

Builder/Contractor Responsibilities

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1)(Mar 05 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC Code of Standard Practice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3) (CSA/S16–09 Section 29).

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design

The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)



Mesco Building Solutions

5244 Bear Creek Court, Irving, Texas 75061

Voice 214-687-9999

Fax 214-687-9737

ENGINEERING DESIGN CRITERIA

Building Code	2012 IBC
Building Risk Category	Normal (Risk Category II)
Roof Dead Load	
Superimposed	2.32 psf
Collateral	0.50 psf
Roof Live Load	
(0.00 psf Ceiling 0.50 psf Other)	20.00 psf reduction allowed
Snow	
Ground Snow Load (Pg)	5.00 psf
Snow Load Importance Factor (I)	1.00
Flat Roof Snow Load (PF)	5.04 psf
Snow Exposure Factor (Ce)	1.20 (Sheltered)
Thermal Factor (Ct)	1.20 (Unheated)
Wind	
Ultimate Wind Speed (Vult)	154.00 mph
Nominal Wind Speed (Vasd)	119 mph (ASCE7-10 section 1609.3.1)
Wind Exposure Category	B
Internal Pressure Coef (GCpi)	0.18/-0.18 (Enclosed)
Loads for components not provided by building manufacturer	
Corner Areas (within 10.00' of corner)	39.04 psf pressure -52.06 psf suction
Other Areas	39.04 psf pressure -42.30 psf suction
These values are the maximum values required based on a 10 sq ft area.	
Components with larger areas may have lower wind loads.	
Seismic	
Seismic Importance Factor (Ie)	1.00
Seismic Design Category	B
Soil Site Class	D Stiff Soil
Ss	0.102 g Sds 0.109 g
SI	0.056 g Sal 0.090 g
Analysis Procedure	Equivalent Lateral Force
Column Line	All
Basic Force Resisting System	H
Response Modification Coefficient (R)	3.00
Seismic Response Coefficient (Cs)	0.04
Design Base Shear in kips (V)	3.05
Basic Structural System (from ASCE 7-10 Table 12.2-1)	
H - Steel System not Specifically Detailed for Seismic Resistance	

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length. The frame sidesway for wind loading is based on ASCE 7 commentary equation CC-3 of 0.7W. The limits shown are at service loads unless indicated otherwise.

BUILDING DEFLECTION LIMITS... BLDG-A

Roof Limits	Rafters	Purlins	Panels
Live L/	180	150	60
Snow L/	180	180	60
Wind L/	180	180	60
Total Gravity L/	120	120	60
Total Uplift L/	N/A	N/A	60
Frame Limits	Sidesway	Portal Frame Sidesway	
Live H/	60		
Snow H/	60		
Wind H/	60		
Seismic Drift H/	40		
Crane H/	100		
Total Gravity H/	60		
Total Wind H/	60		
Service Seismic H/	120	120	
Wall Limits	Limit		
Total Wind Panels L/	60		
Total Wind Girts L/	90		
Total Wind EW Columns L/	120		

The Service Seismic limit as shown here is at service level loads.

PROJECT NOTES

BOLT TIGHTENING - All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the Specification for Structural Joints Using ASTM A325 or A490 Bolts, December 31, 2009. Pretensioning methods, including turn-of-nut, calibrated wrench, twist off type tension control bolts or direct tension indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, other than flange braces, conform to ASTM 36 minimum. Hollow structural shapes conform to ASTM A500 grade B, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold-formed light gage steel members conform to the requirements of ASTM A1011 SS Grade 55, ASTM A1011 HSLAS Grade 55 Class 1, ASTM A653 SS Grade 55, or ASTM A653 HSLAS Grade 55 Class 1 with a minimum yield point of 55 ksi. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

Using Large eave gutter with 4 x 5 downspouts, the roof drainage system has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 30 feet with the first downspout from both ends of the gutter run within 1 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 10.00 in/hr rainfall intensity.

X-Bracing is to be installed to a taut condition with all slack removed. Do not tighten beyond this state.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code.

The materials by the manufacturer will be fabricated in a facility that has received Certification of Accreditation for the Manufacture of Metal Building Systems (AC472) from International Accreditation Service (IAS). This certification is recognized under Section 1704 of the IBC for approved fabricator.

Drawing Index		Ck'd							
Page	Description								
F1	Anchor Rod	By							
F2	Anchor Rod Details								
F3	Reaction Drawings								
E1	Cover Sheet	Description							
E2	Primary Steel BLDGA								
E3	Roof Framing BLDGA								
E4	Roof Sheetting								
E5	Sidewall BLDGA WALLSWA								
E6	Sidewall BLDGA WALLSWC								
E7	Endwall BLDGA WALLEWB								
E8	Endwall BLDGA WALLEWD								
E9-E11	Main Frame Cross Sections								
R1-R12	Construction Drawings	Date							
R13	Trim Profiles								

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

Customer:
AMERICAN WESTERN STEEL LLC
DBA WILSON CONSTRUCTION
HOUSTON, TX 77032
MICHAEL VIVIAN

☐ For Construction Permit
☒ For Erector Installation

☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)

Scale: NOT TO SCALE

Drawn by: KEB 7/28/14

Checked by: KEB 7/28/14

Project Engineer: JS

Job Number: 14-B-54756-1

Sheet Number: E1 of 11

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.

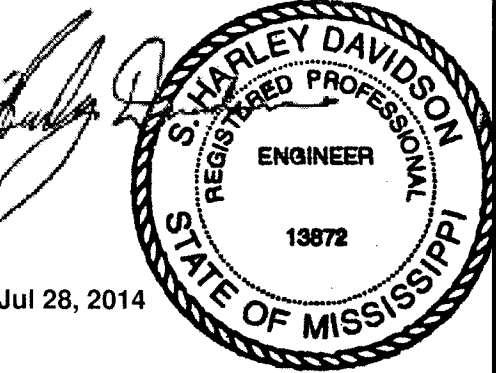
S. Harley Davidson, P.E.
Mississippi P.E. 13872

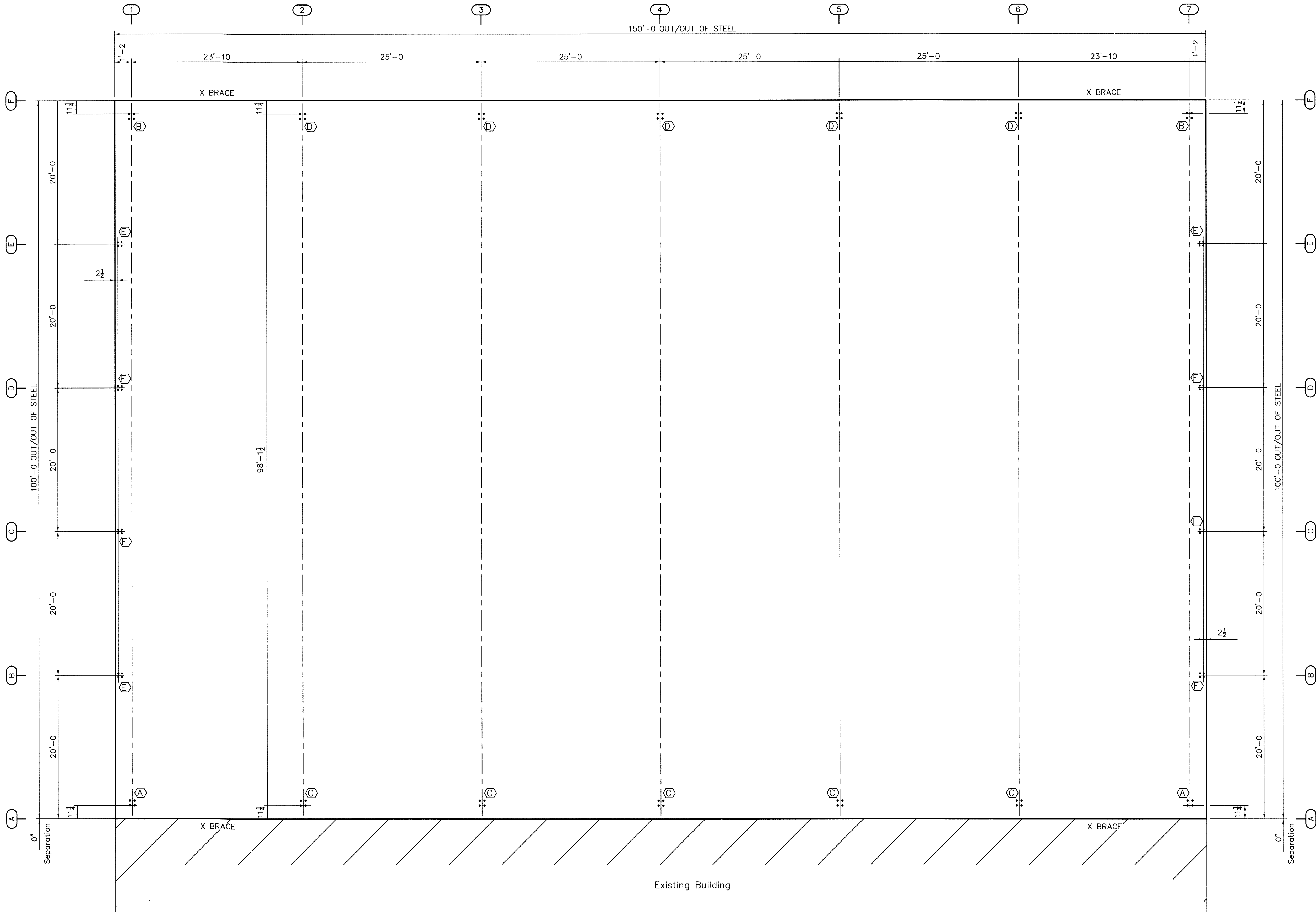
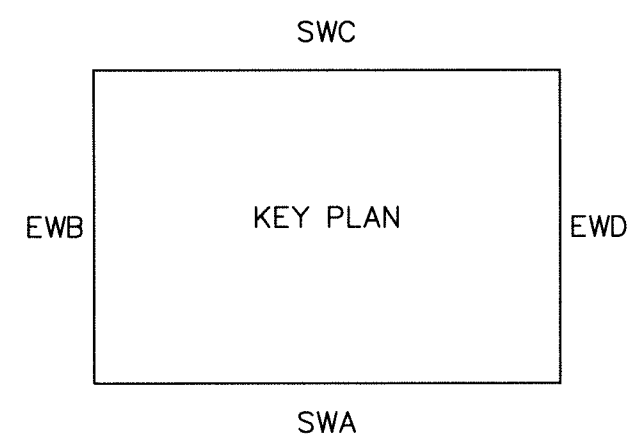
DRME1A ENME1A

3/8" A325 BOLT GRIP TABLE		BOLT LENGTH	NOTE: FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.
GRIP	LENGTH		
0 TO 9/16"	1 1/4" F.T.		WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.
Over 9/16" TO 1 1/16"	1 3/4" F.T.		
Over 1 1/16" TO 1 5/16"	2"		
Over 1 5/16" TO 1 9/16"	2 1/4"		
Over 1 9/16" TO 1 13/16"	2 1/2"		
Over 1 13/16" TO 2 1/16"	2 3/4"		
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS			
F.T. DENOTES FULLY THREADED			

BUILDING DESCRIPTIONS				
Building ID	Width	Length	Height	Slope
Building A	100'-0	150'-0	27'-0	3:12

Jul 28, 2014



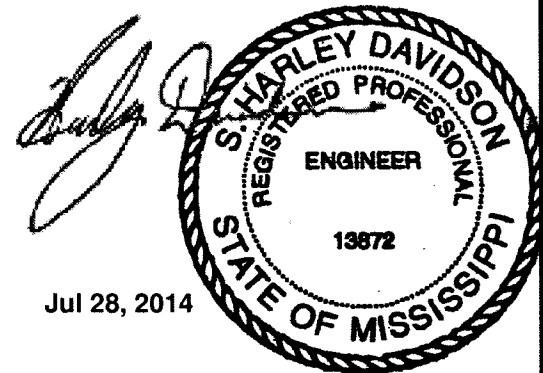


- Anchor Rod Drawings
- 1) This drawing is for anchor rod placement only and is not foundation design.
 - 2) Foundation must be square and level with all anchor rods true in size, location, and projection.
 - 3) Projection shown must be held to keep threads clear of finished concrete.
 - 4) This structural design data includes magnitude and location of design loads and suppose conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
 - 5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
 - 6) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.

ANCHOR ROD SETTING PLAN

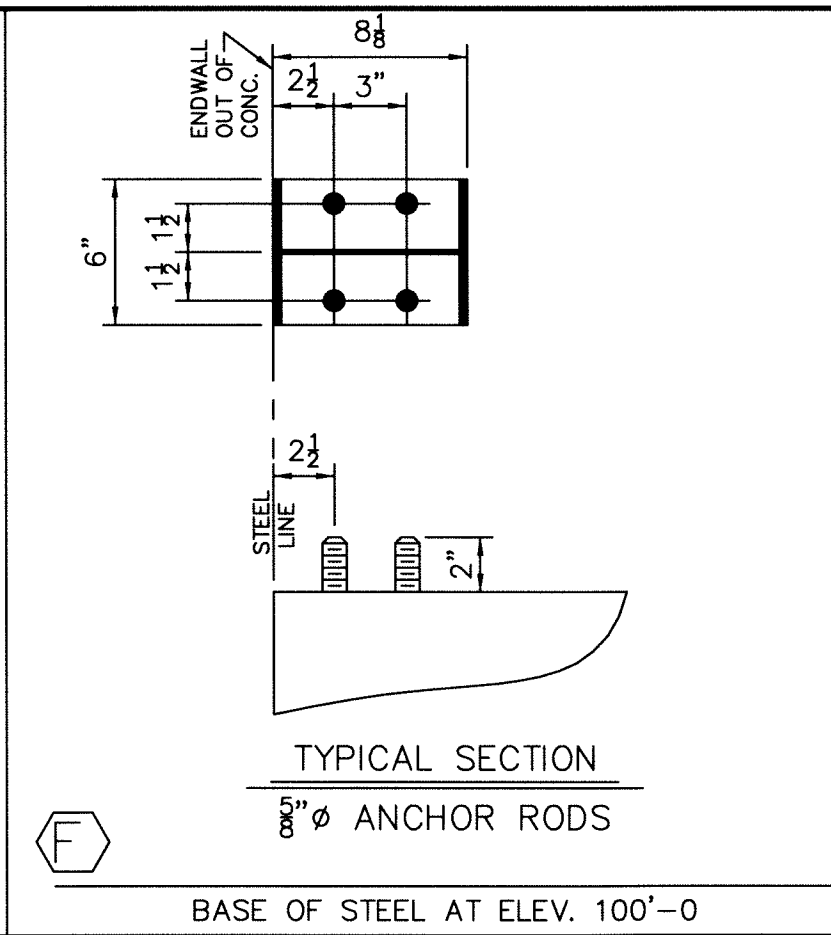
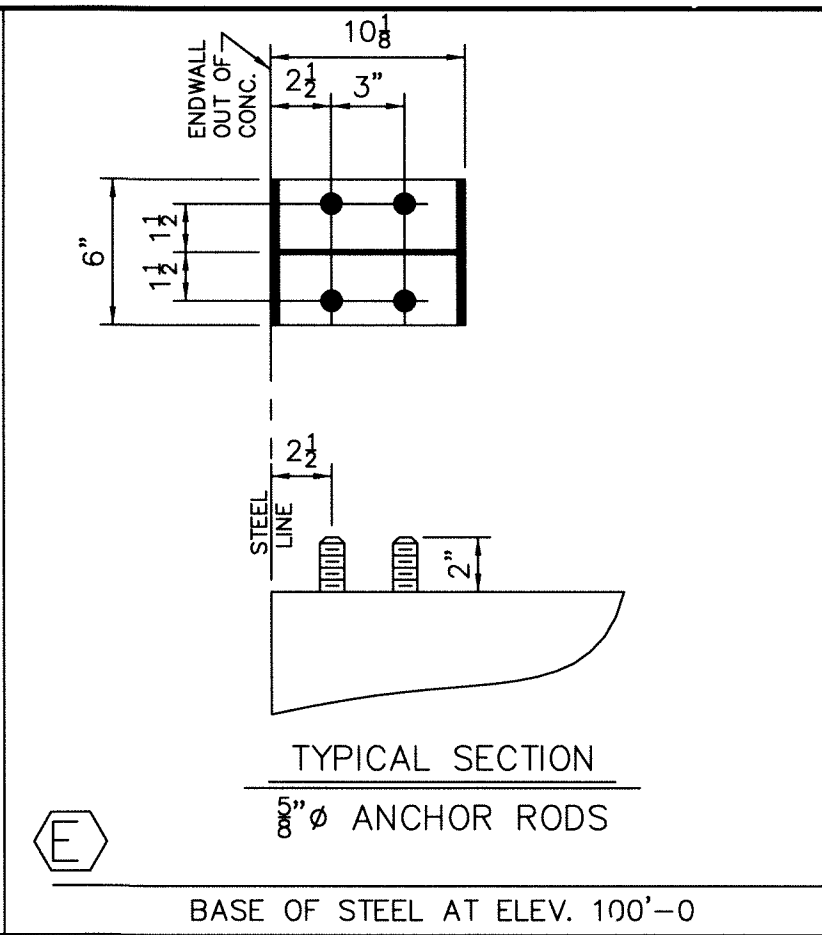
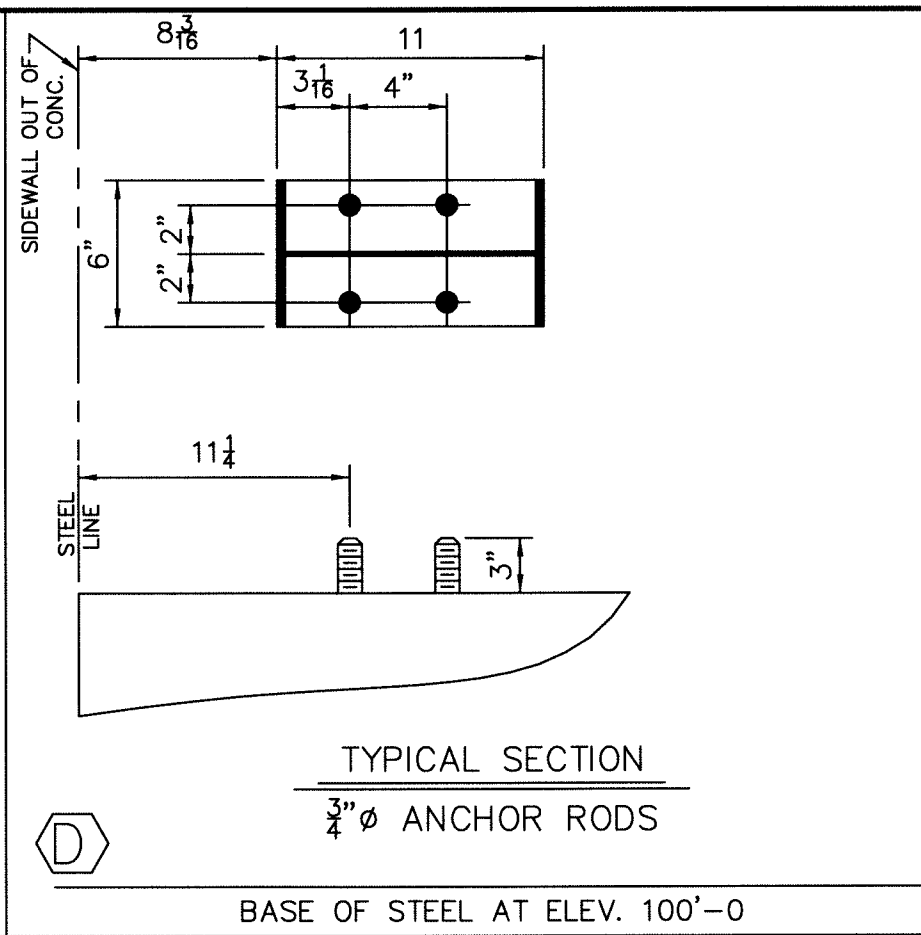
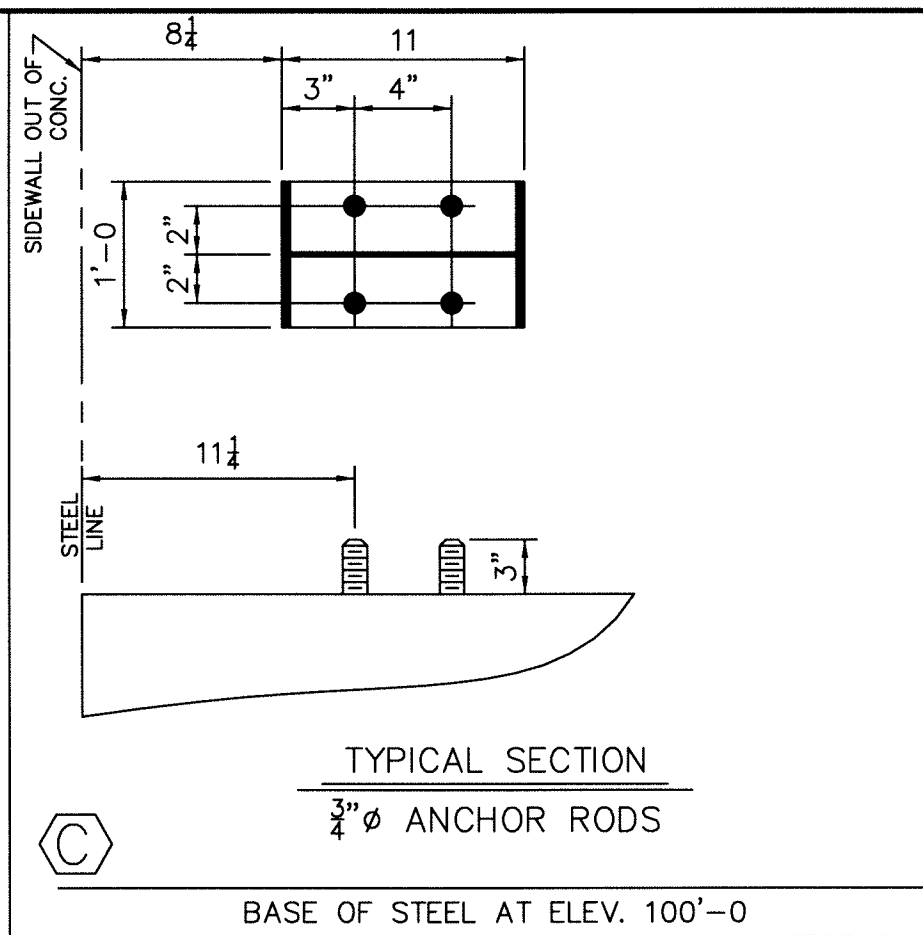
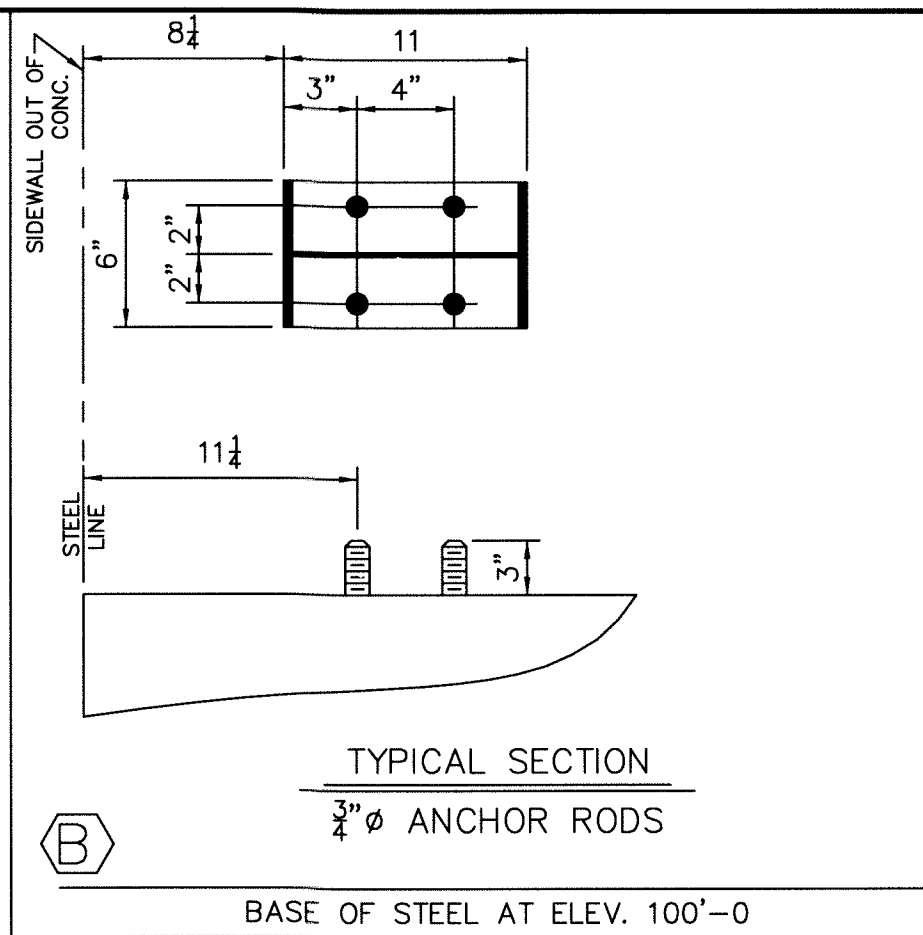
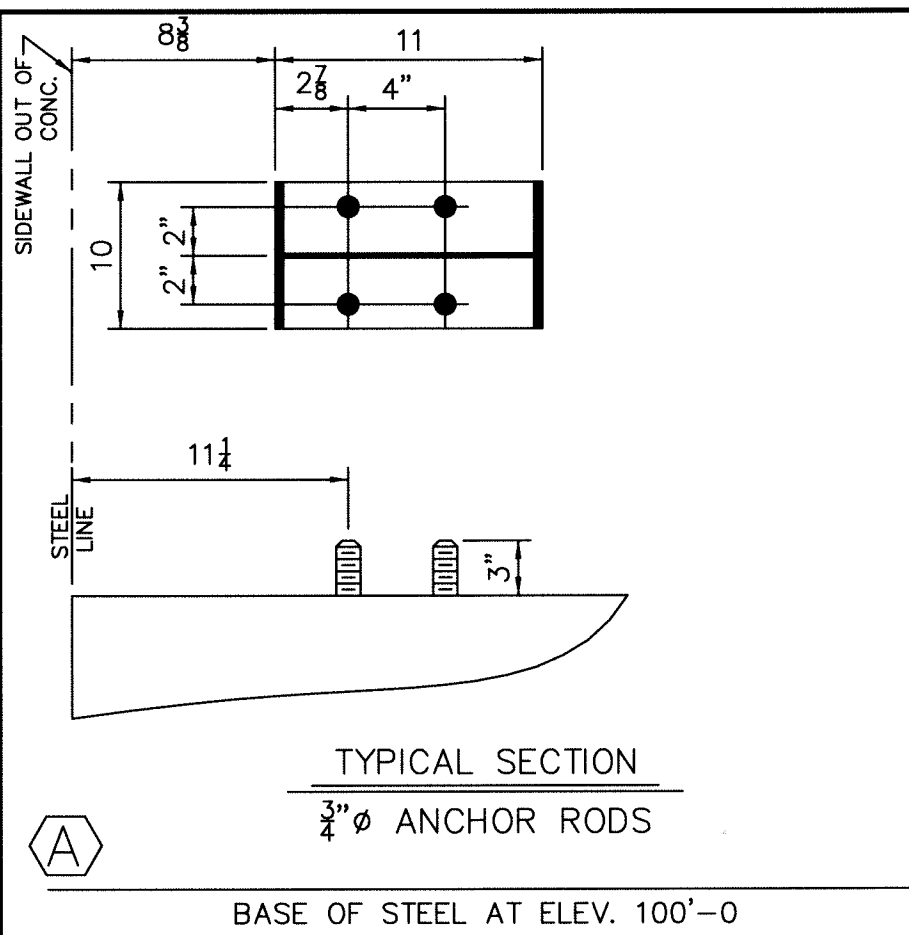
FINISH FLOOR AT ELEVATION 100'-0'

ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.	
ANCHOR ROD DESCRIPTION	QUANTITY
5/8" Ø DIAMETER X	32
3/4" Ø DIAMETER X	56



Jul 28, 2014

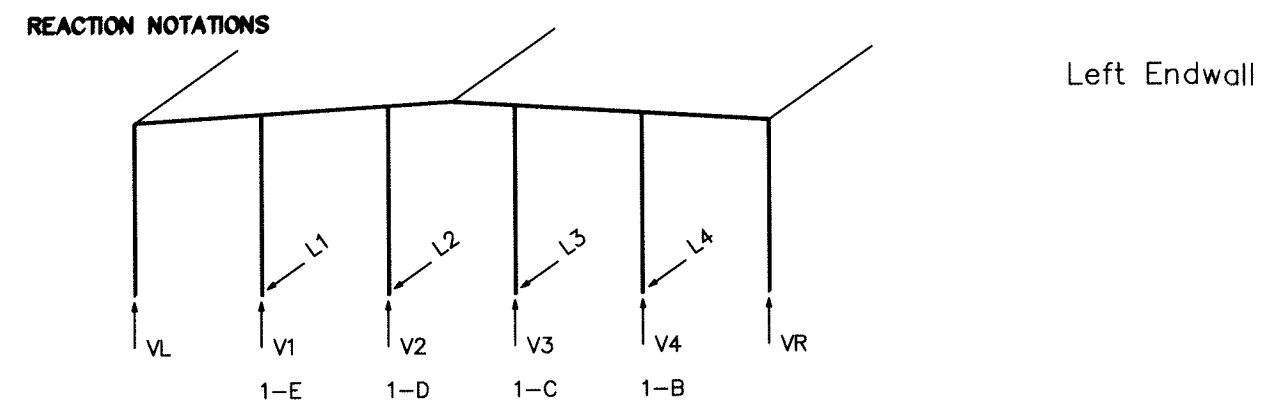
Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737		Project Name & Location: JINDAL SAW USA 1 ROAD A BAY ST LOUIS, MS. 39520-0	
Customer: AMERICAN WESTERN STEEL LLC DBA WILSON CONSTRUCTION 5750 N. SAM HOUSTON PKWY E HOUSTON, TX. 77032-4090		Drawing Status: ☐ Preliminary ☐ For Approval ☒ For Construction Permit ☒ For Erector Installation	
Scale: NOT TO SCALE			
Drawn by: srs 7/23/14			
Checked by: SAP 7/23/14			
Project Engineer: JS			
Job Number: 14-B-54756-1			
Sheet Number: F1 of 3			
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.			
S. Harley Davidson, P.E. Mississippi P.E. 13872 DRME1A ENME1A			



NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
 - 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LOADS (UNLESS NOTED OTHERWISE):
 - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - b) RAISED FRAMES
 - (1) GABLE BUILDINGS
 - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LONG SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - (2) SINGLE SLOPE BUILDINGS
 - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - c) END WALLS
 - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD BY METHOD OF THE FOUNDATION ENGINEER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - e) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 - f) X-BRACING
 - (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (R_{BEQ} AND R_{BQWQ}) DO NOT INCLUDE THE AMPLIFICATION FACTOR, R_u.
 - (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (R_{BEQ} & R_{BQWQ}) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d, WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO I_E/S_{0.2}(0.2) IS GREATER THAN 0.45.
 - 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - 4) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC OR 2010 FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0.
- THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

FRAME DESCRIPTION: Endwall CWB PATH: R:\Jobs\Active\ENG\14-B-54756\ver01-jpeggle\Bldg-A\run01\		USER NAME: jpeggle JOB NAME: 54756A	DATE: 7/21/14 FILE: REWSBLDG1	PAGE: EW-1
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.				TIME: 15:50:32

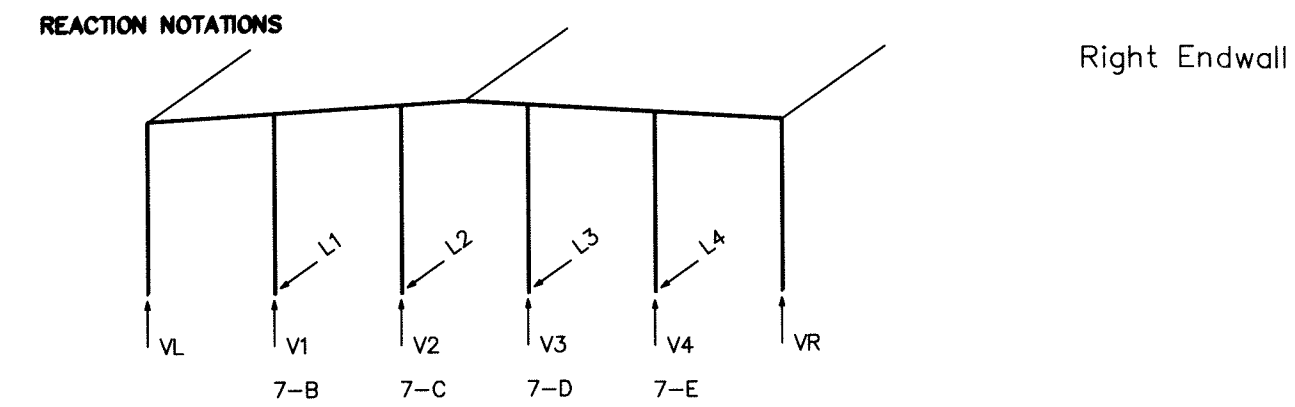


LOAD GROUP REACTION TABLE																
COLUMN		LEFT COLUMN			RIGHT COLUMN			1-E			1-D			1-C		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3	
D	0.	0.	0.	0.	0.	0.	0.	0.5	0.	0.	0.5	0.	0.	0.5	0.	
W+	0.	0.	0.	0.	0.	0.	0.	0.	8.1	0.	0.	8.4	0.	0.	8.4	
W-	0.	0.	0.	0.	0.	0.	0.	0.	-9.0	0.	0.	-9.4	0.	0.	-9.4	

<u>LOAD GROUP DESCRIPTION</u>	
D	: DEAD LOAD
W+	: WIND LOAD AS AN INWARD ACTING PRESSURE
W-	: WIND LOAD AS AN OUTWARD ACTING SUCTION

COLUMN	1-B		
LOAD GROUP	H4	V4	L4
D	0.	0.5	0.
W+	0.	0.	8.1
W-	0.	0.	-9.0

FRAME DESCRIPTION: Endwall EWD		USER NAME: jspeegle JOB NAME: 54756A	DATE: 7/21/14 FILE: REW48LD01	PAGE: EW-3
PATH: R:\Jobs\Active\ENG\14-B-54756\ver01-jspeegle\Bldg-A\run01\				
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in klps and kip-ft.				
				TIME: 15:50:32

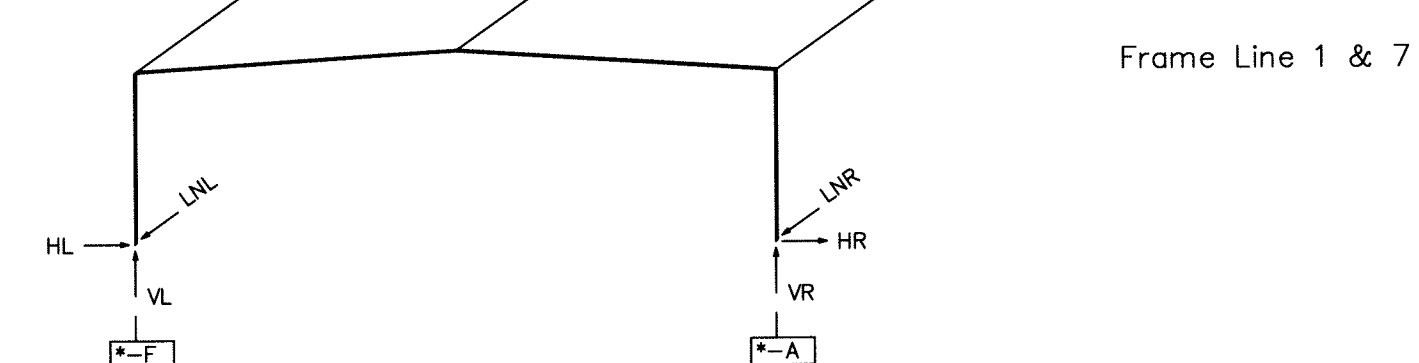


COLUMN	LEFT COLUMN			RIGHT COLUMN			7-B			7-C			7-D		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3
D	0.	0.	0.	0.	0.	0.	0.	0.5	0.	0.	0.5	0.	0.	0.5	0.
W+	0.	0.	0.	0.	0.	0.	0.	0.	8.1	0.	0.	8.4	0.	0.	8.4
W-	0.	0.	0.	0.	0.	0.	0.	0.	-9.0	0.	0.	-9.4	0.	0.	-9.4

LOAD GROUP DESCRIPTION	
D	: DEAD LOAD
W+	: WIND LOAD AS AN INWARD ACTING PRESSURE
W-	: WIND LOAD AS AN OUTWARD ACTING SUCTION

COLUMN	7-E		
LOAD GROUP	H4	V4	L4
D	0.	0.5	0.
W+	0.	0.	8.1
W-	0.	0.	-9.0

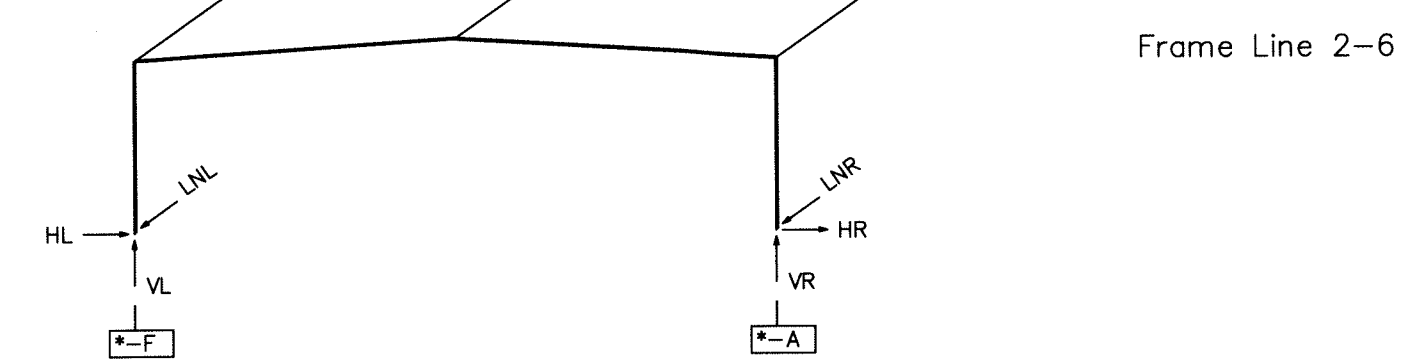
FRAME ID #2		USER NAME: jpeegle	DATE: 7/22/14	PAGE: 2-3
CS 100, 27, 13.083	20, 154.	JOB NAME: 54756A	FILE: frames_1_7.fra	
SUPPORT REACTIONS FOR EACH LOAD GROUP				
*LOCATION: Gridlines: 1 7				
(1) All reactions are in cps and kip-ft.				
(2) The seismic overstrength factor (Omega) is not included in the "RBDWEQ" and "RBUPEQ" Load Group reactions.				
*Notes: (1) All reactions are in cps and kip-ft. (2) The seismic overstrength factor (Omega) is not included in the "RBDWEQ" and "RBUPEQ" Load Group reactions.				
REACTION NOTATIONS				



LOAD GROUP REACTION TABLE * = 1 7						
COLUMN	*-F			*-A		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	1.7	3.2	0.0	-1.7	3.6	0.0
LL	5.1	7.8	0.0	-5.1	7.9	0.0
COLL	0.2	0.3	0.0	-0.2	0.3	0.0
SNOW	2.2	3.3	0.0	-2.2	3.3	0.0
EQ	-0.2	-0.1	0.0	-0.2	0.1	0.0
RBUFEQ	0.0	0.0	-0.7	-0.0	-0.8	-0.7
WL1	-20.8	-28.1	0.0	8.3	-18.3	0.0
WL2	-17.5	-19.6	0.0	6.9	-15.6	0.0
WL3	-18.4	-21.3	0.0	29.9	-28.1	0.0
WL4	-5.1	-9.8	0.0	17.6	-19.6	0.0
LWL1	-10.6	-26.2	0.0	11.4	-20.2	0.0
RBUPLW	0.1	-10.1	-9.3	-0.1	-10.1	-9.3
LWL2	-11.4	-20.1	0.0	10.6	-26.3	0.0
LWL3	-7.3	-17.7	0.0	8.1	-11.6	0.0
LWL4	-8.1	-11.6	0.0	7.3	-17.7	0.0
RS	2.1	2.1	0.0	-2.1	2.5	0.0
LS	2.1	3.5	0.0	-2.1	2.1	0.0
RBDMLW	-0.1	10.1	0.0	0.1	10.1	0.0
RBDWEQ	-0.0	0.8	0.0	0.0	0.8	0.0

LOAD GROUP DESCRIPTION	
DL	: Roof Dead Load
LL	: Roof Live Load
COLL	: Roof Collateral Load
SNOW	: Roof Snow Load
EQ	: Lateral Seismic Load [parallel to plane of frame]
RBDPEQ	: Upward Acting Rod Brace Load from Longit. Seismic
WL1	: Lateral Primary Wind Load
WL2	: Lateral Primary Wind Load
WL3	: Lateral Primary Wind Load
WL4	: Lateral Primary Wind Load
LWL1	: Longitudinal Primary Wind Load
RBDPLW	: Upward Acting Rod Brace Load from Longitud. Wind
LWL2	: Longitudinal Primary Wind Load
LWL3	: Longitudinal Primary Wind Load
LWL4	: Longitudinal Primary Wind Load
RS	: Unbalanced Right Roof Snow Load
LS	: Unbalanced Left Roof Snow Load
RBDLW	: Downward Acting Rod Brace Load from Longit. Wind
RBDWEQ	: Downward Acting Rod Brace Load from Long. Seismic

FRAME ID # cs 100, 27, 24, 417 20, 154.		USER NAME: jpegnie JOB NAME: 54756A	DATE: 12/27/94 FILE: NAME: 2--8 fra	PAGE: 1-3
SUPPORT REACTIONS FOR EACH LOAD GROUP LOCATION: Gridlines 2 3 4 5 6 NOTES: (1) All reactions are in kips and kip-ft. (2) The seismic overstrength factor (Omega) is not included in the "RBDMEQ" and "RBUPE" Load Group reactions. Seismic BASE-ONLY combination reactions include an overstrength factor of: 2.500				
REACTION NOTATIONS				



LOAD GROUP REACTION TABLE * =				2 3 4 5 6		
COLUMN	*-F			*-A		
LOAD GROUP	HL	VL	LNL	HR	VR	UNR
DL	2.8	4.9	0.0	-2.8	5.3	0.0
LI	10.0	15.0	0.0	-10.0	15.0	0.0
COLL	0.4	0.6	0.0	-0.4	0.6	0.0
SNOW	4.2	6.3	0.0	-4.2	6.3	0.0
EQ	-0.2	-0.1	0.0	-0.2	0.1	0.0
RBUQ	0.0	-0.6	-0.7	0.0	-0.8	-0.7
WL1	-27.8	-37.9	0.0	11.9	-28.4	0.0
WL2	-21	-21.6	0.0	7.1	-21.6	0.0
WL3	-12.0	-26.3	0.0	27.9	-37.9	0.0
WL4	-5.5	-10.0	0.0	21.4	-26.6	0.0
LWL1	-13.3	-35.6	0.0	14.2	-28.7	0.0
RBUPLW	0.1	-10.1	-9.3	0.1	-10.1	-9.3
LWL2	-14.2	-28.7	0.0	13.2	-35.6	0.0
WL3	-6.8	-19.3	0.0	7.7	-12.4	0.0
WL4	-7.7	-12.4	0.0	6.8	-19.3	0.0
RLS	4.0	4.0	0.0	-4.0	-4.0	0.0
LS	4.0	4.0	0.0	-4.0	4.0	0.0
RBDWLW	-0.1	10.1	0.0	0.1	10.1	0.0
RBDWEQ	0.0	0.8	0.0	0.0	0.8	0.0

LOAD GROUP DESCRIPTION	
DL	: Roof Dead Load
LL	: Roof Live Load
COLL	: Roof Collateral Load
SNOW	: Roof Snow Load
EQ	: Lateral Seismic Load [parallel to plane of frame]
RBUPFQ	: Upward Acting Rod Brace Load from Longit. Seismic
WL1	: Lateral Primary Wind Load
WL2	: Lateral Primary Wind Load
WL3	: Lateral Primary Wind Load
WL4	: Lateral Primary Wind Load
LWL1	: Longitudinal Primary Wind Load
RBUPWL	: Upward Acting Rod Brace Load from Longitud. Wind
LWL2	: Longitudinal Primary Wind Load
LWL3	: Longitudinal Primary Wind Load
LWL4	: Longitudinal Primary Wind Load
RS	: Unbalanced Right Roof Snow Load
LS	: Unbalanced Left Roof Snow Load
RBDDLW	: Downward Acting Rod Brace Load from Longit. Wind
RBDDLWQ	: Downward Acting Rod Brace Load from Long. Seismic

	Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737
Customer: AMERICAN WESTERN STEEL, LLC DBA W WILSON CONSTRUCTION 5750 N. IAW HOUSTON PKWY E HOUSTON, TX 77032-4090	Project Name & Location: JINDAL SAW USA 1 ROAD A BAY ST. LOUIS, MS. 39520-0 4080
Drawing Status: ☐ Preliminary ☐ (Not For Construction)	☐ For Construction <u>Permit</u> ☒ For Erector <u>Installation</u>

Scale:	NOT TO SCALE	
Drawn by:	srs	7/23/14
Checked by:	SAP	7/23/14
Project Engineer:	JS	
Job Number:	14-B-54756-1	
Sheet Number:	F3 of 3	

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

S. Harley Davidson, P.E.
Mississippi P.E. 13872

DRME1A ENME1A

Jul 28, 2014

- DENOTES: PIPE STRUT.
ATTACHED WITH $\frac{1}{2}$ " ϕ X $1\frac{3}{4}$ " A325 BOLTS.



SWA

SYMBOL	LAP (LENGTH)	SYMBOL	LAP (LENGTH)
	0'-2" (3 $\frac{3}{4}$ ")		2'-4" (2'-5 $\frac{3}{4}$ ")
	1'-4" (1'-5 $\frac{3}{4}$ ")		3'-0" (3'-1 $\frac{3}{4}$ ")

Scale: NOT TO SCALE		
Drawn by:	KEB	7/28/14
Checked by:	KEB	7/28/14
Project Engineer: JS		
Job Number:	14-B-54756-1	
Sheet Number:	E3 of 11	
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.		
S. Harley Davidson, P.E. Mississippi P.E. 13872		

DRME1A ENME1A

Jul 28, 2014

Jul 28, 2014

EWB

KEY PLAN

SWA

PANEL TYPE = PBR (GALVALUME)
PANEL OVERHANG = 3"
FROM OUTER STEEL

ROOF SHEETING PLAN

PANEL TYPE = PBR (GALVALUME)
PANEL OVERHANG = 3"
FROM OUTER STEEL

WPR04004
TYP. @ (4 CORNERS)

WPR05002

F679-1 -

WPR05002

WPR02001

WPR05002

WPR05002

WPR02002

GPR06005	
RIDGE EXPANSION	
F676-1A	
HW5227	
#226	
#228	

GPR06001
GPR06002

Jul 28, 2014

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

DRME1A ENME1A

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061

Project Name & Location:
JINDAL SAW USA
1 ROAD A.
BAY ST. LOUIS MS 39520

Customer:
AMERICAN WESTERN STEEL LLC
DBA W WILSON CONSTRUCTION
5750 N. SAM HOUSTON PKWY E

Drawing Status:

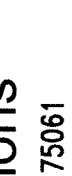
☐ Preliminary
(Not For Construction)

☐ For Approval
(Not For Construction)

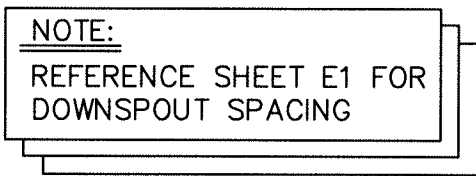
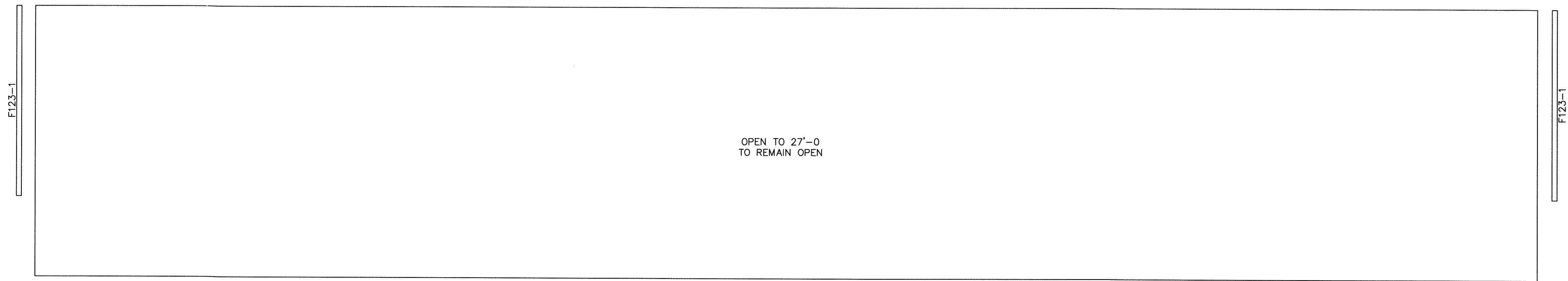
☐ For Construction Permit

☒ For Erector Installation

[illegible]

 Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737	<i>Project Name & Location:</i> JINDAL SAW USA 1 ROAD A BAY ST. LOUIS MS 39520	☐ For Construction Permit ☒ For Erector Installation
<i>Customer:</i> AMERICAN WESTERN STEEL LLC DBA W WILSON CONSTRUCTION 5750 N. SAM HOUSTON PKWY E HOUSTON TX 77032 MICHAEL WYMAN	<i>Drawing Status:</i> ☐ Preliminary (Not For Construction) ☐ For Approval (Not For Construction)	

DRME1A ENME1A



SWC

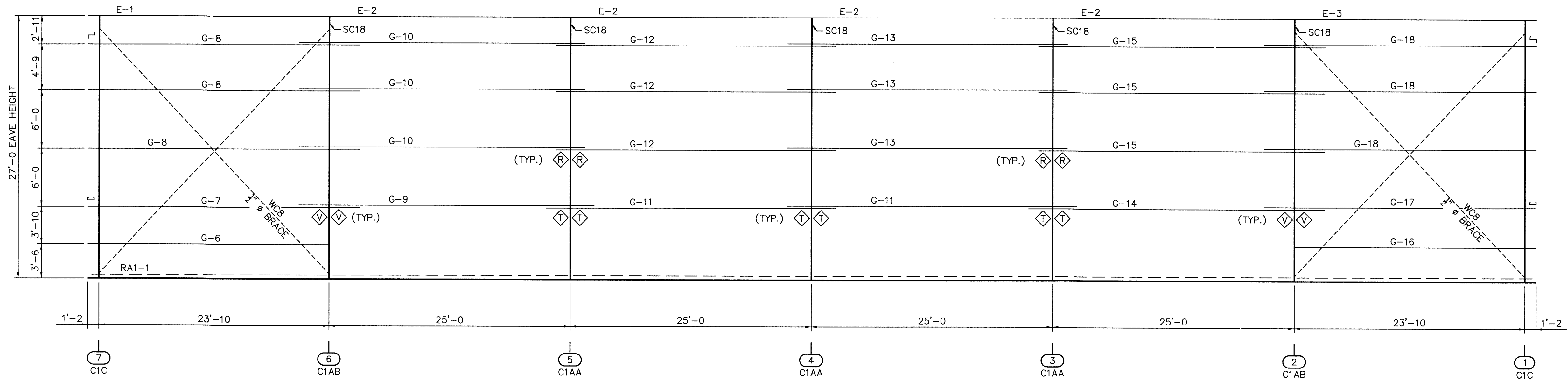
KEY PLAN

SWA

EWB

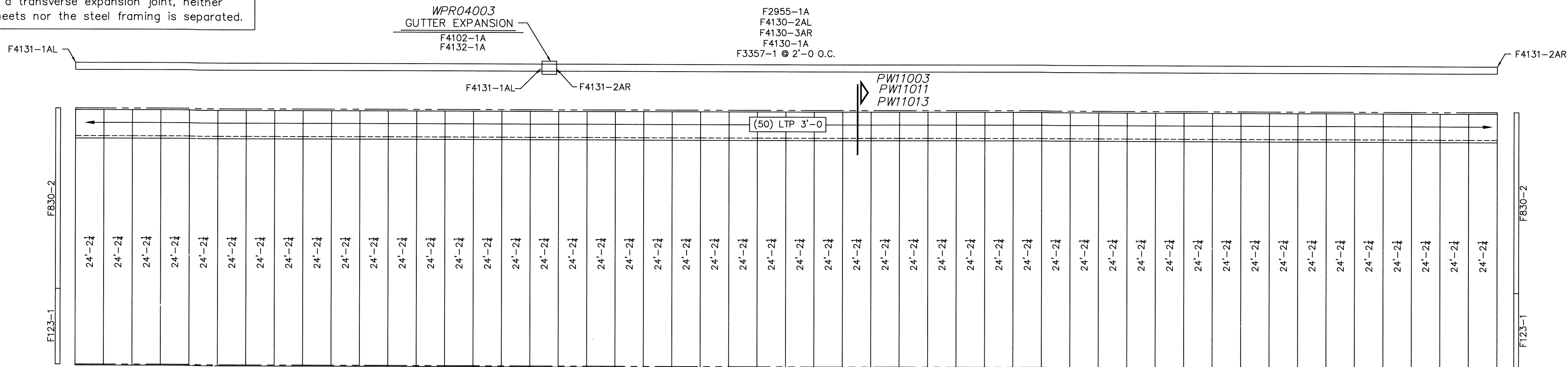
EWD

Jul 28, 2014



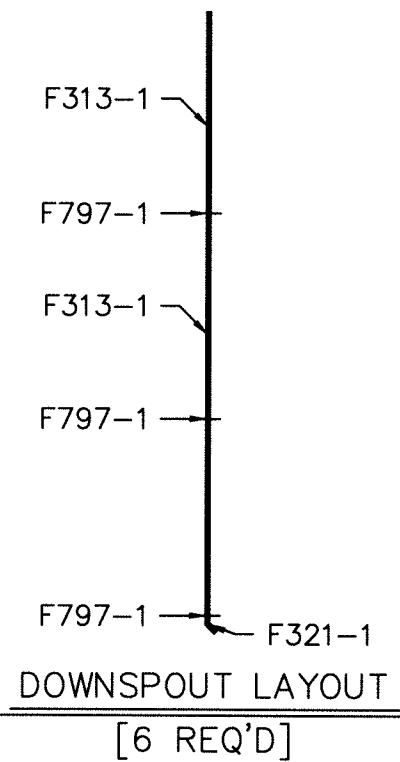
Note:

Gutter Expansion will require a 4" expansion lap every 100'-0" of building length. This expansion lap is only for the gutter, not the eave trim. This is not a transverse expansion joint, neither the roof sheets nor the steel framing is separated.



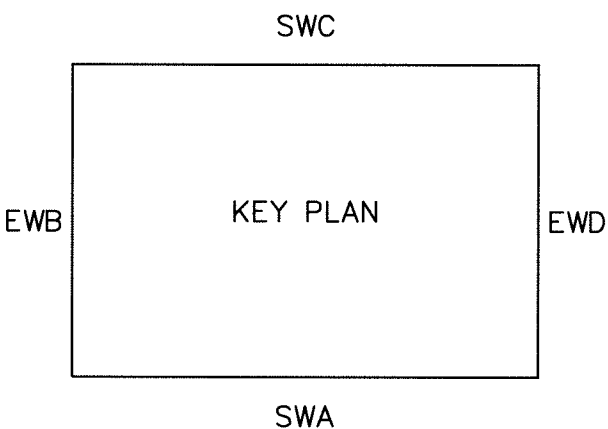
PBR WALL PANELS
PANEL COVERAGE = 3'-0
COLOR = LIGHT STONE
PANEL PKG. REQ'D. = PBS-2

WALL SHEETING ELEVATION "SWC"
BLDG A



NOTE:
REFERENCE SHEET E1 FOR
DOWNSPOUT SPACING

Z_SECTION LAP TABLE			
SYMBOL	LAP (LENGTH)	SYMBOL	LAP (LENGTH)
	0'-2" (3 3/4")		2'-4" (2'-5 3/4")
	1'-4" (1'-5 3/4")		3'-0" (3'-1 3/4")



Revision	Date	Description	By	Ck'd

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737



Customer:
AMERICAN WESTERN STEEL LLC
DBA WILSON CONSTRUCTION
15001 SHAW ROAD, SUITE 100
HOUSTON, TX 77032
MICHAEL VIVIAN

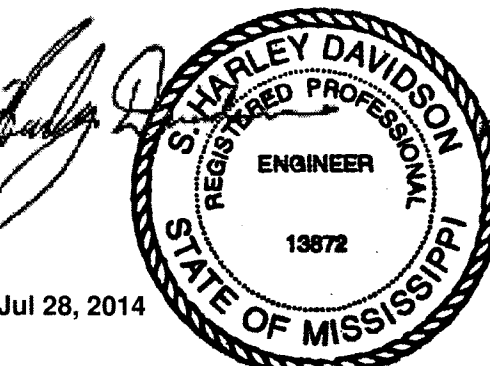
Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

Drawing Status:
☐ Preliminary
☐ (Not For Construction)
☐ For Approval
☒ For Construction Permit
☒ For Erector Installation

Scale: NOT TO SCALE
Drawn by: KEB 7/28/14
Checked by: KEB 7/28/14
Project Engineer: JS
Job Number: 14-B-54756-1
Sheet Number: E6 of 11

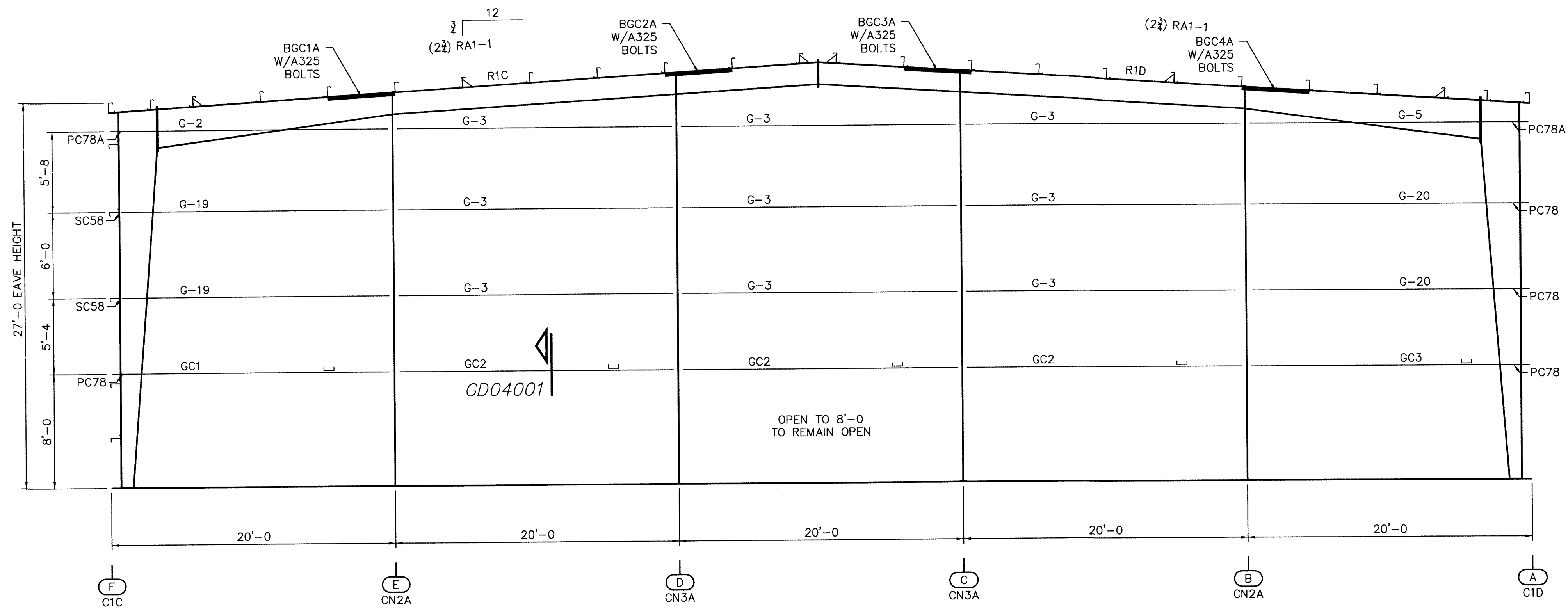
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

S. Harley Davidson, P.E.
Mississippi P.E. 13872
DRME1A ENME1A



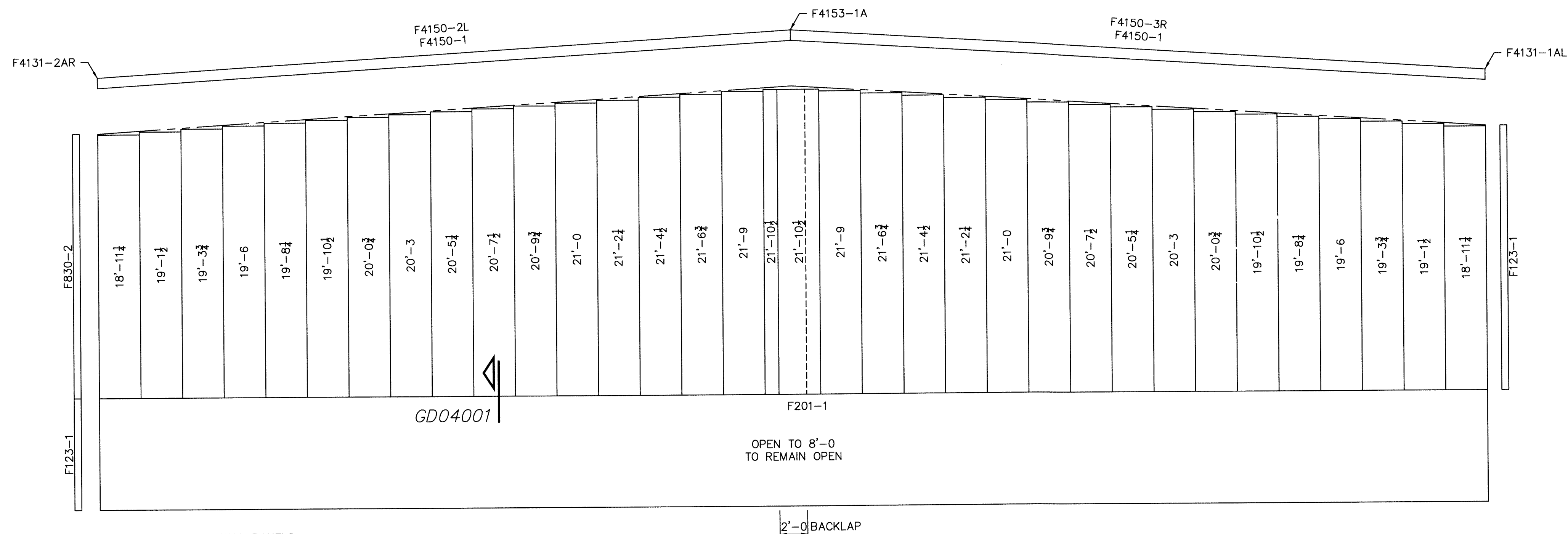
(4) 1 1/2" x 1 1/4" A325 BOLTS @ EACH PURLIN
(4) 3/4" x 1 1/2" A325 BOLTS @ EACH COLUMN

CL292- FASTENS BETWEEN THE GIRTS ON EACH SIDE OF THE ENDWALL COLUMNS, AT ALL GIRT ELEVATIONS. REFER TO DETAILS.



ENDWALL ELEVATION "EWB" AT GRID LINE "1"

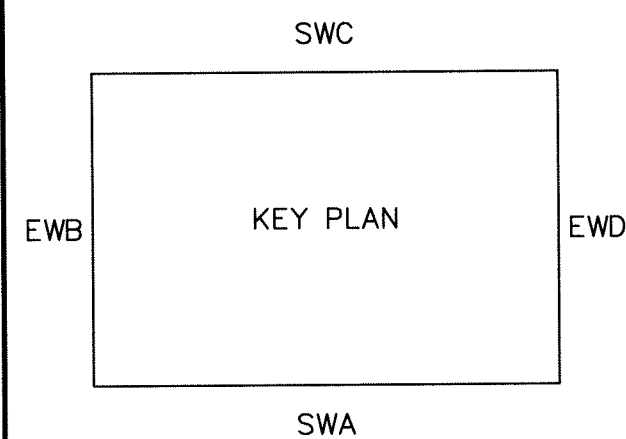
APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
CN3A	536
CN2A	488
C1C	786
C1D	1265
R1C	1075
R1D	1064



PBR WALL PANELS
PANEL COVERAGE = 3'-0"
COLOR = LIGHT STONE
PANEL PKG. REQ'D. = PBS-1

WALL SHEETING ELEVATION "EWB"

BLDG A

[illegible]

 Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737		Project Name & Location: JINDAL SAW USA 1 ROAD A. BAY ST. LOUIS MS 39520		☐ For Construction Permit ☒ For Erector Installation	
Customer: AMERICAN WESTERN STEEL LLC DBA W WILSON CONSTRUCTION 5750 N. SAM HOUSTON PKWY E HOUSTON TX 77032 MICHAEL VIVIAN		Drawing Status: ☐ Preliminary ☐ Not for Construction		☐ For Approval ☐ Not for Construction	
Scale: NOT TO SCALE					
Drawn by: KEB 7/28/14					
Checked by: KEB 7/28/14					
Project Engineer: JS					
Job Number: 14-B-54756-1					
Sheet Number: E7 of 11					
The engineer whose seal appears hereon is an employee of the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.					
S. Harley Davidson, P.E. Mississippi P.E. 13872					

DRME1A ENME1A

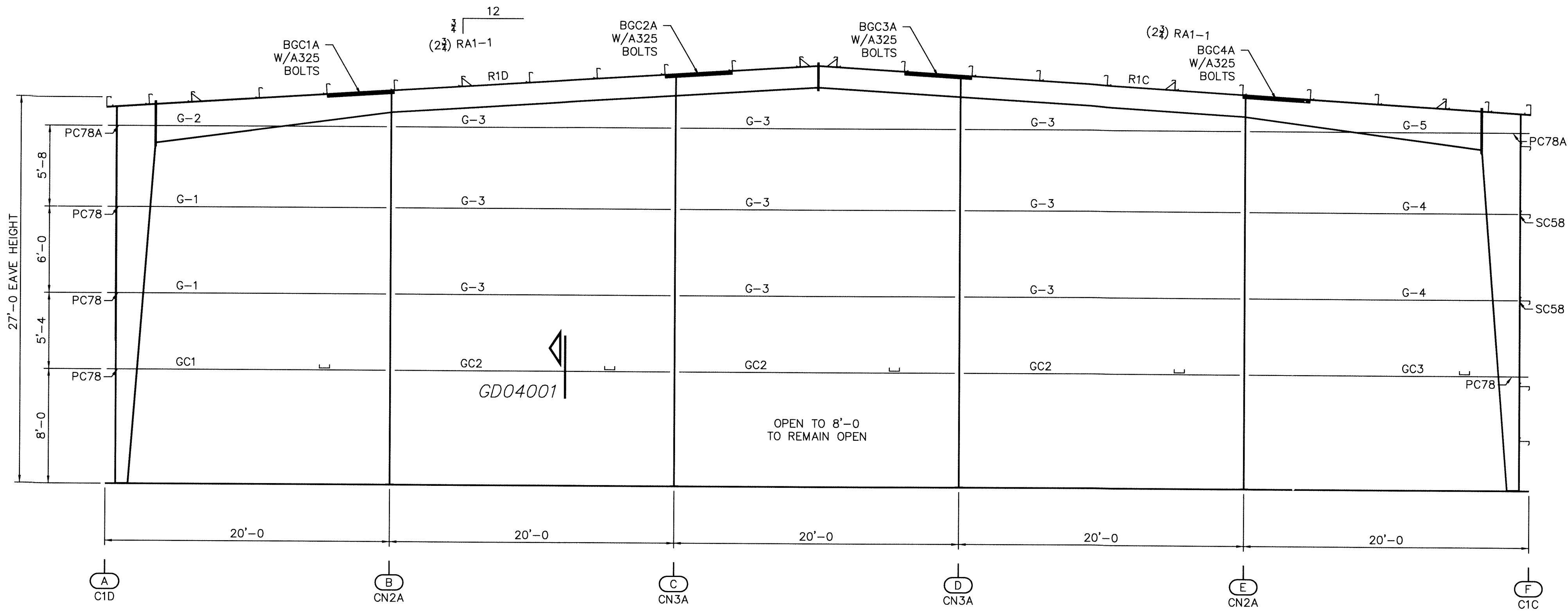
David

The seal is circular with a rope-like border. The outer ring contains the text "STANLEY DAVIDSON" at the top and "STATE OF MISSISSIPPI" at the bottom. The inner circle contains the text "REGISTERED PROFESSIONAL" at the top, "ENGINEER" in the center, and "19872" at the bottom.

Jul 28, 2014

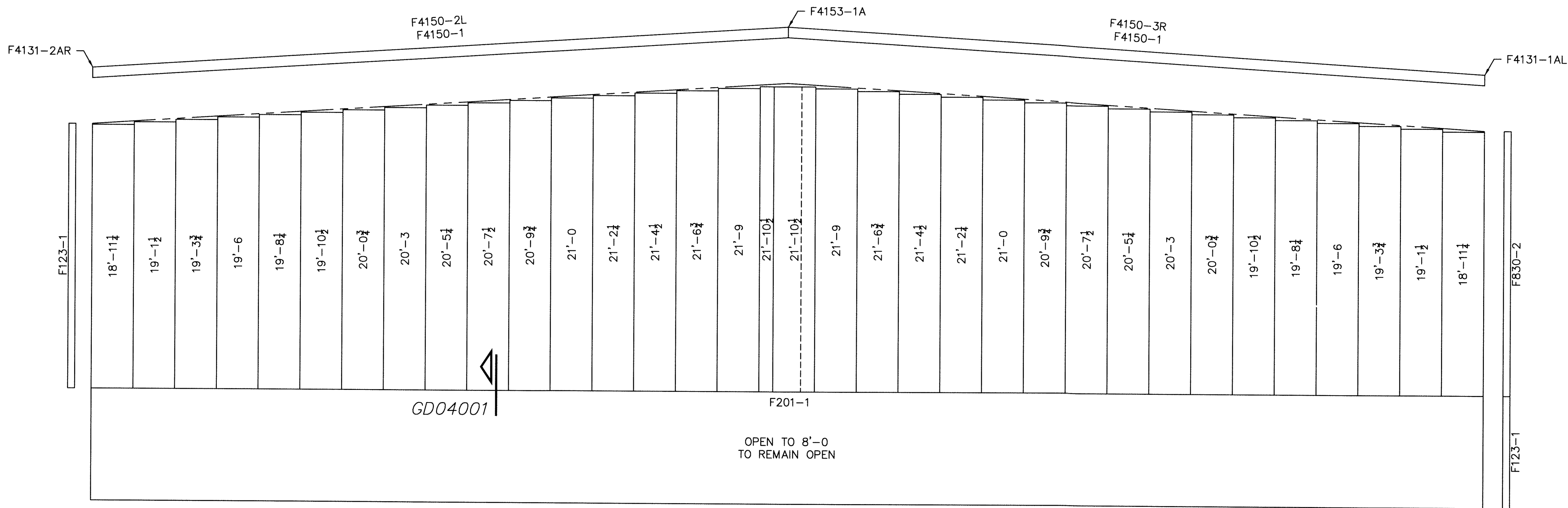
BRIDGE CHANNEL CONNECTION BGC
(4) 1/2" x 1 1/4" A325 BOLTS @ EACH PURLIN
(4) 3/4" x 1 1/2" A325 BOLTS @ EACH COLUMN

CL292- FASTENS BETWEEN THE
GIRTS ON EACH SIDE OF THE
ENDWALL COLUMNS, AT ALL
GIRT ELEVATIONS.
REFER TO DETAILS.



ENDWALL ELEVATION "EWD" AT GRID LINE "7"

APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
CN3A	536
CN2A	488
C1C	786
C1D	1265
R1C	1075
R1D	1064



PBR WALL PANELS
PANEL COVERAGE = 3'-0"
COLOR = LIGHT STONE
PANEL PKG. REQ'D. = PBS-3

WALL SHEETING ELEVATION "EWD"
BLDG A

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737



Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

Customer:
AMERICAN WESTERN STEEL LLC
DBA WILSON CONSTRUCTION
HOUSTON TX 77032
MICHAEL VIVIAN

Drawing Status:
☐ Preliminary
☒ For Construction Permit
☐ For Approval
☒ For Erector Installation

Scale: NOT TO SCALE
Drawn by: KEB 7/28/14
Checked by: KEB 7/28/14
Project Engineer: JS
Job Number: 14-B-54756-1
Sheet Number: E8 of 11

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

S. Harley Davidson, P.E.
Mississippi P.E. 13872

DRME1A ENME1A

S. HARLEY DAVIDSON
REGISTERED PROFESSIONAL
ENGINEER
13872
STATE OF MISSISSIPPI
Jul 28, 2014

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND
MAY VARY DUE TO CONDITIONS (DEFLECTION).

VERTICAL CLEARANCE DIMENSIONS ARE FROM
FINISHED FLOOR REFERENCE ELEVATION.

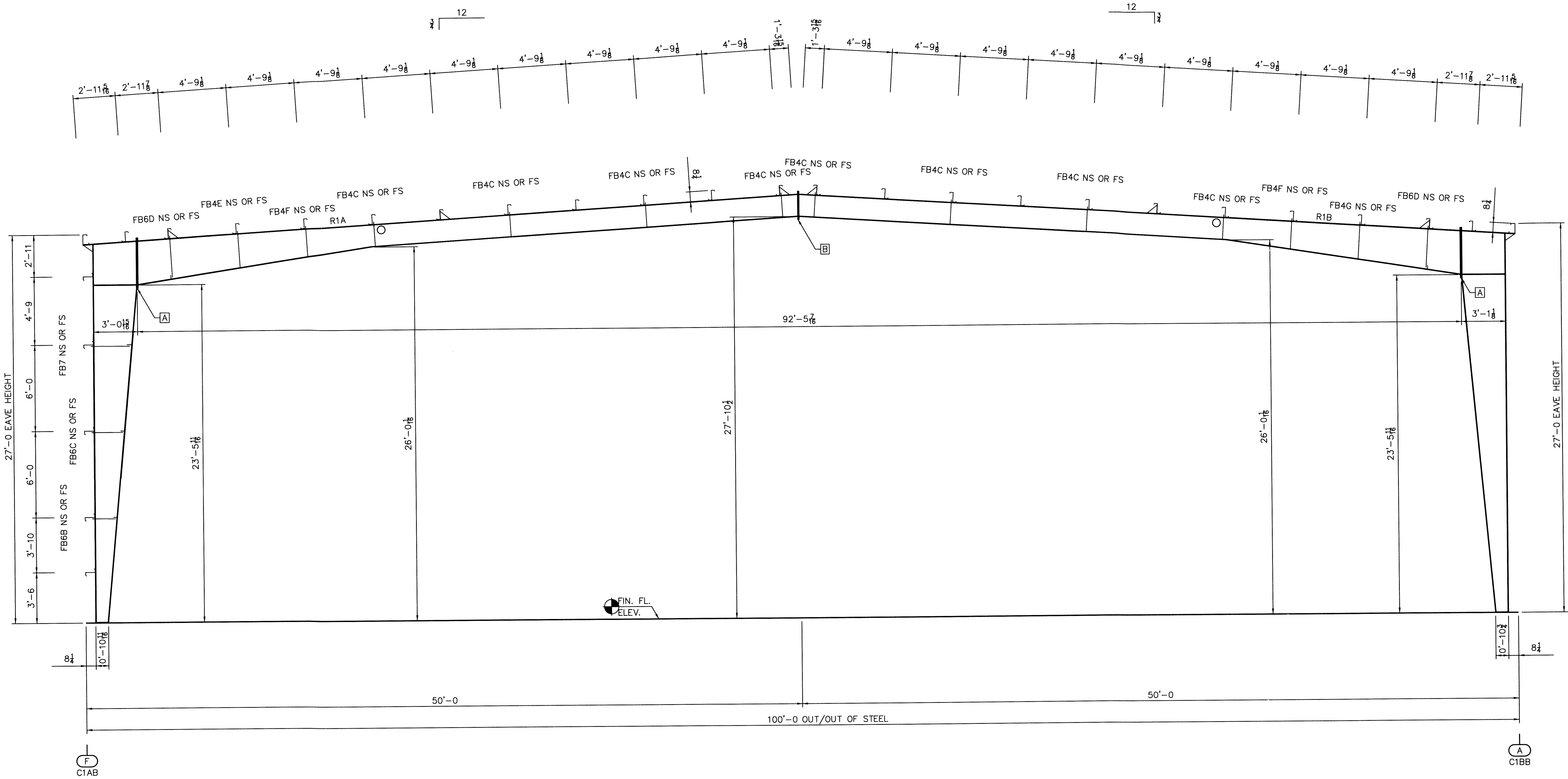


APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R1C	1075
R1D	1064
C1C	786
C1D	1265

	SPLICE BOLT TABLE				
CONN.	QTY.	SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	(12)	$\frac{3}{4}$ X 2"	A325 B&N	0	0
B	(8)	$\frac{3}{4}$ X 2"	A325 B&N	0	0

Jul 28, 2014

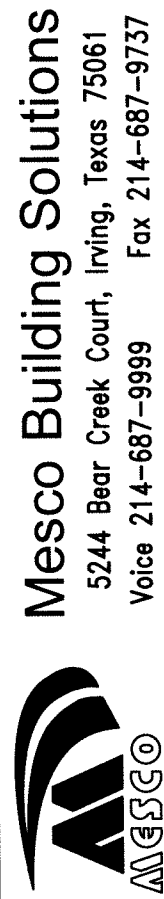
GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND
MAY VARY DUE TO CONDITIONS (DEFLECTION).
VERTICAL CLEARANCE DIMENSIONS ARE FROM
FINISHED FLOOR REFERENCE ELEVATION.



CROSS SECTION AT FRAME LINE "2" & "6"

APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R1A	1231
R1B	1244
C1AB	995
C1BB	1452

SPlice Bolt Table				
CONN.	QTY.	SIZE	TYPE	HARDENED BEVELED WASHERS
A	(16)	3/4" X 2 1/2"	A325 B&N	0
B	(8)	3/4" X 2"	A325 B&N	0



Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

Customer:
AMERICAN WESTERN STEEL LLC
DBA W WILSON CONSTRUCTION
5750 N. SAM HOUSTON PKWY E
HOUSTON TX 77032
MICHAEL WYMAN

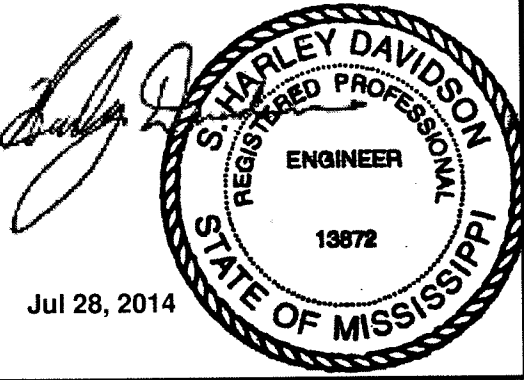
Project Name & Location:
JINDAL SAW USA
1 ROAD A.
BAY ST. LOUIS MS 39520

Drawing Status:
☐ Preliminary
☐ (Not For Construction)
☒ For Approval
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: KEB 7/28/14
Checked by: KEB 7/28/14
Project Engineer: JS
Job Number: 14-B-54756-1
Sheet Number: E10 of 11

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

S. Harley Davidson, P.E.
Mississippi P.E. 13872



GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND
MAY VARY DUE TO CONDITIONS (DEFLECTION).
VERTICAL CLEARANCE DIMENSIONS ARE FROM
FINISHED FLOOR REFERENCE ELEVATION.



APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R1A	1231
R1B	1244
C1AA	990
C1BA	1450

	SPLICE BOLT TABLE				
CONN.	QTY.	SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	(16)	$\frac{3}{4}$ X $2\frac{1}{2}$	A325 B&N	0	0
B	(8)	$\frac{3}{4}$ X 2"	A325 B&N	0	0

[illegible]

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737



Customer:	AMERICAN WESTERN STEEL LLC
Project Name & Location:	JINDAL SAW USA

Customer: AMERICAN WESTERN STEEL LLC

MICHAEL VIVIAN	Drawing Status:		☐ Preliminary (Not For Construction)		☐ For Construction Permit
			☐ For Approval (Not For Construction)		☒ For Erector Installation

Scale: NOT TO SCALE	
Drawn by: KEB	7/28/14
Checked by: KEB	7/28/14
Project Engineer: JS	
Job Number: 14-B-54756-1	
Sheet Number:	E11 of 11
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.	

S. Harley Davidson, P.E.
Mississippi P.E. 13872

DRME1A ENME1A




Jul 28, 2014

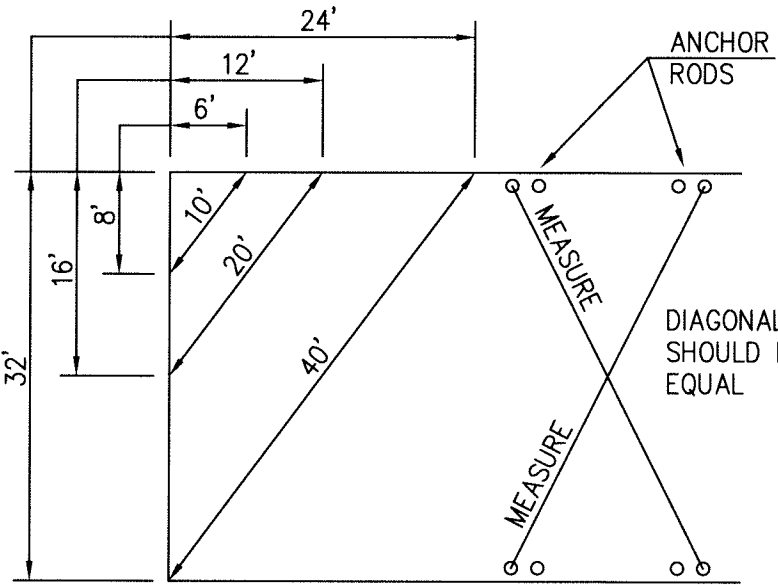
BUILDING ANCHORAGE

DIMENSIONS MUST BE EQUAL

TRANSIT

The following notes, procedures and suggested recommendations are important parts of the pre-erection process.

- The drawing shown below indicates a method which may be used to check the foundation and bolts for square.



7.) Unload and place panels and insulation out of the way.

9.) All hardware boxes should be protected from theft and moisture, especially items such as tube caulking and locksets. Store mastic away from heat.

1. Girts, Eave Struts and Purlins
2. End Frames and Endpost
3. Main Frames
4. Clips, Bolts, Screws, ETC.
5. Endwall Girts

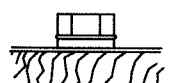
- 1.) Layout primary and secondary framing around the slab as shown.
- 2.) Place components and crates on the slab or on wood blocking to prevent contact with the ground.
- 3.) Block one end of components higher than other end to allow drainage of rain water.
- 4.) Leave one end of the building open for erection equipment access.
- 5.) Construct temporary ramp of timbers from grade to slab to prevent damage to concrete edge from equipment traffic.
- 6.) Install clips and flange braces onto columns and rafters before these members are in the air. Clip and flange brace locations are shown on erection drawings.

-
- A schematic diagram showing a cross-section of a building's foundation. The building is represented by a rectangular block with internal vertical lines, resting on a horizontal line representing the ground surface. Below this surface, several wavy vertical lines represent the soil profile.

CORRECT DEGREE OF



TOO TIGHT SEALANT SQU



TOO TIGHT SEALANT SQUEEZED

LOOSE SEALANT IS

NOTE: Always remove metal filings from surface of panels at the end of each work period. Rusting filings can destroy the paint finish and void any warranty.

Proper mastic application is critical to the weather tightness of a building. Mastic should not be stretched when installed. Apply only to clean, dry surfaces. Keep only enough mastic on the roof that can be installed in a day. During warm weather, store mastic in a cool dry place. During cold weather (below 60°) mastic must be kept warm (60°-90°) until application. After mastic has been applied, keep protective paper in place until panel is ready to be installed.

All details, recommendations and suggestions contained in the ERECTION GUIDE portion of this drawings set are for general guidelines only, and not meant to be all-inclusive. Industry accepted installation practices with regard to all areas not specifically discussed in this section should be followed. Only experienced, knowledgeable installers familiar with accepted practices should be used to assure a quality project.

It is emphasized that the Manufacturer is only a manufacturer of metal building components and is not engaged in the installation of its products. Opinions expressed by the Manufacturer about installation practices noted in the ERECTION GUIDE are intended to represent only a guide as to the sequencing and how the components could be assembled to create a building. Both the quality and safety of installation and the ultimate customer satisfaction with the completed building are determined by the experience, expertise, and skills of the installation crews, as well as the equipment available for handling the materials. Actual installation operations, techniques and site conditions are beyond the Manufacturers control.

The diagram illustrates a building frame under construction. Key components labeled include:

- BRACING**: Temporary structural members used to stabilize the frame.
- PRIMARY FRAME**: The main structural frame of the building.
- GIRT**: A horizontal member connecting the end frame rafter to the primary frame.
- EAVE STRUT**: A diagonal member supporting the eave of the end frame.
- END FRAME RAFTER**: The sloped roof member of the end frame.
- END POST**: The vertical support member of the end frame.
- CORNER COLUMN**: The vertical support member at the corner of the building.
- INTERIOR BAY**: The main span of the building.
- END BAY (BRACED)**: The bay at the end of the building, which is shown with temporary bracing.

NOTE
It is the responsibility of the erector to provide temporary erection bracing until the structure is completed.

The diagram illustrates a roof truss system with the following components labeled:

- PURLINS**: Horizontal members supporting the roof deck.
- PRIMARY FRAME RAFTER**: The main structural member for the roof slope.
- END FRAME**: The structural member at the gable end of the roof.
- END POST**: The vertical support member at the gable end.
- BRACING**: Members used to provide lateral stability to the frame.
- TEMPORARY ERECTION BRACING**: Bracing used during construction to support the structure before it is fully self-supporting.

NOTE
It is the responsibility of the erector to provide temporary erection bracing until the structure is complete.

2D: Plumb and square the first bay. After alignment, tighten wall bracing first and the roof bracing working from eave to peak. Tighten any remaining bolts.

Plumbing and aligning a total structural system begins with the first braced bay and continues through completion. Accurate alignment of the first bay is essential for correct alignment of succeeding bays. The installer is responsible for choosing the best method suited for plumbing and aligning the structural system.

Diagram illustrating the temporary erection bracing for a building frame. The structure is shown in a perspective view, highlighting the temporary bracing system used to stabilize the frame during construction. Labels include:

- END BAY PURLIN
- END FRAME RAFTER
- END POST
- ENDWALL
- CORNER COLUMN
- END BAY GIRT
- TEMPORARY ERECTION BRACING

NOTE
It is the responsibility of the erector to provide temporary erection bracing until the structure is completed.

3B: Attach wall girts to the primary frame columns (see step 1C).

3C: Install eave struts (see step 1D).

3D: Attach roof purlins for this bay to the two rafters. Purlins will bolt to the rafter flange in the same manner as girts to column flanges (see step 1C). Connect flange braces to purlins.

3E: Check alignment, plumb and square the two bays just erected. Tighten all bolts and bracing.

The diagram illustrates a temporary erection bracing system for a roof structure. It shows a rectangular frame with a series of parallel members. Labels include: EAVE STRUT, PURLINS, GIRTS, PRIMARY FRAME, TEMPORARY ERECTION BRACING, BRACING, END POST, and END FRAME. A note at the top right states: 'NOTE: It is the responsibility of the erector to provide temporary erection bracing until the structure is completed.'

Starting at the opposite end of the first bay erected, install the remaining interior frames, girts, purlins, eave struts, bracing, end frames and end posts using the procedures described in the preceding steps. Be sure all wall girts, roof purlins and flange braces as shown on the erection drawings are installed. Constant checks should be made to ensure the building is square, plumb and aligned.

All X-Bracing should be checked that it is installed to a taut condition with all slack removed. Do not tighten beyond this state.

[illegible]

STEP 5: INSTALL SIDEWALL PANELS

NOTE:
It is the responsibility of the erector to provide temporary erection bracing until the structure is completed. This bracing is to remain in place until all roof and wall panels are installed.

5A: Before installing wall panels, the girts must be aligned to a level position so that there is no visible sag. This should be done directly ahead of panel installation.

Girt leveling may be accomplished by standing a section of gable angle vertically against the outside girt flanges at approximate mid-bay location. When girts are level, attach the girt flanges to the angle with vise grip pliers or temporary screws. Wood blocking cut to fit the spaces may also be used for alignment.

NOTE: Temporary girt blocking is not recommended on concealed fastener panels. The removal of the blocks after panel installation can cause oil canning.

NOTE:
Wall panel type and installation details will vary. Refer to the erection drawings and details for the specific panel used for your building.

5B: If walls are to be insulated, place a continuous run of contact tape along the eave strut and base member.

NOTE:
At the base, cut off the insulation a minimum of 1/2" above the bottom of the wall panel. This will prevent the insulation from hanging below the wall panel and wicking moisture.

Eave Detail
(See Erection Drawings)

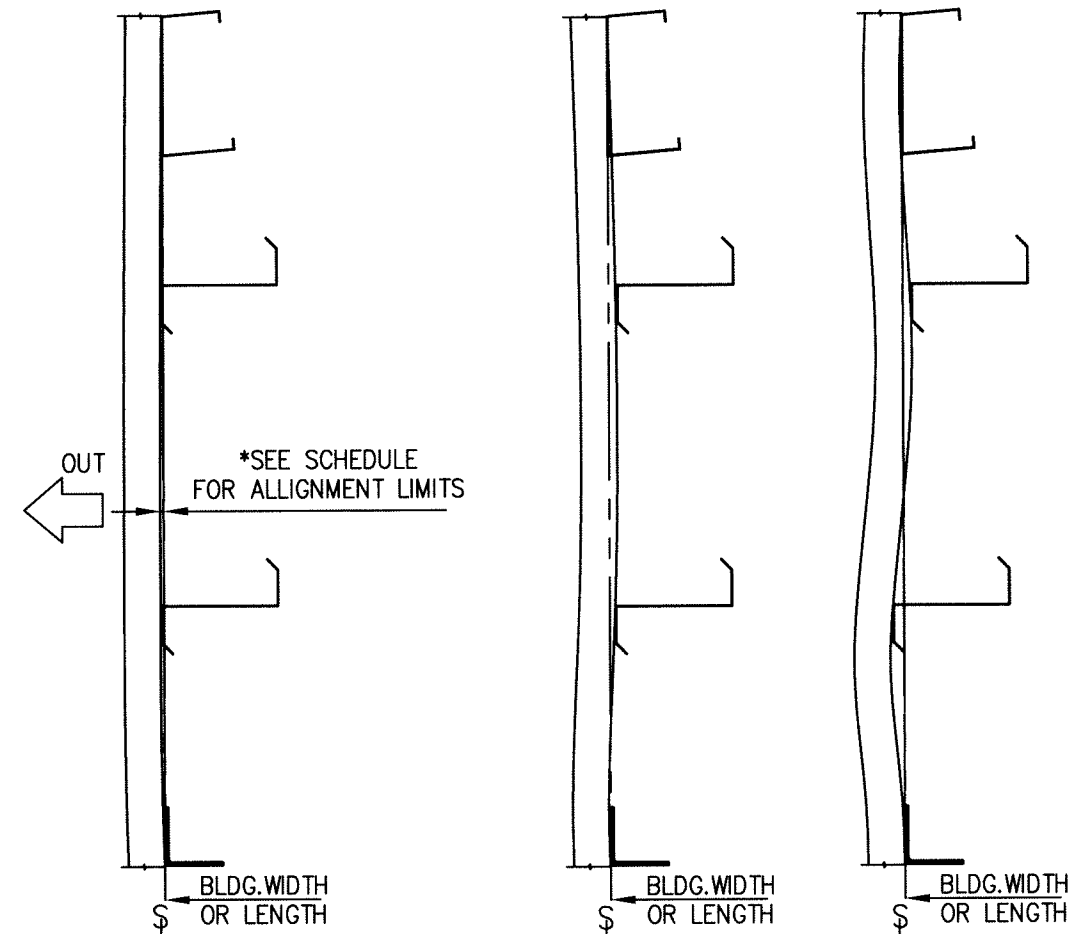
Base Detail
(See Erection Drawings)

Starting at a building corner, attach the first piece of blanket insulation to the contact tape on the eave strut. Pull tight and adhere to tape at the base. It is recommended that insulation not be installed more than 6 feet ahead of panels.

 Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737	Project Name & Location: JINDAL SAW USA DBA W WILSON CONSTRUCTION HOUSTON TX 77032 MICHAEL VIVIAN		Drawing Status: ☐ Preliminary ☐ (Not For Construction)	☐ For Construction Permit ☒ For Erector Installation	Revision Date Description By Ck'd
	Customer: AMERICAN WESTERN STEEL L DBA W WILSON CONSTRUCTION HOUSTON TX 77032 MICHAEL VIVIAN		☐ For Approval ☐ (Not For Construction)	☒ For Erector Installation	
	Scale: NOT TO SCALE		Drawn by: KEB 7/28/14		
	Checked by: KEB 7/28/14		Project Engineer:		
Job Number: 14-B-54756-1		Sheet Number: R4 of 13		The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.	

SECONDARY STEEL ALIGNMENT FOR ALL VERTICAL IMP PROJECTS

SUPPORT ALIGNMENT LIMITS	
SUPPORT SPAN	MAX DEVIATION LIMIT
Up To 4'-0"	1/16"
4'-0" To 8'-0"	1/8"
8'-0" And Up	1/4"



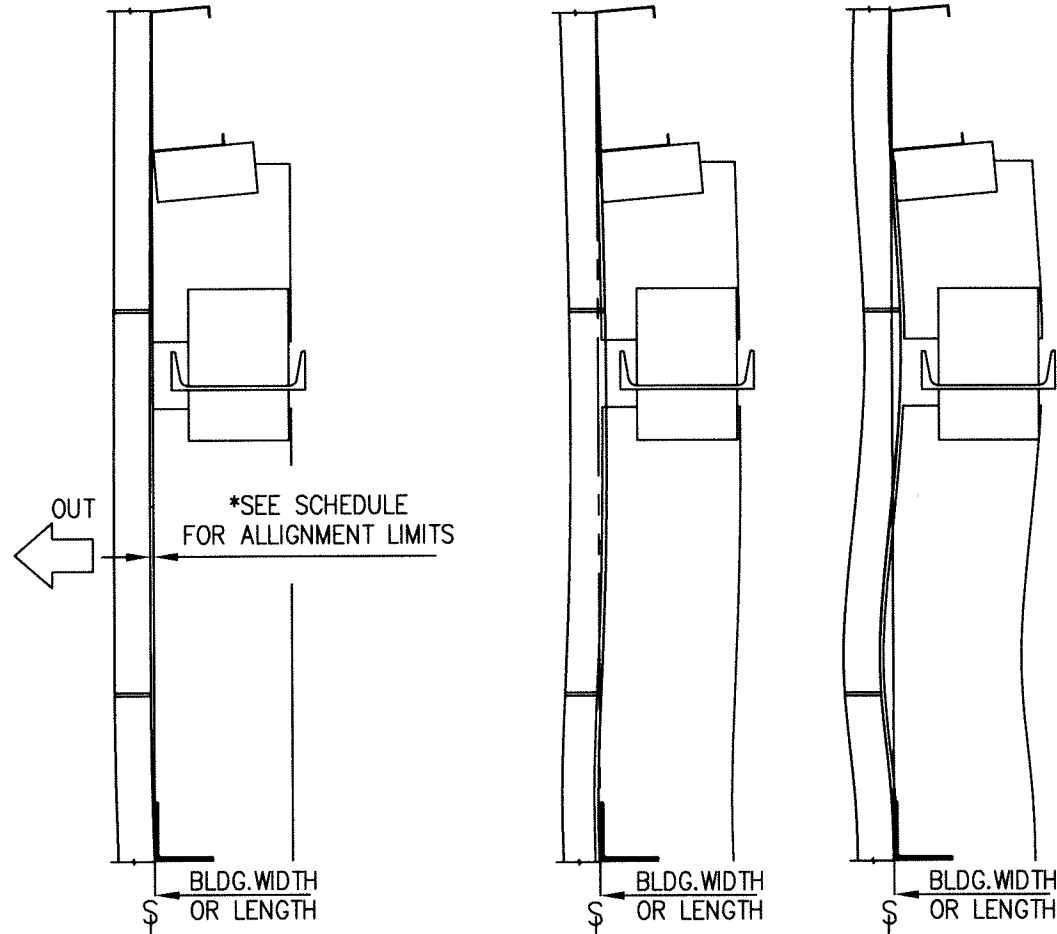
CORRECT ALIGNMENT

MIS-ALIGNMENT
(Unacceptable, Avoid These Conditions)

R5-1

SECONDARY STEEL ALIGNMENT FOR ALL HORIZONTAL IMP PROJECTS

SUPPORT ALIGNMENT LIMITS	
SUPPORT SPAN	MAX DEVIATION LIMIT
Up To 4'-0"	1/16"
4'-0" To 8'-0"	1/8"
8'-0" And Up	1/4"



CORRECT ALIGNMENT

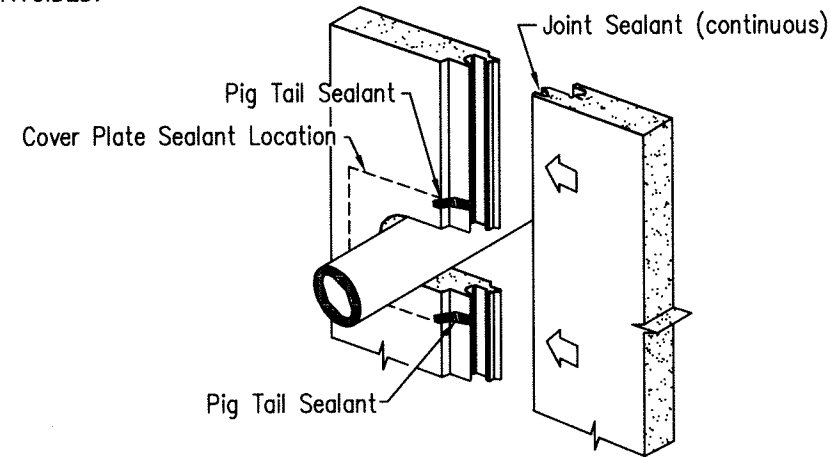
MIS-ALIGNMENT
(Unacceptable, Avoid
These Conditions)

R5-2

PENETRATION FLASHING THROUGH IMP WALLS

WEATHER SEAL - IF THE PENETRATION IS THROUGH AN EXTERIOR WALL WITH VERTICAL WALL PANEL JOINTS, IT IS BEST TO AVOID LOCATING THE PENETRATION WHERE IT WILL INTERSECT A WALL PANEL JOINT AND BE SUBJECT TO WATER DRAINING FROM THE PANEL JOINT INTO THE PENETRATION CAVITY.

SHOWN BELOW ARE WEATHER SEAL DETAILS WHEN INTERSECTING A PANEL JOINT CANNOT BE AVOIDED.

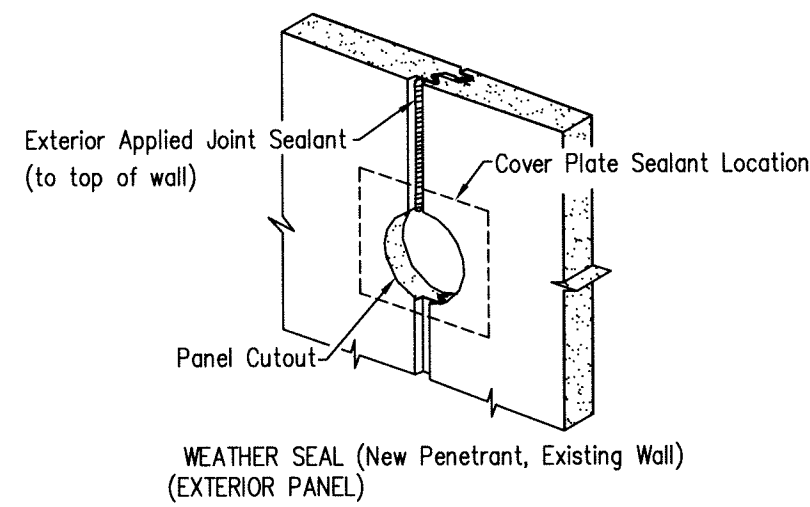


WEATHER & VAPOR SEAL (Existing Penetrant, New Wall)
(EXTERIOR PANEL)

EXISTING PENETRANT - NEW WALL
IF A NEW WALL IS INSTALLED AROUND AN EXISTING PENETRANT, SEALANT MUST BE
APPLIED TO THE EXTERIOR TONGUE & GROOVE OF THE WALL PANEL JOINT TO PREVENT
WATER ENTERING THE PANEL JOINT.

SEALANT PIGTAILS MUST ALSO BE APPLIED TO THE INTERFACE WITH THE PERIMETER SEALANT OF THE PENETRATION COVER PLATES.

NEW PENETRANT - EXISTING WALL
IF THE PENETRANT IS INSTALLED THROUGH AN EXISTING WALL, EITHER THE EXISTING WALL MUST HAVE BEEN INSTALLED WITH THE EXTERIOR JOINT SEALANT OR AN EXTERIOR GRADE SEALANT MUST NOW BE APPLIED ALONG THE EXTERIOR FILLET OF THE PANEL JOINT FOR THE FULL HEIGHT OF THE WALL.



R5-3

PENETRATION FLASHING THROUGH IMP WALLS (con't)

VAPOR SEALS - DEPENDING UPON THE BUILDINGS VAPOR CONTROL REQUIREMENTS, EITHER THE EXTERIOR OR INTERIOR SIDE OF THE WALL PANEL JOINTS MAY HAVE JOINT SEALANT TO FUNCTION AS THE VAPOR BARRIER.

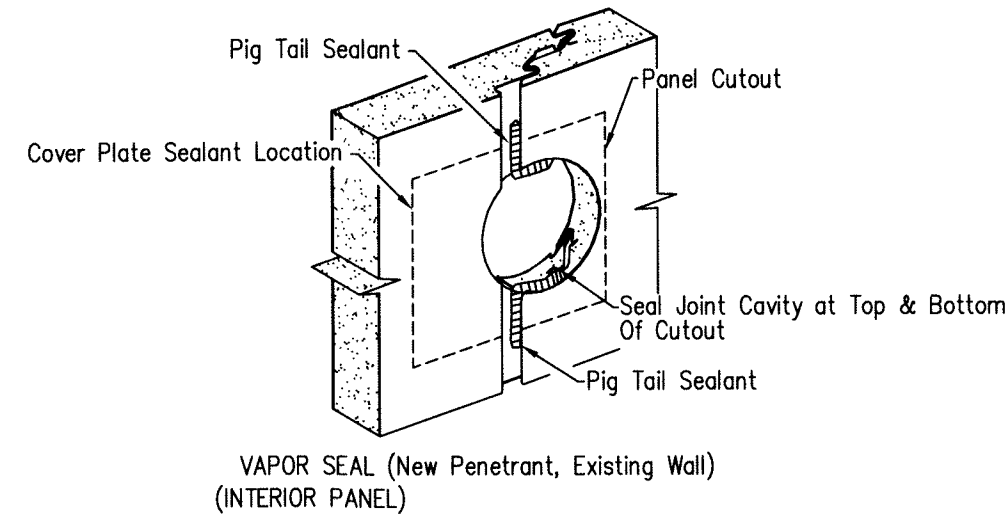
EXISTING PENETRANT - NEW WALL
ON AN EXTERIOR WALL WITH THE VAPOR BARRIER ON THE EXTERIOR SIDE OF THE WALL,
THE WEATHER SEAL DESCRIBED ABOVE ALSO FUNCTIONS AS THE VAPOR SEAL.

FOR INTERIOR WALLS AND FOR EXTERIOR WALLS WITH VAPOR BARRIER ON THE INTERIOR SIDE OF THE WALL, INSTALL THE PIGTAIL SEALANTS TO THE INTERFACE WITH THE COVER PLATE SEALANT IN THE SAME MANNER AS DESCRIBED ABOVE FOR THE WEATHER SEAL.

NEW PENETRANT - EXISTING WALL
TO PREVENT WATER VAPOR ENTERING THE PENETRATION CAVITY ON THE VAPOR BARRIER
SIDE OF THE WALL, PIGTAIL SEALANTS MUST BE APPLIED ON THE PANEL JOINT TO
INTERFACE WITH THE PERIMETER SEALANT OF THE PENETRATION COVER PLATES.

APPLY THE PIGTAIL SEALANT TO THE SEAL OF THE TONGUE-AND-GROOVE JOINT CAVITIES AT THE TOP AND BOTTOM EDGES OF THE PANEL CUT OUT.

EXTEND THE PIGTAIL SEALANT ALONG THE EXTERIOR FILLET OF THE PANEL JOINT TO INTERFACE WITH THE COVER PLATE SEALANT.



VAPOR SEAL (New Penetrant, Existing Wall)
(INTERIOR PANEL)

R5-4

By-Pass Zee Installation

Page
CF01121
Date
Dec '10
Rev
00

By-Pass Purlin and Girt Connections

Page
CF01122
Date
Dec '10
Rev
00

Girts at Column
Flush Sidewall or Endwall Girts

Page
CF01123
Date
Feb '11
Rev
01

Sidewall or Endwall Interior Flush Girt
Connection Detail

Page
PF02020
Date
Feb '11
Rev
01

WELDED CLIP PRIMARY FRAME DETAIL
GIRT ATTACHMENT DETAIL TO GIRT CLIP

SH: No.
CF01120
DATE
Feb '11
REV.
01

ALL BOLTS ARE A325 WITHOUT WASHERS

Weld Clip Corner Detail - Variable Frame Set Back
8" Bypass Girt Sidewall - 8" Girt Endwall

Page
EF03001
Date
Sep '12
Rev
02

Weld Clip Corner Detail
Variable Frame Setback - No Girt Bypass Sidewall - Girt at Endwall

Page
EF03012
Date
May '14
Rev
03

By-Pass Girt to Column
Standard Connection - Flange Brace FB4_

Page
CF01124
Date
Jan '14
Rev
01

Mesco Building Solutions

5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

Customer:
AMERICAN WESTERN STEEL L
DBA W WILSON CONSTRUCTIO
HOUSTON TX 77032
MICHAEL VIVIAN

Drawing Status:
☐ Preliminary
☐ Not For Construction
☒ For Approval
☐ Not For Construction
☒ For Erection Installation

Scale: NOT TO SCALE

Drawn by: KEB 7/28/14

Checked by: KEB 7/28/14

Project Engineer:

Job Number: 14-B-54756-1

Sheet Number: R6 of 13

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

By-Pass Girt to Column Standard Connection - Flange Brace FB6_ or FB7_ CF01127 Jan '14 01 Erection Note Lap and attachment of Girt varies refer to "Elevation Plan" and Lap Detail for requirements. Erection Note Flange Braces may be required one or both sides of Column. For requirements, and locations at Main Frame and Rigid Frame in Endwall refer to "Cross Section". For requirements, and locations at Bearing Frame Endwall refer to "Elevation Plan".	Purlin to Rafter Standard Connection - Flange Brace FB4_ CF02036 Dec '10 00 Erection Note Lap and attachment of Purlin varies refer to Roof Plan and Lap Detail for requirements. Erection Note Flange Braces may be required one or both sides of Rafter. For requirements, and locations at Main Frame and Rigid Frame in Endwall refer to "Cross Section". For requirements, and locations at Bearing Frame Endwall refer to "Roof Plan".	Purlin to Rafter Standard Connection - Flange Brace FB6_ or FB7_ CF02039 Dec '10 00 Erection Note Lap and attachment of Purlin varies refer to Roof Plan and Lap Detail for requirements. Erection Note Flange Braces may be required one or both sides of Rafter. For requirements, and locations at Main Frame and Rigid Frame in Endwall refer to "Cross Section". For requirements, and locations at Bearing Frame Endwall refer to "Roof Plan".	Eave Strut Stiffener Plate Low Eave Interior Bay CF02031 Sep '10 00 Section "A"
Eave Strut Connection at End Bay ByPass And Flush Girts CF02033 Feb '12 03 By-Pass Girts Flush Girts	Eave Strut Connection at Main Frame Interior Bay CF02030 Feb '11 01 By-Pass Flush	Purlin Anti-Roll Clip (at Built-up/Hot-Rolled Rafter) CF02034 Feb '11 01 Section A	Purlin Anti-Roll Clip (W/Back-up Plate) (at Built-up/Hot-Rolled Rafter) CF02035 Mar '13 02 Section A

Mesco Building Solutions 5244 Bear Creek Court, Irving, Texas 75061 Voice 214-687-9999 Fax 214-687-9737	Project Name & Location: JINDAL SAW USA 1 ROAD A. BAY ST. LOUIS MS 39520	Customer: AMERICAN WESTERN STEEL L DBA W WILSON CONSTRUCTIO HOUSTON TX 77032 MICHAEL VIVIAN Drawing Status: ☐ Preliminary ☐ (Not For Construction) ☐ For Approval ☒ (Not For Construction) Permit ☐ For Construction ☒ For Erector Installation
Scale: NOT TO SCALE	Drawn by: KEB 7/28/14	Checked by: KEB 7/28/14
Project Engineer:	Job Number: 14-B-54756-1	Sheet Number: R7 of 13
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.		

END FRAME DETAIL
RIGID END FRAME WITH INSET GIRTS
END POST NOT AT PEAK

SHT. NO. EF08010
DATE May 09 REV. 00

NOTES:
- Use 1/2"Ø A325 bolts unless noted otherwise.
- Frame shapes may vary.
- See erection drawing for elevations and marks.

NOTE: SEE END POST TO PURLIN/BRIDGE CLIP CONNECTION

RAFTER
(NOT ATTACHED TO COLUMN)

ANCHOR RODS WITH NUTS AND WASHERS

END POST

Endwall Column attachment
located between Purlins at Slope

SHT. NO. EF09102
DATE Dec '12 REV. 01

Notes:
1.) See AC00916 / AC00920 for Jamb/Support Column attachment when located at same Purlin space (or adjacent Purlin space) as Endwall Column.
2.) Bridge Channel may span multiple Purlin spaces when required.
3.) Wind Edge Strip Purlins, when required, do not attach to Bridge Channel.
4.) Purlins and Purlin Strut extends past Bridge Channel flange when "Optional" Overhang or Facade is required at endwall.

Downhill

Purlin

Purlin Strut
(When required.)

Purlin

(4) 3/8"Ø Bolts
(Typical at Purlins and Purlin Strut)

Bridge Channel
Cold Formed - BGC_ (Shown)
Hot Rolled - BG_ (Similar)

(4) 3/8"Ø A325 Bolts

Endwall Column or Column Extension
(Bolt to downhill side of Connection Plate.)

PRIMARY FRAME DETAIL
BY-PASS - NO GIRTS WITH X-BRACING

SHT. NO. PF02002
DATE Feb. 10 REV. 01

RAFTER

EAVE STRUT

RAFTER

COLUMN

X-BRACING

ANCHOR RODS AND NUTS WITH WASHERS

X-BRACE CONNECTION

NOTES: FRAME SHAPES MAY VARY.
SEE FRAME CROSS SECTIONS FOR ADDITIONAL FRAME INFORMATION.
X-BRACES ARE PERMANENT AND MAY NOT BE REMOVED AFTER ERECTION UNLESS NOTED FOR PLUMBING PURPOSES ON ERECTION DRAWINGS.

PRIMARY FRAME DETAIL
X-BRACING WEB MOUNTED PIPE STRUT

SHT. NO. PF04006
DATE DEC. 05 REV. 00

RAFTER

RAFTER

X-BRACING

X-BRACING

PIPE STRUT
(A325 BOLTS)

NOTES: FRAME SHAPES MAY VARY.
X-BRACES ARE PERMANENT AND MAY NOT BE REMOVED AFTER ERECTION UNLESS NOTED FOR PLUMBING PURPOSES ON ERECTION DRAWINGS.

Cable Brace
Attachment at Web

Page PF04111
Date Jan '14 Rev. 01

CABLE BRACE ASSEMBLY
1/4"Ø CABLE ~ WC4_
3/8"Ø CABLE ~ WC5_
1/2"Ø CABLE ~ WC6_
3/4"Ø CABLE ~ WC8_
(See Erection Drawings for required Part Mark and location)

BURR THREADS AFTER FINAL TIGHTENING

SLOPE WASHER

FLAT WASHER

HEX NUT

NOTE: THESE ITEMS SHIPPED ON ASSEMBLY

ERECTION NOTE: DETAIL SHOWN AT COLUMN SIMILAR AT RAFTER.

Align slot with Cable Bracing

For Cable Bracing that falls within the Girt Web, the Erector is to add a minimum size slot for the passage of the Cable Eye Bolt and Cable Wrap.

PBR Roof Panel Fastener
and Tape Sealant Location

Page GPR00011
Date Dec '12 Rev. 00

12" 12" 12"
9 1/2" 9 1/2" 9 1/2"
2 1/2" 2 1/2" 2 1/2"
1 1/4"
Sheeting Direction
See "GPR04016"

All Roof Members Except As Noted Below

12" 12" 12"
7" 5" 7" 5" 7"
2 1/2" 2 1/2" 2 1/2"
1 1/4"
Sheeting Direction
See "GPR04016"

At Eave Strut, Panel End Lap, and Peak Purlin

Note:
Screw Patterns Shown
Satisfy U.L. 90 Requirements
For Roof

PBR Panel
Side Lap and End Lap Details

Page GPR04016
Date Jun '14 Rev. 02

Sheeting Direction

Prevailing Wind

Fastener #4
1/4-14 x 8" LL
SD W/Washer
1"-8" O.C.
Tape Sealant
HW507

SECTION THRU PANEL SIDELAP

Fastener #4
1/4-14 x 8" LL SD W/Washer
at 1'-8" O.C.

Tape Sealant 1/2" x 3/8"
HW507

Two Continuous Layers
Tape Sealant 1/2" x 3/8"
HW507

Purlin

PANEL ENDLAP DETAIL

Fastener #3
12-14 x 1 1/2" LL
SD W/Washer
at 5"-7"-8" O.C.

Fastener #4
1/4-14 x 8" LL
SD W/Washer
at 1'-8" O.C.

4" piece of Tape Sealant
(Overlap Tape Sealant on
Closure by a 1/2" Minimum)

Eave Strut

Inside Closure HW455
with Tape Sealant HW507
top and bottom

Eave Trim
(When required, may require attachment
before installing Roof Panels)

PANEL SIDELAP DETAIL
AT LOW EAVE

PBR Panel
Expansion Ridge

Page GPR06002
Date Mar '14 Rev. 01

Fastener #4
1/4-14 x 8" LL
SD W/Washer at
6" O.C.

Tape Sealant

Outside Metal Closure
HW4041 or HW4040

Back-Up Plate
HW2998

2"

Expansion Ridge Flashing
F675

Q. Sym.

PBR Roof Panel

Back-Up Plate
HW2998

Peak Purlin

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

Customer:
AMERICAN WESTERN STEEL L
DBA W WILSON CONSTRUCTIO
HOUSTON TX 77032
MICHAEL VIVAN

Drawing Status:
☐ Preliminary
(Not For Construction)
☐ For Approval
(Not For Construction)
☐ For Construction Permit
☒ For Erector Installation

Scale: NOT TO SCALE

Drawn by: KEB 7/28/14

Checked by: KEB 7/28/14

Project Engineer:

Job Number: 14-B-54756-1

Sheet Number: R8 of 13

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Page
GPR06005
Date
Jan '14 Rev.
01

Roof Stitch Screw
1/4" - 14 x 7/8"
at 6" O.C.

Expansion Ridge Flashing
F675

"PBR" Roof Panel

Back-Up Plate 1/2" x 36"

1/2" Tape Sealant

"PBR" Roof Panel

Outside Metal
Flange
1/2" x 36"
or HW10x10

Fastener #4
1/4" - 14 x 7/8"
20 Required

F675

1"

2 Runs 1/4" Bead
Tube Sealant

F675

1/2" | 1/4"

2" Lap

Section A

A

A

Expansion Lap Cover F676

(2) Fastener in line with Tape Sealant HW506 one end only.

(38) Fastener #226 with Fastener #228 10 x $\frac{1}{2}$ Grommet Washer

Tape Sealant HW506 $\frac{3}{4}$ x 1" with Tube Sealant HW54 on top in line with Fastener

Expansion Ridge Trim F675

Dekstrip HW527 50 Required

2 $\frac{3}{4}$ "

2 $\frac{1}{2}$ "

6 $\frac{1}{2}$ "

2 $\frac{3}{4}$ "

1 $\frac{1}{2}$ "

Metal Closure

Tube Sealant HW54 on top of Tape Sealant at fastener line

Tape Sealant HW506 $\frac{3}{4}$ x 1" perimeter of Dekstrip HW527

Tube Sealant HW506 $\frac{3}{4}$ x 1" with Tube Sealant HW54 on top in line with Fastener

Section A

Fastener #226

Fastener #228

Dekstrip HW527

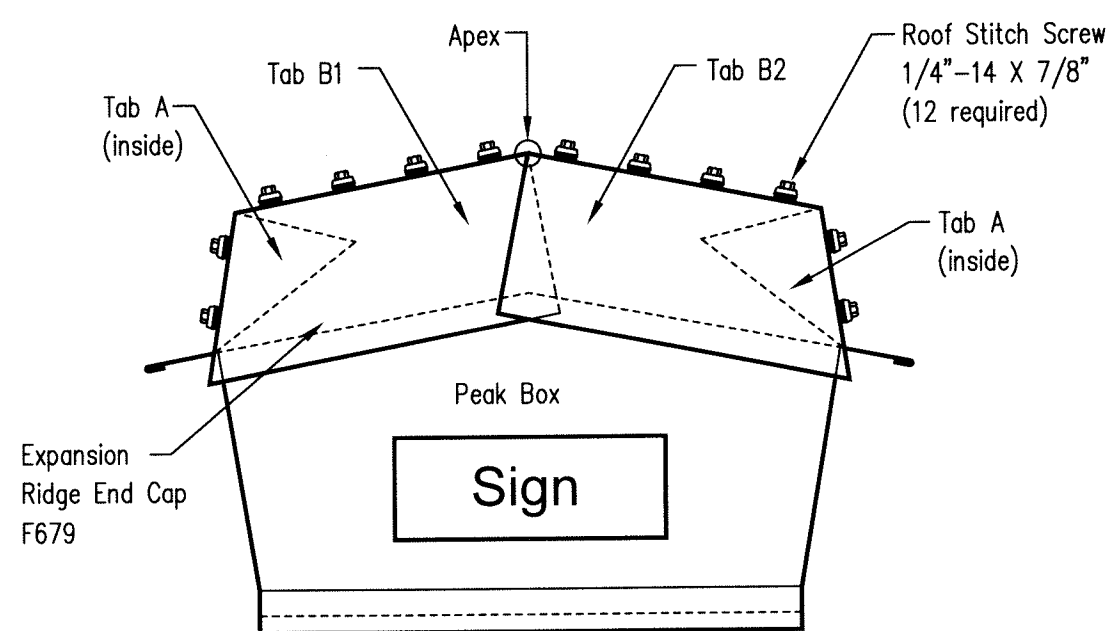
Expansion Ridge Trim F675

Tube Sealant HW54 on top of Tape Sealant at fastener line

Tape Sealant HW506 $\frac{3}{4}$ x 1" perimeter of Dekstrip

Note:
Expansion joint required at 100'-0" intervals.
Locate Expansion Joint between panel ribs only.

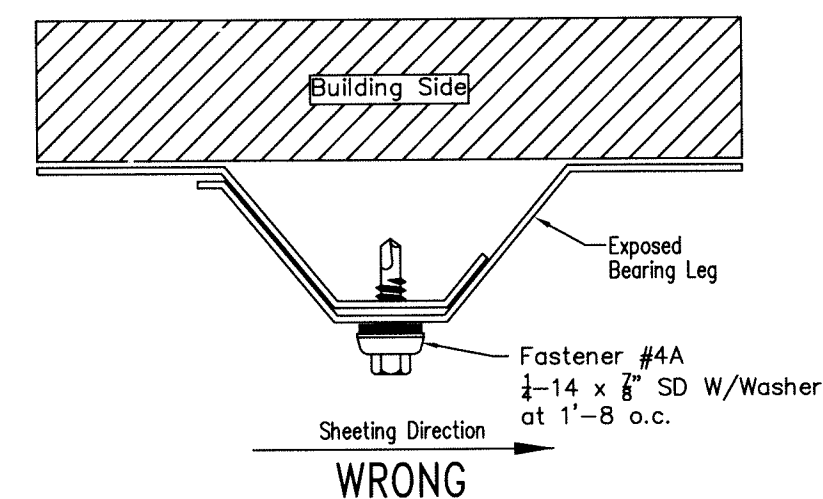
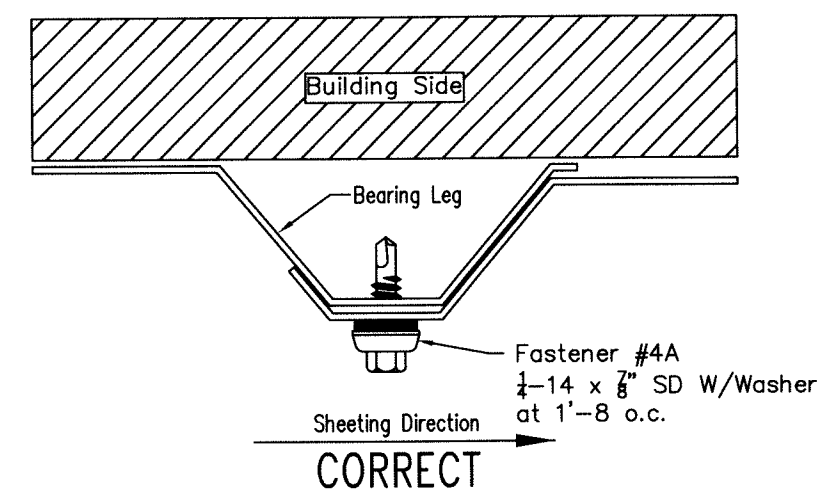
SHT. NO. GPR06000	
DATE Dec '12	REV. 00



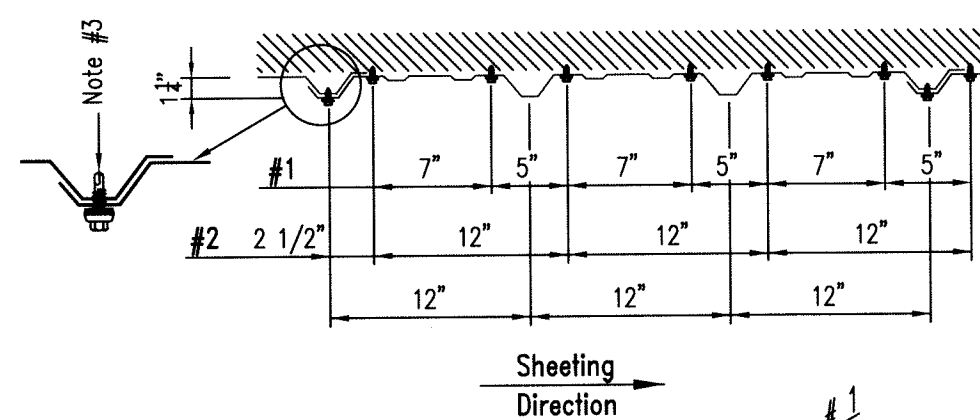
Expansion Ridge End Cap Assembly

1. Apply (2) runs of tube sealant along vertical and horizontal surfaces of expansion end cap.
2. Place end cap inside expansion ridge flashing, allowing the end cap to conform to the ridge flashing profile. Do not deform the top of the ridge by exerting too much pressure.
3. Make sure tab is even with, but not resting on top of the peak box. Tab "A" must be able to pivot in front of the peak box because of panel contraction.
4. Using a screwdriver, insert the blade in the apex of tab "B1" and "B2", and twist the blade enough to cause tab "B1" to spread slightly away from tab "B2".
5. Secure the end cap to the ridge flashing with (12) Stitch Screws 1/4"-14 X 7/8" as shown.

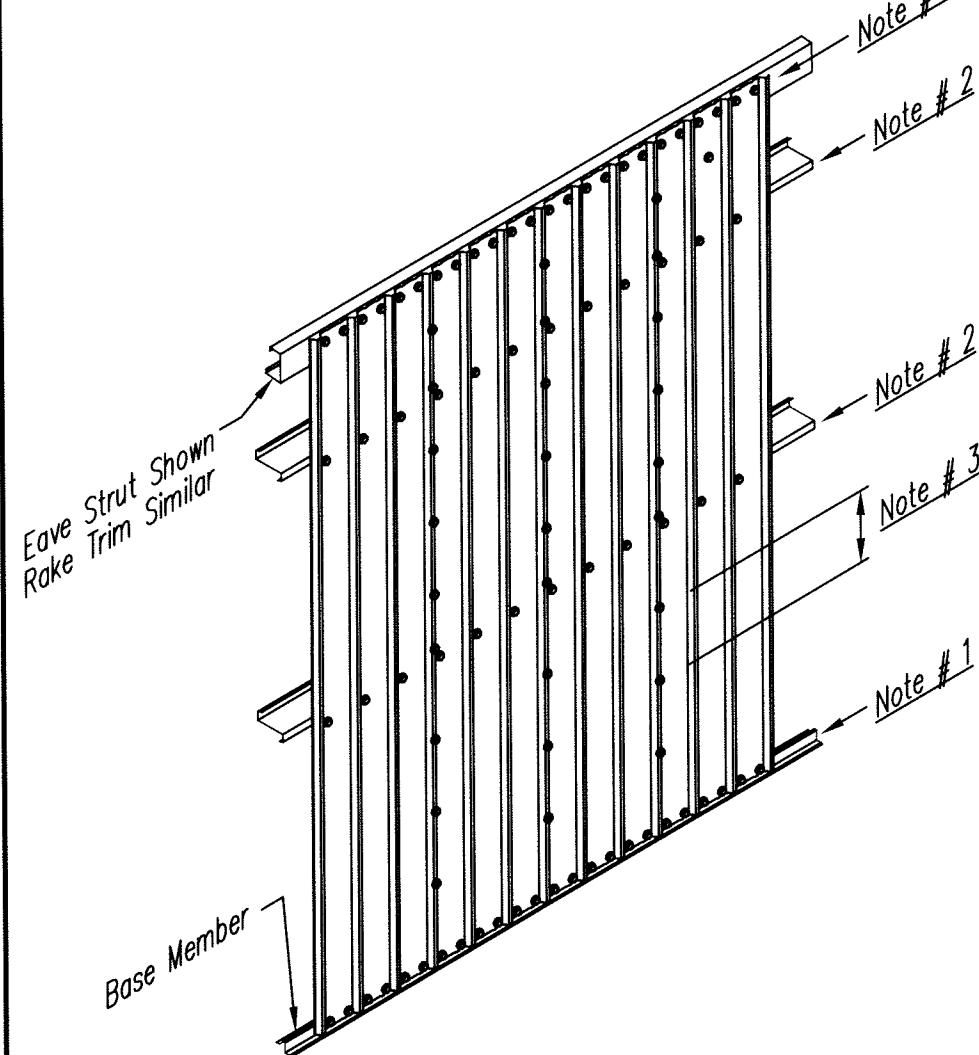
Page		PW05002	
Date	Mar '14	Rev.	02



Page	PW05003	
Date	Mar '14	Rev. 02



Sheeting
Direction



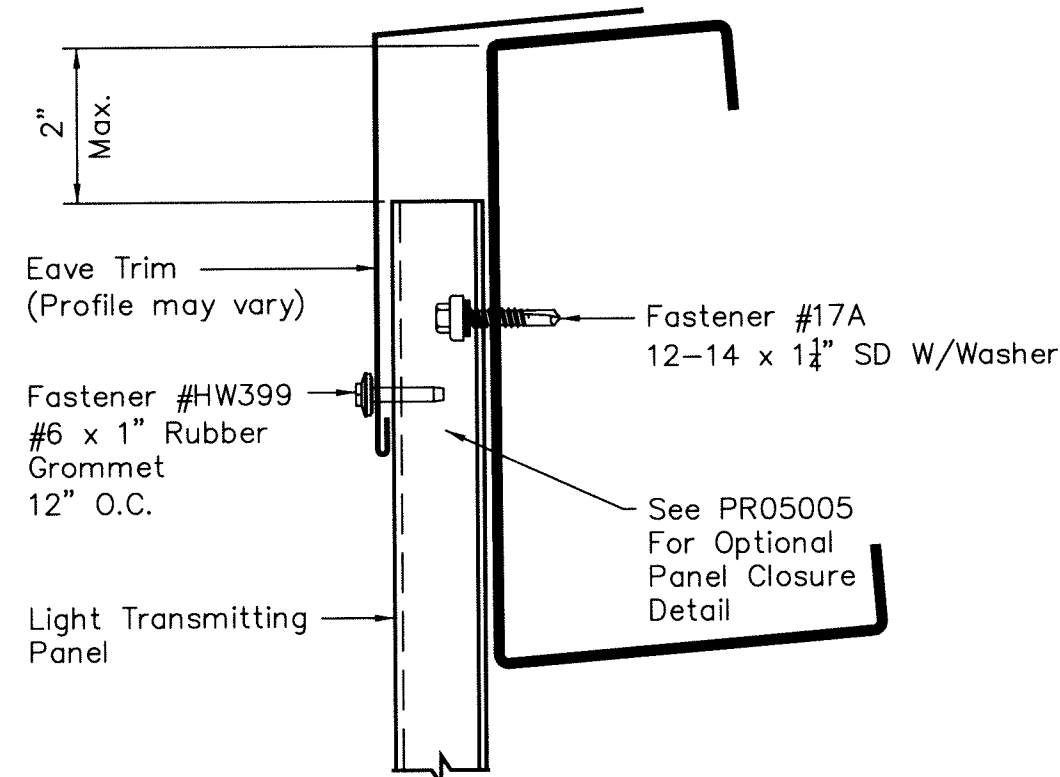
Note #1
Fastener #17A
12-14 x 1 $\frac{1}{4}$ "
SD W/Washer
5", 7", 5" o.c.

Note #2
Fastener #17A
12-14 x 1 1/4"
SD W/Washer
at 1'-0" o.c.

Note #3
Fastener #4A
1/4-14 x 7/8"
SD W/Washer
at 1'-8" o.c.

Page	PW11003	
Date	Mar '14	Rev. 03

Refer to the installation drawing supplied with the building for application of the roof system and specific eave condition.



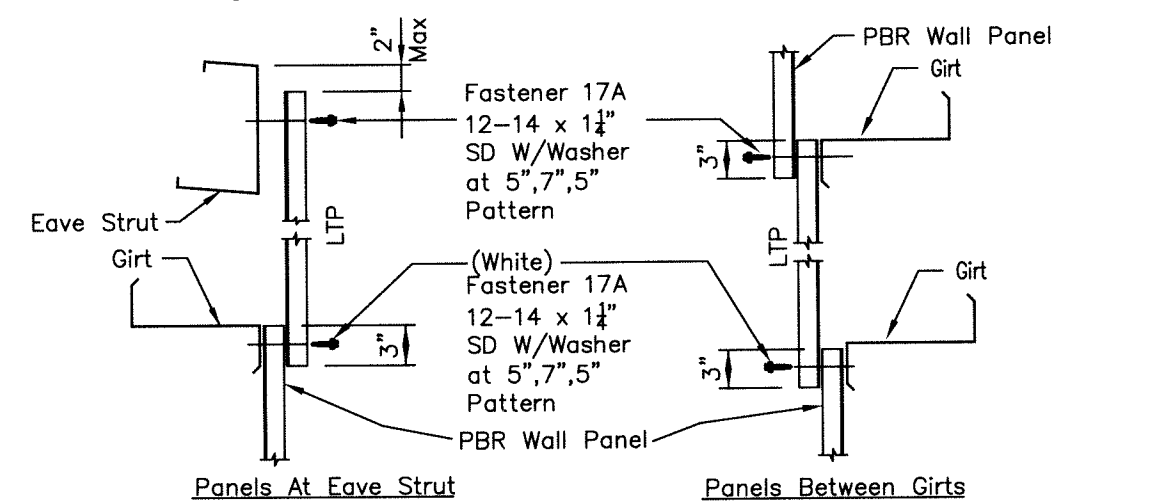
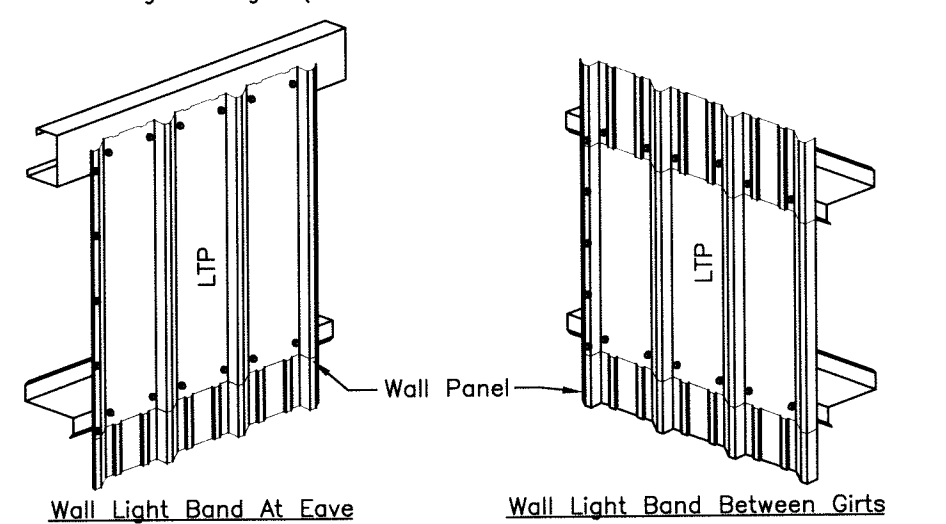
Note: Field Cutting Light Transmitting Panels

Light transmitting panels may need to be cut in the field.
If required, follow these steps:

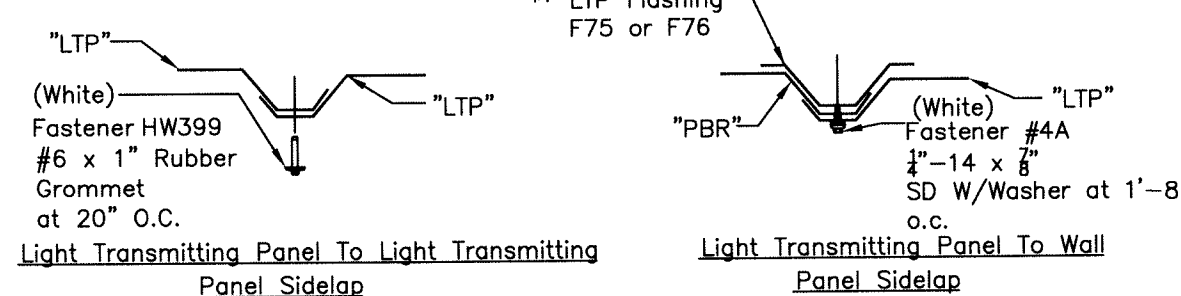
- Up to five (5) LTP's may be cut at one time.
- A circular saw with a 24 tooth per inch blade will work best.
- Cut slow and allow the blade to work without excess pressure.
- Allow the blade to cool between cuts.

Page	PW11011	
Date	Mar '14	Rev. 02

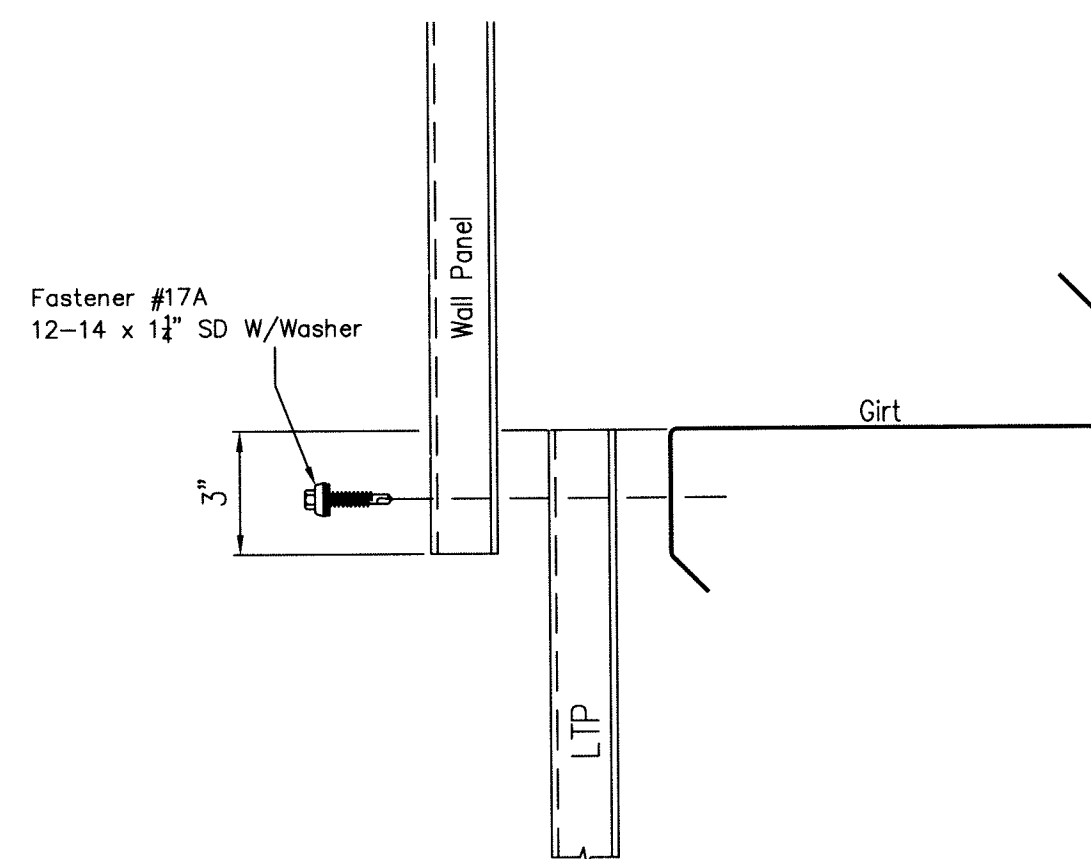
Light Transmitting Panels less than 10'-8" may require field cutting to length (see PW11003 to PW11007 for instructions).



** Not required for walls installed without insulation



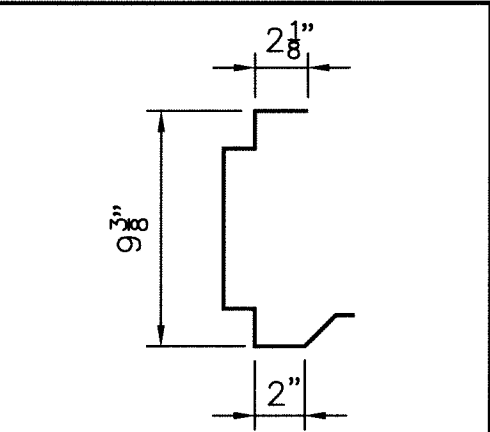
Page	PW11013	
Date	Mar '14	Rev. 01



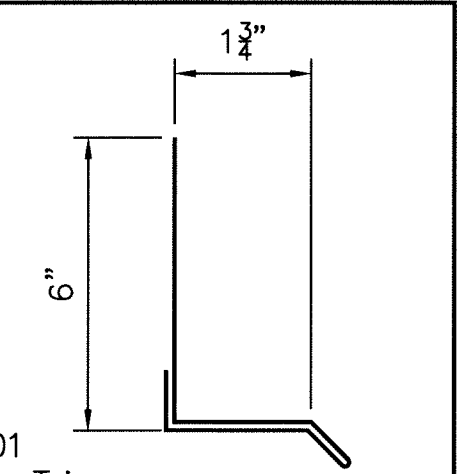
Note: Field Cutting Light Transmitting Panels

Light transmitting panels may need to be cut in the field.
If required, follow these steps:

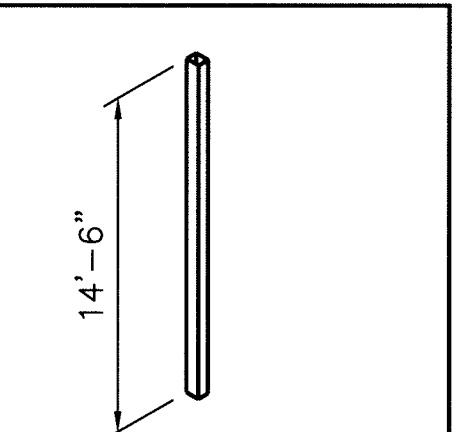
- Up to five (5) LTP's may be cut at one time.
- A circular saw with a 24 tooth per inch blade, will work best.
- Cut slow and allow the blade to work without excess pressure.
- Allow the blade to cool between cuts.



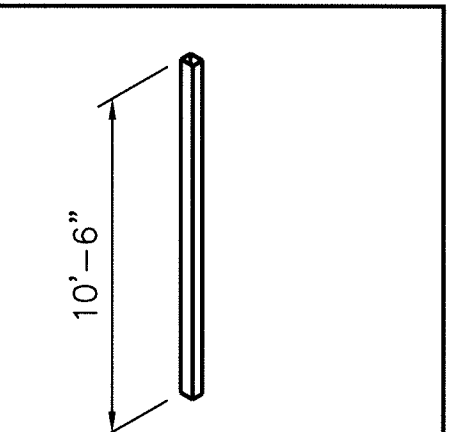
F123
Open Wall Corner Trim



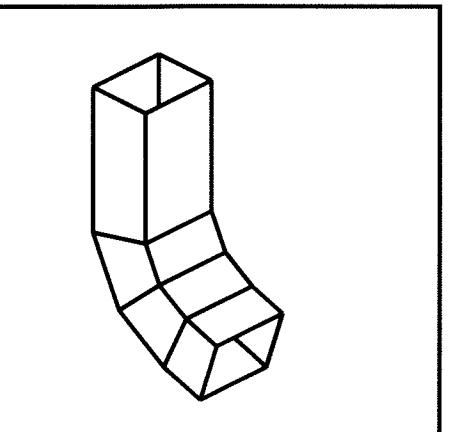
F201
Drip Trim



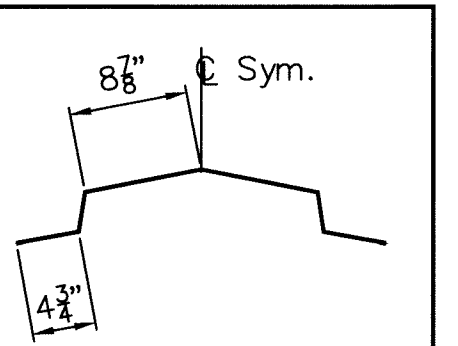
F313
Downspout



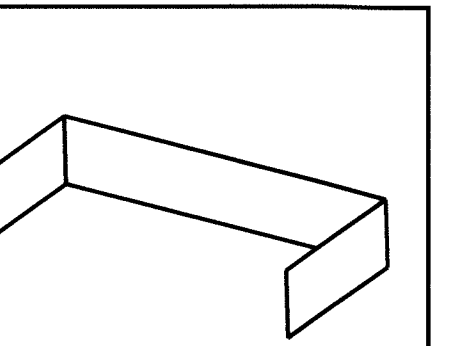
F320
Downspout



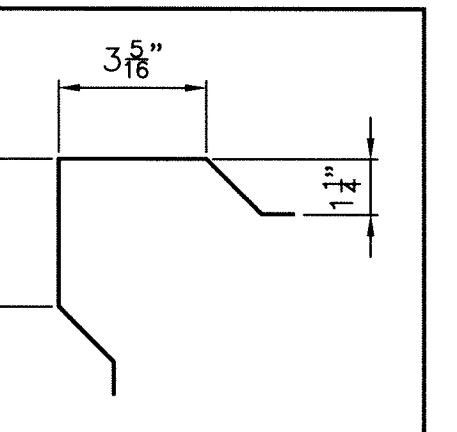
F321
Downspout Elbow



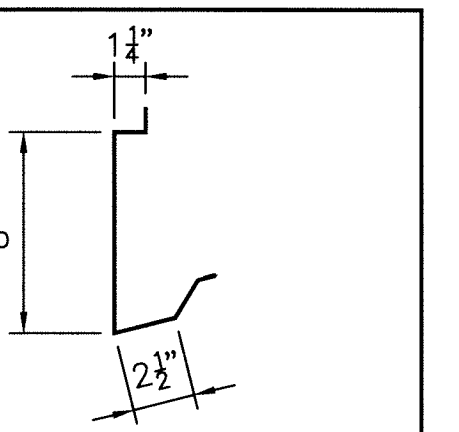
F675
Expansion Ridge Trim



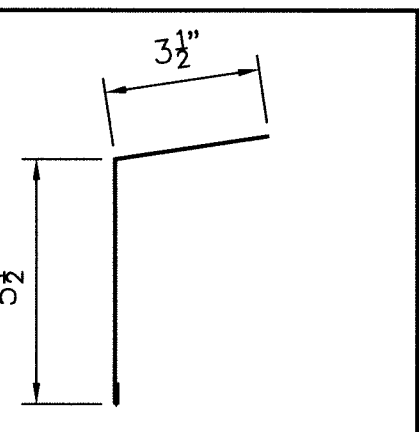
F797
Downspout Strap



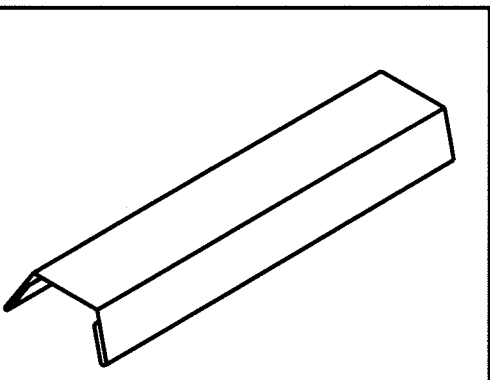
F830
Outside Corner Trim



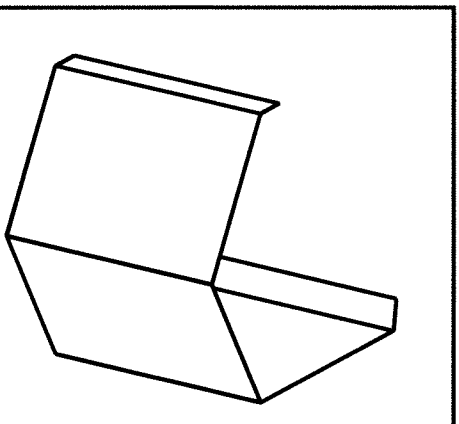
F2901
Low Eave and Rake Trim



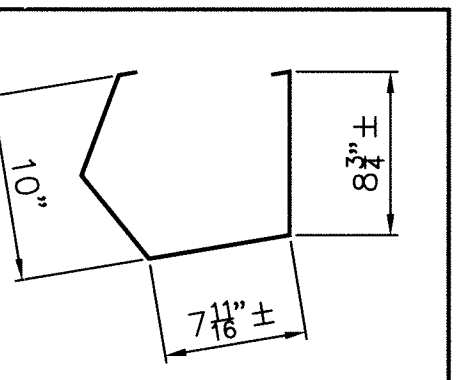
F2955
Flat Eave Trim



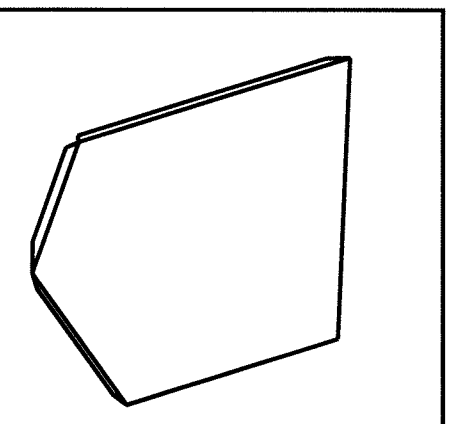
F3357
Gutter Strap



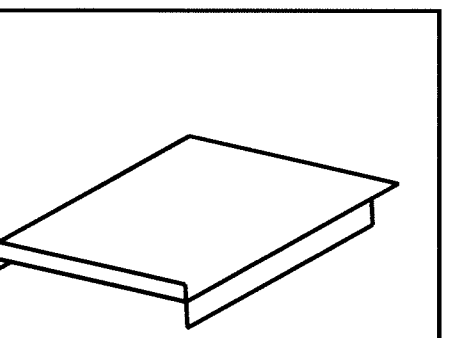
F4102
Gutter Expansion Cover



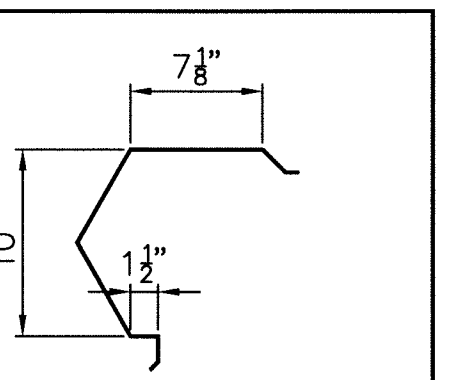
F4130
Gutter



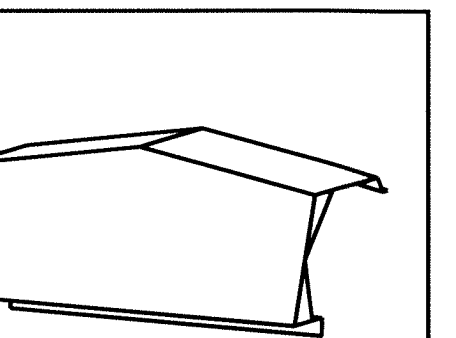
F4131
Gutter End Cap



F4132
Gutter Expansion Cap



F4150
Rake Trim



F4153
Peak Box

Revision	Date	Description	By	Ch'd



Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

Customer: AMERICAN WESTERN STEEL L
DBA: WILSON CONSTRUCTION
HOUSTON TX 77032
MICHAEL VIVIAN

Project Name & Location:
JINDAL SAW USA
1 ROAD A,
BAY ST. LOUIS MS 39520

☐ Preliminary
[Not For Construction]

☐ For Approval
[Not For Construction]

☐ For Construction Permit
[Not For Construction]

☒ For Erector Installation
[Not For Construction]

Scale: NOT TO SCALE

Drawn by: KEB 7/28/14

Checked by: KEB 7/28/14

Project Engineer:

Job Number: 14-B-54756-1

Sheet Number: R13 of 13

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.