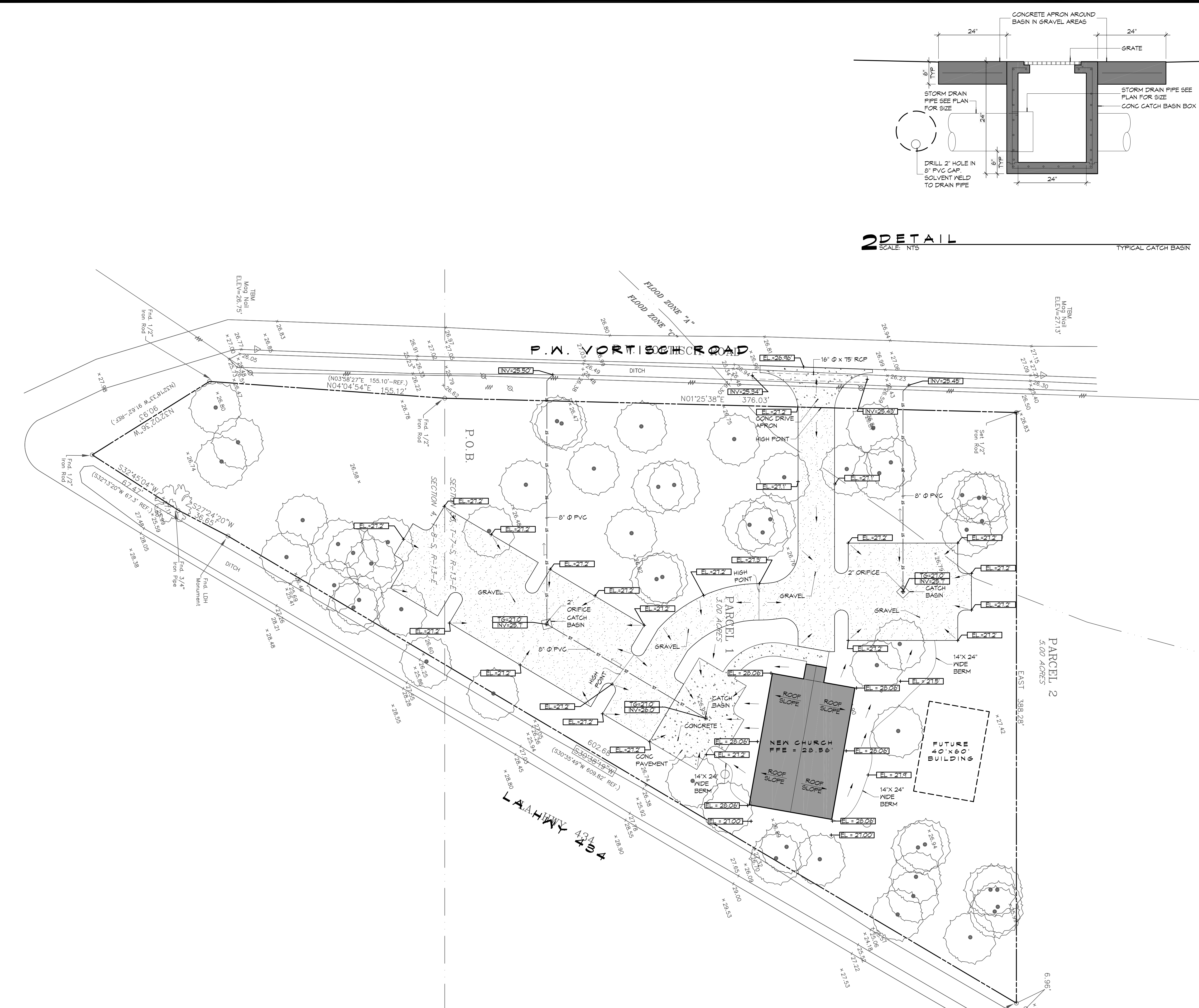
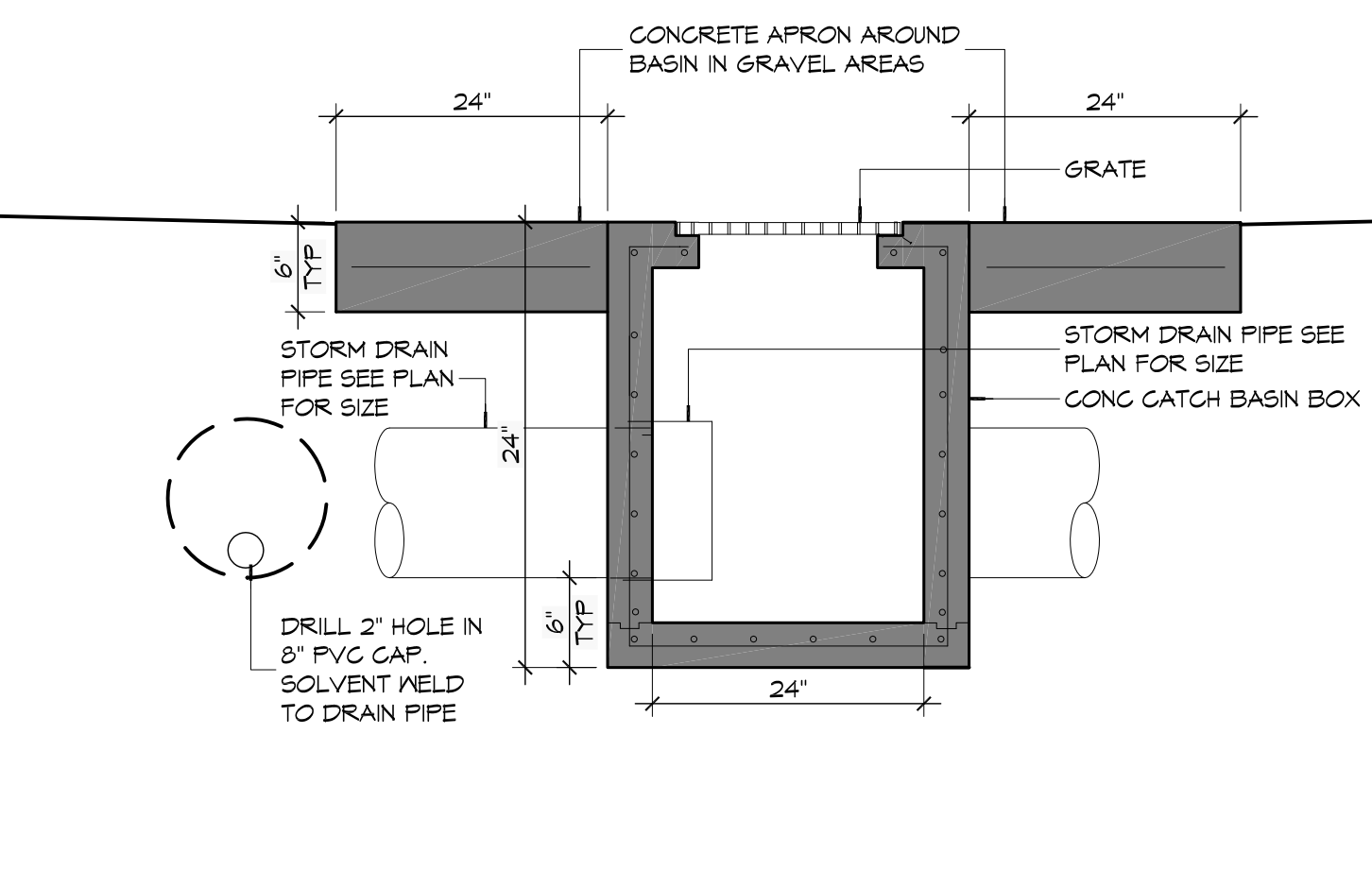


FILE NAME: J:\Projects\15100 - Church\15100.dwg DATE: 09/30/2015 10:53:00 AM



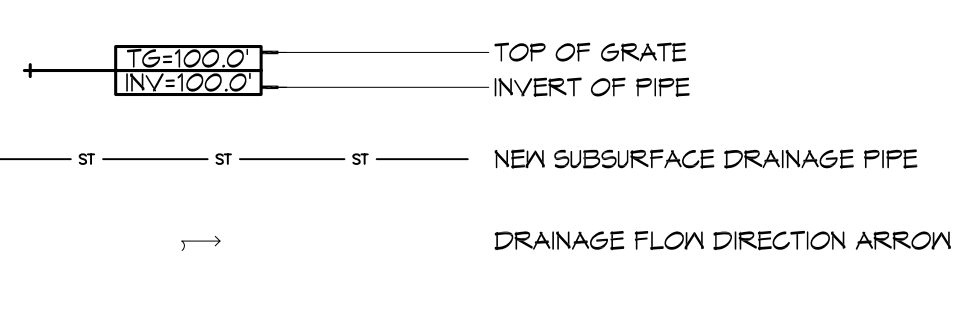
2 DETAIL
SCALE: NTS
TYPICAL CATCH BASIN



GENERAL SITE DRAINAGE NOTES

1. DRAIN PIPE(S) ALONG AIRPORT MUST BE THE BELL AND SPIGOT TYPE WITH "O" RING RUBBER GASKETS. THE BELLS OF THE PIPES SHALL BE LAID UPSTREAM. ALL JOINTS SHALL BE WRAPPED WITH GEOTEXTILE FABRIC. ALL PIPES SHALL REQUIRE A 3" COMPACTED SAND OR LIMESTONE BASE.
2. REMOVE DEBRIS AND CLEAN BOTTOM OF DITCHES DOWN 6" IN DEPTH - REPLACE ANY BROKEN/CRUSHED PIPES OR CULVERTS WITH SAME SIZE AND TYPE.
3. DRAIN PIPE AND FITTINGS WITHIN PROPERTY LINE SHALL BE POLYVINYL CHLORIDE PLASTIC PIPE, MEETING CLASS 100 C-400 PVC.
4. ELEVATIONS SHOWN ARE M.S.L.
5. FIELD VERIFY ALL ELEVATIONS AND DRAINAGE SYSTEM PLACEMENT PRIOR TO START OF WORK.
6. PROVIDE VERTICAL ELBOW AT DOWNSPOUTS FOR CONNECTION TO SUBSURFACE DRAINAGE WHERE INDICATED. ELBOW ID SHALL BE SIZED SUCH THAT THE DOWNSPOUT CAN BE INSERTED INTO THE PIPE OPENING.

SITE DRAINAGE LEGEND



STORM WATER RUN-OFF CALCULATIONS

PROJECT: **New Church**
FORMULAS USED: **STORMWATER RUN-OFF CALCULATIONS**

Formulas used: **[1] RATIONAL METHOD: Q=AcI**
where:
Peak discharge of watershed in cubic feet per second (cfs) due to maximum storm assumed.
Area of watershed in acres.
Coefficient of run-off [2].
Intensity of rainfall in inches per hour based on concentration time [3].
$$I = \frac{(1.48(1000 - S))^{0.77}}{(1140(S^{0.5}))}$$

where:
Time of concentration time required for rain falling at most remote point to reach discharge point.
Site run-off coefficient based on conditions shown.
Percent slope of overland flow.

PRIOR DEVELOPMENT
25 Year Frequency

Surface	c(1)	c(2)	c(3)	Summary
Waterlight Surfaces	0.4	0	0	0.000 Acres
Gravel Surface	0.25	0	0	0.000 Acres
Green Space	0.15	191691	3.023	3.023 Acres
Summary	c = 0.15	191691	3.023	3.023 Acres

Duration (D) = Time of concentration (TC)
where
L = 602 run-off length ft
c = 0.15 run-off coef
S = 0.1661 percent slope
TC = D = 28.40 minutes
therefore
Expected rainfall intensity
I = 3.64 in/hr
Q₁ = 1.651 cfs 10% reductic **0.165 cfs**

POST DEVELOPMENT
25 Year Frequency

Surface	c(1)	c(2)	c(3)	Summary
Waterlight Surfaces	0.4	0175	0	0.186 Acres
Gravel Surface	0.25	24954	0	0.554 Acres
Green Space	0.15	49163	2.276	2.276 Acres
Summary	c = 0.22	191691	3.023	3.023 Acres

Duration (D) = Time of concentration (TC)
where
L = 120 run-off length ft
c = 0.22 run-off coef
S = 1.0000 percent slope
TC = D = 14.90 minutes
therefore
Expected rainfall intensity
I = 3.64 in/hr
Q₂ = 2.361 cfs

DETENTION REQUIREMENTS

Detention required Q ₂ -Q ₁	0.71 cfs
ONE HOUR DETENTION	25712 cuft
DETENTION DIMENSIONS	WIDTH 64 feet
	LENGTH 211 feet
	DEPTH 0.14 feet

DISCHARGE END AREA REQUIREMENTS
10 Year Frequency

where:
A = Discharge Area required
g = Acceleration of gravity
C = Discharge coefficient
h = Hydraulic head
Q = Flow volume from run-off

Pipe Servicing Site Drainage

Q =	0.165 cfs	h =	3.00 feet
c =	0.82 coefficient	A =	0.014 sqft
g =	32.16 ft/sec/sec		

REQUIRED CONDUIT = 1.87 inch inside diameter

References:
1. Chen, N.F. The Civil Engineering Handbook, 1995, Eq# 311, pg. 1036
2. Seelye, Elwyn E. Data Book for Civil Engineers, Vol 1 1960, Tbl. B, pg. 18-02
3. Seelye, Elwyn E. Data Book for Civil Engineers, Vol 1 1960, Fig. B, pg. 18-01
4. Chen, N.F. The Civil Engineering Handbook, 1995, Tbl. B12 Regan Equation (n=0.015)
5. Chen, N.F. The Civil Engineering Handbook, 1995, Eq# 28.32, pg. 964

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REVISIONS

#	DESCRIPTION	DATE

SEAL:

**P R O M U L T I S
L A C O M B E C H A P E L**
P.W. VORTSCH ROAD AND LA HWY 434
LACOMBE, LOUISIANA 70445
JOB No: 2250 DATE: SEPTEMBER 3, 2015
DRAWN BY: K-K CHECKED BY: CKD

SHEET TITLE:
SITE PLAN - DRAINAGE

DRAWING NUMBER:
C102

SHEET No: 4 of 14

1 SITE PLAN
SCALE: 1" = 30'

DRAINAGE