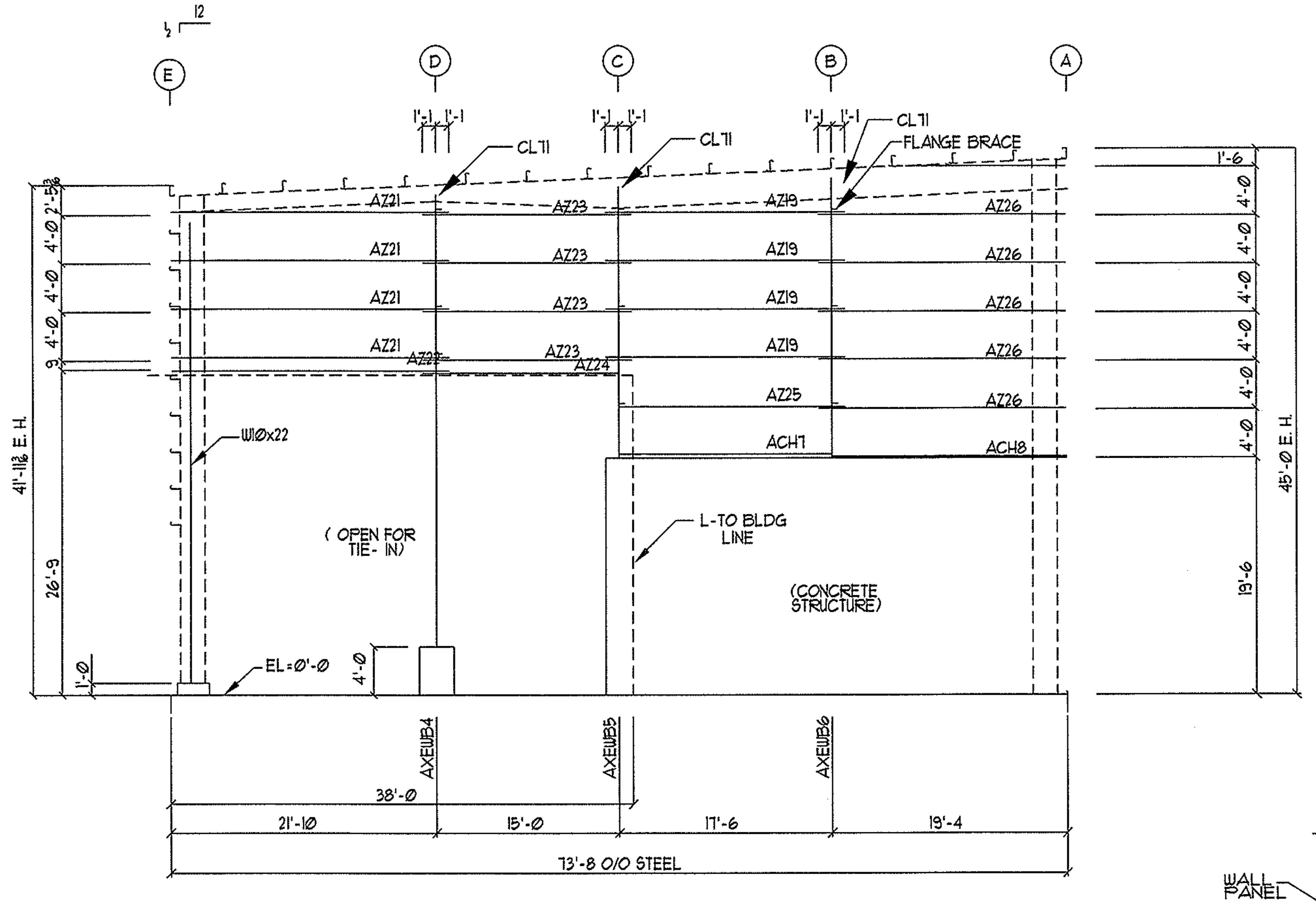
ISSUE	REV. DESCRIPTION	DATE	BY / DSN
P1	PERMIT	5/21	KTL

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
192 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

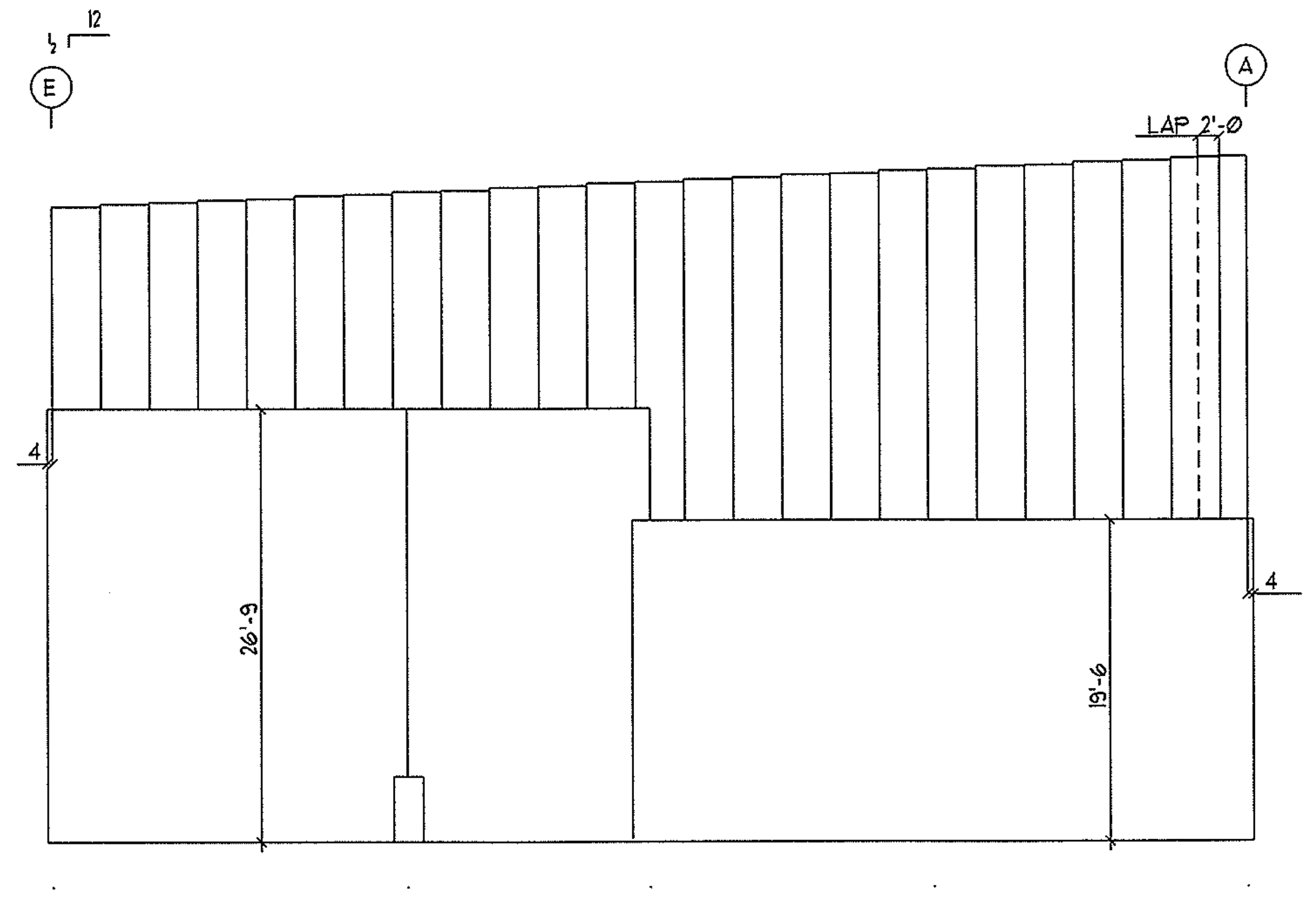
DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E3 of 10	ISSUE P1

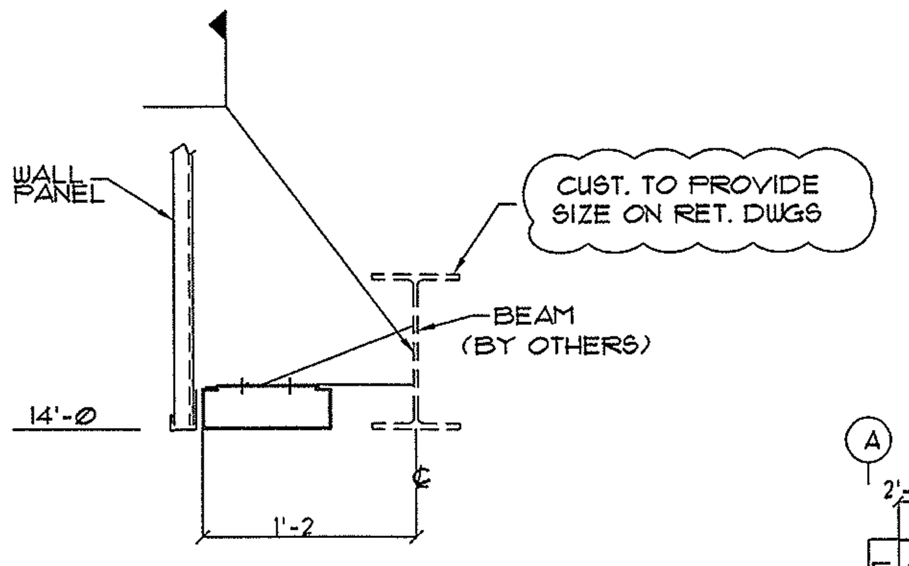
36506-BSUB.DWG



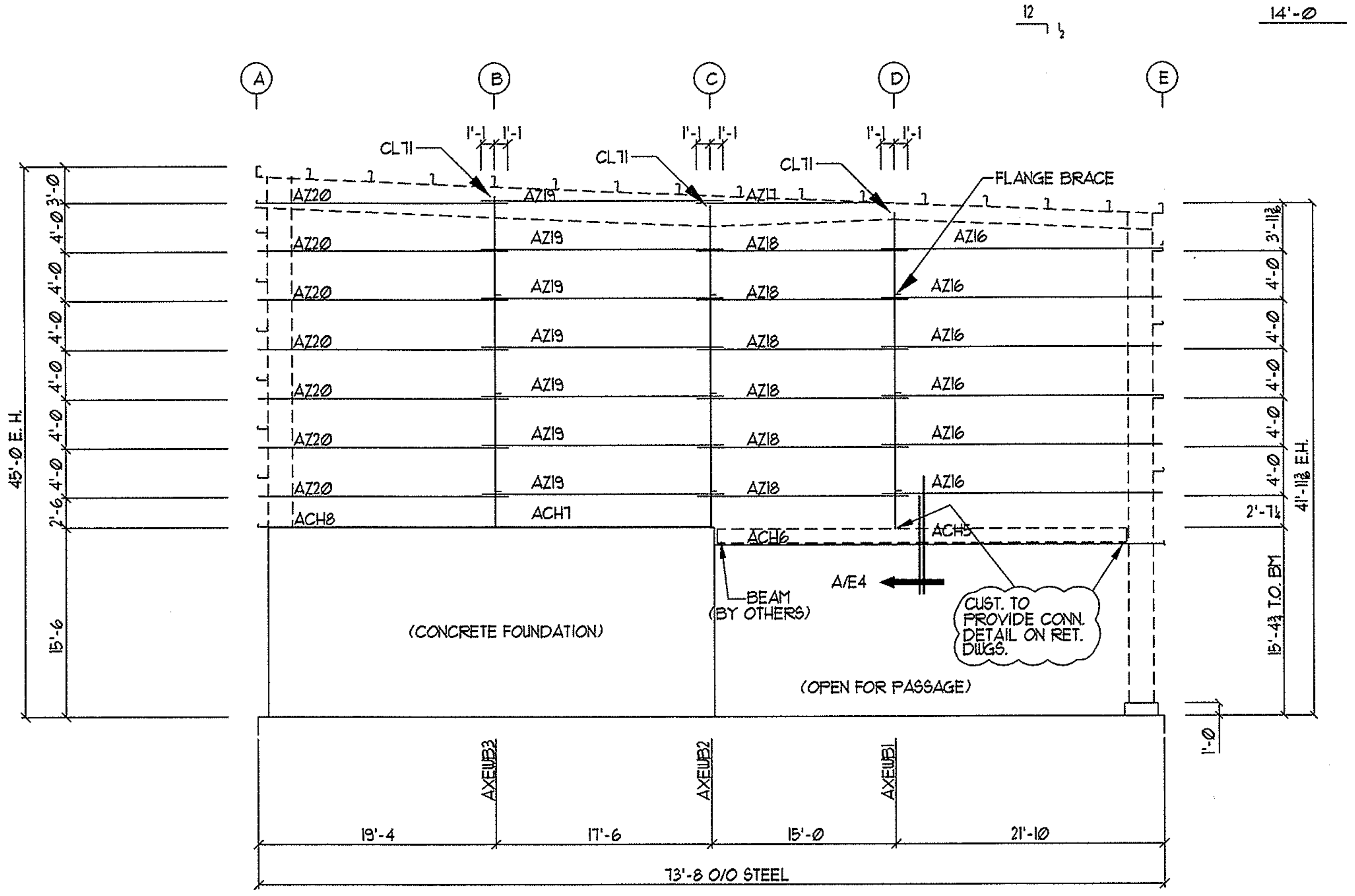
RIGHT ENDWALL FRAMING PLAN AT COLUMN LINE "1"



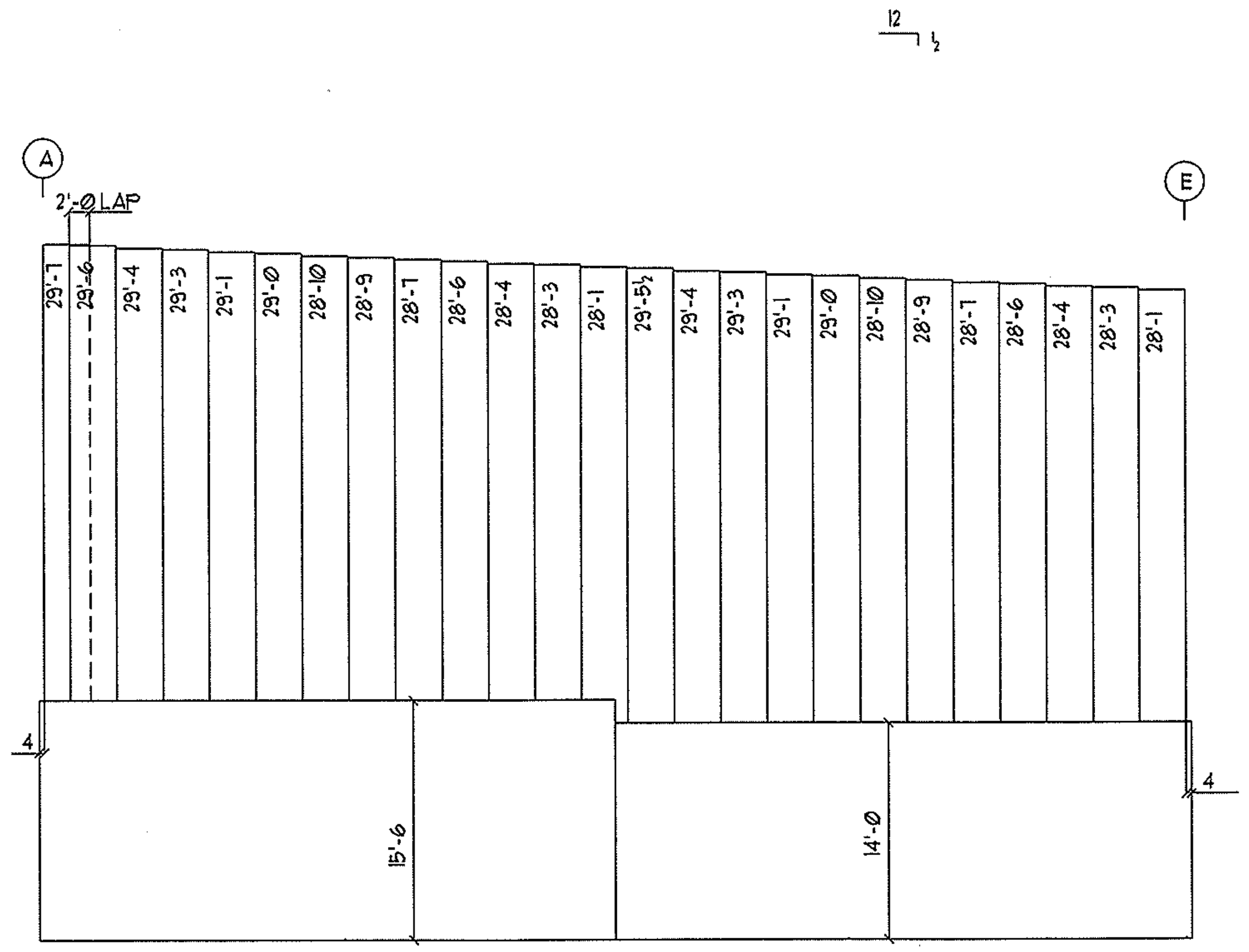
RIGHT ENDWALL SHEETING PLAN AT COLUMN LINE "4"



SECTION "A"/E4



LEFT ENDWALL FRAMING PLAN AT COLUMN LINE "4"



LEFT ENDWALL SHEETING PLAN AT COLUMN LINE "1"

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

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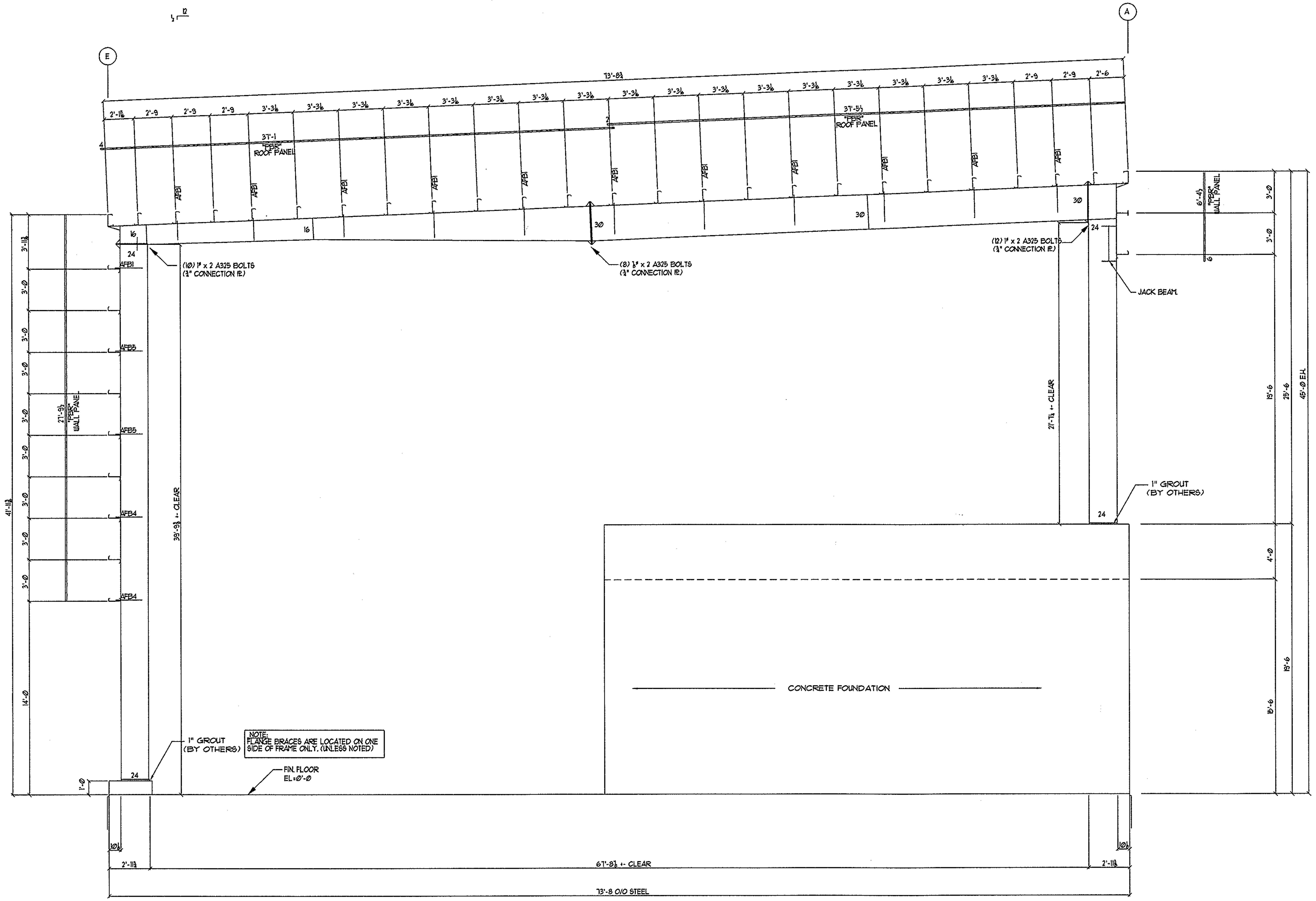
U.S.A.

UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: CONVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/13/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E4 of 10	ISSUE P1



CROSS SECTION AT COLUMN LINE 1

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

MAY 23 2008

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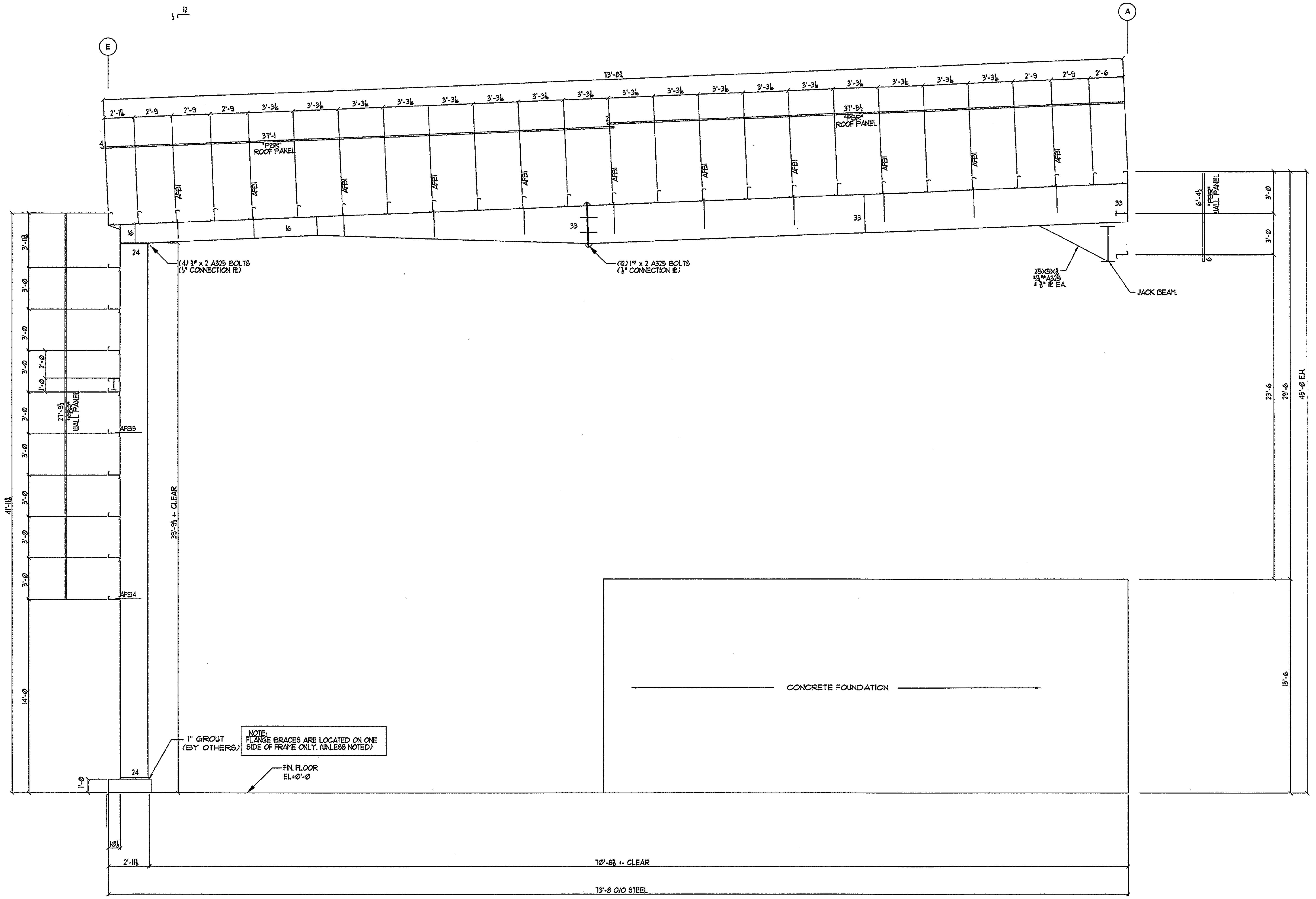
UNITED STRUCTURES OF AMERICA, INC.
150 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8241

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E5 of 10	ISSUE P1

36506-ACST.DWG



CROSS SECTION AT COLUMN LINE 2

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33608
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

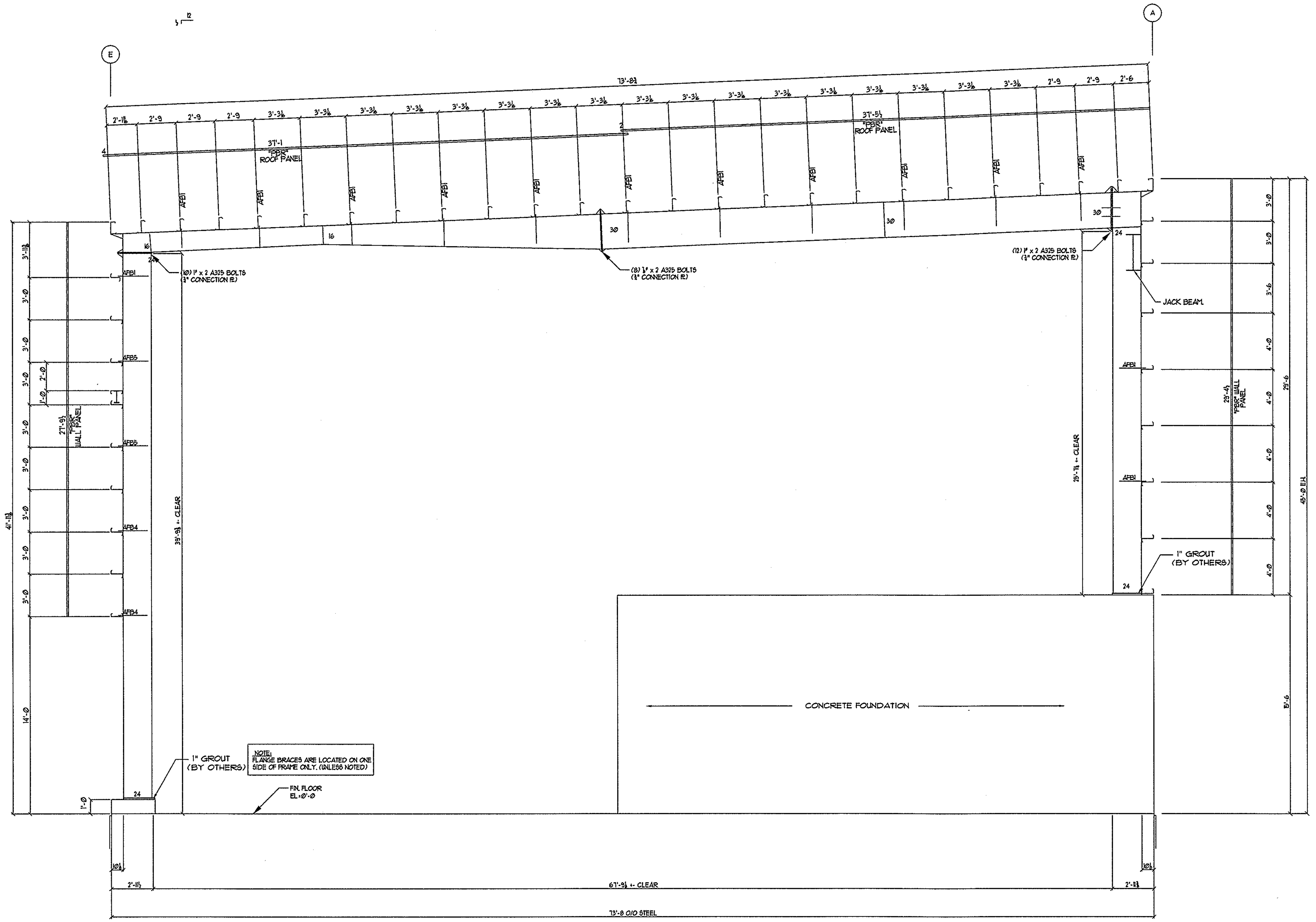
ISSUE	REV. DESCRIPTION	DATE	BY / DSN
P1	PERMIT	5/21	KTL

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
192 BUECHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E6 of 10	ISSUE P1

36506-AC83.DWG



CROSS SECTION AT COLUMN LINE 3

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

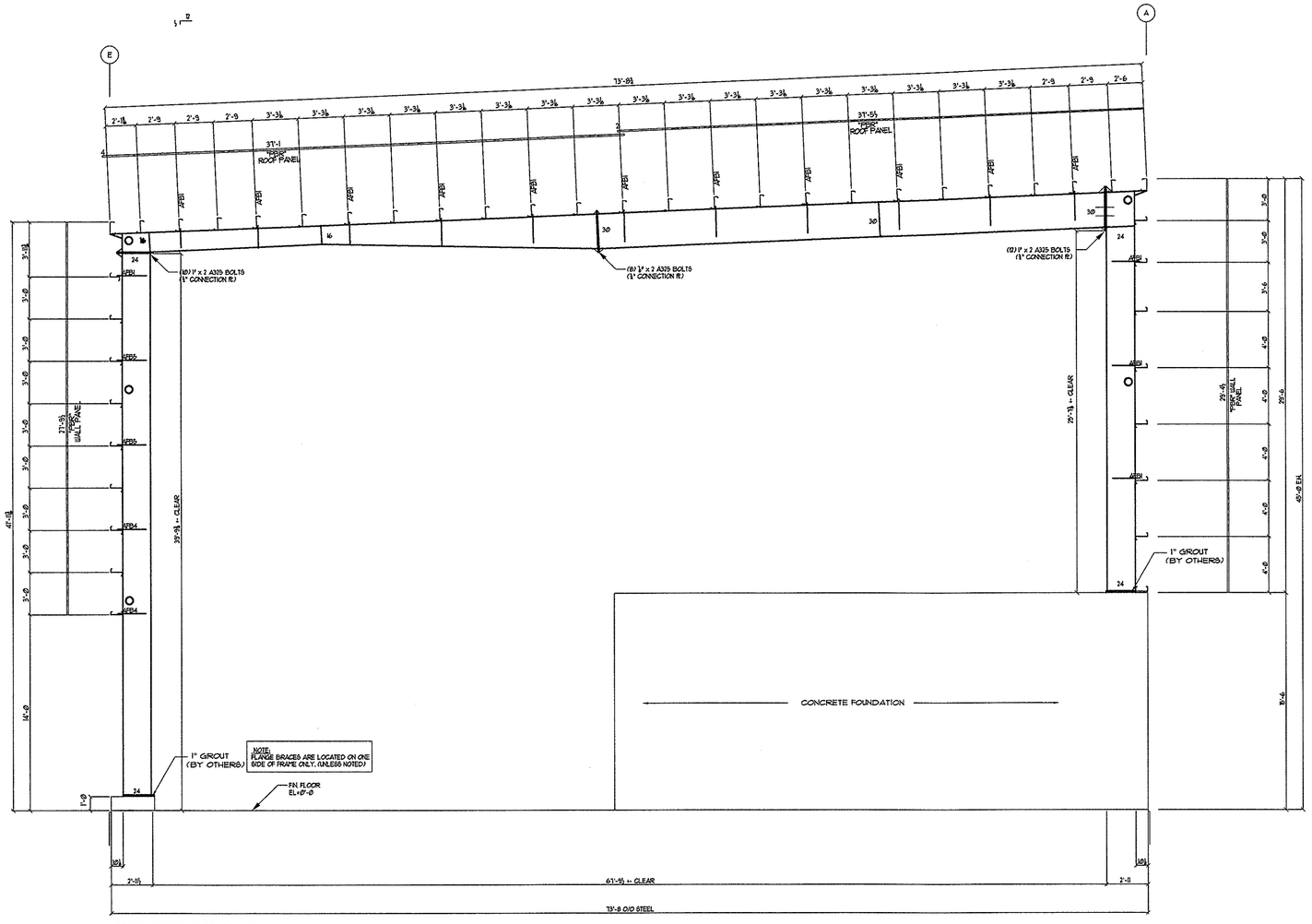
ISSUE	REV. DESCRIPTION	DATE	BY DSN
P1	PERMIT	5/21	KTL

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E7 of 10	ISSUE P1

36506-AC83.DWG



CROSS SECTION AT COLUMN LINE 4

CUSTOMER NOTE:
REFERENCE ST DRAWING(S)
FOR STANDARD SECTIONS.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

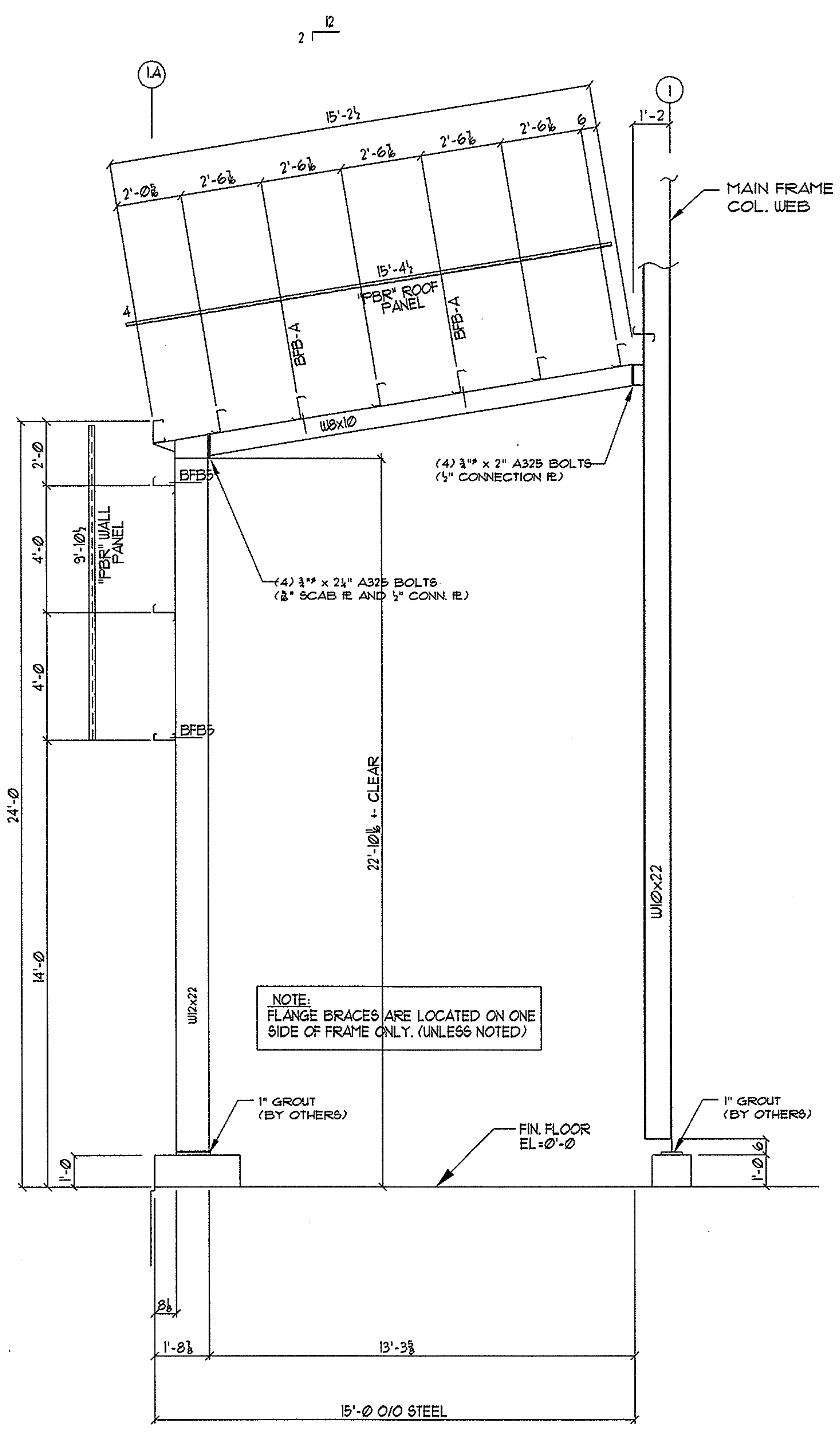
ISSUE	REV. DESCRIPTION	DATE	BY
PI	PERMIT	5/21	KEV

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
910 BISCHOFF
HOUSTON, TEXAS 77039
281-442-8241

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

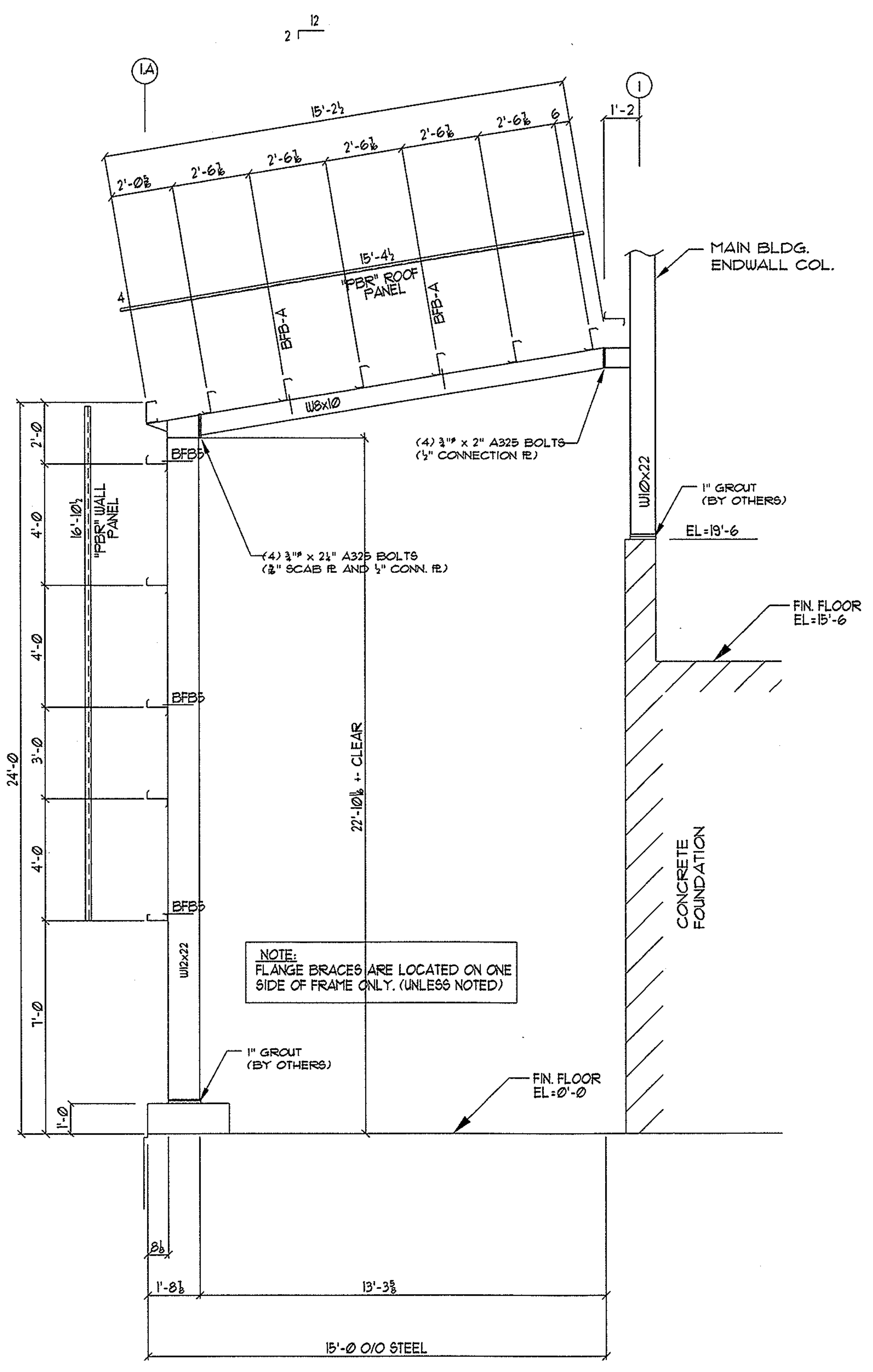
DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB #	SCALE	DRAWING #	ISSUE
36506A	N.T.S.	E8 of 10	PI

36506-ACAD2005



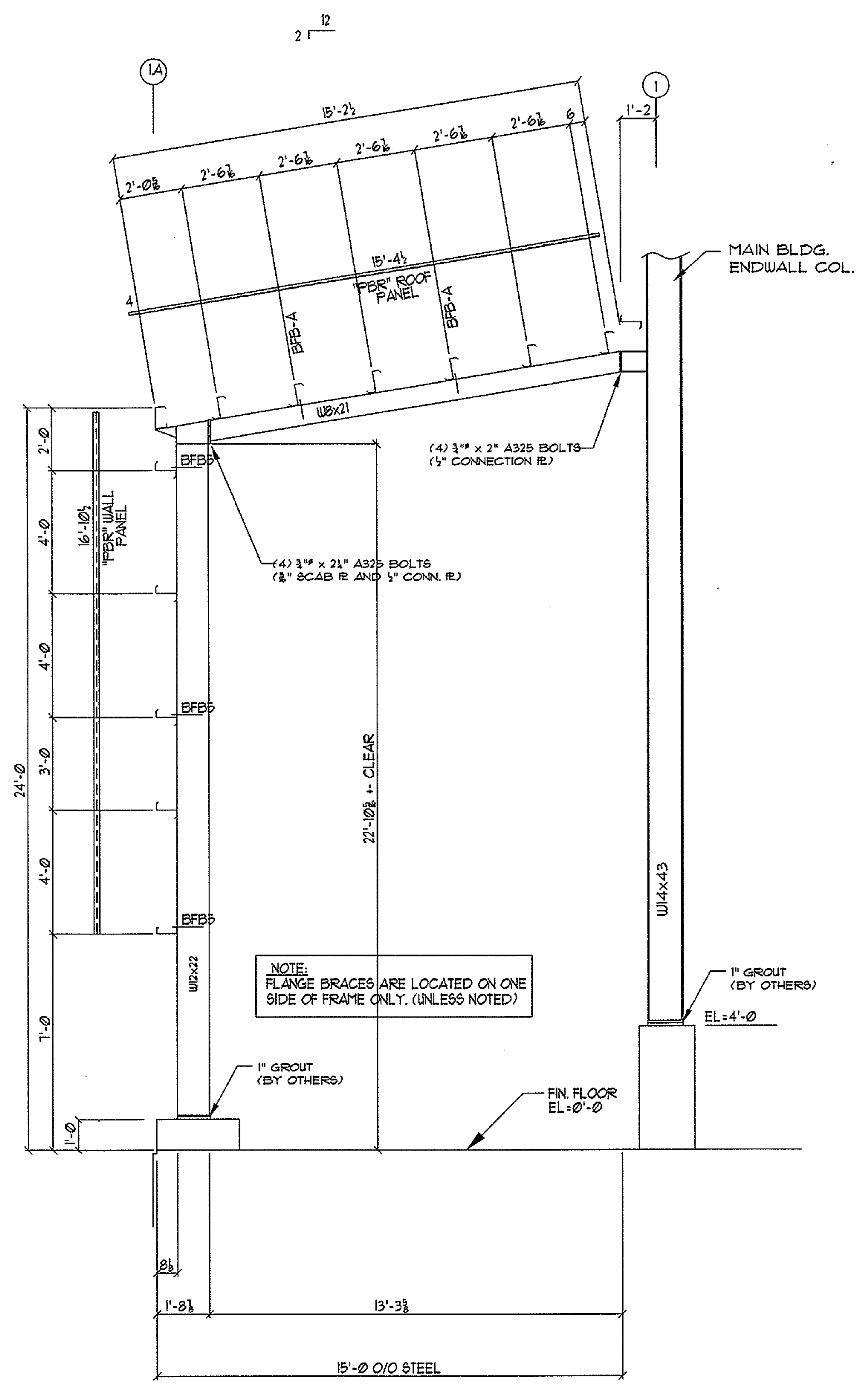
15'-2½
2'-0⅝
2'-6⅝
6
1'-2
MAIN FRAME
COL. WEB
15'-4½
"PBR" ROOF
PANEL
BFB-A
W8x10
(4) ¾"φ x 2" A325 BOLTS
(½" CONNECTION PL)
BFB5
9'-10½
"PBR" WALL
PANEL
(4) ¾"φ x 2¼" A325 BOLTS
(⅝" SCAB PL AND ½" CONN. PL)
24'-0
22'-10⅝ +- CLEAR
W10x22
W12x22
NOTE:
FLANGE BRACES ARE LOCATED ON ONE
SIDE OF FRAME ONLY. (UNLESS NOTED)
1" GROUT
(BY OTHERS)
FIN. FLOOR
EL=0'-0
1'-8⅝
13'-3⅜
15'-0 O/O STEEL

CROSS SECTION AT COLUMN LINE "E"
BLDG. LEAN-TO



15'-2½
1'-2
MAIN BLDG.
ENDWALL COL.
15'-4½
"PBR" ROOF
PANEL
BFB-A
W8x10
(4) ¾"φ x 2" A325 BOLTS
(½" CONNECTION PL)
W10x22
1" GROUT
(BY OTHERS)
EL=19'-6
FIN. FLOOR
EL=15'-6
16'-10½
"PBR" WALL
PANEL
(4) ¾"φ x 2¼" A325 BOLTS
(⅝" SCAB PL AND ½" CONN. PL)
22'-10⅝ +- CLEAR
CONCRETE
FOUNDATION
NOTE:
FLANGE BRACES ARE LOCATED ON ONE
SIDE OF FRAME ONLY. (UNLESS NOTED)
FIN. FLOOR
EL=0'-0
1'-8⅝
13'-3⅜
15'-0 O/O STEEL

CROSS SECTION AT COLUMN LINE "C"
BLDG. LEAN-TO



15'-2½
1'-2
MAIN BLDG.
ENDWALL COL.
15'-4½
"PBR" ROOF
PANEL
BFB-A
W8x21
(4) ¾"φ x 2" A325 BOLTS
(½" CONNECTION PL)
16'-10½
"PBR" WALL
PANEL
(4) ¾"φ x 2¼" A325 BOLTS
(⅝" SCAB PL AND ½" CONN. PL)
22'-10⅝ +- CLEAR
W14x43
1" GROUT
(BY OTHERS)
EL=4'-0
NOTE:
FLANGE BRACES ARE LOCATED ON ONE
SIDE OF FRAME ONLY. (UNLESS NOTED)
FIN. FLOOR
EL=0'-0
1'-8⅝
13'-3⅜
15'-0 O/O STEEL

CROSS SECTION AT COLUMN LINE "D"
BLDG. LEAN-TO

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33809
PROFESSIONAL ENGINEER
CIVIL ENGINEERING

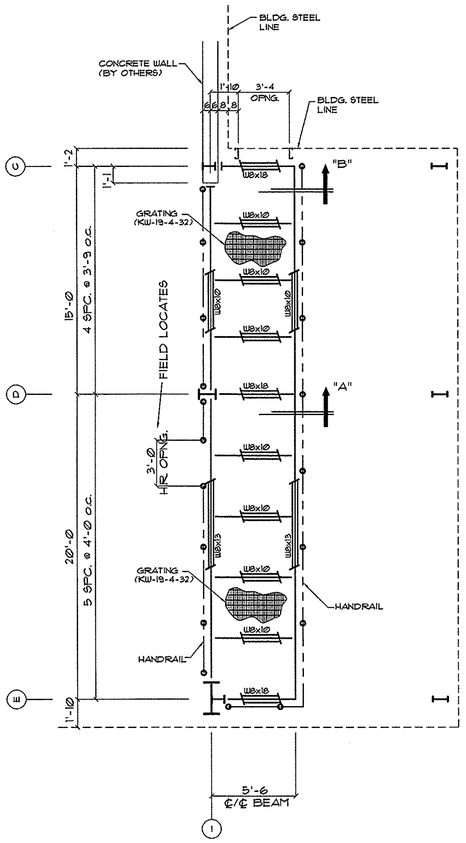
MAY 23 2008

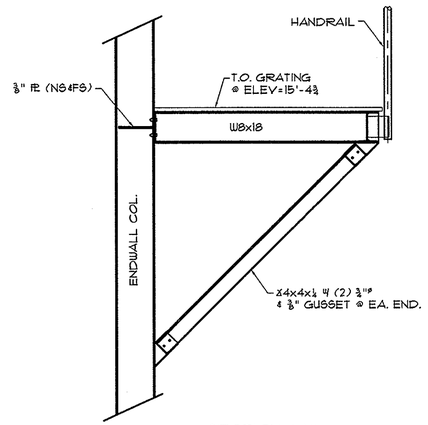
ISSUE	REV. DESCRIPTION	DATE	BY
P1	PERMIT	5/21	KTL

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.

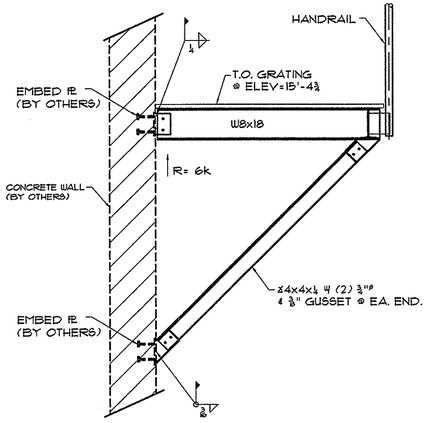
DESCRIPTION:	ERECTION PLAN
CUSTOMER:	CORSO FABRICATORS, INC
LOCATION:	COVINGTON, LA
BUYER NO.:	COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/19/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # E9 of 10	ISSUE P1

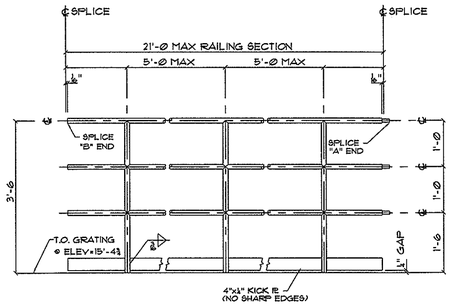




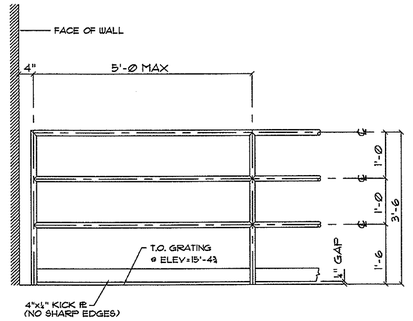
SECTION "A"



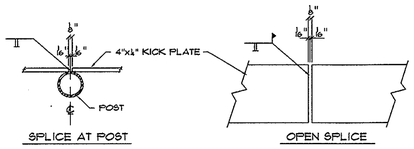
SECTION "B"



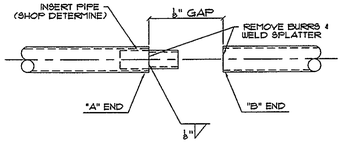
TYPICAL HANDRAIL ELEVATION
(LEVEL, Ψ KICK PLATE)



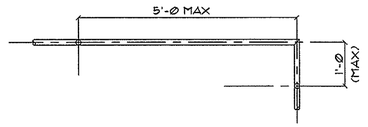
TYPICAL HANDRAIL END



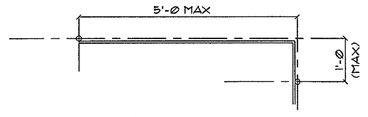
TYPICAL KICK PLATE SPLICE DETAILS



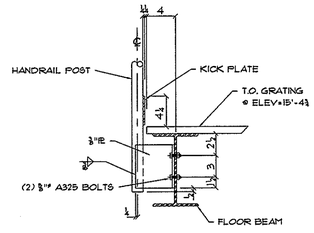
HANDRAIL SPLICE



HANDRAIL PLAN @ CORNER



KICK PLATE PLAN @ CORNER



HANDRAIL CONNECTION TO FLOOR BEAM

Notes Regarding Mezzanine Floor Vibration:

Vibration and / or dynamic analysis are beyond the scope of USA's work. The actual extent and perceptibility of floor vibrations are functions of several factors which are beyond USA's control. These factors include the dynamic loading (such as walking, mechanical equipment, etc). Also included amongst these factors are the layout and loading of the floor, such as weight and layout of partitions, furniture, and other dead or live loads.

If there is a genuine concern regarding possible floor vibration, USA recommends a detailed examination on the part of the customer and the design professional.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

CUSTOMER NOTE:
REFERENCE ST DRAWING(S) FOR STANDARD SECTIONS.

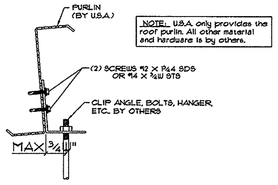
ISSUE	REV. DESCRIPTION	DATE	BY CHK
PI	PERMIT		

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
102 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION:	ERECTION PLAN
CUSTOMER:	CORSO FABRICATORS, INC.
LOCATION:	COVINGTON, LA
BUYER NO.:	COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
JC	5/17/08		
JOB #	SCALE	DRAWING #	ISSUE
36506A	N.T.S.	E10 OF 10	PI

36506-MEZZSOS



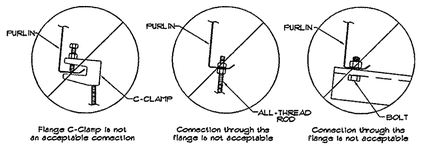
NOTE: U.S.A. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

NOTE: Drilling of holes in flanges of purlins is structurally detrimental to the member. Any collateral loads, which are included in the purlin design loads, are to be attached by connection directly to the web only.

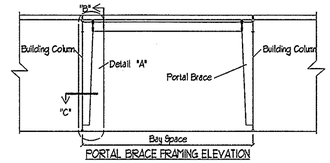
Field Modifications - Warning

Ref. Cover sheet L1 paragraph 2.11

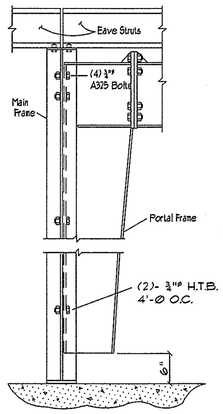


DRAWING DESCRIPTION

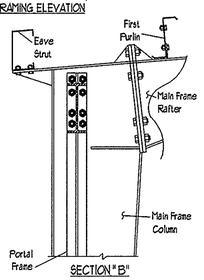
ATTACHMENT OF SUSPENDED ITEMS (BY OTHERS)



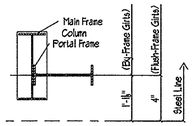
PORTAL BRACE FRAMING ELEVATION



DETAIL "A"



SECTION "B"



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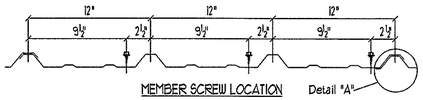
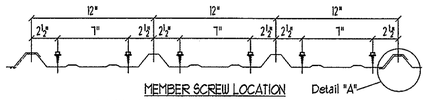
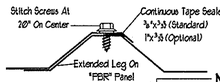


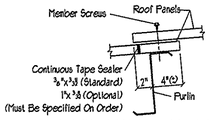
MEMBER SCREW LOCATION (ALL MEMBERS EXCEPT AS NOTED)



MEMBER SCREW LOCATION (PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS)



DETAIL "A"

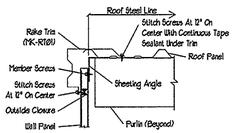


SECTION THRU PEAK PURLIN AND PANEL END LAPS

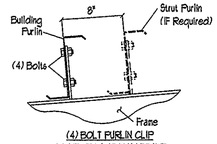
"PBR" PANEL ROOF

NOTE!

SCREW PATTERNS SHOWN, SATISFY UL-90 REQUIREMENTS FOR ROOF. THE USA PBR MUST BE ATTACHED TO THE STRUT ZEE. ANY ADDED ROOF PURLINS FOR EDGE STRIPPING REQUIREMENTS MUST BE ATTACHED TO THAT MEMBER.

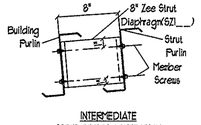


RAKE DETAIL WITH SHEETED ENDWALL

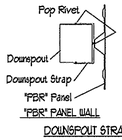


(4) BOLT PURLIN CLIP CONNECTION AT MAIN FRAME

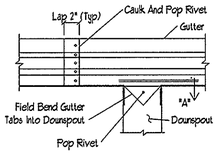
All Connection Bolts Are ½"ø x 1" M.B. Unless Otherwise Noted
The USA PBR roof nail not be attached to the strut zee. On any added roof purlins for edge stripping requirements, the panel can be attached to that member.
Strut zees are located 5'-0 max.



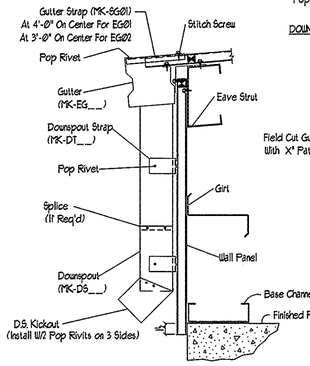
INTERMEDIATE STRUT PURLIN CONNECTION (AT DIAPHRAGM)



"PBR" PANEL WALL DOWNSPOUT STRAP ATTACHMENT

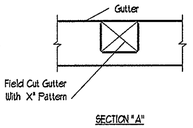


DOWNSPOUT TO GUTTER DETAIL

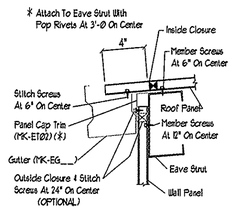


DOWNSPOUT DETAIL AT SHEETED WALL

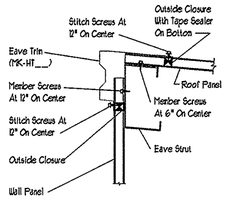
D.S. STRAP QUANTITY	
EAVE HEIGHT	QUANTITY
10'-0	2
12'-0	2
14'-0	2
16'-0	2
20'-0	2
25'-0	3



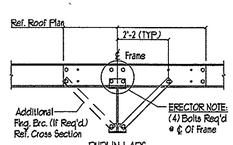
SECTION "A"



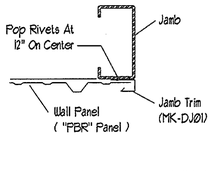
LOW EAVE DETAIL WITH EAVE GUTTER (SHEETED SIDEWALL)



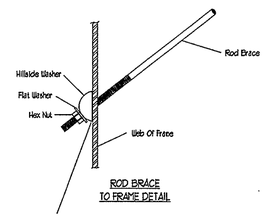
HIGH EAVE DETAIL WITH SCULPTURED EAVE TRIM (SHEETED SIDEWALL)



PURLIN LAPS (8 Bolts, Add 2 additional per flng. brc.)

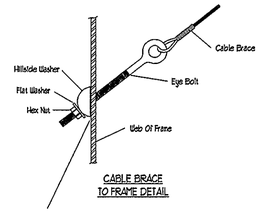


JAMB DETAIL FOR FRAMED OPENINGS



ROD BRACE TO FRAME DETAIL

NOTE: Use A Backup Plate Under The Hillside Washer For Cold Form Members.



CABLE BRACE TO FRAME DETAIL

NOTE: Use A Backup Plate Under The Hillside Washer For Cold Form Members.

CUSTOMER NOTE: REFERENCE ST DRAWING(S) FOR STANDARD SECTIONS.

GENERAL NOTES :

1. ALL BOLTS ARE 1/2" DIA. x 1" MACHINE BOLTS UNLESS NOTED.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

ISSUE	REV. DESCRIPTION	DATE	BY / CHK
P1	PERMIT	5/21	KEW

U.S.A.

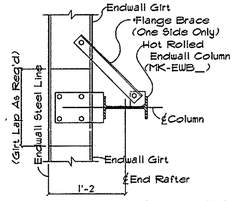
UNITED STRUCTURES OF AMERICA, INC.
910 EMMONS
HOUSTON, TEXAS 77009
281-442-6247

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

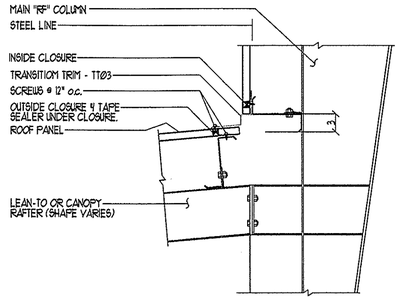
DRAWING BY	DATE	CHECK BY	DATE
CJ	5/12/2008		
JOB # 365064	SCALE N.T.S.	DRAWING # ST1 of 2	ISSUE P1

365064-A-PBRDWG

Flange Brace	
W8 Col.- FB2	W14 Col.- FB1
W10 Col.- FB5	W16 Col.- FB5
W12 Col.- FB3	W18 Col.- FB3

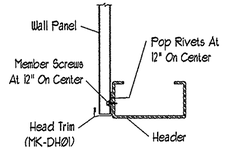


HOT ROLLED ENDWALL COLUMN

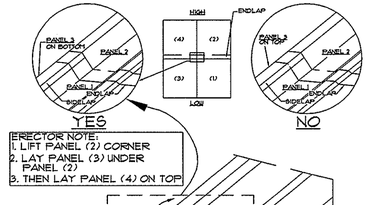


BY - FRAMED

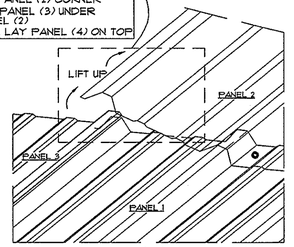
LEAN-TO AND/OR CANOPY TIE-IN
(ROOF TO WALL TRANSITION)



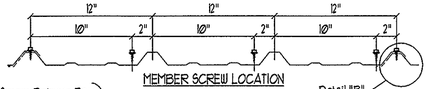
HEADER DETAIL FOR FRAMED OPENINGS



ERECTOR NOTE:
1. LIFT PANEL (2) CORNER
2. LAY PANEL (3) UNDER PANEL (2)
3. THEN LAY PANEL (4) ON TOP

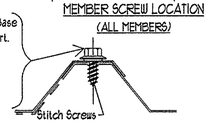


PBR ROOF ENDLAP DETAIL
AT ADJOINING CORNERS

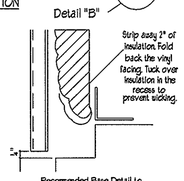


MEMBER SCREW LOCATION
(ALL MEMBERS)

(2) Screws Between Base Member And 1'-3 ½ Girt.
(1) Screw Between All Other Members.
And (1) Screw At All Member Locations.



DETAIL "B"



"PBR" or "R" PANEL WALL
DRAWING DESCRIPTION

CUSTOMER NOTE:
REFERENCE ST DRAWING(S) FOR STANDARD SECTIONS.

GENERAL NOTES :
1. ALL BOLTS ARE 1/2" DIA. x 1" MACHINE BOLTS UNLESS NOTED.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING
MAY 23 2008

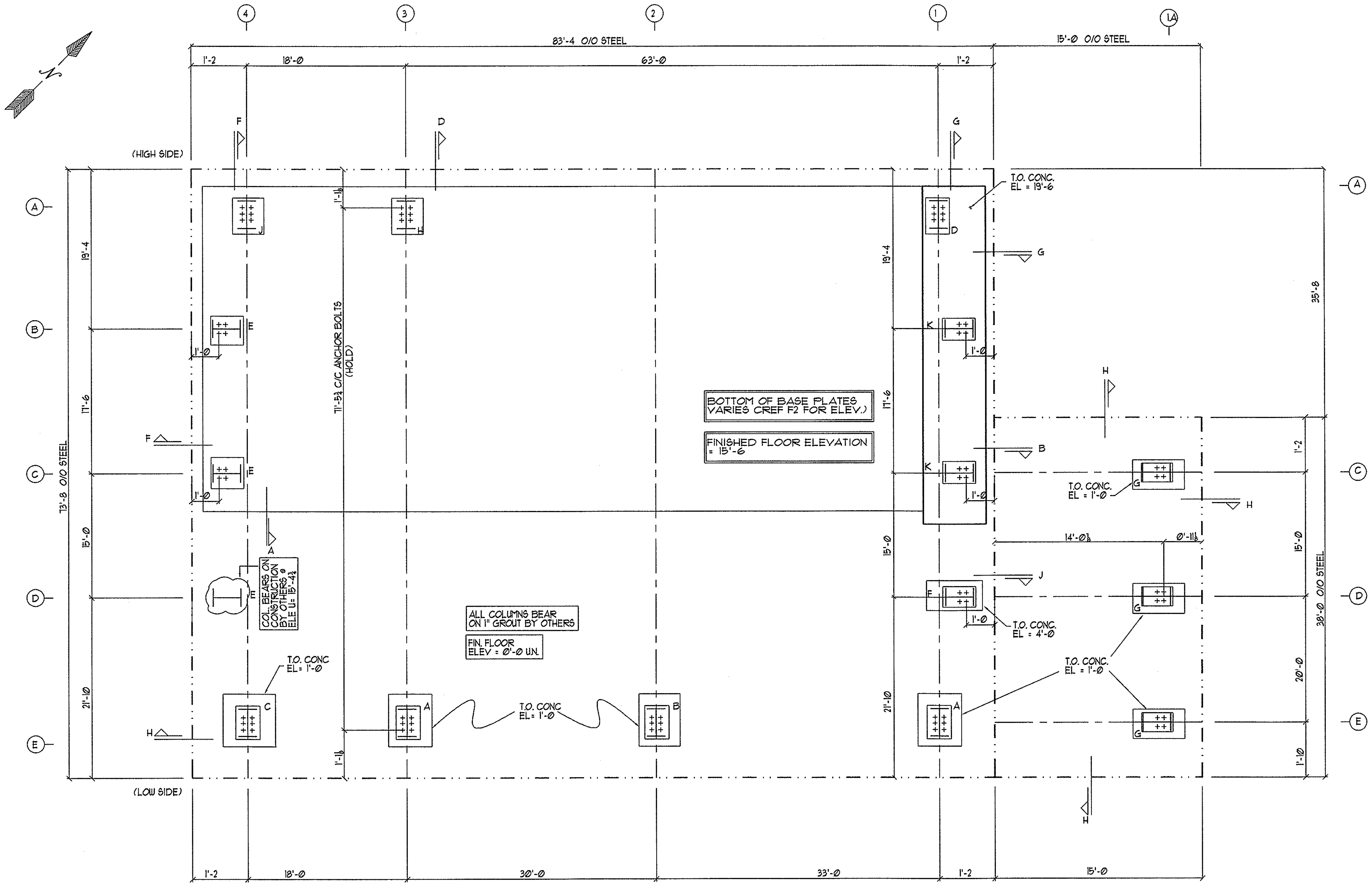
ISSUE	REV. DESCRIPTION	DATE	BY
P1	PERMIT	5/21	KW

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
102 BUSCHONG
HOUSTON, TEXAS 77029
281-442-8241

DESCRIPTION: ERECTION PLAN
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/12/2008		
JOB #	SCALE	DRAWING #	ISSUE
36506A	N.T.S.	ST 2 of 2	P

36506-A-P0712X36



ANCHOR BOLT SETTING PLAN

ISSUE	REV. DESCRIPTION	DATE	BY KTL / DSN
0	PERMIT/CONST	5/15	

U.S.A.

UNITED STRUCTURES OF AMERICA, INC.

DESCRIPTION: ANCHOR BOLT SETTING PLAN

CUSTOMER: CORSO FABRICATORS, INC

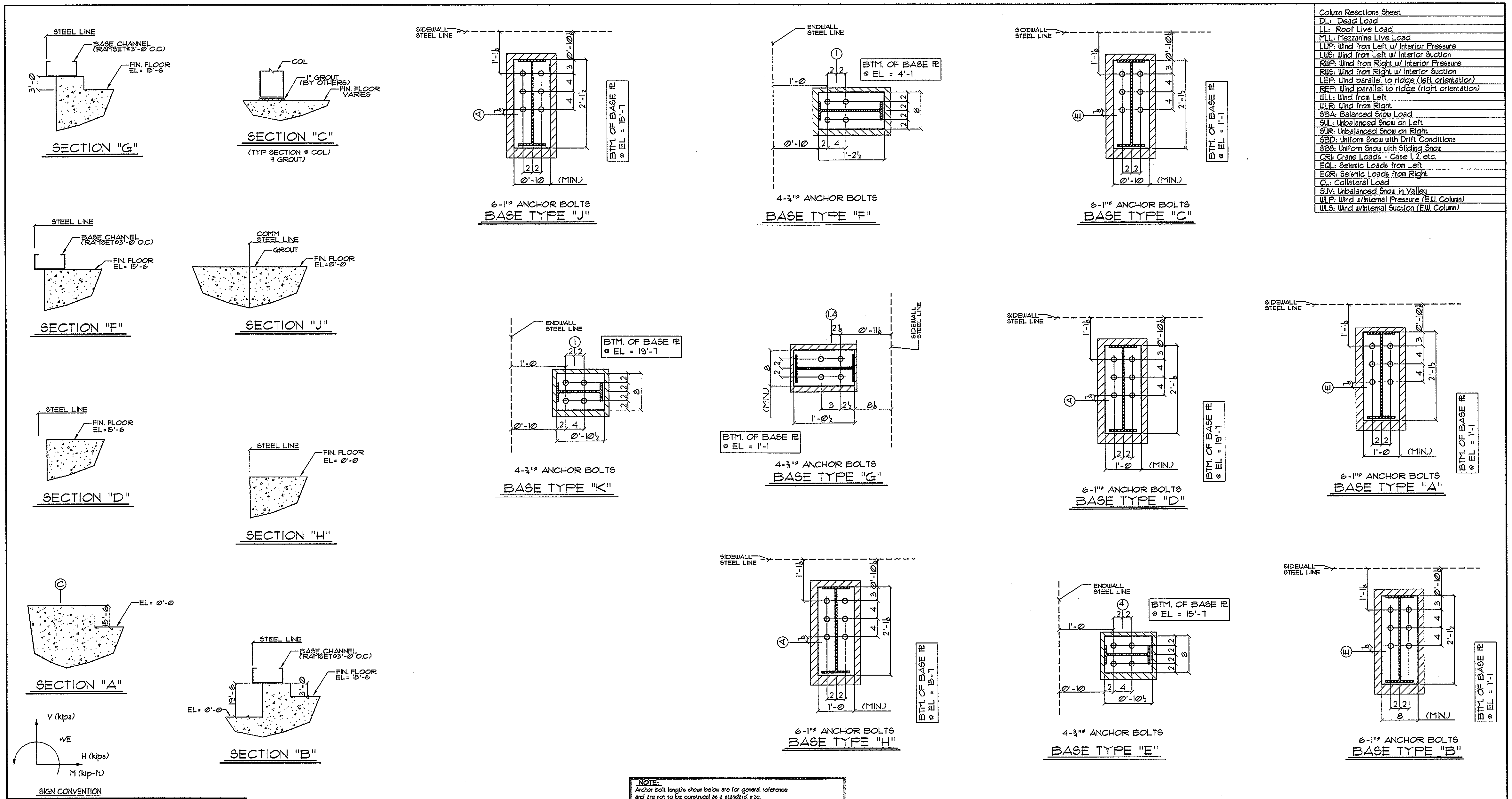
LOCATION: COVINGTON, LA

BUYER NO.: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/12/08	KTL	5/15/08
JOB # 36506A	SCALE N.T.S.	DRAWING # F1 of 3	ISSUE 0

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

MAY 23 2008



Column Reactions Sheet
DL: Dead Load
LL: Roof Live Load
MLL: Mezzanine Live Load
LWP: Wind from Left w/ Interior Pressure
LWS: Wind from Left w/ Interior Suction
RWP: Wind from Right w/ Interior Pressure
RWS: Wind from Right w/ Interior Suction
LEP: Wind parallel to ridge (left orientation)
REP: Wind parallel to ridge (right orientation)
WLL: Wind from Left
WLR: Wind from Right
SBA: Balanced Snow Load
SUL: Unbalanced Snow on Left
SUR: Unbalanced Snow on Right
SBD: Uniform Snow with Drift Conditions
SBS: Uniform Snow with Sliding Snow
CRI: Crane Loads - Case I, 2, etc.
EQL: Seismic Loads from Left
EQR: Seismic Loads from Right
CL: Collateral Load
SUV: Unbalanced Snow in Valley
WLP: Wind w/Internal Pressure (E.W. Column)
WLS: Wind w/Internal Suction (E.W. Column)

NOTE:
Anchor bolt lengths shown below are for general reference and are not to be construed as a standard size.
Embedment length of bolts will vary based on the foundation design. It is the responsibility of the customer to determine the proper anchor bolt embedment length.
Ref. General Note #2 of this drawings.
(If Anchor Bolts are supplied by U.S.A., Customer must specify required length to USA).

NOTE:
ANCHOR BOLT PROJECTION ARE FROM TOP OF CONCRETE.

GENERAL NOTES:

① The Anchor Bolt Details shown on this drawing locate the Anchor Bolts in reference to both the Building Steel Line and the outside of U.S.A.'s SUGGESTED panel recess of 1-1/2".

② United Structures of America is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing, and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, United Structures of America does not design and is not responsible for the design, material, and construction of the foundation or foundation embedments. The End Use Customer should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures.

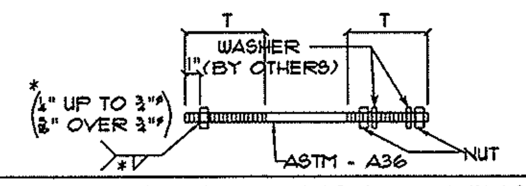
③ Bottom of all Base Plates are at the same elevation. (UNLESS NOTED)

④ NOTE: SHEAR ANGLES AND/OR ANCHOR TIES ARE NOT FURNISHED BY U.S.A.

STATE OF LOUISIANA
DAIN R. DRAKE
License No. 33808
PROFESSIONAL ENGINEER
IN
CIVIL ENGINEERING

MAY 23 2008

ANCHOR BOLT DETAIL



ANCHOR BOLTS (BY OTHERS)

PcMk	SYMBOL	DIA.	QTY.		T	PROJ.
A4	✦	1/2"			1 1/4"	1"
A5	⊕	5/8"			2"	2"
A6	⊕	3/4"	32		3"	4"
A7	⊕	7/8"			3"	3"
A8	⊕	1"	42		3"	4"
A10	⊕	1 1/4"			4"	4"
A12	⊕	1 1/2"			4"	4"

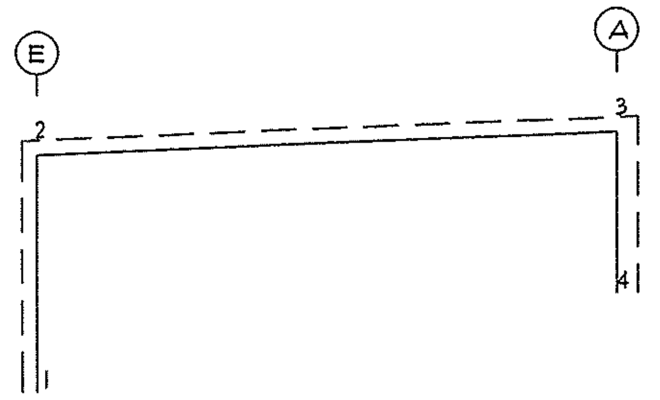
ISSUE	REV. DESCRIPTION	DATE	BY / DSN
0	PERMIT/COMST	5/15	KTL

U.S.A.
UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

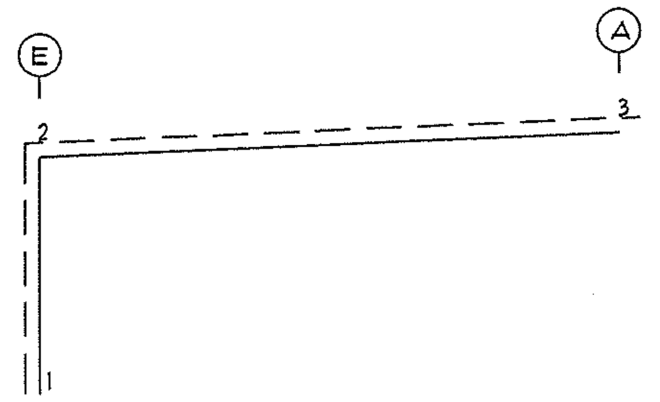
DESCRIPTION: REACTION
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER: COVINGTON PICK UP STATION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/13/08		
JOB # 36506A	SCALE N.T.S.	DRAWING # F 2 of 3	ISSUE 0

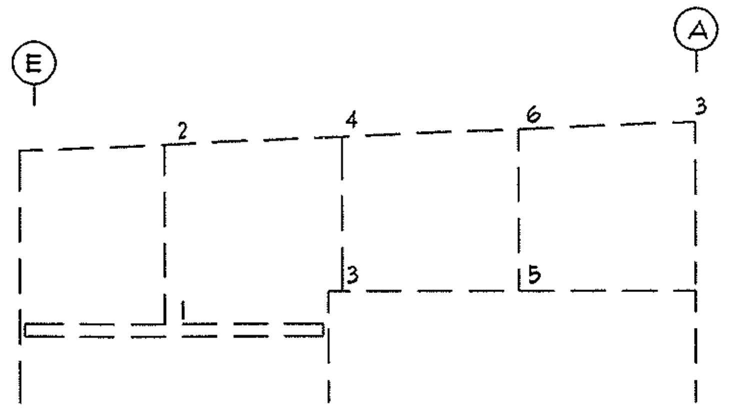
36506-AAB2.DWG



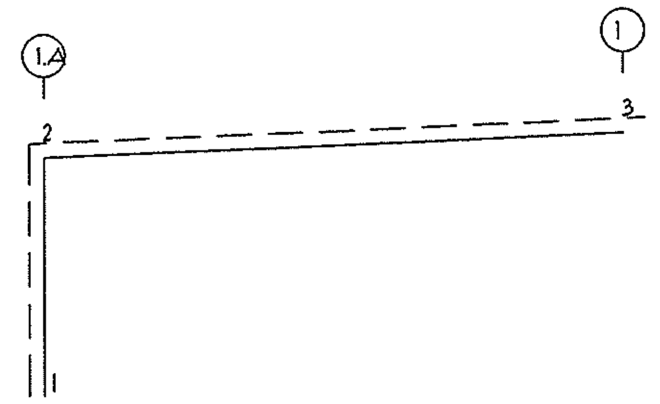
REACTION FOR SET# 4 FRAME LINE 1					
NO.	LOAD CASE / CONDITION	V1	H1	V4	H4
1	1.0DL+1.0LL	19	3	24	-2
2	0.6DL + 1.0LWP	-26	-11	-20	-12
3	0.6DL + 1.0LWS	-13	-11	-1	-6
4	0.6DL + 1.0RWP	-1	7	-38	22
5	0.6DL + 1.0RWS	-2	3	-12	14
6	0.6DL + 1.0LEP	-19	3	-35	10
7	0.6DL + 1.0REP	-3	7	-32	11
8	1.0DL+1.0LL+1.0MDL+1.0MLL	24	3	24	-2
9	1.0DL+1.0MDL+1.0MLL+0.75LL+0.75LWP	-3	-6	2	-11
10	1.0DL+1.0MDL+1.0MLL+0.75LL+0.75RWP	16	8	-11	15
11	1.0DL+1.0MDL+1.0MLL+0.75LL+0.75LEP	3	4	-9	5
12	1.0DL+1.0MDL+1.0MLL+0.75LL+0.75REP	14	7	-7	6



REACTION FOR SET# 3 FRAME LINE 2					
NO.	LOAD CASE / CONDITION	V1	H1	V4	H4
1	1.0DL+1.0LL	24	0	21	0
2	0.6DL + 1.0LWP	-21	-4	-18	-2
3	0.6DL + 1.0LWS	-9	-10	-6	-9
4	0.6DL + 1.0RWP	-16	9	-23	11
5	0.6DL + 1.0RWS	-4	3	-11	3
6	0.6DL + 1.0LEP	-27	12	-26	14
7	0.6DL + 1.0REP	-24	12	-29	14

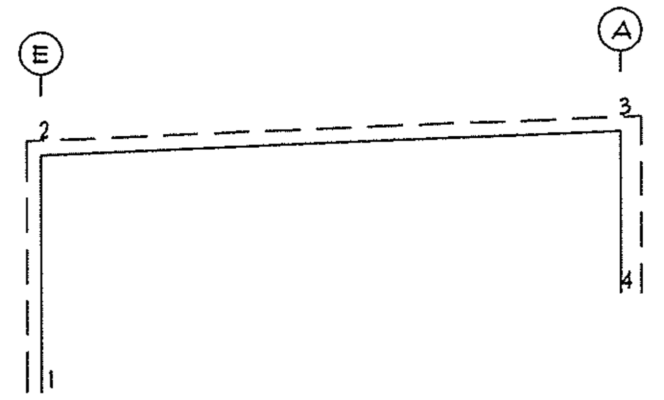


LEW COLUMN REACTION							
NO.	LOAD CASE / CONDITION	V1	H1	V3	H3	V5	H5
1	DL	1	0	1	0	1	0
2	WLP	0	-6	0	-6	0	-6
3	WLS	0	8	0	8	0	8

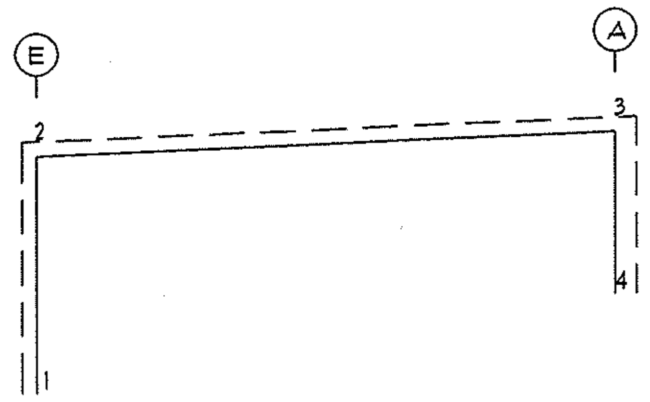


LINE D

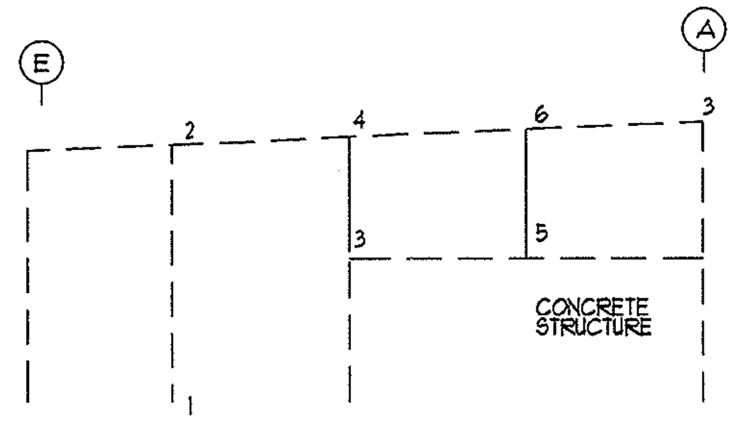
REACTION FOR SET# 2 @GL/s D					
NO.	LOAD CASE / CONDITION	V1	H1	V3	H3
1	1.0DL+1.0LL	5	0	5	0
2	0.6DL + 1.0LWP	-5	1	-4	3
3	0.6DL + 1.0LWS	2	-7	0	-7
4	0.6DL + 1.0RWP	-5	7	-4	8
5	0.6DL + 1.0RWS	2	-1	0	-1
6	0.6DL + 1.0LEP	-7	8	-4	10
7	0.6DL + 1.0REP	-6	8	-5	10
8	0.6DL + 1.0SBD + 1.0SBA + 0.09LL	3	0	5	0
9	1.0DL + 0.75LWS + 0.75SBD + 0.75SBA	3	-5	3	-5
10	1.0DL + 0.75RWS + 0.75SBD + 0.75SBA	3	-1	3	-1



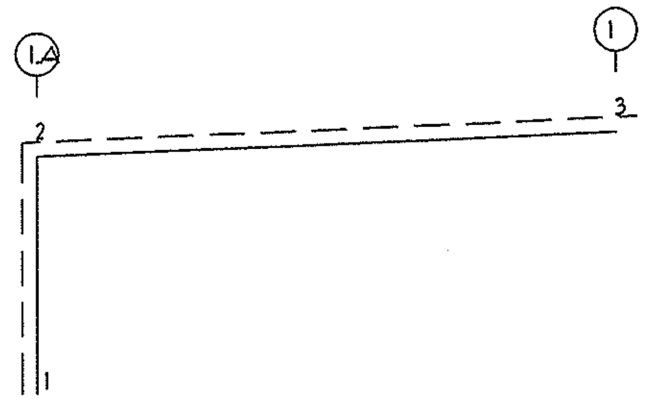
REACTION FOR SET# 2 FRAME LINE 3					
NO.	LOAD CASE / CONDITION	V1	H1	V4	H4
1	1.0DL+1.0LL	21	4	28	-3
2	0.6DL + 1.0LWP	-21	-9	-20	-11
3	0.6DL + 1.0LWS	-9	-11	2	-6
4	0.6DL + 1.0RWP	-4	7	-37	19
5	0.6DL + 1.0RWS	1	2	-9	13
6	0.6DL + 1.0LEP	-21	5	-43	10
7	0.6DL + 1.0REP	-12	7	-39	8



REACTION FOR SET# 1 FRAME LINE 4					
NO.	LOAD CASE / CONDITION	V1	H1	V4	H4
1	1.0DL+1.0LL	19	5	17	-4
2	0.6DL + 1.0LWP	-21	-9	-10	-5
3	0.6DL + 1.0LWS	-12	-12	0	-6
4	0.6DL + 1.0RWP	-9	6	-21	15
5	0.6DL + 1.0RWS	0	3	-11	14
6	0.6DL + 1.0LEP	-21	5	-20	4
7	0.6DL + 1.0REP	-18	5	-23	3

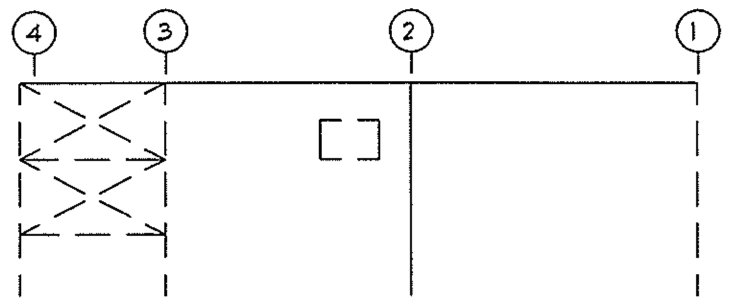


REW COLUMN REACTION							
NO.	LOAD CASE / CONDITION	V1	H1	V3	H3	V5	H5
1	DL	3	0	2	0	2	0
2	WLP	-4	-10	-3	-10	0	-6
3	WLS	4	12	3	12	0	8
4	LL	5	0	3	0	0	0
5	MDL	3	0	0	0	0	0
6	MLL	6	0	0	0	0	0
7	SBA	5	0	3	0	0	0



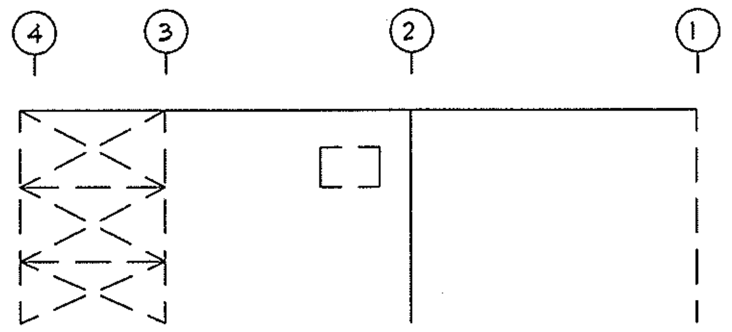
LINE C.E

REACTION FOR SET# 1 @GL/s C.E (HALF LOAD END FRAME)					
NO.	LOAD CASE / CONDITION	V1	H1	V3	H3
1	1.0DL+1.0LL	3	0	3	0
2	0.6DL + 1.0LWP	-3	0	-3	1
3	0.6DL + 1.0LWS	1	-4	-1	-4
4	0.6DL + 1.0RWP	-3	4	-3	6
5	0.6DL + 1.0RWS	1	0	0	0
6	0.6DL + 1.0LEP	-3	5	-2	6
7	0.6DL + 1.0REP	-3	5	-2	6
8	0.6DL + 1.0SBD + 1.0SBA + 0.09LL	2	0	3	0
9	1.0DL + 0.75LWS + 0.75SBD + 0.75SBA	2	-3	1	-3
10	1.0DL + 0.75RWS + 0.75SBD + 0.75SBA	2	0	1	0



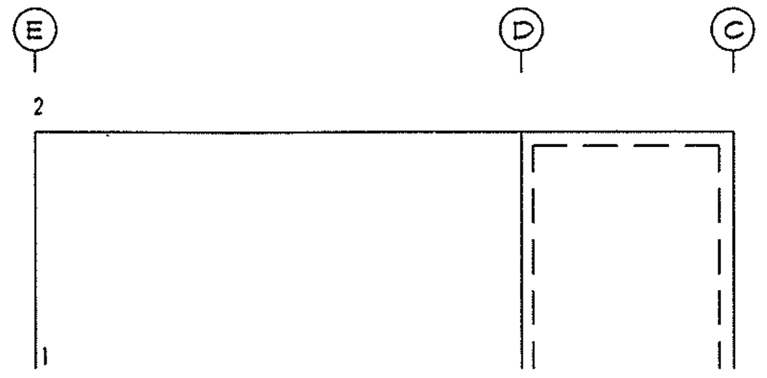
(@ LINE A)

BSW BRACING REACTIONS (FOR LEP/REP WIND LOAD CASES)				
NO.	BRACING TYPE	H	V	M
1	ROD @ BAY# 1	19	31	0



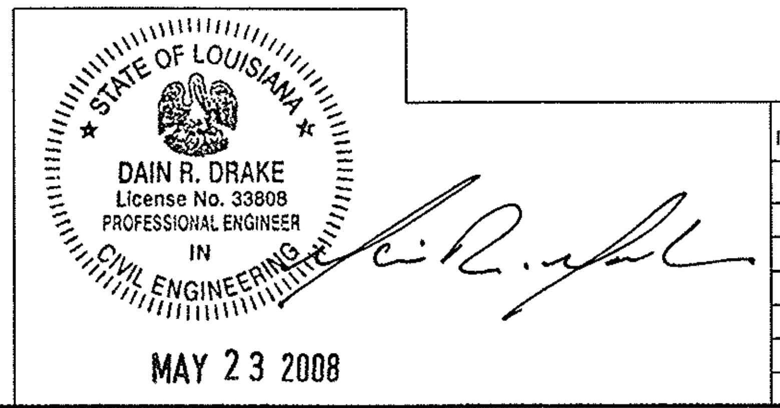
(@ LINE E)

FSW BRACING REACTIONS (FOR LEP/REP WIND LOAD CASES)				
NO.	BRACING TYPE	H	V	M
1	ROD @ BAY# 1	19	42	0



LINE 1.A

FSW BRACING REACTION (FOR LEP/REP WIND LOAD CASES)				
SET#	BRACING TYPE	H	V	M
1	Stitched Portal @ BAY#2	1	4	0



ISSUE	REV. DESCRIPTION	DATE	BY / DSN
0	PERMIT/CONST	5/15	KTL

U.S.A.

UNITED STRUCTURES OF AMERICA, INC.
1912 BUSCHONG
HOUSTON, TEXAS 77039
281-442-8247

DESCRIPTION: REACTION
CUSTOMER: CORSO FABRICATORS, INC
LOCATION: COVINGTON, LA
BUYER NO.: COVINGTON PICK UP STSTION

DRAWING BY	DATE	CHECK BY	DATE
CJ	5/15/08	KTL	5/15/08
JOB # 36506A	SCALE N.T.S.	DRAWING # F3 of 3	ISSUE 0

DRAWING2.DWG