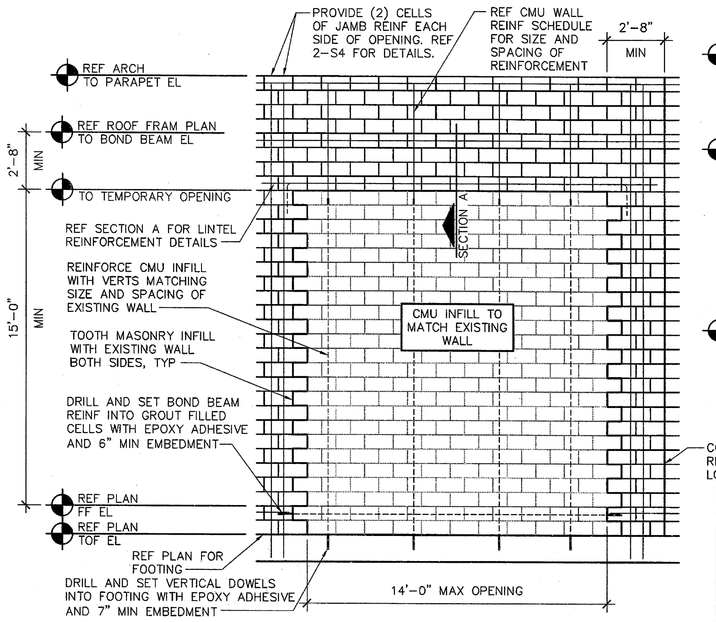

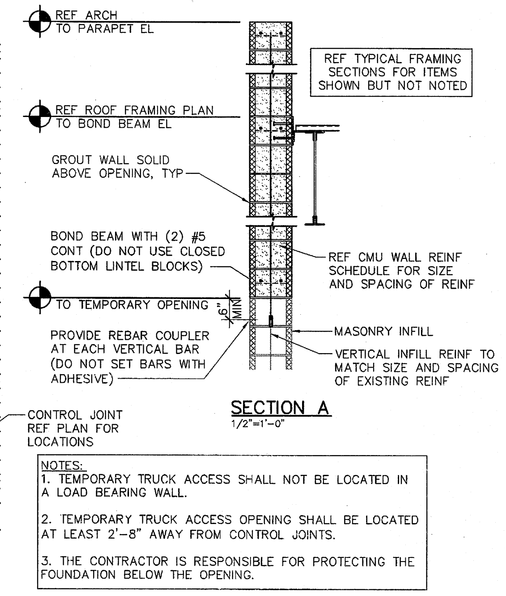

SECTION A
1/2"=1'-0"

NOTES:
1. TEMPORARY TRUCK ACCESS SHALL NOT BE LOCATED IN A LOAD BEARING WALL.
2. TEMPORARY TRUCK ACCESS OPENING SHALL BE LOCATED AT LEAST 2'-8" AWAY FROM CONTROL JOINTS.
3. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE FOUNDATION BELOW THE OPENING.

8 TEMPORARY TRUCK ACCESS WALL REINFORCEMENT

1/4"=1'-0" 042911 S0-008_41

SPECIAL INSPECTIONS REQUIRED

SPECIAL INSPECTIONS AS REQUIRED BY SECTION 1704 OF THE 2009 INTERNATIONAL BUILDING CODE:

PERIODIC SPECIAL INSPECTIONS:

1. STEEL – REFER TO SPECIFICATION 05120
2. STEEL JOISTS AND JOIST GIRDERS – REFER TO SPECIFICATION 05210
3. STEEL DECK – REFER TO SPECIFICATION 05300
4. CONCRETE – REFER TO SPECIFICATION 03310 AND 03312
5. MASONRY – REFER TO SPECIFICATION 04200

CONTINUOUS SPECIAL INSPECTIONS:

1. CONCRETE – REFER TO SPECIFICATION 03310 AND 03312
2. MASONRY – REFER TO SPECIFICATION 04200
3. POST-INSTALLED ANCHORS – REFER TO SPECIFICATION 05090

7 IBC SPECIAL INSPECTIONS

NTS 073109 s0-007_IBC

GENERAL NOTES

1.0 GENERAL
- 1.1 STEEL FRAMING IS NON-SELF SUPPORTING AND REQUIRES INTERACTION WITH OTHER ELEMENTS NOT CLASSIFIED AS STRUCTURAL STEEL TO PROVIDE THE REQUIRED STABILITY AND RESISTANCE TO LATERAL FORCES.
- 1.2 THE STEEL FRAMING AND ALL CONCRETE AND CMU WALLS SHALL BE TEMPORARILY BRACED UNTIL ALL STEEL BRACING, FLOOR AND ROOF DECKS, AND CONCRETE AND CMU WALLS HAVE BEEN INSTALLED AND ALL CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE.
- 1.3 THE CONTRACTOR SHALL COORDINATE THE SIZE AND LOCATION OF EQUIPMENT PADS AND PENETRATIONS THROUGH THE STRUCTURE FOR MECHANICAL, ELECTRICAL AND PLUMBING WORK.

2.0 FOUNDATIONS AND SLABS-ON-GRADE
- 2.1 FOOTING DESIGNS ARE BASED ON A NET SOIL BEARING PRESSURE OF 2500 PSF FOR SPREAD FOOTINGS AND 2000 PSF FOR CONTINUOUS WALL FOOTINGS.
- 2.2 THE CONTRACTOR MUST READ THE SOILS REPORT AND BE THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION GIVEN THEREIN.
- 2.3 THE BUILDING PAD WILL INCLUDE, BUT NOT BE LIMITED TO THAT WHICH IS PRESENTED IN THE "FOUNDATION SUBSURFACE PREPARATION" DESCRIPTION. THE "FOUNDATION SUBSURFACE PREPARATION" SHALL GOVERN OVER DISCREPANCIES OR OPTIONS NOTED IN THE SOILS REPORT.
- 2.4 EXTERIOR FOOTINGS SHALL BEAR AT OR BELOW MINIMUM BEARING DEPTH. MINIMUM BEARING DEPTH IS 16" BELOW ADJACENT FINISHED GRADE OR FROST DEPTH, 24", WHICHEVER IS DEEPER. THICKENED SLAB EDGES FOR STOOPS, CANOPIES, ETC. SHALL BE 16 INCHES (UNO).
- 2.5 STANDARD PROCEDURES OF FROST PROTECTION FOR FOUNDATIONS AND EXCAVATIONS SHALL BE EMPLOYED FOR WINTER CONSTRUCTION. BACKFILLING OF EXCAVATIONS SHALL BE DONE AS SOON AS POSSIBLE TO PROTECT FOUNDATIONS FROM FROST.
- 2.6 SLABS-ON-GRADE ARE UNREINFORCED CONCRETE UNLESS NOTED OTHERWISE.
- 2.7 PROVIDE SAW CUT JOINTS AT 10'-0" OC MAXIMUM SPACING UNLESS NOTED OTHERWISE ON THE CONTRACT DRAWINGS.
- 2.8 PROVIDE (2) #4 BAR, 4'-0" LONG DIAGONALLY IN TOP OF SLAB AT REENTRANT CORNERS.
- 2.9 "CJ" INDICATES SAW CUT CONTRACTION JOINT IN SLAB-ON-GRADE, "CONST JT" INDICATES DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. REF. SPECIFICATION SECTION 03312.
- 2.10 TRUCK DOCK RAMP SHALL BE 6" CONCRETE REINFORCED WITH #4 BARS AT 16" OC, EW PLACED 2" FROM TOP OF SLAB. PROVIDE CONTRACTION JOINTS AT 15'-0" MAX. REF SPECIFICATIONS 03310 FOR SLUMP AND AIR CONTENT, TRUCK DOCK SUBRGADE PREPARATION AND BASE COURSE MATERIAL SHALL BE THE SAME AS FOR THE HEAVY DUTY PAVEMENT.
- 2.11 PHARMACY DRIVE-THRU PAVING SHALL BE 5" THICK CONCRETE REINFORCED WITH #4 BARS AT 16" OC, EW PLACED 2" FROM TOP OF SLAB. REF ARCH FOR CONTRACTION JOINT SPACING. REFERENCE SPECIFICATIONS 03310 FOR SLUMP AND AIR CONTENT, SUBGRADE PREPARATION AND BASE COURSE MATERIAL SHALL BE THE SAME AS THE HEAVY DUTY PAVEMENT.
- 2.12 HORIZ BARS IN FOOTINGS AND CONCRETE STEM WALLS SHALL BE CONTINUOUS. PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS UNO. REF CORNER BAR DETAIL.
- 2.13 FOUNDATION WALLS SHALL HAVE TEMPORARY BRACING BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL WALL IS PERMANENTLY BRACED.
- 2.14 FOUNDATION PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ARCHITECT/ENGINEER. PENETRATIONS SHALL BE THROUGH FOUNDATION STEMWALL OR 6" CLEAR BELOW FOOTING. REF 14-S2 FOR FOOTING STEP DETAIL.

3.0 CONCRETE AND REINFORCING STEEL
- 3.1 MINIMUM COMPRESSIVE STRENGTH (F'c) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS:

A. STRUCTURAL CAST-IN-PLACE CONCRETE	3,000 PSI	SPECIFICATION SECTION 03310
B. EXTERIOR CAST-IN-PLACE CONCRETE SLABS	REF SPEC	SPECIFICATION SECTION 03310
C. INTERIOR CAST-IN-PLACE CONCRETE SLABS	3,500 PSI	SPECIFICATION SECTION 03312
D. COMPACTOR PAD	5,000 PSI	SPECIFICATION SECTION 03310

FOR ALL OTHER CONCRETE PROPERTIES REFER TO SPECIFICATIONS.
- 3.2 DOCK WALLS EXPOSED TO FREEZE/THAW CYCLES SHALL BE 3500 PSI AIR ENTRAINED CONCRETE.
- 3.3 REINFORCING STEEL SHALL MEET ASTM SPECIFICATION A 615, DEFORMED BAR, GRADE 60 OR ASTM SPECIFICATION A 706, DEFORMED BAR, GRADE 60. REF SHEET S4 FOR LAP SPLICE LENGTHS UNO ON DETAILS.
- 3.4 REFER TO ACI 318 FOR CONCRETE COVER, ACI 315 FOR DETAILING PRACTICES AND FABRICATION, AND ACI 301 FOR STANDARD PRACTICE FOR MIXING AND PLACING CONCRETE.
- 3.5 LEAN CONCRETE – MIN 2 1/2 SACKS PORTLAND CEMENT PER CUBIC YARD.

4.0 STRUCTURAL STEEL
- 4.1 STRUCTURAL STEEL SHALL MEET THE FOLLOWING MINIMUM YIELD STRENGTH AND SPECIFICATIONS:

	YIELD	ASTM SPECIFICATION
A. WIDE FLANGE STEEL SHAPES:	50 KSI	A 992
B. HOLLOW STRUCTURAL SHAPES:	46 KSI	A 500 GRADE B
C. STRUCTURAL STEEL PIPE:	35 KSI	A 53 TYPE E, GRADE B
D. BARS, PLATES, CHANNELS, ANGLES, AND ANCHOR BOLTS:	36 KSI	A 36 (UNO)
E. HEADED STUD ANCHORS:	50 KSI	A 108 (GRADE DESIGNATIONS 1010 TO 1020 INCLUSIVE)

- 4.2 BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4" DIAMETER ASTM A 325-N HIGH-STRENGTH BOLTS, UNO. ALL BOLTED CONNECTIONS ARE BEARING TYPE. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT, UNO.
- 4.3 WELDING SHALL MEET ANSI/AWS D1.1 STRUCTURAL WELDING CODE. ELECTRODES SHALL BE 70 KSI LOW HYDROGEN.
- 4.4 THE FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGN AND ADEQUACY OF CONNECTIONS THAT ARE NOT DESIGNED OR FULLY DETAILED ON THE CONTRACT DOCUMENTS.
- 4.5 PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION.
- 4.6 PROVIDE 1 1/2 INCH NON-SHRINK GROUT UNDER BASE PLATE AFTER ERECTION EXCEPT USE 2 1/2 INCH NON-SHRINK GROUT WHEN COLUMN ANCHOR BOLTS ARE 1 1/4 INCH DIAMETER OR LARGER.
- 4.7 PROVIDE L5x3x1/4 (LLV) FIELD-FABRICATED FRAME BETWEEN JOISTS AT OPENINGS IN ROOF GREATER THAN 10"x10", UNO. (INCLUDING ROOF DRAIN AND EXHAUST FAN OPENINGS).
- 4.8 PROVIDE 1/2"x6"x0'-10" BEARING PLATES WITH (2) 1/2" DIAx0'-5" HEADED STUD ANCHORS FOR STEEL BEAMS BEARING UPON MASONRY, UNO. FILL CELLS BELOW WITH GROUT AND PROVIDE (2) #6 VERTICAL REINFORCING BARS WITH DOWELS FROM FOUNDATION, UNO.

5.0 STEEL JOISTS AND JOIST GIRDERS
- 5.1 HANGERS SUPPORTING MECHANICAL EQUIPMENT FROM JOIST CHORDS SHALL BE LOCATED WITHIN 3 INCHES OF JOIST PANEL POINTS OR JOIST SHALL BE REINFORCED PER JOIST REINFORCING DETAIL. HANGER LOADS GREATER THAN 150 POUNDS SHALL NOT BE ATTACHED TO THE EDGE OF CHORD ANGLES AND SHALL BE CENTERED ON JOIST CHORD.
- 5.2 JOISTS AND JOIST GIRDERS SHALL RESIST THE NET UPLIFT PRESSURE ON ROOF SHOWN IN THE DESIGN LOADS.
- 5.3 SPECIAL JOISTS AND JOIST GIRDERS THAT REQUIRE SPECIFIC ORIENTATION SHALL BE TAGGED AT ONE END. DEFINE LOCATION OF TAGGED END ON ERECTION DRAWINGS.
- 5.4 DIAGONAL BRIDGING SHALL BE PROVIDED BETWEEN ADJACENT JOISTS WHENEVER BOTTOM CHORD HORIZONTAL BRIDGING IS DISCONTINUOUS.
- 5.5 JOIST MANUFACTURER SHALL DESIGN THE COMPRESSION CHORD OF ALL JOISTS SUPPORTING ROOF TOP UNITS, SKY LIGHTS, AND OTHER STRUCTURES FOR AN UNBRACED LENGTH APPLICABLE TO THE CONDITIONS AT THE PROJECT WHERE THE UNBRACED LENGTH IS GREATER THAN THE SJI MAXIMUM.

6.0 STEEL DECK
- 6.1 ROOF DECK SHALL BE PAINTED TYPE "B" (WIDE RIB) THREE SPAN MIN (UNO ON FRAMING PLAN) AS SHOWN ON ROOF FRAMING PLAN. THE STEEL ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM AND SCHEDULE FOR ATTACHMENT PATTERN OPTIONS.
- 6.2 PROVIDE 2 LAYERS OF ROOF DECK WHERE SINGLE SPAN CONDITION EXISTS. MAKE DIAPHRAGM CONNECTIONS AFTER PLACEMENT OF BOTH LAYERS OF ROOF DECK.
- 6.3 WHEN THE ROOF DECK IS WELDED, WELDING RODS SHALL BE E 6022.

7.0 LIGHT GAGE STEEL FRAMING
- 7.1 FOR 18 GAUGE AND LIGHTER FRAMING, CONNECTIONS SHALL BE MADE USING SELF-DRILLING, SELF-TAPPING SCREWS OR POWDER ACTUATED FASTENERS.
- 7.2 FOR 16 GAUGE AND HEAVIER FRAMING, CONNECTIONS SHALL BE MADE BY SELF-DRILLING SELF-TAPPING SCREWS, POWDER ACTUATED FASTENERS, OR BY WELDING AS INDICATED ON THE DRAWINGS.
- 7.3 SELF-DRILLING SELF-TAPPING SCREW OR POWDER ACTUATED FASTENER CONNECTIONS ARE NOT PERMITTED TO BE USED WHERE WELDED STUD CONNECTIONS ARE SHOWN ON THE DRAWINGS.
- 7.4 ALL 18 GAUGE AND LIGHTER FRAMING SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI. ALL 16 GAUGE AND HEAVIER FRAMING SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI.
- 7.5 WHERE DETAILED CONNECTIONS OCCUR AT BRIDGING HOLES, INSTALL 16 GA x 1'-0" TRACK OVER STUD WITH (4) #10 SELF DRILLING SCREWS EACH LEG. CENTER TRACK ON CONNECTION.

8.0 MASONRY
- 8.1 CONCRETE MASONRY UNITS SHALL MEET ASTM SPECIFICATION C 90. THE SPECIFIED DESIGN COMPRESSIVE STRENGTH OF CONCRETE MASONRY (F'm) SHALL BE 2000 PSI. THE NET AREA COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY UNITS SHALL BE 2800 PSI.
- 8.2 MORTAR SHALL BE A PREBLENDED DRY MIX CONFORMING TO ASTM C 1714 AND MEETING THE PROPERTY SPECIFICATIONS OF ASTM C 270 TYPE "S" MORTAR. MASONRY CEMENT SHALL NOT BE USED FOR MORTAR.
- 8.3 GROUT SHALL MEET ASTM SPECIFICATION C 476 AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI. REF SPECIFICATION SECTION 04200 FOR ADDITIONAL REQUIREMENTS.
- 8.4 GROUT PLACED BY THE LOW LIFT GROUTING METHOD SHALL BE MECHANICALLY CONSOLIDATED USING A VIBRATOR WITH A MAXIMUM 3/4" DIAMETER HEAD. REF SPECIFICATION SECTION 04200 FOR HIGH LIFT GROUTING PROCEDURES.
- 8.5 HORIZONTAL JOINT REINFORCEMENT SHALL BE LADDER TYPE, (REF SPEC SECTION 04200). JOINT REINFORCEMENT SHALL BE SPACED AT 8" OC BELOW FINISHED FLOOR AND IN PARAPETS, AND 16" OC ABOVE FINISH FLOOR, UNO.
- 8.6 CONCRETE MASONRY SHALL BE LAID IN RUNNING (COMMON) BOND.
- 8.7 CONCRETE MASONRY BELOW FINISHED FLOOR SHALL BE NORMAL WEIGHT UNITS AND SHALL HAVE ALL CELLS FULLY GROUTED. CONCRETE MASONRY ABOVE FINISHED FLOOR SHALL BE LIGHT WEIGHT OR NORMAL WEIGHT AND SHALL BE GROUTED ONLY AT REINFORCED CELLS AND BOND BEAMS, UNO.
- 8.8 INSTALL TEMPORARY BRACING AT ALL CMU WALLS. DO NOT REMOVE TEMPORARY BRACING UNTIL WALL IS PERMANENTLY BRACED BY CONNECTION TO THE ROOF STRUCTURE.
- 8.9 REFER TO CMU WALL REINFORCING DIAGRAM AND CMU WALL VERTICAL REINFORCING SCHEDULE FOR PRIMARY WALL REINFORCEMENT.
- 8.10 REFER TO CMU WALL REINFORCING DIAGRAM, TYPICAL MASONRY WALL OPENING DIAGRAM AND SCHEDULE, AND SPECIAL WALL SECTION SCHEDULE AND DETAILS FOR ADDITIONAL REINFORCING AT OPENINGS, CONTROL JOINTS, CORNERS AND ENDS OF WALL PANELS.
- 8.11 REFER TO WALL EMBED SCHEDULE, FOUNDATION DETAILS, AND FRAMING DETAILS FOR ADDITIONAL BOND BEAM LOCATIONS AND EMBEDDED ITEMS.
- 8.12 PROVIDE HORIZONTAL REINFORCEMENT AS INDICATED IN THE CMU WALL REINFORCING DIAGRAM. USE OPEN KNOCK OUT BOND BEAM BLOCK. DO NOT USE TROUGH TYPE BLOCKS FOR BOND BEAMS. DO NOT CONTINUE BOND BEAM REINFORCING THROUGH CONTROL JOINTS, UNO.
- 8.13 MASONRY CONSTRUCTION REQUIRES SPECIAL INSPECTION. REF SPECIFICATION SECTION 04200 FOR REQUIREMENTS.
- 8.14 INSTALL EMBEDDED STEEL ITEMS FOR OVERHEAD DOORS IN GROUTED CELLS. COORDINATE LOCATIONS OF EMBEDDED ITEMS WITH OVERHEAD DOOR MANUFACTURER.

9.0 POST-INSTALLED ANCHORS
- 9.1 POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. REFER TO SPECIFICATION 05090 FOR ADDITIONAL INFORMATION.

10.0 SUBGRADE AND BASE
- 10.1 THE TESTING AGENCY SHALL VERIFY THE SUBGRADE IS COMPACTED TO THE OPTIMUM MAXIMUM DRY DENSITY AS SPECIFIED IN THE PAD PREPARATION. A QUALIFIED REPRESENTATIVE OF THE TESTING AGENCY SHALL WITNESS PROOF ROLLING OF THE SUBGRADE TO IDENTIFY UNACCEPTABLE AREAS OF THE BUILDING PAD. THE CONTRACTOR SHALL RECOMPACT OR REMOVE AND REPLACE SOFT AREAS AS DETERMINED TO BE UNACCEPTABLE BY THE TESTING AGENCY. THE TESTING AGENCY SHALL PROVIDE A REPORT TO THE OWNER.
- 10.2 THE TESTING AGENCY SHALL VERIFY THE AGGREGATE BASE IS COMPACTED TO THE OPTIMUM MAXIMUM DRY DENSITY AS SPECIFIED IN THE PAD PREPARATION JUST PRIOR TO PLACING THE SLAB. A QUALIFIED REPRESENTATIVE OF THE TESTING AGENCY SHALL WITNESS PROOF ROLLING OF THE BASE TO IDENTIFY UNACCEPTABLE AREAS OF THE BUILDING PAD. THE CONTRACTOR SHALL REPAIR SOFT AREAS AS DIRECTED BY THE TESTING AGENCY. RUTTING DUE TO PROOF ROLLING DEEPER THAN 1/2 INCH IN THE BASE SHALL BE UNACCEPTABLE. THE TESTING AGENCY SHALL PROVIDE A REPORT TO THE OWNER AND THE ENGINEER STATING THE SUBGRADE AND BASE IS ACCEPTABLE.
- 10.3 NOTIFY IMMEDIATELY THE OWNER'S REPRESENTATIVE AND ENGINEER IF UNUSUAL SOIL CONDITIONS ARE FOUND.
- 10.4 PROTECT EXISTING STRUCTURES, UTILITIES, PROPERTY, ETC. RESTORE ALL ITEMS DAMAGED, AS REQUIRED BY OWNER'S REPRESENTATIVE, AT NO COST TO OWNER OR WITHOUT EXTENSION OF CONTRACT TIME.
- 10.5 DO NOT ALLOW STORED EXCAVATION MATERIAL TO DISRUPT PROPER DRAINAGE OF AREA.
- 10.6 DISPOSE OF EXCAVATED MATERIAL AS REQUIRED BY OWNER'S REPRESENTATIVE.

1 GENERAL NOTES

NTS 072911 SPANNER

ABBREVIATION LEGEND

ABBR	DEFINITION
AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
BFF	BELOW FINISHED FLOOR
BL	BLOCK LINTEL
BM	BEAM
BO	BOTTOM OF
BOS	BOTTOM OF STEEL
BRG	BEARING
CJ	CONTRACTION JOINT
CL	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
COND	CONDENSER UNIT
CONST	CONSTRUCTION
CONT	CONTINUOUS
DIA	DIAMETER
EDC	ELECTRICAL DISTRIBUTION CENTER
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
EQ	EQUAL
EW	EACH WAY
FDN	FOUNDATION
FF	FINISHED FLOOR
FS	FAR SIDE
FTG	FOOTING
GA	GAGE
GC	GENERAL CONTRACTOR
GYP	GYPSUM BOARD
H	HEIGHT
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HSS	HOLLOW STRUCTURAL SHAPE
INFO	INFORMATION
ISO	ISOLATION
JBE	JOIST BEARING ELEVATION
JST	JOIST
JT	JOINT
KSI	KIPS PER SQUARE INCH
L	LENGTH
LB	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
MAX	MAXIMUM
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MTL	METAL
NBPS	NOT BY PRECAST SUPPLIER
NIC	NOT IN CONTRACT
NO	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OH	OPPOSITE HAND
OSL	OUTSTANDING LEG
PAF	POWDER ACTUATED FASTENER
PC	PRECAST
PCF	POUNDS PER CUBIC FOOT
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PMEJ	PREMOLDED EXPANSION JOINT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY	QUANTITY
RACK	REFRIGERATION RACK
REF	REFER TO
REINF	REINFORCING
REQD	REQUIRED
REV	REVERSE
RO	ROUGH OPENING
RTU	ROOF TOP UNIT
SCHED	SCHEDULE
SDI	STEEL DECK INSTITUTE
SIM	SIMILAR
SJI	STEEL JOIST INSTITUTE
SPECS	SPECIFICATIONS
STRUC	STRUCTURAL
T&B	TOP AND BOTTOM
THK	THICKNESS
TO	TOP OF
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOGB	TOP OF GRADE BEAM
TOM	TOP OF MASONRY
TOP	TOP OF PAVING
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WIDTH
WP	WORK POINT

6 ABBREVIATION LIST

NTS 051009 S0-006

FOUNDATION SUBSURFACE PREPARATION

UNLESS SPECIFICALLY INDICATED OTHERWISE IN THE DRAWINGS AND/OR SPECIFICATIONS, THE LIMITS OF THIS SUBSURFACE PREPARATION ARE CONSIDERED TO BE THAT PORTION OF THE SITE DIRECTLY BENEATH AND 5 FEET BEYOND THE BUILDING AND APPURTENANCES.

APPURTENANCES ARE THOSE ITEMS ATTACHED TO THE BUILDING PROPER (REFER TO DRAWING SHEET SP1). TYPICALLY INCLUDING, BUT NOT LIMITED TO, THE BUILDING SIDEWALKS, GARDEN CENTER, PORCHES, RAMPS, STOOPS, TRUCK WELLS/DOCKS, CONCRETE APRONS AT THE AUTOMOTIVE CENTER, COMPACTOR PAD, ETC. THE BASE AND THE VAPOR BARRIER, WHERE REQUIRED, DO NOT EXTEND BEYOND THE LIMITS OF THE ACTUAL BUILDING AND THE APPURTENANCES.

ESTABLISH THE FINAL SUBGRADE ELEVATION TO ALLOW FOR THE CONCRETE SLAB AND BASE, AND SUBBASE. REFERENCE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR REQUIRED SLAB THICKNESS. THE 4 INCH THICK BASE MATERIAL SHALL CONFORM TO LOUISIANA STANDARD SPECIFICATIONS FOR ROAD AND BRIDGES SECTION 1003.03D, CRUSHED LIMESTONE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ACCURATE MEASUREMENTS FOR ALL CUT AND FILL DEPTHS REQUIRED. ANY PROPOSED EQUIVALENT ALTERNATIVE BASE MATERIAL MUST BE SUBMITTED FOR APPROVAL WITHIN 30 DAYS AFTER AWARD OF CONTRACT. ANY EQUIVALENT ALTERNATIVE SHALL ONLY BE USED IF APPROVED IN WRITING BY THE CEC AND AOR.

REMOVE EXISTING BUILDINGS, PAVEMENT, SURFACE VEGETATIONS, TOPSOIL, ROOT SYSTEMS, ORGANIC MATERIAL, EXISTING FILL, AND SOFT OR OTHERWISE UNSUITABLE MATERIAL FROM THE BUILDING AREA. ADDITIONALLY, REMOVE THE TOP 12 INCHES OF MATERIAL FROM UNDERNEATH THE EXISTING BUILDING SLABS AND PAVEMENT WITHIN THE BUILDING AREA. THE CONCRETE SLAB, BASE, AND SUBBASE SHALL BE SUPPORTED ON A MINIMUM OF 24 INCHES OF COMPACTED STRUCTURAL FILL. BEFORE THE PLACEMENT OF ANY FILL, 12 INCHES OF THE EXPOSED SUBGRADE SHALL BE RECOMPACTED IN PLACE TO AT LEAST 98% OF THE STANDARD PROCTOR.

PROOFROLL THE EXPOSED SUBGRADE USING A RUBBER TIRED SINGLE AXLE TRUCK OR EQUIVALENT WEIGHING 15 TO 20 TONS. REMOVE AND REPLACE UNSUITABLE SOILS WITH SUITABLE MATERIAL. THE SUBGRADE MATERIAL SHALL BE FREE OF ORGANIC AND OTHER DELETERIOUS MATERIALS.

SINCE THE SOIL AT THE SITE EXHIBITS LOW TO MEDIUM PLASTICITY CHARACTERISTICS, A BUFFER ZONE, CONSISTING OF TWO (2) FEET OF LOW PLASTICITY COMPACTED FILL SHOULD BE PROVIDED BELOW THE FLOOR SLAB.

THE FILL MATERIAL SHALL MEET THE FOLLOWING REQUIREMENTS:

LOCATION WITH RESPECT TO FINAL GRADE	P.I	L.L.
BUILDING AREA, BELOW UPPER 4 FEET	35 MAX	50 MAX
BUILDING AREA, UPPER 4 FEET	20 MAX	40 MAX

THE FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8 INCHES IN THICKNESS AND COMPACTED TO AT LEAST 98 PERCENT OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D698) AT A MOISTURE CONTENT WITHIN 1 PERCENT BELOW TO 3 PERCENT ABOVE THE OPTIMUM MOISTURE.

THE FOUNDATION SYSTEM SHALL BE ISOLATED SPREAD FOOTINGS AT COLUMNS AND CONTINUOUS FOOTINGS AT WALLS.

THIS FOUNDATION SUBSURFACE PREPARATION DOES NOT CONSITUTE A COMPLETE SITE WORK SPECIFICATION. IN CASE OF CONFLICT, INFORMATION COVERED IN THIS PREPARATION SHALL TAKE PRECEDENCE OVER THE WAL-MART SPECIFICATIONS. REFER TO THE SPECIFICATIONS FOR SPECIFIC INFORMATION NOT COVERED IN THIS PREPARATION. THIS INFORMATION WAS TAKEN FROM A GEOTECHNICAL REPORT PREPARED BY STRATUM ENGINEERING, LLC, DATED JUNE 26, 2011 AND REVISED DECEMBER 27, 2011 (GEOTECHNICAL REPORT IS FOR INFORMATION ONLY AND IS NOT A CONSTRUCTION SPECIFICATION).

5 FOUNDATION SUBSURFACE PREPARATION

NTS 062609 S0-005

LEGEND

000'-0"	INDICATES TOP OF BOND BEAM ELEVATION AT ROOF BETWEEN CMU CONTROL JOINTS		SATELLITE DISH
			EXHAUST HOOD
	INDICATES MASONRY CONTROL JOINT		EXHAUST FAN
	ROOF TOP UNIT (RTU)	*	INDICATES CONCENTRATED LOAD REF NOTE 5-S3

ITEMS IN LEGEND MAY NOT OCCUR ON ALL PLANS

3 LEGEND

NTS 042911 S0-003_41

DESIGN LOADS

1. CODE — IBC 2009
2. GRAVITY LOADS

	MAX	MIN
A. DEAD LOADS-ROOF		
-ROOFING INSULATION AND METAL DECK	6 PSF	3 PSF
-JOIST	3 PSF	2 PSF
-MECH, ELEC, PLUMBING, SPRINKLERS AND MERCHANDISE	6 PSF	1 PSF
TOTALS	15 PSF	6 PSF
B. LIVE LOADS		
1. ROOF	20 PSF	MIN OR SNOW LOAD

C. SNOW LOAD

1. IMPORTANCE FACTOR (I)	1.0
2. SNOW EXPOSURE FACTOR (Ce)	1.0
3. ROOF THERMAL FACTOR (Ct)	1.0
4. GROUND SNOW LOAD (P_g)	0 PSF
5. FLAT ROOF SNOW LOAD (Pf)	0 PSF
6. OWNER DIRECTED SNOW SURCHARGE LOAD	0 PSF
TOTAL DESIGN ROOF SNOW LOAD	0 PSF

3. LATERAL LOADS

A. WIND LOADS

1. BASIC WIND SPEED (3-SECOND GUST)	120 MPH
2. IMPORTANCE FACTOR (I)	1.0
3. EXPOSURE CATEGORY	C
4. VELOCITY PRESSURE EXPOSURE COEFFICIENT (Kh)	0.86
5. INTERNAL PRESSURE COEFFICIENT (GCpi)	0.18
6. WIDTH OF EDGE ZONE FOR JOISTS (a)	6.4 FT
7. WIDTH OF EDGE ZONE FOR JOIST GIRDERS (2h)	13.9 FT

B. DESIGN WIND PRESSURE (DOES NOT INCLUDE LOAD COMB. FACTOR)

1. WALLS (BASED UPON 100 SQ FT)	
INTERIOR ZONE	27.1 PSF
CORNER ZONE	30.4 PSF
PARAPETS	54.0 PSF
2. ROOF UPLIFT (NET)	
JOIST GIRDERS EDGE ZONE	19.4 PSF
JOIST GIRDERS INTERIOR ZONE	5.7 PSF
JOIST CORNER ZONE	28.4 PSF
JOIST EDGE ZONE	28.4 PSF
JOIST INTERIOR ZONE	23.0 PSF

C. SEISMIC LOADS

1. 5% DAMPED SPECTRAL RESPONSE COEFF (SDS)	.1269
2. 1-SEC PERIOD SPECTRAL RESP. COEFF. (SD1)	.0848
3. SEISMIC OCCUPANCY CATEGORY	II
4. SEISMIC DESIGN CATEGORY	B
5. SITE CLASS	D