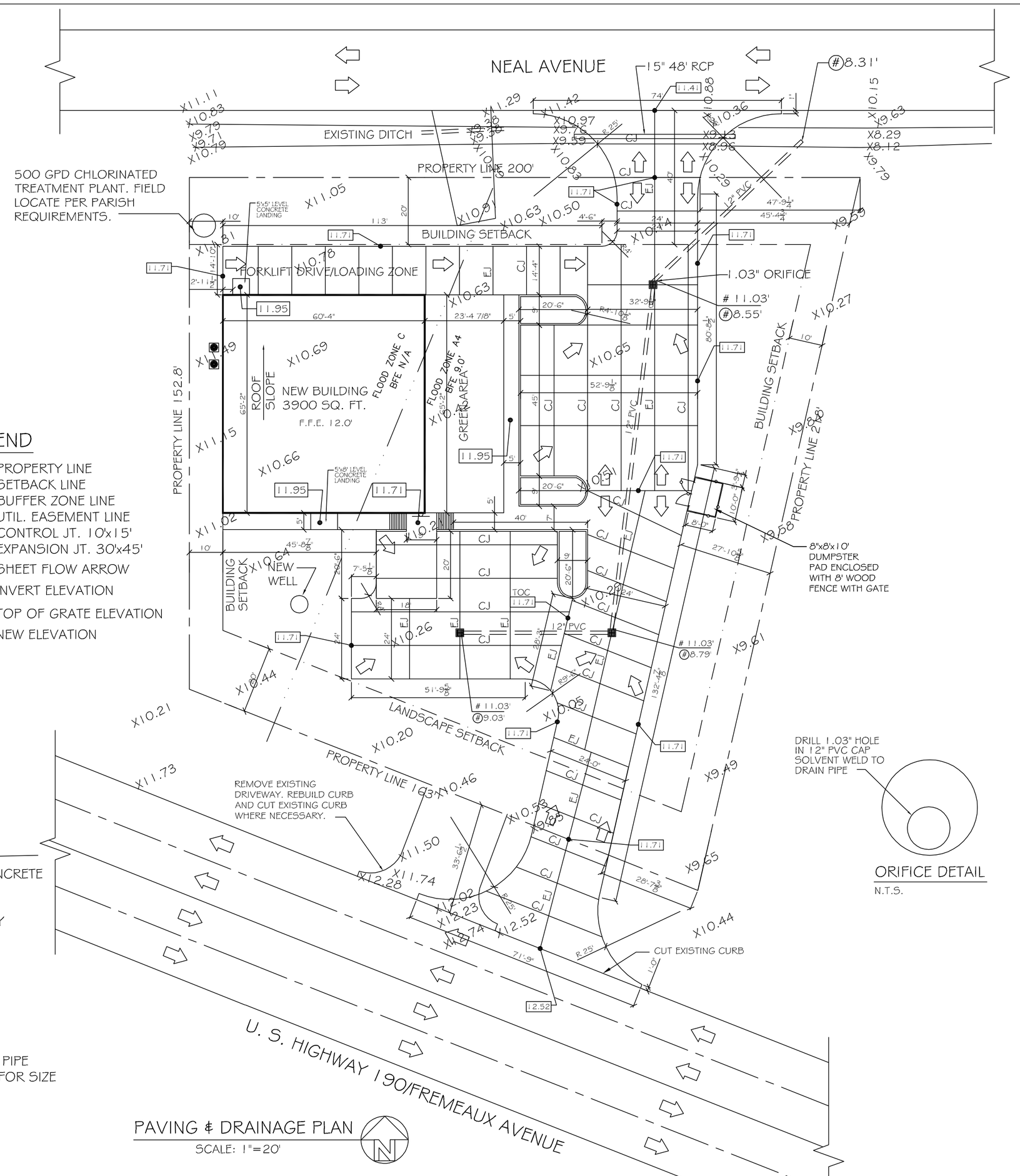
PAVING NOTES:

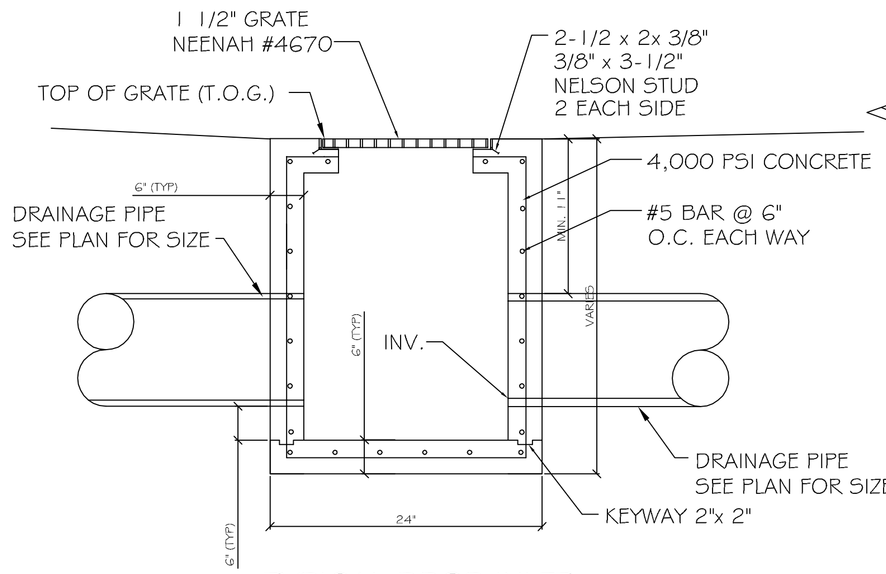
1) ALL NEW CONCRETE SHALL SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS AND A MINIMUM THICKNESS OF 5". CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE LATEST REVISION OF ASTM C-150 TYPE I.
2) CONCRETE PAVING THICKNESS SHALL VARY AS FOLLOWS:
   a) APRONS = 7" THICKNESS
   b) DRIVELANES = 6" THICKNESS
   c) PARKING AREAS = 5" THICKNESS
3) ALL REINFORCING STEEL SHALL MEET ASTM-A615 (GRADE 60).
4) ALL REINFORCING STEEL SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING CONCRETE PLACEMENT. ALL CONTROL AND EXPANSION JOINTS SHALL BE LOCATED AND INSTALLED AS SHOWN ON THE PAVING PLAN AND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
5) ALL SUB GRADE FILL SHALL BE SELECT GRANULAR MATERIAL COMPACTED TO 95% STANDARD PROCTOR DENSITY IN A MAXIMUM OF 6" LIFTS.
6) CONTRACTOR SHALL CONTACT THEIR REGULATORY DEPARTMENT OF ENGINEERING PRIOR TO CONDUCTING ANY WORK.
7) ANY WORK WITHIN THE ROADWAY OR ADJACENT TO THE ROADWAY CAUSING AN INTERFERENCE TO VEHICULAR TRAFFIC REQUIRES PRIOR APPROVAL FROM THE CITY TRAFFIC ENGINEERING DIVISION, AND MUST CONFORM TO THE REQUIREMENTS SET FORTH BY THE UNIFORM MANUAL OF TRAFFIC CONTROL DEVICES OF THE STATE OF LOUISIANA. THE CONTRACTOR MUST FURNISH ALL NECESSARY TRAFFIC SIGNS AND/OR BARRICADES AND MAINTAIN THEM DURING CONSTRUCTION ACTIVITY.

LEGEND

- PROPERTY LINE
- SETBACK LINE
- BUFFER ZONE LINE
- UTIL. EASEMENT LINE
- CJ — CONTROL JT. 10'x15'
- EJ — EXPANSION JT. 30'x45'
- SHEET FLOW ARROW
- (#)0.00' — INVERT ELEVATION
- # 0.00' — TOP OF GRATE ELEVATION
- 00.00 — NEW ELEVATION



PAVING & DRAINAGE PLAN
SCALE: 1"=20'

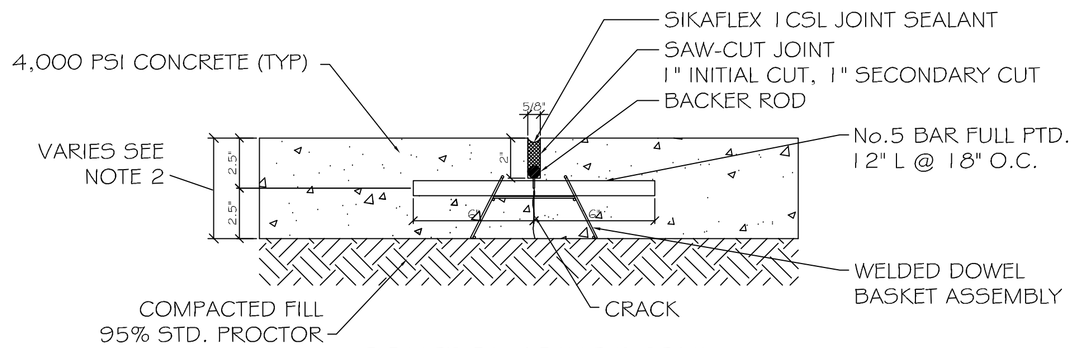


TYPICAL DROP INLET
N.T.S.

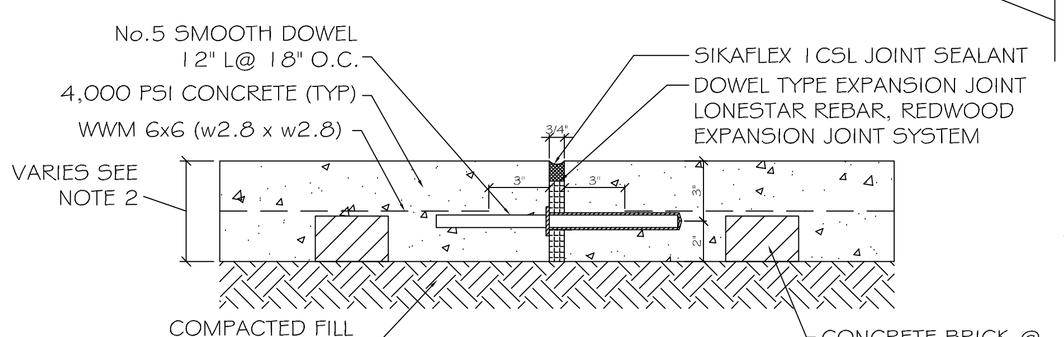
ORIFICE DETAIL
N.T.S.

DRAINAGE PLAN NOTES:

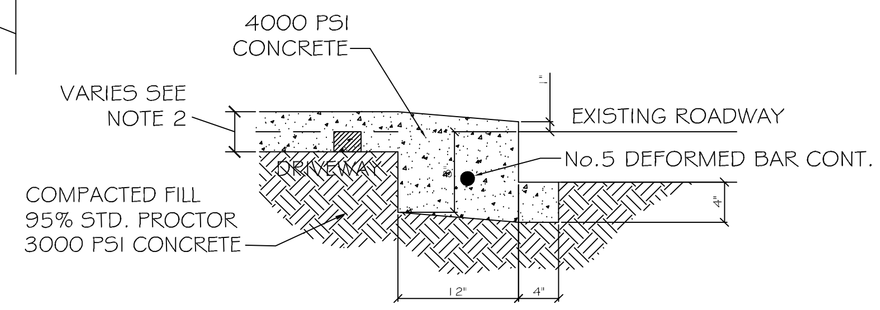
1. DRAIN PIPE & FITTINGS WITHIN PROPERTY LINE SHALL BE POLYVINYL CHLORIDE PLASTIC PIPE, MEETING CLASS 100 C-900 PVC.
2. ELEVATIONS SHOWN ARE M.S.L.
3. FIELD VERIFY ALL ELEVATIONS AND DRAINAGE SYSTEM PLACEMENT PRIOR TO START OF WORK.
4. THERE IS NO EVIDENCE OF EXISTING OFF-SITE FLOW CROSSING THE PROPERTY.



CONTROL JOINT DETAIL
N.T.S.



EXPANSION JOINT DETAIL
N.T.S.



DRIVEWAY SECTION
N. T. S.

PROJECT: RAY HORVATH BUILDING
STORMWATER RUN-OFF CALCULATIONS

Formulas used:

[1] RATIONAL METHOD: Q=Aci

where: Q= Peak discharge of watershed in cubic feet per second (cfs) due to maximum storm assumed.
A= Area of watershed in acres.
c= Coefficient of run-off [2].
i= Intensity of rainfall in inches per hour based on concentration time. [3]

[4] TC=

where: TC=Time of concentration= time required for rain falling at most remote point to reach discharge point.
c= Site run-off coefficient based on conditions shown.
s= Percent slope of overland flow.

PRIOR DEVELOPMENT
25 Year Frequency

Q_i = Aci

Watertight Surfaces		838	sqft =	0.019 Acres
c(1) =	0.9			
Gravel Surface		3554	sqft =	0.082 Acres
c(2) =	0.25			
Green Space		28428	sqft =	0.653 Acres
c(3) =	0.15			
Summary		32820	sqft =	0.753 Acres
c =	0.18			

Duration (D) = Time of concentration (TC)
where L = 218 run-off length ft — Elev diff 4.4
c = 0.18 run-off coef
S = 0.6422 percent slope
therefore TC = D 61.80 minutes
Expected rainfall intensity i = 3.64 in/hr

Q_i = **0.494 cfs** 10% reduction **0.049 cfs**

POST DEVELOPMENT
25 Year Frequency

Q_i = Aci

Watertight Surfaces		18018	sqft =	0.414 Acres
c(1) =	0.9			
Gravel Surface		0	sqft =	0.000 Acres
c(2) =	0.25			
Green Space		14712	sqft =	0.338 Acres
c(3) =	0.15			
Summary		32730	sqft =	0.751 Acres
c =	0.56			

Duration (D) = Time of concentration (TC)
where L = 205 run-off length ft — Elev diff 8.1
c = 0.56 run-off coef
S = 1.5122 percent slope
therefore TC = D 14.30 minutes
Expected rainfall intensity i = 3.64 in/hr

Q_i = **1.539 cfs**

DETENTION REQUIREMENTS

Detention required Q-Q 1.05cfs
ONE HOUR DETENTION 3765.2cuft
DETENTION DIMENSIONS
WIDTH 60feet
LENGTH 92feet
DEPTH **0.68 feet**

DISCHARGE END AREA REQUIREMENTS

where: A= Discharge Area required
g= Acceleration of gravity
c= Discharge coefficient
h= Hydraulic head
Q= Flow volume from run-off

Pipe Servicing Site Drainage
Q = 0.049 cfs — h = 3.00 feet
c = 0.62 coefficient — A = 0.006sqft
g = 32.16ft/sec/sec

REQUIRED CONDUIT = 1.03 inch inside diameter

DAMMON ENGINEERING, INC.
Architects & Engineers
dammonengineering.com
dammon@bellsouth.net
PHONE: 985-649-5832
FAX: 985-641-5950

CHIEF ENGINEER: EMMETT DAMMON, P.E.
5544 ARCHITECT ROBERT WITTE
985 JEAN MARIE TRAIL
SLIDELL, LA 70458

The above drawings and specifications, designs and arrangements shown thereon are and shall remain the property of Dammon Engineering and are not to be used in connection with any other project. Dammon Engineering shall not be responsible for any changes, interpretations and developments made without written consent of Dammon Engineering. Any unauthorized use, reuse or alteration of these drawings and specifications shall constitute conclusive evidence of acceptance of these restrictions.

RAY HORVATH
FLOODWORKS AND BLINDS
RETAIL
1430 SHORTCUT ROAD
SLIDELL, LA 70458

JOB No: 2152 | DATE: 11-13-12 | CHECKED BY: BSN | DRAWN BY: BSN

REVISIONS

#	DESCRIPTION	DATE

PAVING AND DRAINAGE PLAN

SHEET No: 3 OF 10

C-2