

POST-TENSIONED CONCRETE

1. REQUIREMENTS FOR DESIGN, DETAILING, MATERIALS AND INSTALLATION BY THE POST-TENSIONING INSTITUTE, AMERICAN CONCRETE INSTITUTE AND THE INTERNATIONAL BUILDING CODE SHALL BE FOLLOWED.
2. PROVIDE COMPLETE DESIGN CALCULATIONS (WHERE APPLICABLE) AND SHOP DRAWINGS STAMPED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF GEORGIA.
3. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING INFORMATION:
 - A. SLAB LAYOUT & DIMENSIONS (INCLUDING OPENINGS)
 - B. POST-TENSIONING LAYOUT & DIMENSIONS
 - C. POST-TENSIONING ANCHORAGE COMPONENTS
 - D. CONVENTIONAL REINFORCEMENT DETAILS
 - E. SHEAR HEAD REINFORCING DETAILS
 - F. CONNECTION DETAILS
 - G. STRESSING PROCEDURES & SEQUENCING
4. POST-TENSIONED CONCRETE SHALL BE UNBONDED SINGLE-STRAND TENDONS (UNO).
5. STRANDS SHALL BE ½" DIA, 7 - WIRE STRANDS CONFORMING TO ASTM A-416, GRADE 270 KSI.
6. STRANDS SHALL BE COATED WITH RUST PREVENTIVE GREASE AND ENCLOSED IN PLASTIC SHEATHING. DAMAGED SHEATHING MUST BE REPAIRED PRIOR TO CONCRETE PLACEMENT. UNSHEATHED STRAND SEGMENTS ARE NOT PERMITTED.
7. POST-TENSION SLABS SHALL BE REGULAR WEIGHT CONCRETE (145 - 150 PCF) HAVING A 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI, UNO.
8. STRESSING OF POST-TENSIONED CONCRETE SHALL BE EXECUTED WHEN TESTS VERIFY THAT POURED CONCRETE HAS ACHIEVED A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI.
9. TENDONS SHALL BE PLACED IN SMOOTH PARABOLIC CURVES BETWEEN HIGH AND LOW POINTS, UNLESS NOTED OTHERWISE.
10. NON-POST-TENSIONING REINFORCEMENT AND NON-STRUCTURAL SLAB EMBEDMENTS SHALL IN NO CASE INTERFERE OR DISPLACE POST-TENSIONING REINFORCEMENT WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD.
11. POST-TENSIONING FORCES ON PLANS ARE FINAL AFTER ALL LOSSES.
12. CORING OR CUTTING OF SLABS OR USE OF POST-INSTALLED ANCHORED ANCHORS IS NOT PERMITTED WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER OF RECORD.
13. TWISTING OF STRANDS WITHIN A BUNDLE OR BEAM IS NOT PERMITTED.
14. CONTRACTOR SHALL SUBMIT REQUEST FOR INFORMATION TO ARCHITECT/ENGINEER IN THE EVENT OF TENDON BREAKAGE OR BLOWOUTS AND SHALL CARRY OUT ALL RESULTING REMEDIAL ACTIONS.
15. TENDONS WITH FORCES SHOWN ON PLANS NOTED IN KIPS SHALL BE PLACED UNIFORMLY IN A BAND WIDTH ON EACH SIDE OF COLUMN. PLACE A MINIMUM OF 3 TENDONS THROUGH COLUMN WITH ONE-HALF OF REMAINING TENDONS PLACED ON EACH SIDE OF COLUMN.
16. TENDONS WITH FORCES SHOWN ON PLANS NOTED IN KIPS PER FOOT (KLF) SHALL BE PLACED UNIFORMLY BETWEEN INDICATED WIDTHS, WITH A MINIMUM OF 2 TENDONS PLACED THROUGH EACH COLUMN.
17. TENDON CENTER OF GRAVITY SHALL BE MID-DEPTH OF SLAB AT DEAD ENDS AND STRESSING ENDS, UNLESS NOTED OTHERWISE.
18. TENDON CENTER OF GRAVITY AT HIGH POINTS SHALL BE AT CENTER LINE OF INTERIOR SUPPORT (SUCH AS COLUMNS AND GIRDERS), UNLESS NOTED OTHERWISE. UNIFORM TENDONS SHALL BE LOCATED UNDER BANDED TENDONS AT COLUMNS AND AS NOTED AT OTHER SUPPORTS.
19. TENDON LOW POINT SHALL BE AT MID-SPAN, UNO.
20. CONTRACTOR SHALL SUBMIT ALL STRESSING AND ELONGATION RECORDS TO POST-TENSION SUPPLIER AND STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL. DO NOT CUT TENDON TAILS. STRIP FROM WORK OR POUR ADJACENT SLABS UNTIL APPROVAL HAS BEEN OBTAINED FROM BOTH.
21. TENDON SUPPORT BARS AND BOLSTERS SHALL BE SECURED TO FORMWORK TO PREVENT MOVEMENT OR DISPLACEMENT OF TENDONS DURING CONCRETE PLACEMENT.
22. EXPOSED SEGMENT OF TENDON AND GRIPPING PART OF ANCHORAGE SHALL BE COATED WITH AN EPOXY-RESIN COMPOUND BEFORE BEING DRY-PACKED WITH A NON-SHRINK GROUT.

MASONRY

1. ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530-08, ASCE 5-08, TMS 402-08 AND ACI 530.1-08, ASCE 6-08, TMS 602-08.
2. MASONRY SHALL BE MEDIUM WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH, f'_m , OF 1500 PSI BASED ON NET CROSS SECTIONAL AREA. MORTAR SHALL CONFORM TO ASTM C270 TYPE S OR M. GROUT SHALL CONFORM TO ASTM C478, WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI.
3. REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE.
4. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED LADDER TYPE FABRICATED UNITS WITH A SINGLE PAIR OF # GAGE SIDE RODS AND # GAGE CROSS RODS FABRICATED FROM COLD DRAWN STEEL WIRE COMPLYING WITH ASTM A62. JOINT REINFORCING SHALL BE SPACED AT 16" OC VERTICALLY IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE.
5. ALL REINFORCED CELLS AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
6. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
7. REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
8. VERTICAL BARS SHALL BE HELD IN POSITION WITH PRE-MANUFACTURED TIES AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING NOR 10 FEET.
9. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 2½" x 3".
10. GROUTING SHALL BE STOPPED 1½" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
11. GROUTING SHALL BE CONSOLIDATED USING A MECHANICAL VIBRATOR PER REQUIREMENTS STATED IN ACI 530-05.
12. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
13. WALLS SHALL BE GROUTED USING LOW LIFT GROUTING TECHNIQUES.
14. ALL MASONRY WALLS SHALL BE ASSUMED TO BE RUNNING BOND, UNLESS NOTED OTHERWISE IN PLAN OR SECTION.

WOOD FRAMING

1. ALL WOOD DESIGN AND CONSTRUCTION SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (ANSI/APA NDS-2005) AND RELATED SUPPLEMENTS.
2. UNLESS NOTED, USE SPRUCE PINE FIR (E=1400 KSI MIN), 19% MAX MOISTURE CONTENT, AS FOLLOWS:

BEAMS & HEADERS	No 2
LOAD BEARING STUDS	No 2
NON-LOAD BEARING STUDS (EXTERIOR)	No 2
NON-LOAD BEARING STUDS (INTERIOR)	STUD GRADE
JOISTS & PURLINS	No 2
PLATES, BLOCKING & SUB-PURLINS	No 2
3. ALL WOOD IN CONTACT WITH CONCRETE, MASONRY OR SOIL OR PERMANENTLY EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
4. AT STUD WALL OPENINGS, THE TOTAL NUMBER OF DISPLACED AND/OR CUT STUDS SHALL BE INSTALLED AND ATTACHED TO THE JAMBS, ONE-HALF OF THE TOTAL TO EACH SIDE OF THE OPENING (TOTAL NUMBER INCLUDING JACK AND KING STUDS.)
5. METAL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS SO THAT THE MAXIMUM PUBLISHED CAPACITY IS DEVELOPED.
6. WHERE NO CONNECTION IS INDICATED ON THE DRAWINGS, ATTACHMENT SHALL BE MADE IN ACCORDANCE TO TABLE 2304.9.1 FASTENING SCHEDULE IN THE INTERNATIONAL BUILDING CODE.
7. ALL CONNECTORS SHALL BE G90 GALVANIZED STEEL, EXCEPT CONNECTORS IN CONTACT WITH PRESSURE TREATED, FIRE-RETARDANT OR WOLMANIZED WOOD SHALL BE COATED WITH G185 ZINC COATING.
8. FURNISH BOLTS AND ANCHOR RODS WITH STANDARD NUT WASHER.
9. TOE NAILS SHALL BE DRIVEN AT A 30° ANGLE RELATIVE TO PIECE. START NAIL AT ONE-THIRD NAIL LENGTH FROM THE END OF PIECE.
10. ALL LOAD BEARING STUD WALLS (INTERIOR & EXTERIOR) SHALL HAVE CONTINUOUS HORIZONTAL BLOCKING AT 4'-6" OC (MAX) VERTICALLY PRIOR TO APPLYING ANY LOADS (INCLUDING FRAMING FOR FLOORS ABOVE).
11. STUD PACK OR SOLID SAWN WOOD COLUMNS SHALL BE CONTINUOUS FROM LOCATION SHOWN TO THE FOUNDATION. BLOCK FLOOR CAVITY SOLID BELOW WOOD COLUMN (WIDTH EQUAL TO WOOD COLUMN) TO ACHIEVE CONTINUITY.
12. STRUCTURAL ELEMENTS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING UNLESS METAL OR WOOD SIDE PLATES ARE PROVIDED TO STRENGTHEN THE MEMBER. PENETRATIONS IN FLOOR AND WALL SHEATHING IS PERMITTED PROVIDED THAT 2x BLOCKING IS INSTALLED AT OPENING PERIMETER (FOR OPENINGS LARGER THAN 10" IN LENGTH/DIAMETER) AND WALL FRAMING IS NOT INTERRUPTED.
13. WALL SHEATHING NOTED ON STRUCTURAL DRAWINGS SHALL BE ATTACHED DIRECTLY TO THE FACE OF FRAMING MEMBERS. SEE ARCHITECTURAL DRAWINGS FOR ALL NON-STRUCTURAL SHEATHING REQUIREMENTS. WHERE ARCHITECTURAL DRAWINGS REQUIRE ADDITIONAL SHEATHING, SUCH SHEATHING SHALL BE ATTACHED TO THE OUTSIDE FACE OF STRUCTURAL SHEATHING.
14. PROVIDE ONE ROW OF BRIDGING FOR EACH 8'-0" LENGTH OF ROOF FRAMING MEMBERS.

PRE-ENGINEERED OPEN WEB TRUSSES

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (ANSI/APA NDS - 2005) & RELATED SUPPLEMENTS.
2. WOOD TRUSSES SHALL BE FULLY DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE LOAD CONDITIONS AND DESIGN PARAMETERS INDICATED ON THE STRUCTURAL DRAWINGS. THE MANUFACTURER'S ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN, ADEQUACY AND SAFETY OF ALL WOOD TRUSSES.
3. MANUFACTURER SHALL SUBMIT DESIGN CALCULATIONS AND SHOP DRAWINGS BEARING SEAL AND SIGNATURE OF MANUFACTURER'S ENGINEER WITH ENGINEER'S SEAL FOR PROJECT STATE. SHOP DRAWINGS SHALL INCLUDE LAYOUT AND DETAIL NECESSARY FOR DETERMINING FIT AND PLACEMENT OF TRUSS IN THE BUILDING, INCLUDING SUPPORT ATTACHMENTS, CONFORM TO INTERNATIONAL BUILDING CODE SECTION 2303.4 FOR SUBMITTALS.
4. UNLESS NOTED OTHERWISE, WOOD TRUSSES SHALL BE DESIGNED AS SIMPLY SUPPORTED UNIFORMLY LOADED TRUSSES WITH THE TOP CHORD BRACED AGAINST LATERAL BUCKLING.
5. TRUSS MANUFACTURER SHALL DETERMINE AND PROVIDE ALL NECESSARY END BLOCKING AND/OR BRIDGING BETWEEN TRUSSES.
6. GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL, ELECTRICAL AND PLUMBING COMPONENTS WITH WOOD TRUSS MANUFACTURER. OPENINGS AND ADDITIONAL LOADS REQUIRED FOR PLACEMENT OF THESE COMPONENTS SHALL BE INCORPORATED INTO DESIGN AND COORDINATED ON SHOP DRAWINGS. NO NOTCHES, CUTS OR OTHER MODIFICATIONS MAY BE MADE TO WOOD TRUSS IN FIELD WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.
7. MINIMUM TRUSS CHORD MEMBERS SHALL BE 2x4.
8. WOOD FLOOR TRUSSES SHALL BE DESIGNED FOR A LIVE LOAD DEFLECTION OF SPAN / 480 AND A DEAD PLUS LIVE LOAD DEFLECTION OF SPAN / 360.
9. WOOD ROOF TRUSSES SHALL BE DESIGNED FOR A LIVE LOAD DEFLECTION OF SPAN / 360 AND A DEAD PLUS LIVE LOAD DEFLECTION OF SPAN / 240.
10. WHERE CONCENTRATED LOADS OR OTHER NON-TYPICAL LOADS ARE INDICATED, GENERAL CONTRACTOR SHALL INSURE LOAD IS APPLIED AT TRUSS PANEL POINT, OR ACCEPTABLE DISTANCE FROM PANEL POINT TO BE DETERMINED BY TRUSS DESIGNER. WHERE THIS DISTANCE IS EXCEEDED, GENERAL CONTRACTOR SHALL REINFORCE TRUSS PER DIRECTION AND APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND THE MANUFACTURER'S ENGINEER.

PLYWOOD FLOOR DECKING

1. USE ¾" APA - CDX TONGUE AND GROOVE DECKING (RATED 48/24).
2. CONTINUOUS PANEL JOINTS ARE NOT PERMITTED. LONG SIDE OF PANEL SHALL BE PERPENDICULAR TO THE SUPPORT WITH THE SHORT SIDE OF THE PANEL ALIGNED WITH THE CENTER OF THE TWO ADJACENT PANELS.
3. ATTACHMENT OF PANEL TO WOOD FRAMING MEMBERS SHALL BE 10d NAILS AT THE FOLLOWING SPACING, UNLESS NOTED OTHERWISE:
 - 4" AT FLOOR PERIMETER
 - 6" AT PANEL EDGES
 - 12" AT INTERMEDIATE SUPPORTSGLUE DECKING TO SUPPORTING FLOOR FRAMING.

PLYWOOD ROOF DECKING

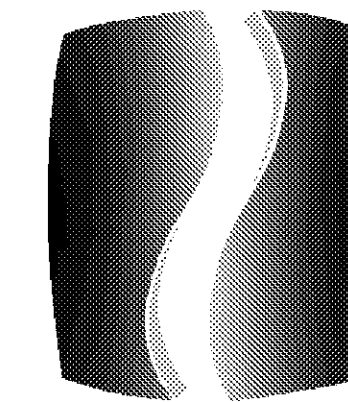
1. DECKING SHALL BE ½" APA-CDX RATED SHEATHING 32/16 (EXPOSURE 1).
2. ORIENT LONG SIDE OF PANEL PERPENDICULAR TO SUPPORT. END JOINT SHALL BE ALIGNED WITH THE MIDPOINT OF THE TWO ADJACENT PANELS. NO CONTINUOUS PANEL JOINTS ARE PERMITTED. PANELS SHALL BE CONTINUOUS OVER TWO OR MORE SPANS (NO SINGLE SPAN CONDITIONS).
3. ATTACHMENT OF PANEL TO WOOD FRAMING MEMBERS SHALL BE 8d NAILS AT THE FOLLOWING SPACINGS, UNLESS NOTED OTHERWISE:
 - 4" AT ROOF PERIMETER
 - 6" AT PANEL EDGES
 - 12" AT INTERMEDIATE SUPPORTS
4. EDGE SUPPORTS SHALL BE PROVIDED AS RECOMMENDED BY THE AMERICAN PLYWOOD ASSOCIATION (APA) BY USE OF PANEL CLIPS OR WOOD BLOCKING BETWEEN JOISTS OR TRUSSES. PANEL END JOINTS SHALL OCCUR OVER FRAMING. PANELS SHALL BE BLOCKED AT PERIMETER OF ROOF AND AT DIRECTIONAL CHANGES.

WOOD UPLIFT LOAD RESISTING SYSTEM

1. THESE NOTES ARE INTENDED TO SUPPLEMENT THE STRUCTURAL PLANS, DETAILS AND WALL ELEVATIONS. WHERE CONFLICT EXISTS, PLANS, DETAILS, AND WALL ELEVATIONS SHALL GOVERN. CONNECTIONS BELOW ARE MINIMUM AND DO NOT RELIEVE PRE-ENGINEERED TRUSS ENGINEER FROM CONNECTION DESIGN.
2. ROOF TRUSSES, TRUSS GIRDERS, JOISTS AND BEAM TO WALL CONNECTIONS:
 - A. TYPICAL TRUSSES AND JOISTS: 30' SPAN OR LESS: (1) SIMPSON H2.5A EA END GREATER THAN 30' SPAN: (2) SIMPSON H2.5A EA END
 - B. 2 SPAN TRUSSES AND JOISTS: (2) SIMPSON H2.5A AT EACH END AND AT INTERMEDIATE SUPPORTS (ONE EACH SIDE)
 - C. HEADERS & BEAMS: (1) SIMPSON HTS20 OR CS18 EACH END
3. TOP PLATES TO FLOOR STUDS: (1) SIMPSON H6 @ 32" OC
4. GROUND FLOOR STUDS TO MUD SILL PLATE: (1) SIMPSON H4 AT 2'-0" OC FOR ROOF BEARING WALLS.
5. FASTEN ALL ROOF HEADERS UP TO 8'-0" SPAN W/ (1) CS18 STRAP TO JACK STUDS EA END AND PROVIDE (1) CS18 FLOOR TO FLOOR STRAP AT KING STUDS BELOW.
6. FASTEN ALL ROOF HEADERS UP TO 10'-0" SPAN W/ (1) CS16 STRAP TO JACK STUDS EA END AND PROVIDE (1) CS16 FLOOR TO FLOOR STRAP AT KING STUDS BELOW.
7. WHERE WALL OPENING IS GREATER THAN 5'-0" WIDE, PROVIDE (1) CS18 STRAP AT MID SPAN OF OPENING HEADER. STRAP SHALL EXTEND TO BOTTOM OF HEADER ON EA SIDE.
8. PROVIDE (1) CS18 STRAP @ 48" OC THROUGH ALL FLOOR CAVITIES AT ROOF BEARING WALLS.

POST-INSTALLED ANCHORS

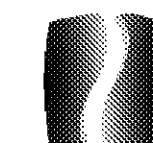
1. POST-INSTALLED ANCHORS SHALL INCLUDE ADHESIVE AND MECHANICAL ANCHORS.
2. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
3. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS.
4. ANCHORS SHALL BE PER MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE.
5. SPECIAL INSPECTION SHALL BE PROVIDED FOR ALL ADHESIVE AND MECHANICAL ANCHORS. SPECIAL INSPECTOR SHALL PERFORM INSPECTIONS PURSUANT TO CONTRACT DOCUMENTS, SCHEDULE OF SPECIAL INSPECTIONS AND APPROPRIATE ICC-ES REPORTS.
6. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER FOR ACCEPTANCE. PRODUCT ICC-ES CODE REPORTS SHALL BE INCLUDED WITH SUBMITTAL PACKAGE.
7. UNLESS NOTED OTHERWISE ON PLANS, ACCEPTABLE PRODUCTS SHALL BE:
 - A. ADHESIVE ANCHORS
 - I. FOR ANCHORING INTO CONCRETE AND GROUT-FILLED CMU
 - SIMPSON STRONG-TIE: ACRYLIC-TIE WITH ASTM A307 THREADED RODS
 - HILTI: HIT-CE/HT-HY 150 MAX WITH STANDARD HAS-E THREADED RODS
 - II. FOR ANCHORING INTO HOLLOW BASE MATERIAL (HOLLOW CMU)
 - SIMPSON STRONG-TIE: ACRYLIC-TIE AND SCREEN TUBES (ATSP OR ETSF) WITH ASTM A36 THREADED RODS
 - HILTI: HIT-HY 70 AND SCREEN TUBES WITH STANDARD HIT-A OR HAS-E THREADED RODS
 - B. MECHANICAL ANCHORS
 - I. FOR ANCHORING INTO CONCRETE
 - SIMPSON STRONG-TIE: TITEN-HD (SCREW ANCHOR) OR STRONG-BOLT (WEDGE ANCHOR)
 - HILTI: HUS-H (SCREW ANCHOR) OR HDA UNDERCUT ANCHOR (WEDGE ANCHOR)
 - II. FOR ANCHORING INTO GROUT-FILLED CMU
 - SIMPSON STRONG-TIE: TITEN-HD (SCREW ANCHOR) OR WEDGE-ALL (WEDGE ANCHOR)
 - HILTI: HUS-H (SCREW ANCHOR) OR KWIK BOLT 3 (WEDGE ANCHOR)
 - III. FOR ANCHORING INTO HOLLOW BASE MATERIAL (HOLLOW CMU)
 - SIMPSON STRONG-TIE: TITEN HD (SCREW ANCHOR)
 - HILTI: HLC SLEEVE ANCHOR (WEDGE ANCHOR)
8. CONTACT ANCHOR MANUFACTURER FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY.
 - SIMPSON STRONG-TIE (800) 999-5099
 - HILTI (800) 879-8000



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SEAL

ISSUE & REVISION RECORD

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PROJECT

**SPRINGS AT
FREMAUX
TOWN CENTER**
SLIDELL, LA

CLIENT

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DRAWING TITLE

GENERAL NOTES

SHEET NUMBER

S0.2

NORTH TERRACES
400 PERIMETER CENTER TERRACE
SUITE 650
ATLANTA, GEORGIA 30346