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March 26, 2008

URS GROUP, INC.
7650 West Courtney Campbell Causeway
Tampa, FL 33607

ATTENTION: Mr. Fred Walker, P.E.

REFERENCE: Proposed Riverine Training Facility – Preferred Site
Project No. P-210
Stennis Space Center
Hancock County, Mississippi
SESI Project M08-028

Dear Mr. Walker:

Southern Earth Sciences, Inc., has completed the subsurface exploration and engineering studies and analyses for the above referenced project. Authorization to proceed was given us by Ms. Kathy Hartman. The following report provides a discussion of the development, exploration procedures used, subsurface conditions encountered, considerations for site preparation, and foundation recommendations.

SITE DESCRIPTION

The area is bound by Lower Gainesville Road to the north, a wooded area to the south, a utility storage area to the east and an unpaved access road to the west. At the time of our investigation we noted the site was heavily wooded with moderate underbrush. The topography varied approximately from Elevation 7.0 to Elevation 22.0, based on a provided topographic drawing. A geographic map of the area is attached in Appendix A.

GEOLOGIC SETTING

The site is located in the southern, lower, Gulf Coastal Plain physiographic province of Mississippi. Soils encountered and sedimentary deposits are primarily from the Holocene period, and consist of loam, sands, gravels and clay of the Coastal Deposits formation. A geologic map of the area is included in Appendix A.

PROJECT INFORMATION

It is our understanding that the project will consist of several one- to two-story buildings of steel and masonry construction with concrete floor slabs on grade. We also understand that the finished floors of the structures are planned at approximately Elevation 20.0, which may require as much as 9 feet of fill in some areas. In addition, paved parking areas will be constructed adjacent to the proposed structures. For the purpose of this report, we have assumed design column loads of 100 kips and maximum wall loads of 8 kips per lineal foot, as no structural loads were provided.

FIELD INVESTIGATIVE PROCEDURES

On March 13 through 18, 2008, we performed seventeen (17) split-spoon sample soil borings with Standard Penetration Tests in locations as determined by our engineers and to depths as directed. The boring locations were staked in the field by our firm using a portable, hand-held, global-positioning receiver. The approximate locations of the borings are shown on a Test Location Plan included in Appendix A.

Boring RB-4, performed to a depth of 41½ for a previous exploration for this project, was in close vicinity of the Operations Building and its information was used for this project. A copy of the boring log is also attached in Appendix A.

The soil borings with Standard Penetration Tests (SPT) were advanced to depths of 16½ to 61½ feet using track-mounted, mud-rotary type drilling equipment. All samples obtained in the field were returned to our laboratory for visual classification in accordance with the Unified Soil Classification System and further testing. Logs for the soil borings are attached to this report in Appendix B.

LABORATORY TESTING

In order to define the soil properties more closely, classification tests were performed on selected samples obtained in the borings. Results of these tests are shown on the individual boring logs and a summary is attached in Appendix C.

SUBSURFACE CONDITIONS ENCOUNTERED

Subsurface conditions for each area will be discussed separately. All references to depth are made with respect to the existing ground surface at the time the fieldwork was performed.

Operations and Boat Maintenance Buildings

Subsurface conditions encountered in these borings varied and they will be addressed separately. All references to depth are made with respect to the existing ground surface at the times the borings were performed.

Boring No. B-1 showed very soft to soft clay to 4 feet, followed by loose sand to 7 feet, medium clay to 14 feet, loose to medium clayey sand to 23 feet, dense to very dense sand 38 feet, and loose sand to 43 feet. This was underlain by soft to medium clay to 55 feet, very dense sand to 58 feet, and soft clay to the depth of the boring at 61½ feet.

Boring No. B-2 exhibited very loose to medium sand to 4 feet, medium clay to 7 feet, medium sand to 33 feet and medium clay to the depth of the boring at 41½ feet.

Boring No. B-16 displayed soft to medium clay to 7 feet, followed by dense sand to 9½ feet, and loose to medium sand to the depth of the boring at 16½ feet.

Boring No. RB-4 revealed loose to medium sand and clayey sand to 7 feet, underlain by medium clay to 13 feet, medium to dense sand to 38 feet, and soft clay to the depth of the boring at 41½ feet.

10-Boat Storage

Boring Nos. B-3 and B-4 revealed loose to medium sand and medium to stiff clay to 4½ feet, underlain by medium sand and clay to the depth of the borings at 26½ feet.

6-Boat Storage

Boring No. B-5 showed loose to medium sand to 9½ feet, medium to very stiff clay to 13 feet, and medium to dense sand to the depth of the boring at 26½ feet.

Parking Areas

The remainder of the borings showed layers of sand and clayey sands with densities ranging from very loose to dense, alternating with strata of medium to hard consistency clays and silts. The only notable exceptions occurred in the following areas:

- Boring No. B-10, which showed a stratum of soft clay between depths of 7 to 10½ feet
- Boring No. B-15, which exhibited a layer of soft clay from 2 feet to 5½ feet

Ground Water Table

The ground water table varied from a depth of 0.9 feet in Boring No. B-13 to 11.0 feet below grade in Boring No. B-5. The most likely reasons for the variance are due to differences in elevation of the borings as well as ground water being trapped over a relatively impermeable layer, often referred to as a "perched" water table. It must be noted that fluctuations in the level of the ground water table may occur due to changes in rainfall, drainage, temperature and other factors not evident at the time measurements were made. Since the probability of such variations is anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on such assumptions of variation.

USDA NRCS SOILS CLASSIFICATION

Based on information obtained from the Natural Resource Conservation Service of the United States Department of Agriculture, the predominant soil classes at these sites are the Saucier fine sandy loam (SaB, SaC), Saucier-Susquehanna complex (ScD), and Smithton fine sandy loam (Su). Summaries of the soil surveys for these sites are included in Appendix D.

EXPANSIVE SOIL CHARACTERISTICS

With regards to Section 1802.3.2 of the 2003 Edition of the International Building Code (IBC), laboratory tests performed on clayey soils encountered at relatively shallow depths (less than 7½ feet in depth) do not exhibit the characteristics of expansive soils.

SEISMIC CONSIDERATIONS

Based on the subsurface conditions encountered, with reference to Table 1615.1.1 of the IBC, these sites would best be categorized as Site Class "E". It is noted, borings for this project were performed to a maximum depth of 61½ feet and the IBC normally requires soil information to a depth of 100 feet be used in site classification. In Section 1615.1.1, however, it states that appropriate soil properties can be estimated by a registered design professional if site-specific data is not available. Data available in our files from previous projects near this site was used for this purpose.

In accordance with Sections 1615.1.2, 1615.1.3, and 1615.1.4 of the IBC, design parameters were calculated for an earthquake having a 2% probability of exceedence in a 50-year period using seismic data available from the United States Geological Survey (USGS). The results of these calculations, expressed as a percent of the gravitational force (g) are as follows:

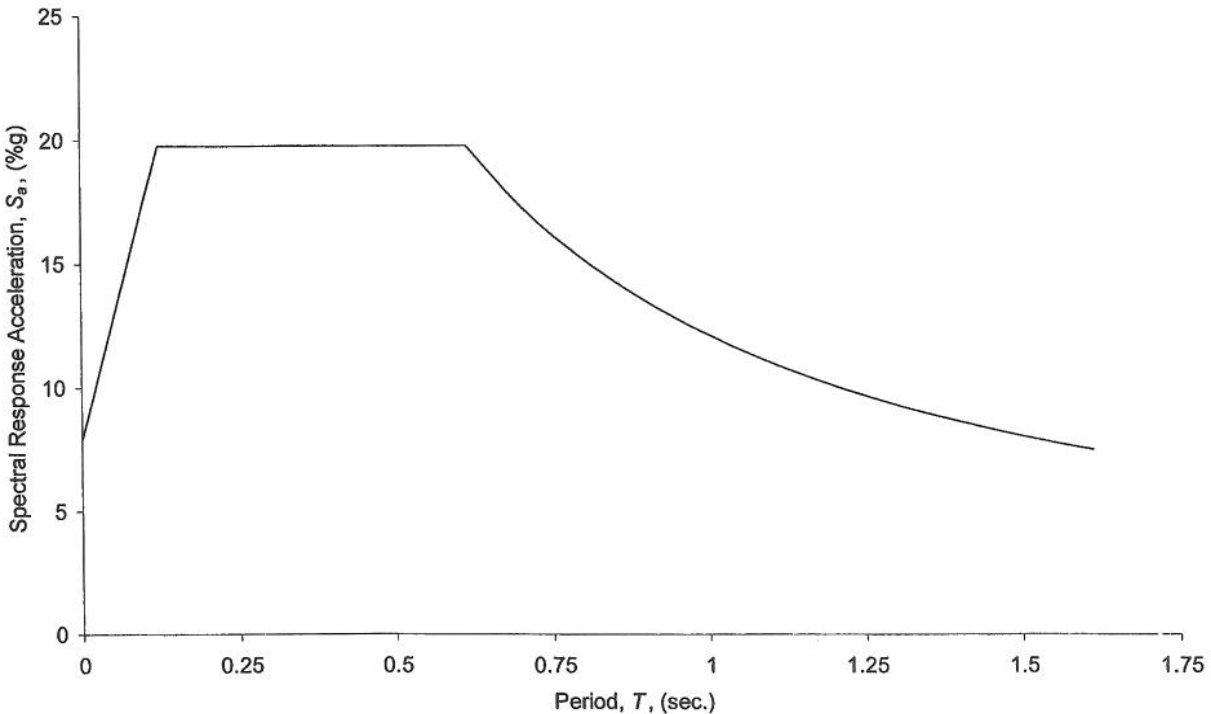
Five-Percent Damped Design Spectral Response Acceleration Parameters

Short Periods (0.2 sec) $S_{DS} = 19.77 \%g$

1-Second Periods $S_{D1} = 12.11 \%g$

Based on the IBC Site Class and the above seismic data, a typical spectral response curve for this site was developed and is shown below:

Design Response Spectrum



ENGINEERING ANALYSIS

A study of the subsurface conditions in the area of the planned structures shows that in most cases, soils of reasonable density and consistency exist to a depth of at least 35 feet. As previously mentioned, however, soft clays were discovered in Boring Nos. B-1, B-10, B-15 and B-16 at relatively shallow depths. Based on these conditions and the assumed design data, these structures could be supported on shallow foundations; however, remedial action will be required to mitigate the layers of potentially compressible clays found at shallow depths.

soft clay
B1 10 15
16

SUBGRADE PREPARATION

Because of the moderately low permeability of some of the shallow soils at this site, water will tend to collect in low areas. These materials, when wetted, will retain moisture for long periods, and these soils can undergo significant weakening if disturbed in the presence of excess moisture. For these reasons, design and implementation of a permanent grading and drainage plan is essential. In the initial stages of site development, effective drainage must be established and modified as necessary during construction. In areas where it will be necessary to undercut

grading plan

weaker soils, control of water and drainage is vital. Further, construction traffic, especially rubber-tired vehicles, should be minimized when the soils are wet.

Initially, the site should be cleared, grubbed, and stripped of all soils containing roots and organic matter. Topsoil and other organic laden soils obtained during site stripping operations could be stockpiled on site for reuse in landscaped areas and on the sides of roads.

Remedial Action

Operations and Boat Maintenance Buildings

In the footprint of these structures, we would recommend that the subgrade be undercut to a depth of at least four feet below original grade in order to remove the layers of soft clays and very loose clayey sands.

Other Areas

In the area of lower elevation to the south of the site, we would recommend that the potentially weak layers, as noted in Boring Nos. B-3, B-15, and B-17, be undercut to at least Elevation 4.0.

An approximation of the area requiring undercut is shown on the attached Test Location Plan; however the actual depth and lateral extent of these objectionable soils can only be accurately determined by careful field inspection at the time earthwork operations are performed. If desired, additional shallow borings could be performed to further isolate the areas requiring undercutting. If you would like to discuss this in more detail, please contact us.

The remaining surface soils should be proof rolled with a loaded, tandem-axle, dump truck in the presence of a geotechnical engineer or his representative to expose any soft or yielding areas. Where encountered, these soils should be excavated and replaced with thoroughly compacted backfill. The remaining surface soils should then be thoroughly and uniformly compacted in place. Thoroughly compacted backfill and fill should then be placed to the desired subgrade elevations.

Backfill and Fill

Backfill and Fill material should be non-plastic and granular in nature with a maximum of 20% passing the Wash 200 Sieve. It should be placed in thin successive layers 8" to 10" loose measurement and each layer should be compacted to at least 95% of its maximum laboratory dry density, within $\pm 2\%$ of its optimum moisture content, in accordance with ASTM D1557 (Modified Proctor). In-place field density tests should be performed as this material is being placed and compacted in order to insure that required density is being achieved. Since these testing services are within the scope of our activities, we urge that our firm be retained to assist you during the earthwork phase of this project.

RECOMMENDATIONS

Footings

We would recommend that footings be designed for a maximum net allowable unit soil pressure of 2,000 pounds per square foot in order to minimize post-construction settlement. Bearing depths should be a minimum of 18 inches below finished grade with a minimum width of 18 inches. Based on the assumed design data and this maximum soil pressure, total settlement of the foundations would not be expected to exceed 1 inch, approximately 50% of which could be differential between interior columns and load bearing walls.

Concrete Floor Slabs

After the subgrade has been prepared as outlined, conventionally designed concrete slabs could be placed on grade. These slabs should be appropriately jointed such that minor differential movement does not cause excessive distortion or damage. For design purposes, a modulus of subgrade reaction (*k*) of 200 pounds per cubic inch (pci) could be used.

PAVEMENT DESIGN DATA

A bulk samples was obtained for California Bearing Ratio (CBR) testing. In a sample obtained in the vicinity of Boring No. B-7, a CBR value of 8 was determined. For subgrade soils composed of fill materials placed and compacted as outlined above, a CBR value of 10 would apply.

Is must be noted; as discussed above, a layer of soft clay was encountered in Boring No. B-10 at a depth of 7 feet. Calculations indicate that if four feet of fill material are placed to achieve the final subgrade elevation, long-term settlements of 1/2 to 3/4 inches could occur. If this amount of settlement is unacceptable, design changes or remedial action will be necessary to mitigate this condition.

GENERAL COMMENTS

The soil samples obtained during the subsurface investigation will be retained the end of 2008. If no further instructions are received, they will be disposed of at that time.

While the borings are representative of subsurface conditions at their respective locations and for their respective vertical reaches, local variations characteristic of the subsurface materials of the region are anticipated and may be encountered. The boring logs and related information are based on the driller's logs and visual examination of selected samples in the laboratory. The delineation between soil types shown on the logs is approximate and the description represents our interpretation of subsurface conditions at the designated boring location and on the particular date drilled.

Professional judgments on design alternatives and criteria are presented in this report. These are based partly on our evaluations of technical information gathered, partly on our understanding of the characteristics of the project being planned, and partly on our general experience with subsurface conditions in the area. We do not guarantee performance of the project in any respect, only that our engineering works and judgments rendered meet the standard of care of our profession.

This report has been prepared in order to aid in the evaluation of this project and to assist the architects and engineers in the structural design. It is intended for use with regard to the specific project discussed herein and any substantial changes in the project, loads, locations, or assumed grades should be brought to our attention so that we may determine how such changes may affect our conclusions and recommendations. We would appreciate the opportunity to review the plans and specifications for construction to ensure that our conclusions and recommendations are interpreted correctly.

As the project geotechnical engineer of record that developed the foundation design recommendations, please be aware that we cannot accept responsibility for the performance of the foundation system if we are not afforded the opportunity to confirm that our recommendations have been followed. Accordingly, we recommend that Southern Earth Sciences, Inc. be retained on this project to perform observation and field-testing services during the construction phase of the foundation system.

This report is exclusively for the use and benefit of the addressee(s) identified on the first page of this report and is not for the use or benefit of, nor may it be relied upon by any other person or entity. The contents of this report may not be quoted in whole or in part or distributed to any person or entity other than the addressee(s) hereof without, in each case, the advance written consent of the undersigned.

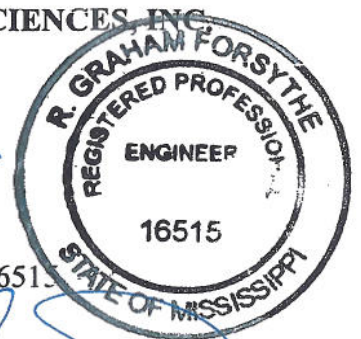
We appreciate the opportunity to serve you on this project. If you have any questions or if we may be of further assistance, please call at your convenience.

URS GROUP, INC.
SESI Project M08-028
March 26, 2008
Page 9

Very truly yours,

SOUTHERN EARTH SCIENCES, INC.


R. Graham Forsythe, P.E.
Registered, Mississippi 16515



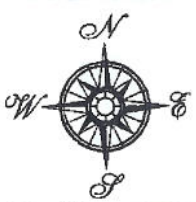
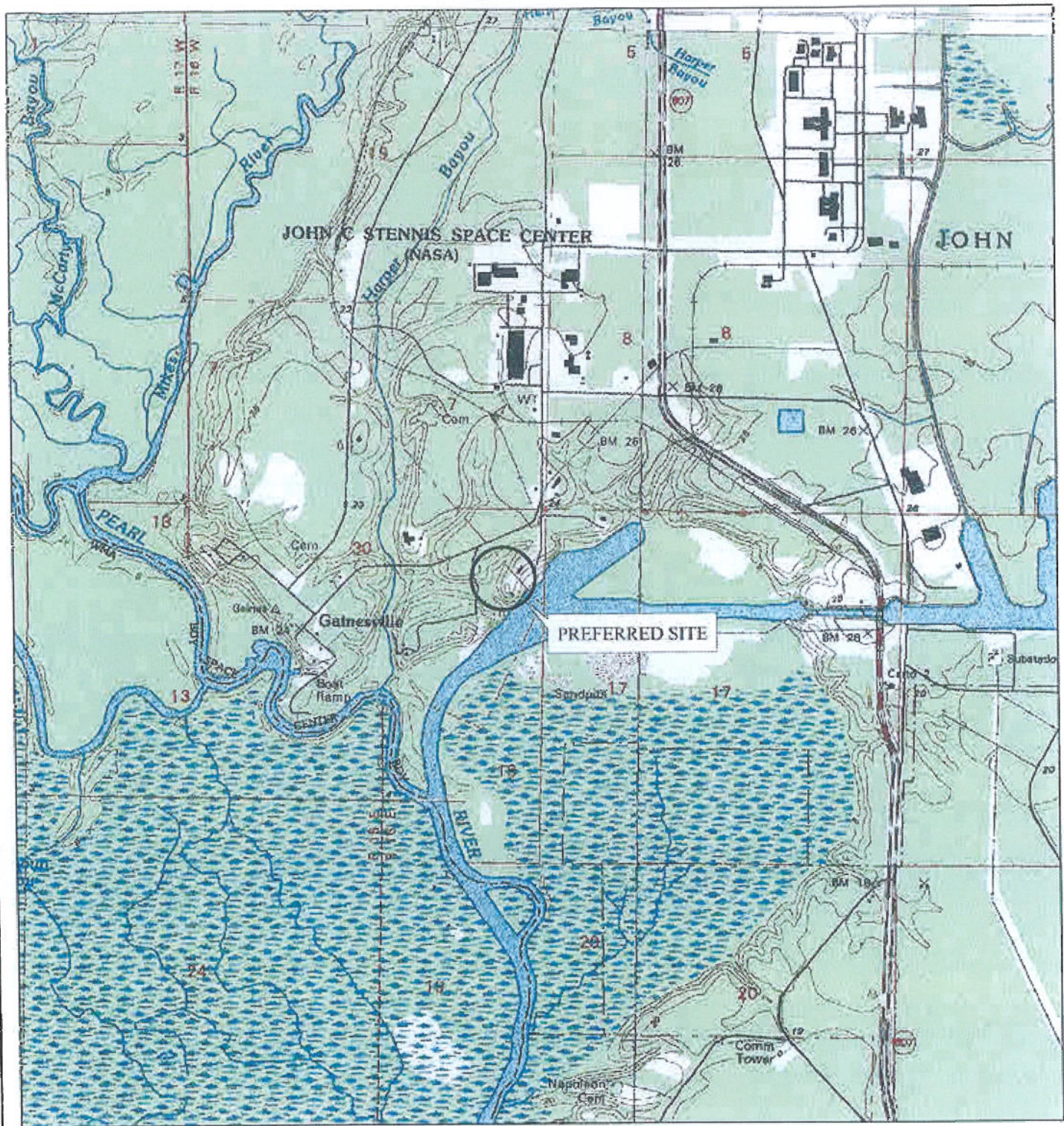

Lewis Copeland, Jr., P.E.
Registered, Mississippi 13312

RGF/LCJR/gf
Enclosures

APPENDIX A

TEST LOCATION PLAN

PROPOSED RIVERINE TRAINING FACILITY – PREFERRED SITE
URS PROJECT NO. P-210
STENNIS SPACE CENTER
HANCOCK COUNTY, MISSISSIPPI
SESI PROJECT NO. M08-028



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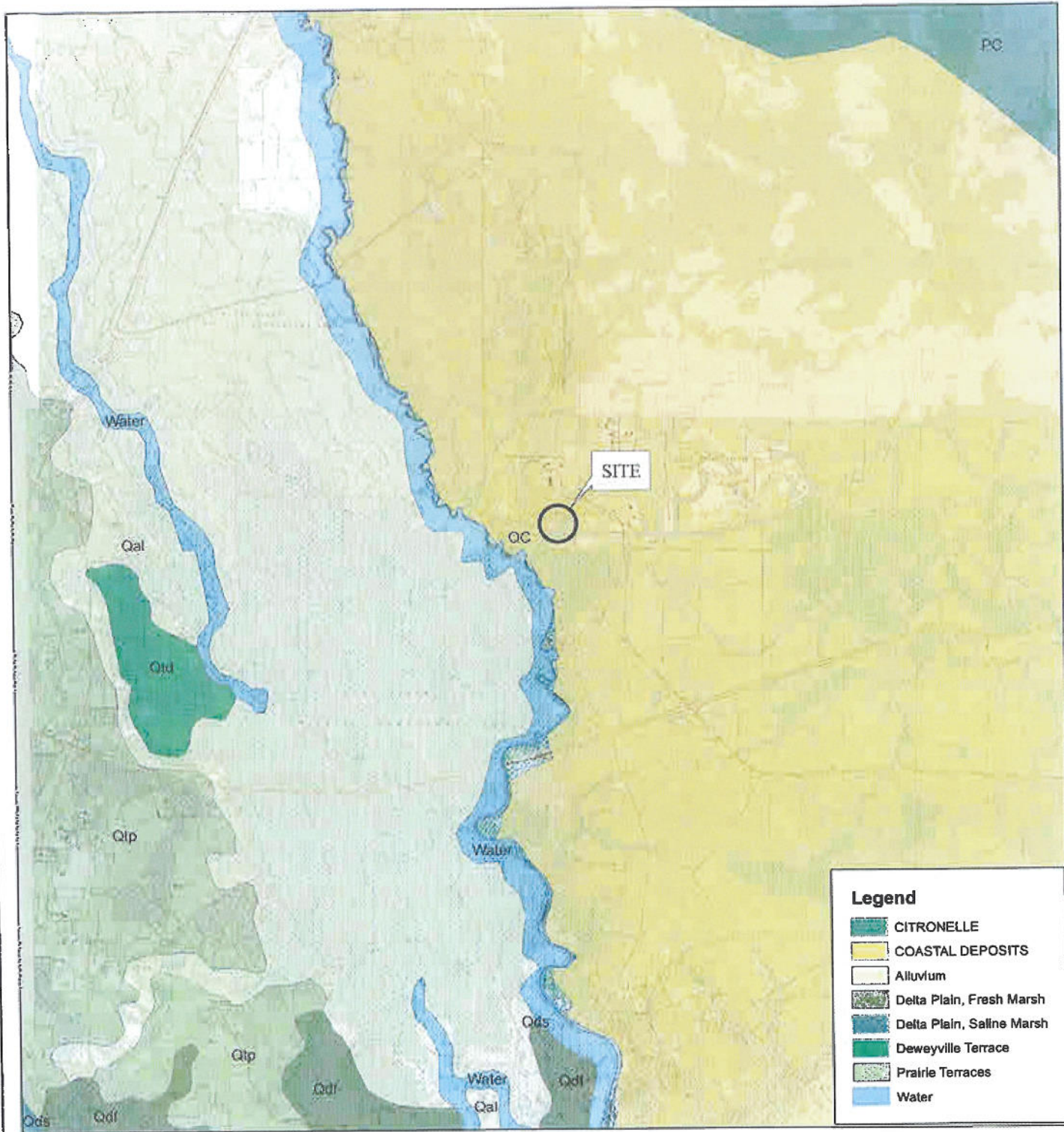
EXTRACTED FROM THE USGS PHOTOMOSAIC OF HANCOCK COUNTY

**RIVERINE TRAINING FACILITY
PREFERRED SITE
HANCOCK COUNTY, MISSISSIPPI**

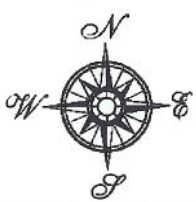
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**TOPOGRAPHIC MAP
SITE LOCATION
SESI JOB NO.: 08-028**



Legend	
	CITRONELLE
	COASTAL DEPOSITS
	Alluvium
	Delta Plain, Fresh Marsh
	Delta Plain, Saline Marsh
	Deweyville Terrace
	Prairie Terraces
	Water



1:120,000

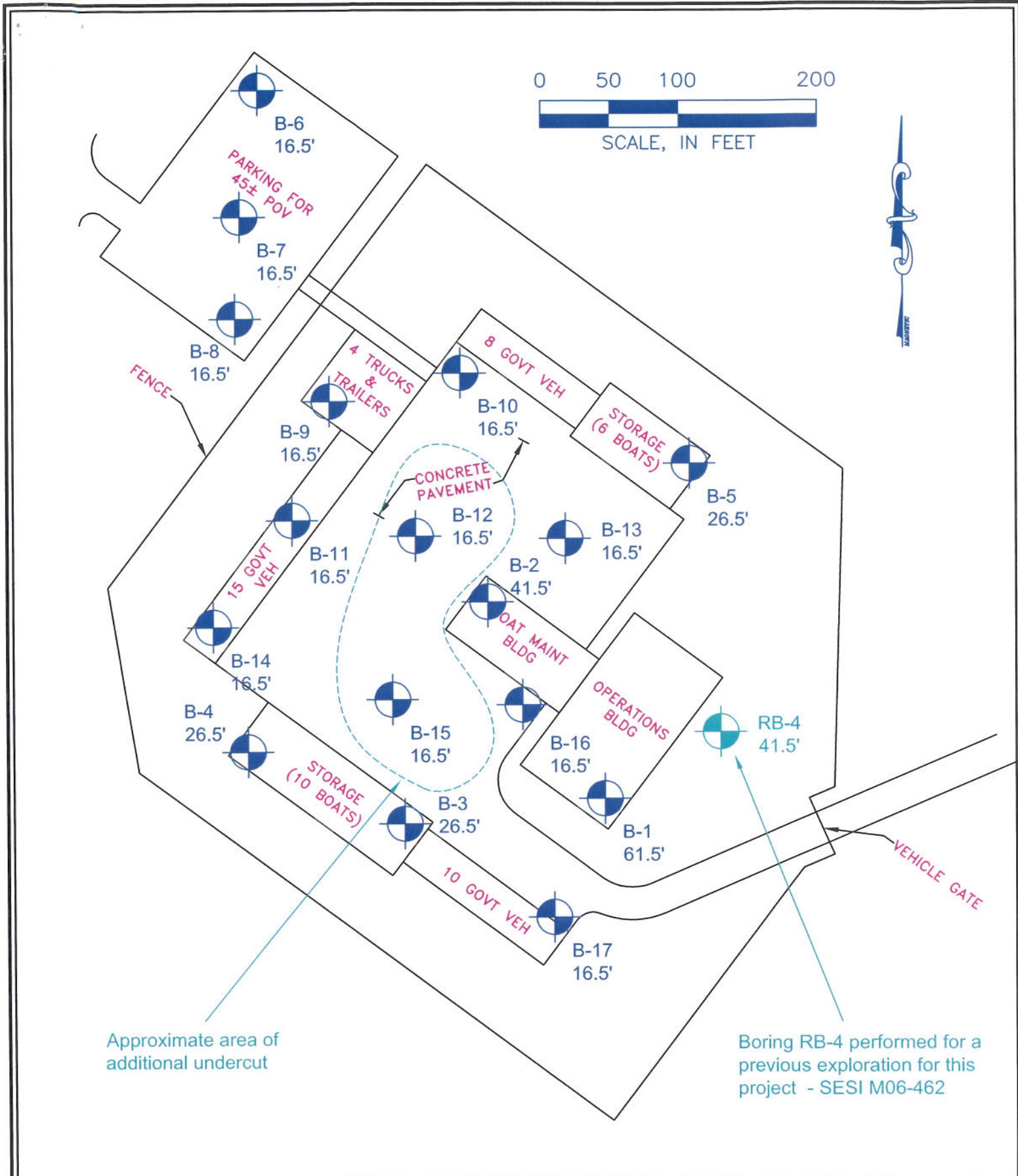


EXTRACTED FROM THE USGS PHOTOMOSAIC OF HANCOCK COUNTY
GEOLOGIC MAPS OF LA AND MS FROM THE USGS

**RIVERINE TRAINING FACILITY
PREFERRED SITE
HANCOCK COUNTY, MISSISSIPPI**



**GEOLOGIC MAP
SITE LOCATION
SESI JOB NO.: 08-028**



PROPOSED RIVERINE TRAINING FACILITY – PREFERRED SITE
 STENNIS SPACE CENTER
 HANCOCK COUNTY, MISSISSIPPI
 SESI PROJECT M08-028

SOUTHERN EARTH SCIENCES, inc.



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FIGURE 1
 TEST LOCATION PLAN

APPENDIX B

BORING LOGS

PROPOSED RIVERINE TRAINING FACILITY – PREFERRED SITE
URS PROJECT NO. P-210
STENNIS SPACE CENTER
HANCOCK COUNTY, MISSISSIPPI
SESI PROJECT NO. M08-028

LOG OF BOREHOLE NO. B-1

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 10 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER/WASH
BORING ELEVATION: 18.5 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 03/14/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0	1/6	CL	Very Soft Dark Brown Sandy CLAY with Organics				
15	1/6 2/6	CL	Soft Orange and Gray Sandy CLAY	2	32.0		76
5	2/6 2/6	SP-SM	Loose Tan Fine SAND with Silt	4	30.8	20	82
10	3/6 2/6	CL	Medium Gray Sandy CLAY	6	22.7		7
10	2/6 3/6			5	20.7	30	81
15	4/6 4/6			6			
15	6/6 5/6	SC	Loose to Medium Gray and Orange Clayey Fine SAND	10			
20	6/6 4/6			12			
25	4/6 5/6	SP	Very Dense Dark Brown Silty Fine SAND and Organics	9			
30	4/6 23/6			49	26.1		4
35	24/6 25/6			70			
40	21/6 32/6	SP	Loose to Dense Gray Fine SAND	46			
45	38/6 18/6			9			
50	20/6 26/6	CH	Soft to Medium Gray CLAY with Trace Sand	4			
55	5/6 5/6			7			
60	4/6 30/6	SP	Very Dense Gray Fine SAND	78			
65	40/6 38/6	CH	Soft Gray CLAY with Trace Sand	4			
70	1/6 2/6						

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Remarks:

LOG OF BOREHOLE NO. B-2

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/16/08
WATER LEVEL: 4.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER/WASH
BORING ELEVATION: 9.5 ft
DATE COMPLETED: 03/16/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0	2/6 1/6 3/6	SC	Very Loose Dark Brown and Tan Clayey Fine SAND	4	22.4		44
5	3/6 7/6 8/6	SC-SM	Medium Brown and Orange Clayey SAND	15	19.3	20	42
5	2/6 2/6 4/6	CH	Medium to Very Stiff Gray CLAY with Trace Sand	6	23.0	51	70
0	2/6 9/6 10/6	SM	Medium Tan and Orange Silty Fine SAND	19			
-5	8/6 11/6 6/6			17			
-5	7/6 5/6 8/6			13			
-10	5/6 8/6 9/6	SP	Medium Dark Brown Fine SAND with Organics	17			
-10		SP	Medium Gray and Orange Fine SAND	24			
-15	7/6 8/6 16/6			25	26.6		4
-15	7/6 10/6 15/6			25			
-20	11/6 11/6 14/6	SM	Medium Gray Silty Fine SAND	25			
-25	2/6 2/6 3/6	CH	Medium Gray CLAY with Trace Sand	5			
-30	3/6 4/6 3/6			7			
-35							
-40							

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Remarks:

LOG OF BOREHOLE NO. B-3

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 2 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER/WASH
BORING ELEVATION: 7.5 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 01/24/00
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0		CL	Medium to Stiff Orange Sandy CLAY	5		
5		SM	Medium Orange and Gray Silty Fine SAND	12	26.0	74
5		SM	Medium Tan Silty Fine SAND with Wood	23	13.9	76
0		SP	Medium to Dense White Fine SAND	21		
-10		SP	Medium to Dense White Fine SAND	24		
-5		SP	Medium to Dense White Fine SAND	33		
-15		SP	Medium to Dense White Fine SAND	33		
-10		SP	Medium Dark Brown Fine SAND	27		
-20				32		
-25						
-20						
-30						

Remarks:

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LOG OF BOREHOLE NO. B-4

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 7.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER/WASH
BORING ELEVATION: 17 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0		SM	Loose Tan and Orange Silty Fine SAND	7			
15		SP	Medium Orange and Brown Fine SAND	23			
5		SM	Medium Gray Silty Fine SAND	24			
10		CH	Medium Brown CLAY with Trace Sand	11			
10		CH	Medium Brown CLAY with Trace Sand	10	53.6	80	98
5		SP	Medium White to Gray Fine SAND	16	18.3		2
15		SP	Medium White to Gray Fine SAND	20			
20		SP	Medium White to Gray Fine SAND	22	17.0		5
-5		SP	Medium Dark Brown Fine Sand				
-25		SP	Medium Dark Brown Fine Sand				
-10		SP	Medium Dark Brown Fine Sand	15			
-30							

Remarks:

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LOG OF BOREHOLE NO. B-5

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 11 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER/WASH
BORING ELEVATION: 17.5 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 03/14/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %	
0		SM	Loose to Medium Orange Silty Fine SAND	5				
15		13						
5		16			13.9		17	
10		8	SM	Loose Gray and Orange Silty Fine SAND				
10		14	CH	Medium to Very Stiff Brown CLAY with Trace Sand	32.7	54	93	
5		29	SP	Medium to Dense White Fine SAND				
15		27						
20		35						
-5		24	SP	Medium Dark Brown Fine SAND				
25								
-10								
30								

Remarks:

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LOG OF BOREHOLE NO. B-6

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 6.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 22 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0		CL	Medium Tan Sandy CLAY	8	18.8	73
20		CL	Stiff to Very Stiff Orange and Tan Clayey Sandy CLAY	15	19.2	73
5		SP	Medium Tan to Gray Fine SAND	31	22.9	67
15		CL	Medium to Stiff Gray and Brown Sandy CLAY	27		
10		CL	Medium to Stiff Gray and Brown Sandy CLAY	18		
10		CL	Medium to Stiff Gray and Brown Sandy CLAY	11		
15		CL	Medium to Stiff Gray and Brown Sandy CLAY	10		
5						
20						

Remarks:

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LOG OF BOREHOLE NO. B-7

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 9.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 20 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/17/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
20 0		SC	Loose Brown Clayey Fine SAND with Organics	8			
		SC	Medium to Dense Orange and Brown Clayey Fine SAND	11	20.8		41
15 5		SP	Medium Tan Fine SAND	39			
		SM	Loose Orange and Tan Silty Fine SAND	17			
10 10		CH	Medium Brown and tan CLAY with Trace Sand	8	27.6	21	40
		SM	Medium Brown and Orange Silty Fine SAND	8	51.2	61	95
5 15					21		
0 20							

Remarks:

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LOG OF BOREHOLE NO. B-8

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 4.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 16 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/17/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0		SC	Loose Brown and Tan Clayey Fine SAND				
15		6	14.6	47			
3		8	18.9	44			
5		SM	Medium Orange Silty Fine SAND	16	22.0	20	23
10		MH	Medium to Stiff Gray SILT with Trace Sand	10	72.9	58	99
15	SP	Medium White and Gray Fine SAND	16				
20				19			
0				21			

Remarks:

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LOG OF BOREHOLE NO. B-9

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 4 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 16 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/17/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %	
0		SC	Loose Dark Brown and Tan Clayey Fine SAND	6	21.1	49	
15		SP-SM	Loose to Medium Orange and Tan Fine SAND with Silt	8			
5			CL	Stiff Gray CLAY with Trace SAND	21	20.8	7
10			SP	Medium Brown to White and Gray Fine SAND	19		
5					20		
15					18		
0							
20							

Remarks:

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LOG OF BOREHOLE NO. B-10

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 6.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 16 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/17/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0		SM	Loose Brown and Tan Silty Fine SAND	6		
15		SM	Very Loose Orange Silty Fine SAND	4	17.5	35
5		SC	Medium Gray and Orange Clayey Fine SAND	18	20.9	45
10		CL	Soft Tan and Orange Sandy CLAY	4		
5		SP	Medium Orange To White Fine SAND	16		
15		SP	Medium Orange To White Fine SAND	16		
0				16		

Remarks:

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LOG OF BOREHOLE NO. B-11

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/18/08
WATER LEVEL: 9.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 18 ft
DATE COMPLETED: 03/18/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %	
0		SM	Loose Brown and Tan Silty Fine SAND	5				
15		6	17.2	19				
5		8						
10		10	23.7	63	93			
10		11						
5		16						
15		27						
0								
20								

Remarks:

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LOG OF BOREHOLE NO. B-12

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 3.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 11 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/17/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0		SM	Very Loose Dark Brown Silty Fine Sand with Organics	3			
10		SM	Very Loose Tan Silty Fine SAND	3			
5		CL	Medium Gray Sandy CLAY	6	50.5	33	89
5		SP	Medium Tan, Brown and White Fine SAND	15			
10				14			
0				17			
15				16			
-5							
20							

Remarks:

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LOG OF BOREHOLE NO. B-13

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 0.9 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 9.5 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 03/14/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0	▼	SM	Very Loose Gray Silty Fine SAND	6	18.4	30
5	▲					
5	▲	SC	Loose Gray Clayey SAND	7	33.7	41
10	▲					
10	▲	SP	Medium to Dense White to Brown and Tan Fine SAND	37		
15	▲					
15	▲	SM	Medium Brown Silty Fine SAND with Organics	16	91.5	21
20	▲					
20	▲			12		

Remarks:

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LOG OF BOREHOLE NO. B-14

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 10.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 19 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %	
0		SC	Loose Brown and Tan Silty Fine SAND	5				
15		CL	Stiff Orange and Tan Sandy CLAY	16	17.3	25	45	
5		SM	Loose to Medium Orange and Tan Silty Fine SAND	22				
10		8		8	17.4		17	
10		11		11	21.8		35	
5		8	CH	Medium Orange and Gray CLAY with Trace Sand	8			
15		15	SP	Medium White and Tan Fine SAND	15			

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Remarks:

LOG OF BOREHOLE NO. B-15

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/17/08
WATER LEVEL: 1.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 9 ft
DATE COMPLETED: 03/17/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0		SM	Very Loose Gray and Tan Silty Fine SAND	2	19.8	34
5		CL	Soft Orange and Tan Sandy CLAY	4		
5		SP	Medium Tan to White and Gray Fine SAND	28		
0				21		
10				11		
-5		SP	Medium Dark Brown Fine Sand with Organics	22	22.2	2
-15				15		

Remarks:

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LOG OF BOREHOLE NO. B-16

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 2.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 9.5 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0		CL	Soft to Medium Gray and Orange Sandy CLAY	3	19.4		60
5		SP	Dense White and Gray Fine SAND	6	33.3	27	59
10		SM	Loose to Medium Gray Silty Fine SAND	7			
15		SP	Medium Brown and Tan Fine SAND with Organics	12			
20					26		

Remarks:

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LOG OF BOREHOLE NO. B-17

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 03/13/08
WATER LEVEL: 6.5 ft
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: AUGER
BORING ELEVATION: 8 ft
DATE COMPLETED: 03/13/08
WATER LEVEL DATE: 03/18/08
DRILLER: D. GARDNER

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	LL %	200 %
0		CL	Medium Orange and Gray Sandy CLAY	6	21.0	27	60
5		SC	Loose Orange and Gray Clayey Fine SAND	9	16.1		41
5		SM	Medium Orange and Gray Silty Fine SAND	22			
0		CL	Medium to Hard Gray Sandy CLAY	14			
10				17			
-5				26	23.3	32	85
-15				34			
-20							

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Remarks:

LOG OF BOREHOLE NO. RB-4

PROJECT: RIVERINE TRAINING FACILITY
PROJECT LOCATION: HANCOCK COUNTY, MISSISSIPPI
BORING LOCATION: SEE TEST LOCATION PLAN
DATE DRILLED: 12/20/06
WATER LEVEL: NOT MEASURED
GEOL / ENGR: G. FORSYTHE

SESI PROJECT: 08-028
METHOD: MUD ROTARY
BORING ELEVATION: 19 ft
DATE COMPLETED: 12/20/06
WATER LEVEL DATE:
DRILLER: M. CONNIF

Elevation / Depth	Soil Symbols Sampler Symbols and Field Test Data	USCS	Description	SPT N	NM %	200 %
0		SP-SM	Loose Tan & Gray Fine SAND with Some Silt & Organics	4		
15		SC	Loose to Medium Tan, Gray & Yellow Clayey Fine SAND	7		
5		CL	Medium Gray & Orange CLAY with Trace Sand	16	14.2	31
10		CL	Medium Gray & Orange CLAY with Trace Sand	7		
10		CL	Medium Gray & Orange CLAY with Trace Sand	5		
5		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	15		
15		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	28		
20		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	28		
25		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	28		
30		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	45		
35		SP-SM	Medium to Dense Gray, Tan & Brown Fine SAND with Some Silt	18		
40		CL	Soft Gray CLAY with Trace Sand	4		
45						
50						

Remarks: Ground water table not measured due to collapse of borehole.
 This boring performed for a previous exploration for this project (SESI M06-462)

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