

1 GENERAL NOTES

1. PILES:
- A.S.T.M. D25 TREATED TIMBER PILES
 - SYMBOL ON PLAN:
 - LENGTH: 55 FEET
 - MINIMUM TIP DIAMETER: 7 INCHES
 - MINIMUM DIAMETER 3'-0" FROM BUTT: 13 INCHES
 - TREATMENT: C.C.A.; 0.80 LBS. PER CU. FT. RETENTION; A.W.P.A. SPECIFICATIONS.
 - CUT-OFF TREATMENT: BRUSH TREAT TOP OF PILE WITH COPPER NAPHTHENE CONFORMING TO A.W.P.A. SPECIFICATION M4.
 - FOLLOWING: PILES MAY BE FOLLOWED, ALLOW 1'-0" FOR CUT-OFF.
 - PREDRILL: 10 FEET; 7 INCH THREE BLADE DEMON BIT.
 - HAMMER: NO. 1 VULCAN (15,000 FT-LBS)
 - DESIGN LOAD 25 TONS
- B. WOOD-CONCRETE COMPOSITE PILES
- SYMBOL ON PLAN:
 - TIP ELEVATION: EL. 42'-0" (FIRST FLOOR ELEVATION 100'-0")
 - LOWER TIMBER SECTION:
 - TYPE: UNTREATED TIMBER PILE, ASTM D25
 - MINIMUM TIP DIAMETER: 7"
 - MINIMUM DIAMETER 3'-0" FROM BUTT: 12"
 - LENGTH: 40 FEET
 - CONCRETE UPPER SECTION:
 - TYPE: CAST-IN-PLACE CONCRETE
 - STRENGTH: 4,000 PSI AT 28 DAYS
 - LENGTH: AS REQUIRED TO EXTEND FROM BUTT OF TIMBER LOWER SECTION TO CUT-OFF ELEVATIONS.
 - PREDRILL: 10 ft., 7" THREE BLADE DEMON BIT
 - HAMMER: VULCAN NO. 1 (15,000 FT-LBS)
 - DESIGN LOAD: 25 TONS
- C. PILES (SMALL TIMBER PILES):
- ASTM D25 TREATED TIMBER PILES. (FOR PREFABRICATED CANOPY & SITE FENCING FOUNDATIONS)
 - LENGTH: 40'-0" ft.
 - MINIMUM TIP DIAMETER: 7"
 - MINIMUM BUTT DIAMETER: 12"
 - TREATMENT: C.C.A.; 0.80 PCF RETENTION PER A.W.P.A. SPECIFICATIONS.
 - CUT-OFF TREATMENT: BRUSH TREAT TOP OF PILE WITH COPPER NAPHTHENE CONFORMING TO A.W.P.A. SPECIFICATION M4.
- F. FOLLOWING: PILES MAY BE FOLLOWED, ALLOW 1'-0" FOR CUT-OFF.
- G. PREDRILL: 5 FEET; 6 INCH THREE BLADE DEMON BIT.
- H. HAMMER: NO. 2 VULCAN OR 3,000 LB DROP HAMMER DELIVERING 9,000 FT-LBS OF ENERGY PER BLOW.
- I. EXPLORATORY PILES: NONE REQUIRED
- J. LOAD TEST: NONE REQUIRED PER 1813.5.2.5 OF AMENDMENTS TO THE 2009 INTERNATIONAL BUILDING CODE.
- K. DESIGN LOAD: $(9.5 \times 11 \times 40 \times 150) = 14,922 \text{ LBS} = 7 \text{ TONS}$

2. CONCRETE:
- ACI 301-05 SPECIFICATIONS
 - NORMAL WEIGHT CONCRETE: 150 PCF
 - LIGHTWEIGHT CONCRETE: 115 PCF
 - COMPRESSIVE STRENGTH AT 28 DAYS:
 - PILE CAPS: 4,000 PSI
 - GRADE BEAMS AND 1ST FLOOR SLAB: 4,000 PSI
 - CONCRETE ON METAL DECK FLOOR SLAB: 4,000 PSI
3. REINFORCING STEEL:
- ASTM A615, GRADE 60
 - WELDED WIRE MESH: ASTM A185
 - CONTRACTOR SHALL INCLUDE IN THE COST OF HIS BID, AN ADDITIONAL 5.0 TONS OF REINFORCING STEEL. THIS COST SHALL INCLUDE FABRICATION, TRANSPORTATION, INSTALLATION, ETC. OF ADDITIONAL REINFORCING STEEL. COST OF ADDITIONAL REINFORCING STEEL NOT UTILIZED DURING CONSTRUCTION WILL BE CREDITED BACK TO THE OWNER VIA CHANGE ORDER AT THE UNIT PRICE PROVIDED ON THE BID FORM.
4. REINFORCING CLEARANCES REQUIRED ARE AS FOLLOWS:
- ACI 117 STANDARDS. UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, REINFORCING CLEARANCES SHALL BE AS FOLLOWS:
 - SLABS:
 - 1/2" CLEAR TOP & BOTTOM FORMED, 1" CLEAR BOTTOM, 3/4" CLEAR TOP ON GRADE.
 - EXTERIOR 1/2" CLEAR TOP & 3/4" BOTTOM FORMED.
 - SLABS: 1/2" CLEAR TOP & 1" CLEAR BOTTOM ON GRADE.
 - BEAMS: 1/2" CLEAR BOTTOM FORMED, 3" CLEAR BOTTOM CAST ON EARTH, 1/2" CLEAR SIDES AND TOP FORMED, 3" CLEAR SIDES EARTH FORMED, 1/2" CLEAR TOP.
 - COLUMNS: 1/2" CLEAR, TYPICAL.
 - WALLS: 1/2" CLEAR, TYPICAL.

5. REINFORCING DETAILS FOR STRUCTURALLY SUPPORTED CONCRETE:
- ACI 315 STANDARDS. UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, BAR LAPS AND CONFIGURATIONS SHALL BE AS FOLLOWS:
 - CONTINUOUS TOP BARS: HOOK AT NON-CONTINUOUS ENDS. LAP 30 DIAS. AT MID-SPAN.
 - CONTINUOUS BOTTOM BARS: LAP 6" AT CENTER OF SUPPORT.
 - TEMPERATURE BARS IN SLAB AND INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS: TENSION LAP SPICES, SEE TABLE BELOW.
 - SLAB TOP REINFORCING SUPPORT BARS: SLAB TOP REINFORCING BARS LESS THAN 6 FEET IN LENGTH SHALL HAVE 2-#4 CONT. SUPPORT BARS AND SLAB TOP REINFORCING BARS GREATER THAN 6 FEET IN LENGTH SHALL HAVE #4 SUPPORT BARS EQUALLY SPACED AT NO MORE THAN 4'-0" O.C.
 - CORNER BARS: PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS, CORNER BARS SHALL LAP WITH HORIZONTAL BARS. PROVIDE "U" BARS AT WALL ENDS. LAP #3 TO #6 BARS 30" EACH WAY AND LAP #7 TO #11 BARS 48" EACH WAY. HOOK INSIDE BARS IN WALLS AT ENDS.
 - LAP SPLICE TABLE:

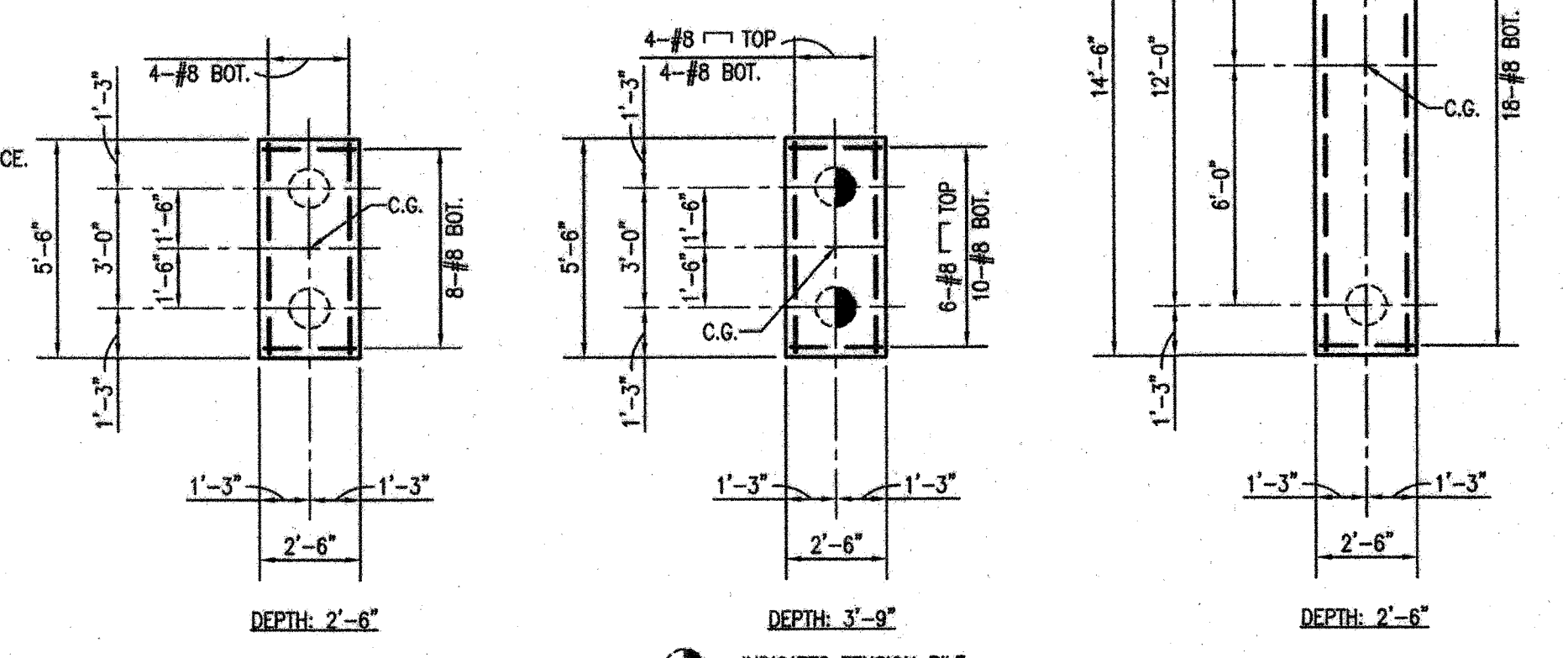
	LOCATION	BAR SIZE										
		#3	#4	#5	#6	#7	#8	#9	#10	#11		
SLABS	TOP	15	24	36	48	78	96	117	140	165		
	OTHER	12	19	28	37	60	74	90	108	127		
OTHER	TOP	24	32	40	48	70	80	91	102	113		
	OTHER	19	25	31	37	54	62	70	79	87		

6. CONDUITS AND PIPES EMBEDDED IN CONCRETE:
- CONDUITS, PIPES, AND SLEEVES OF ANY MATERIAL NOT HARMFUL TO CONCRETE SHALL BE PERMITTED TO BE EMBEDDED IN CONCRETE WITH APPROVAL OF THE ENGINEER, PROVIDED THAT REGULATIONS ARE FOLLOWED AS OUTLINED IN THE APPLICABLE ACI CODES.
 - CONDUITS, PIPES, AND SLEEVES PASSING THROUGH A SLAB OR BEAM SHALL NOT SIGNIFICANTLY IMPAIR THE STRENGTH OF CONSTRUCTION AS DETERMINED BY THE ENGINEER.
 - SINGLE CONDUITS AND PIPES OR INTERSECTING CONDUITS AND PIPES SHALL NOT OCCUPY MORE THAN AN 1/4" OF SLAB THICKNESS AND 1/2 THE OVERALL THICKNESS OF BEAMS IN WHICH THEY ARE EMBEDDED, AND THEY SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS OR WIDTHS ON CENTER. ANY CONDUIT OR PIPE LARGER SHALL BE LOCATED BELOW THE RESPECTIVE SLAB OR BEAM. THE CONTRACTOR SHALL SUBMIT FOR APPROVAL A DIAGRAM DEPICTING THE HOME RUNS OF CONDUIT TO ALL PANELS, TYPICAL.
 - IT WILL NOT BE PERMITTED TO CUT, BEND, OR DISPLACE THE REINFORCING STEEL FROM ITS PROPER LOCATION.
 - COORDINATION MUST BE MADE BY THE CONTRACTOR AT HIS EXPENSE TO FOLLOW THE ABOVE GUIDELINES.
7. STRUCTURAL STEEL:
- A.I.S.C. SPECIFICATIONS; STEEL CONSTRUCTION MANUAL 13TH EDITION; ALL WIDE FLANGE SHAPES ASTM A992 GRADE 50, ALL MISCELLANEOUS SHAPES ASTM A36, HSS SHAPES ASTM A500 GRADE B Fy=46 ksi, STEEL PIPE ASTM A53 GRADE B Fy=35 ksi; HIGH STRENGTH BOLTS A325, 3/4" MIN. EXCEPT AS NOTED OTHERWISE; ANCHOR RODS AND BOLTS ASTM F1554 GRADE 105 EXCEPT AS NOTED OTHERWISE; AUTOMATICALLY END WELDED SHEAR CONNECTORS (A.E.W.S.C.) AS MANUFACTURED BY NELSON STUDS OR APPROVED EQUAL; E-70 ELECTRODES.
 - ALL BEAM CONNECTIONS SHALL BE A.I.S.C. STANDARD FRAMED CONNECTIONS. SHOP CONNECTIONS SHALL BE WELDED. FIELD CONNECTIONS SHALL BE BOLTED, EXCEPT CONNECTIONS TO EXISTING CONSTRUCTION SHALL BE WELDED. CONNECTIONS NOT SCHEDULED OR DETAILED OTHERWISE TO BE DESIGNED FOR 1/2 UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN AND 2/3 UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN FOR COMPOSITE BEAMS. CONNECTION DESIGN SHALL ALSO INCLUDE ANY CONCENTRATED LOAD WITHIN 3FT OF MEMBER ENDS.
 - ALL STRUCTURAL STEEL ITEMS AND RESPECTIVE ANCHORS AND FASTENERS PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. PROVIDE VENT HOLES AS REQUIRED. TOUCH UP ALL DAMAGED COATING WITH STICK GALVANIZING. SEE ARCHITECT FOR PAINTING.
 - CONTRACTOR SHALL INCLUDE IN THE COST OF HIS BID, AN ADDITIONAL 5.0 TONS OF STRUCTURAL STEEL. THIS COST SHALL INCLUDE FABRICATION, TRANSPORTATION, INSTALLATION, ETC. OF ADDITIONAL STRUCTURAL STEEL. COST OF ADDITIONAL STRUCTURAL STEEL NOT UTILIZED DURING CONSTRUCTION WILL BE CREDITED BACK TO THE OWNER VIA CHANGE ORDER AT THE UNIT PRICE PROVIDED ON THE BID FORM.
 - FOR CONNECTIONS NOT SCHEDULED OR DETAILED PROVIDE SHOP DRAWINGS AND CALCULATIONS PREPARED UNDER SUPERVISION OF A LOUISIANA LICENSED ENGINEER, INCLUDING COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS, PROCEDURES AND DIAGRAMS.
 - REFER TO SPECIFICATIONS FOR ADDITIONAL SHOP DRAWING REQUIREMENTS.
8. STEEL JOISTS AND JOIST GIRDERS:
- A.S.I. SPECIFICATIONS AND CODE OF STANDARD PRACTICE (1994); FIELD WELDED IN PLACE, MANUFACTURER MUST BE A MEMBER OF STEEL JOIST INSTITUTE.
 - JOISTS AND BRIDGING AT ROOF SHALL BE DESIGNED FOR A NET UPLIFT. THE NET UPLIFT LOAD COMBINATION SHALL BE CALCULATED BY SUBTRACTING 12 PSF FROM THE APPROPRIATE VALUE (BASED ON EFFECTIVE WIND AREA (EWA) AND ZONE) TAKEN FROM THE WIND CHART REFERENCED UNDER THE DESIGN LOADS. SEE GENERAL NOTE 11.
 - PROVIDE BRIDGING AND BRACING AS REQUIRED BY THE STEEL JOIST INSTITUTE SPECIFICATIONS, INCLUDING THE ADDITIONAL BRIDGING AND BRACING REQUIRED FOR UPLIFT DESIGN AT THE FIRST BOTTOM CHORD PANEL POINT OF JOISTS AND JOIST GIRDERS. ALL BRIDGING MUST BE CONNECTED TO BEAMS WHICH ARE PARALLEL TO JOISTS.
 - SUBMIT COMPUTATIONS FOR UPLIFT DESIGN CERTIFIED BY A CIVIL ENGINEER LICENSED TO PRACTICE IN THE STATE OF LOUISIANA.
 - SEE 5/54.01 FOR TYPICAL JOIST REINFORCING AT CONCENTRATED LOAD.
9. METAL DECK FOR FLOORS:
- 1.5W120 AS MFG. BY VULCRAFT OR APPROVED EQUAL GALVANIZED STEEL COMPOSITE TYPE DECK, CONTINUOUS OVER 3 OR MORE SPANS; FIELD WELDED TO SUPPORTS AND/OR FASTENED WITH AUTOMATIC END WELDED SHEAR CONNECTORS WHERE INDICATED.
 - DECK: 20.
 - MIN. SECTION MODULUS (POSITIVE): 0.224 IN³/IN PER FT.
 - MIN. SECTION MODULUS (NEGATIVE): 0.231 IN³/IN PER FT.
 - MIN. MOMENT OF INERTIA: 0.186 IN⁴/IN PER FT.
 - DESIGN THICKNESS: 0.036"
 - ALLOWABLE STRESS: 50 KSI
 - DECK FASTENING PATTERN
 - SUPPORT FASTENERS: 3/4" PUDDLE WELDS
 - FASTENER PATTERN: 36/4
 - SIDELAP FASTENER: #10 SCREWS
 - SIDELAP FASTENERS PER SPAN: 3
10. METAL DECK (AT STEEL ROOFS):
- 1.5B220 OR 1.5B240 AS MFG. BY VULCRAFT OR APPROVED EQUAL GALVANIZED METAL ROOF DECK, CONTINUOUS OVER THREE OR MORE SPANS; FIELD WELDED TO SUPPORTS.
 - DEPTH: 1/2"
 - GAGE: 20.
 - MINIMUM SECTION MODULUS (POSITIVE): 0.234 IN³/IN PER FT.
 - MINIMUM SECTION MODULUS (NEGATIVE): 0.247 IN³/IN PER FT.
 - MINIMUM MOMENT OF INERTIA: 0.212 IN⁴/IN PER FT.
 - ALLOWABLE STRESS: 33 KSI
 - DECK FASTENING PATTERN
 - SUPPORT FASTENERS: 3/4" PUDDLE WELDS
 - FASTENER PATTERN: 36/7
 - SIDELAP FASTENER: #10 SCREWS
 - SIDELAP FASTENERS PER SPAN: 3
11. DESIGN LOAD AND OTHER PERTINENT DESIGN INFORMATION:
- BUILDING CODE: INTERNATIONAL BUILDING CODE 2009/ASCE 7-05
 - FLOOR LIVE LOADS:
 - CLASSROOMS: 40 PSF
 - OFFICE AREAS: 50 PSF
 - CORRIDORS: 80 PSF; 100 PSF (1ST FLOOR).
 - LOCKER ROOMS, GYMNASIUM, RESTROOMS: 100 PSF
 - MEDIA CENTER: 150 PSF
 - MECHANICAL/ STORAGE ROOMS: 125 PSF(MIN.)
 - FLOOR SUPERIMPOSED DEAD LOADS (EXCLUDES STRUCTURE'S SELF WEIGHT, WEIGHT OF SLAB & BEAMS):
 - CEILING: 10 PSF
 - PARTITION: 10 PSF(MIN.)
 - ROOF LOADS (UNOCCUPIED AREAS):
 - LIVE LOAD: 20 PSF
 - SUPERIMPOSED DEAD LOAD: 15 PSF
 - ROOF SNOW LOADS:
 - GROUND SNOW LOAD (Pg): 0.0 PSF
 - WIND LOADS:
 - BASIC WIND SPEED (V): 130 MPH
 - WIND IMPORTANCE FACTOR (I): 1.15
 - WIND EXPOSURE: B
 - INTERNAL PRESSURE COEFFICIENT (GC): ±18
 - COMPONENTS & CLADDING PRESSURES:

ZONE	COMPONENTS AND CLADDING DESIGN PRESSURES (PSF)										PARAPET	OVERHANG
	1	2	3	4	5	6	7	8	9	10	1	2
EWA (FT ²)	<10	17	40	17	68	17	102	40	44	40	54	±96 ±130
	20	15	39	15	61	15	85	39	42	39	51	NA
	50	14	38	14	51	14	61	35	39	35	46	NA
	100	13	37	13	44	13	44	34	38	34	38	NA
	200	13	37	13	44	13	44	33	36	33	39	NA
	≥200	13	37	13	44	13	44	30	34	30	34	NA

- NOTES:
- EWA IS THE EFFECTIVE WIND AREA OF A STRUCTURAL COMPONENT.
 - FOR ZONE DEFINITIONS, SEE ASCE 7-05 FIGURE 6-11A & 6-11B.
 - PLUS AND MINUS SIGNS SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTIVELY.
 - WIDTH OF EDGE STRIP "d" = 10'-0".
- G. EARTHQUAKE DESIGN DATA
- SEISMIC IMPORTANCE FACTOR: 1.25
- OCCUPANCY CATEGORY: III
- SPECTRAL RESPONSE, S_s: 0.110
- SPECTRAL RESPONSE, S₁: 0.048
- SEISMIC DESIGN CATEGORY: B
- SITE CLASS: E
- BASIC SEISMIC-FORCE-RESISTING SYSTEMS:
- PLAN N/S DIRECTION- ORDINARY STEEL MOMENT FRAMES NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.
- PLAN E/W DIRECTION- ORDINARY STEEL CONCENTRICALLY BRACED FRAMES NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.
- DESIGN BASE SHEAR (V):
- PLAN N/S DIRECTION: 485KIPS
- PLAN E/W DIRECTION: 785KIPS
- SEISMIC RESPONSE COEFFICIENT (C_s):
- PLAN N/S DIRECTION: 0.047
- PLAN E/W DIRECTION: 0.076
- RESPONSE MODIFICATION FACTOR (R):
- ORDINARY STEEL MOMENT FRAMES: 3
- ORDINARY STEEL CONCENTRICALLY BRACED FRAMES: 3
- ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE
12. MECHANICAL EQUIPMENT:
- PRIOR TO THE DETAILING OF ANY STRUCTURAL MATERIALS INVOLVED IN THE SUPPORT OF MECHANICAL EQUIPMENT, THE CONTRACTOR SHALL FURNISH TO THE ARCHITECT ALL INFORMATION RELATIVE TO LOADS, LOAD POINTS, DIMENSIONS, ETC. OF THE ACTUAL EQUIPMENT WHICH IS TO BE FURNISHED.
 - ALL DETAILS AND MEMBER SIZES SHOWN ON THE STRUCTURAL DRAWINGS ARE TENTATIVE UNTIL SUCH TIME AS THIS INFORMATION IS REVIEWED BY THE ARCHITECT.
 - LOCATION OF SUPPORT BEAMS MUST BE COORDINATED WITH MECHANICAL EQUIPMENT LOCATIONS.
 - COORDINATION MUST BE MADE BY THE CONTRACTOR AT HIS EXPENSE TO FOLLOW THE ABOVE GUIDELINES.
13. MASONRY:
- COMPLY WITH BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI308-08/ASCE5-08/TMS402-08.
 - HOLLOW CONCRETE MASONRY UNITS (CMU) - ASTM C90, TYPE 1.
 - MORTAR - TYPE "S" AND COMPLY WITH ASTM C270.
 - MINIMUM STRENGTH SHALL BE 1,900 PSI.
 - GROUT - COMPLY WITH ASTM C476; MINIMUM STRENGTH SHALL BE 2,000 PSI.
 - MINIMUM NET AREA COMPRESSIVE STRENGTH SHALL BE 1,500 PSI.
 - SPLICE VERTICAL BARS #7 & LARGER WITH MECHANICAL COUPLERS WHICH DEVELOP 125% OF ASTM SPECIFIED BAR TENSILE STRENGTH.
 - SEE TYPICAL MASONRY DETAILS ON 54.02.
14. CAMBER:
- ALL BEAMS SHALL BE CAMBERED UPWARD THE DESIGNATED AMOUNTS SHOWN ON THE PLANS. BEAMS WITHOUT A SPECIFIED CAMBER SHALL BE ORIENTED SUCH THAT ANY INCIDENTAL MILL CAMBER IS UPWARD.
15. AUTOMATICALLY END WELDED SHEAR CONNECTORS (A.E.W.S.C.) AS MANUFACTURED BY NELSON STUDS.
16. OTHER WORK:
- COORDINATE ALL OTHER WORK WITH STRUCTURAL. UNLESS DETAILED OR SPECIFIED, THE ARCHITECT IS TO APPROVE ALL OPENINGS, SLEEVES, EMBEDDED ITEMS ETC. INVOLVED IN STRUCTURAL WORK PRIOR TO THEIR BEING SET. DO NOT CUT OR DRILL HOLES IN STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ARCHITECT. ALL SUCH ITEMS SHALL NOT IMPAIR THE STRUCTURAL INTEGRITY OF THE MEMBER AS DETERMINED BY THE ENGINEER OF RECORD.
17. POST-INSTALLED ANCHORS TO HARDENED CONCRETE:
- IN THE INSTALLATION OF ANCHORS, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING, CONDUIT, ETC. SEE GENERAL NOTE #18 FOR PROCEDURE FOR DRILLING HOLES IN EXISTING CONCRETE.
 - ANCHOR TYPES SHALL BE AS FOLLOWS:
 - WEDGE TYPE ANCHORS: "Kwik" BOLTS AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUAL.
 - ADHESIVE TYPE ANCHORS (CONCRETE & GROUT FILLED CMU): "HAS" RODS AND HIT-HY 150 MAX ADHESIVE SYSTEM AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUAL.
 - ADHESIVE TYPE ANCHORS (HOLLOW CMU): "HIT-A" RODS AND HIT-HY 70 ADHESIVE SYSTEM AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUAL. ANCHORS PERMANENTLY EXPOSED TO AN EXTERIOR WEATHER ENVIRONMENT SHALL BE STAINLESS STEEL TYPE 316.
18. DRILLING HOLES FOR ANCHORS AND CORING HOLES IN EXISTING CONCRETE:
- PRIOR TO DRILLING OR CORING HOLES, THE CONTRACTOR SHALL SUBMIT A PROPOSED PENETRATION PLAN TO THE A/E FOR REVIEW.
 - THE CONTRACTOR SHALL LOCATE EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC. IN THE AREA WHERE NEW HOLES ARE TO BE INSTALLED THROUGH NON-DESTRUCTIVE TESTING SUCH AS WITH AN X-RAY, RADAR, OR WITH OTHER NON-DESTRUCTIVE DEVICES.
 - MARK THE LOCATION OF ALL REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, AND OTHER EXISTING INTERFERENCES ON THE SURFACE OF THE SLAB.
 - IF NEW HOLE LOCATIONS CONFLICT WITH EXISTING REINFORCING, POST-TENSIONING, CONDUIT, PIPING, ETC., THE CONTRACTOR SHALL NOTIFY THE A/E BEFORE INSTALLING THE NEW HOLES.
 - VERIFY NO CONFLICTS EXIST AT NEW HOLE LOCATIONS BY SMALL DRILLED PILOT HOLES. IF NO CONFLICTS EXIST, COMPLETE THE INSTALLATION. IN THE CASE OF STEEL TO BE FASTENED TO THE CONCRETE WITH MULTIPLE ANCHORS, FABRICATE, FROM A FIELD TEMPLATE, THE STEEL TO BE FASTENED TO THE CONCRETE BY THE ANCHORS AND COMPLETE THE INSTALLATION.
 - WHEN INSTALLING NEW HOLES, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING STEEL, POST-TENSIONING, CONDUIT, PIPING, ETC.
 - ALL NEW PENETRATIONS THROUGH EXISTING BEAMS SHALL BE APPROVED BY ENGINEER PRIOR TO DRILLING OR CORING.
 - MAINTAIN MINIMUM 6" CLEAR BETWEEN ALL PENETRATIONS UNLESS APPROVED BY ENGINEER.

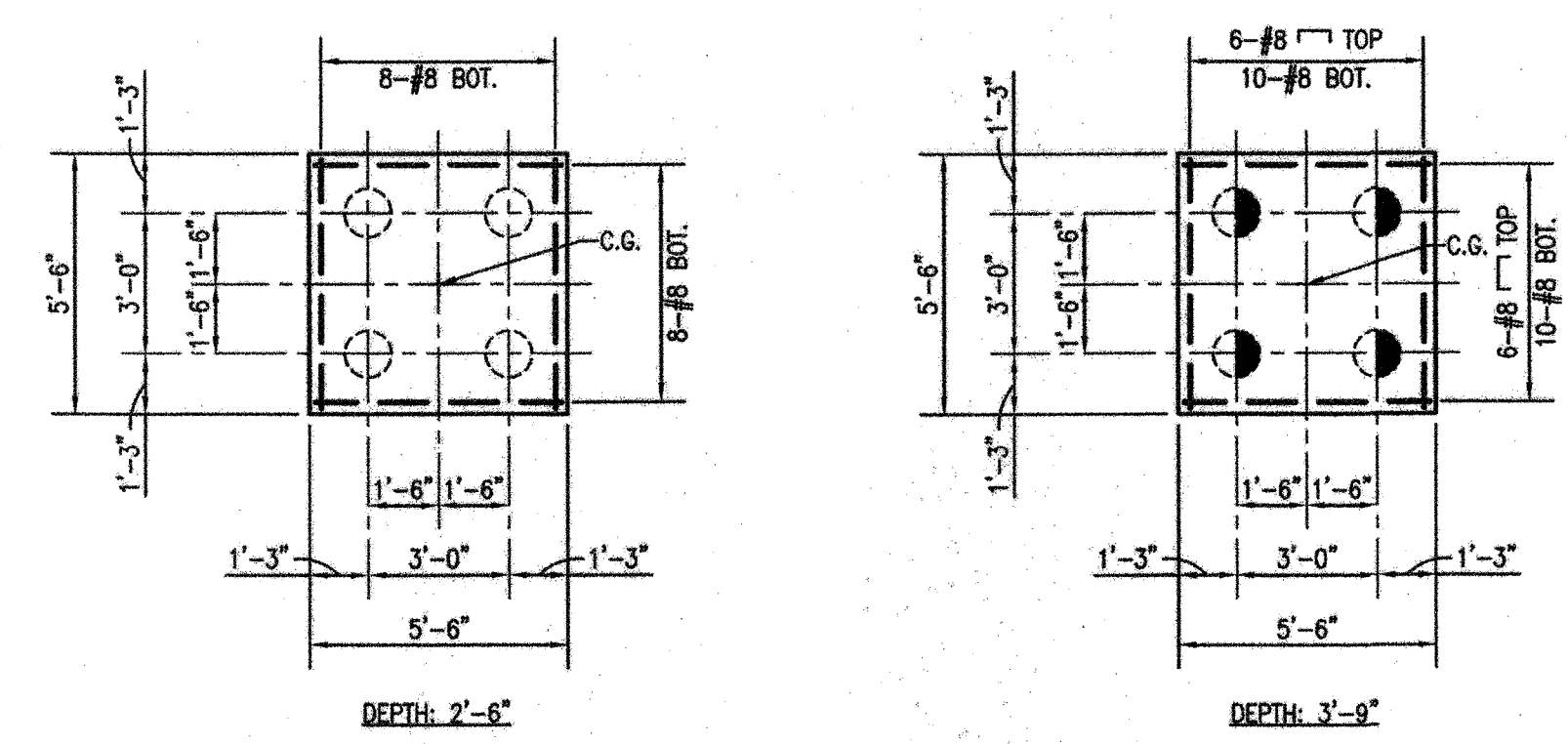
19. COLD FORMED METAL FRAMING:
- MEMBERS DESIGNATED SUCH AS 600S250-68, ETC. ARE COLD FORMED METAL FRAMING MEMBERS AS MANUFACTURED BY CLARK STEEL FRAMING SYSTEMS OR APPROVED EQUAL.
 - ALL MATERIAL 18 GAGE OR THINNER TO BE 33 KSI. ALL MATERIAL GREATER THAN 18 GAGE SHALL BE 50 KSI.
 - INSTALL, CONNECT, PROVIDE BRIDGING, ETC. IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
 - SUBMIT SHOP DRAWINGS FOR REVIEW INDICATING PLACEMENT FOR FRAMING MEMBERS SHOWING SIZE AND GAGE DESIGNATIONS, NUMBER, TYPE, LOCATION AND SPACING. INCLUDE ALL SUPPLEMENTAL STRAPPING, BRACING, BRIDGING, ACCESSORIES, AND DETAILS FOR PROPER INSTALLATION. SHOP DRAWINGS SHALL BEAR THE SEAL OF THE PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THE PROJECT.
 - FURNISH COMPLETE SEALED DESIGN COMPUTATIONS FOR RECORD PURPOSES.
 - TYPICAL FRAMING AND DETAILS INDICATED ON DRAWINGS SHIT. S5.02. REPRESENT BASIS OF DESIGN WHICH IS TO BE EXECUTED BY THE CFMF DESIGNER. ALL CONNECTIONS, BRACING AND FRAMING AT OPENINGS ARE TO BE DESIGNED BY THE CFMF DESIGNER.
 - REFER TO SPECIFICATIONS FOR ADDITIONAL INFORMATION.



2 DETAIL-2 PILE CAP (2PA) 1/4"=1'-0"

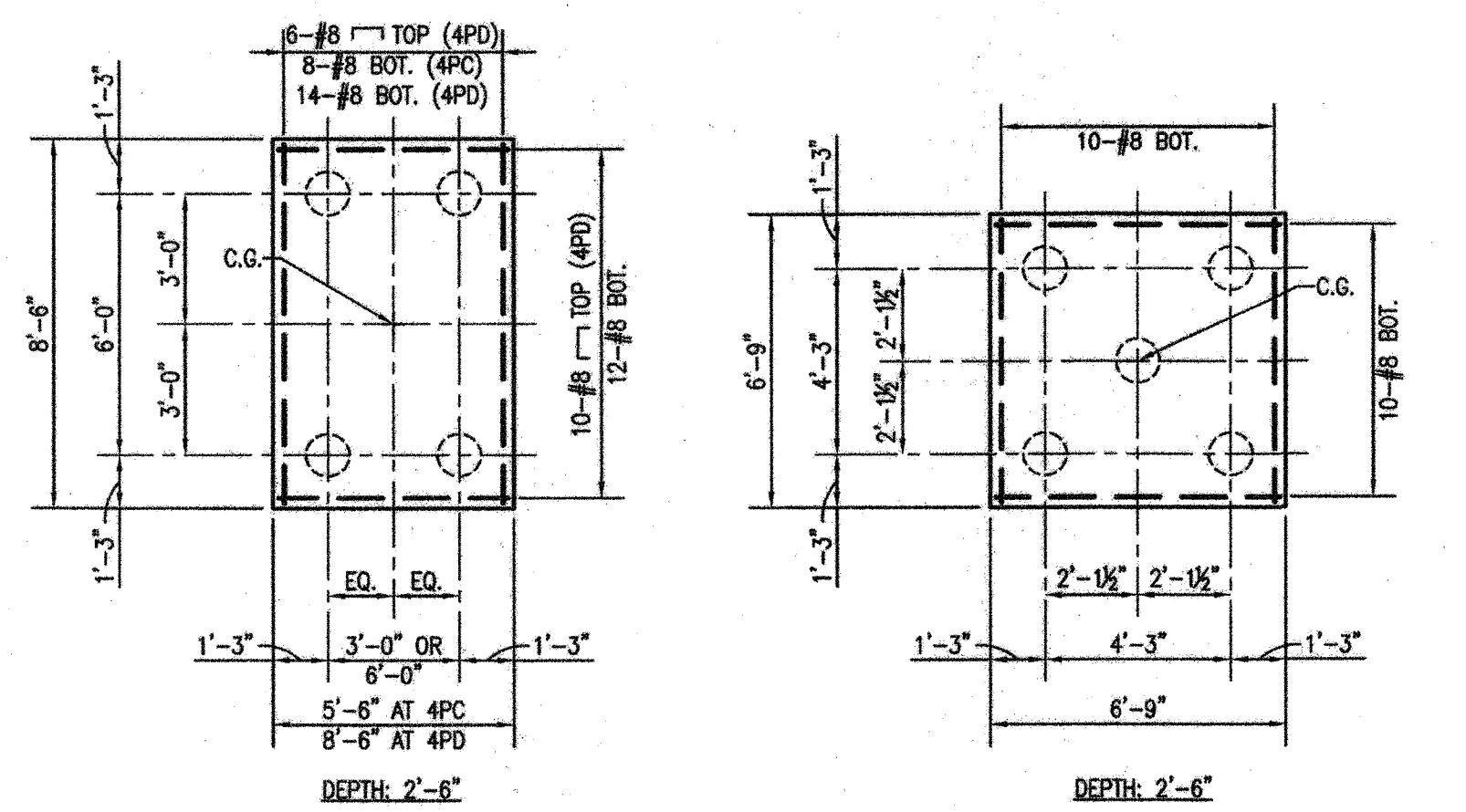
3 DETAIL-2 PILE CAP (2PA)(T) 1/4"=1'-0"

4 DETAIL-2 PILE CAP (2PB) 1/4"=1'-0"



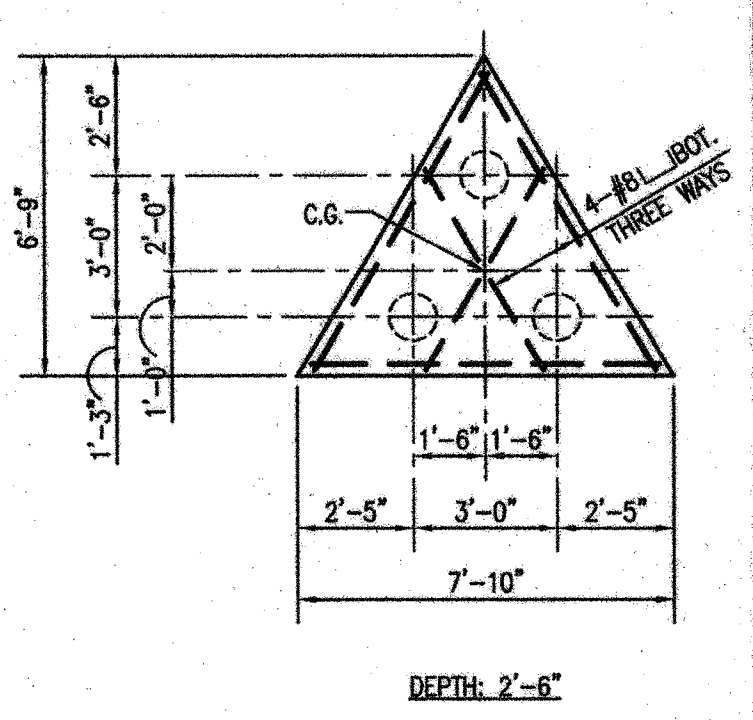
6 DETAIL-4 PILE CAP (4PA) 1/4"=1'-0"

7 DETAIL-4 PILE CAP (4PA)(T) 1/4"=1'-0"

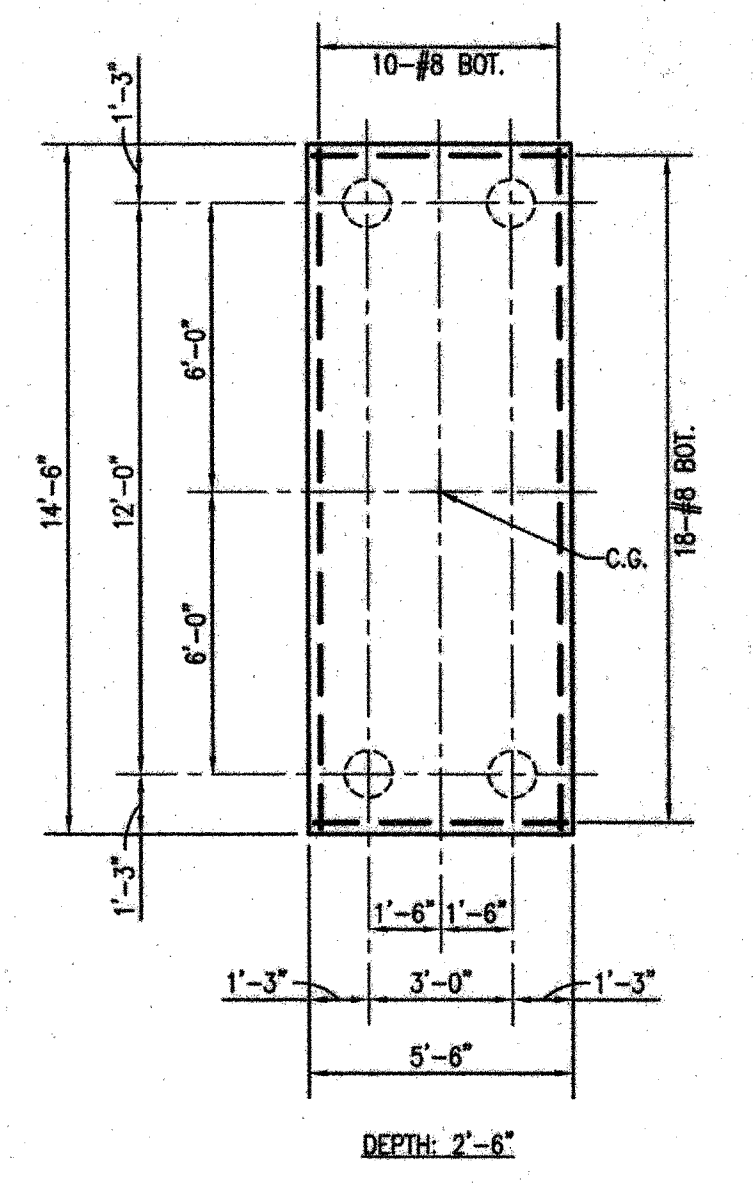


9 DETAIL-4 PILE CAP (4PC) & (4PD) 1/4"=1'-0"

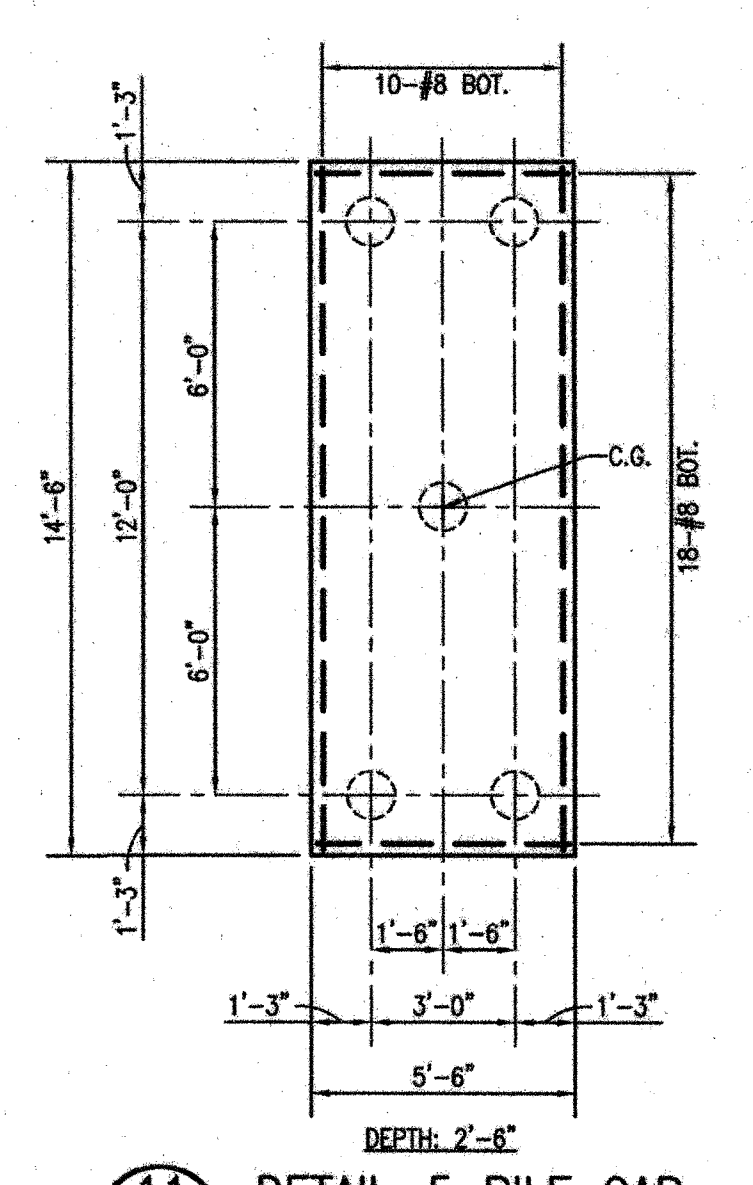
10 DETAIL-5 PILE CAP (5PA) 1/4"=1'-0"



5 DETAIL-3 PILE CAP (3PA) 1/4"=1'-0"



8 DETAIL-4 PILE CAP (4PB) 1/4"=1'-0"



11 DETAIL-5 PILE CAP (5PB) 1/4"=1'-0"

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GENERAL NOTES & PILE CAP DETAILS

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