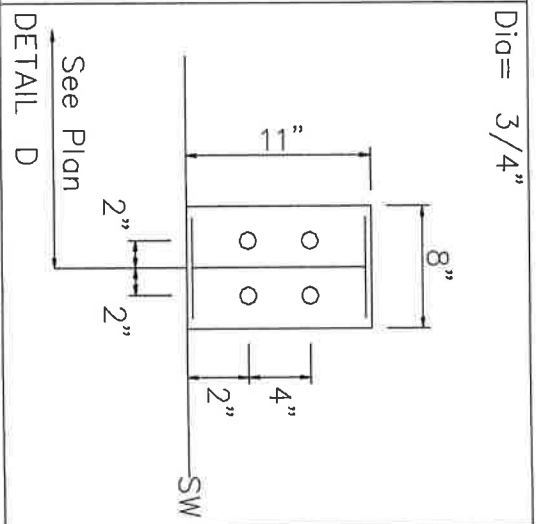
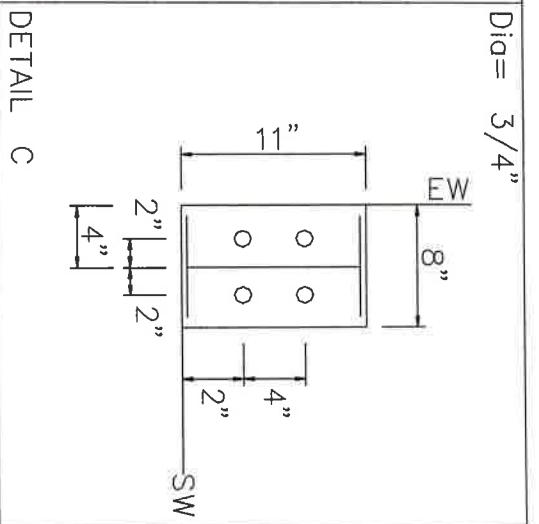
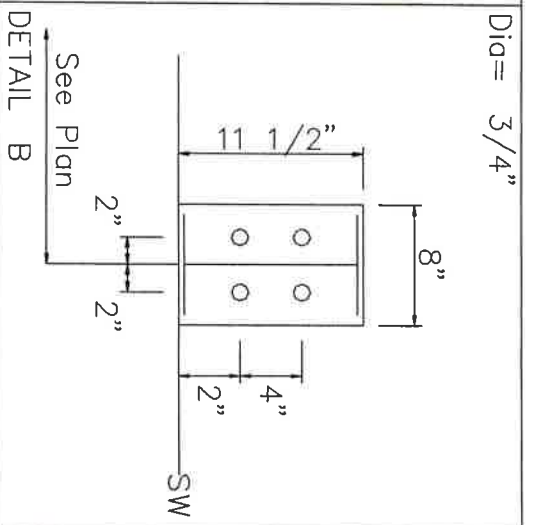
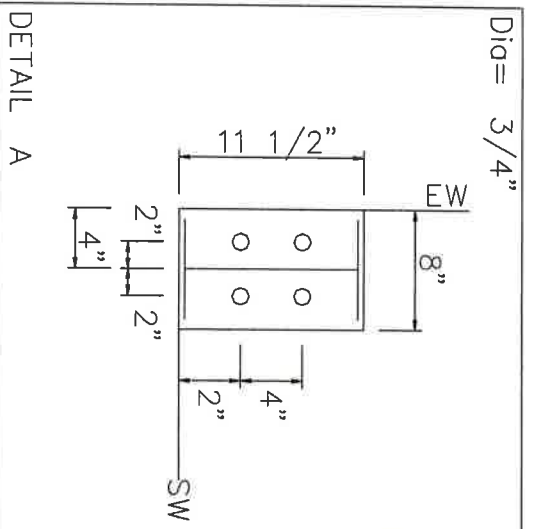


[Handwritten signature in blue ink]

11-16-2023



BLDGs ARE DESIGNED FROM THE BASE PLATE UP
CONCRETE AND ANCHOR BOLT LENGTH IS NOT IN OUR SCOPE OF WORK

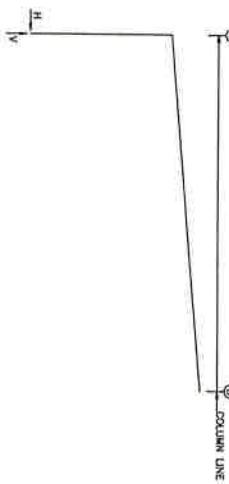


11-16-2023

ADCO Metals

PROJECT	Melinda Nunez	ANCHOR BOLT DETAILS
ID	37629PR	DESIGN: JM DRAFT: GK CHECK: LP
PROJECT	24516 Hemphill Rd	DATE: 10/3/23 SHEET A2 OF 4
ADDRESS	Bush, LA 70431	

FRAME LINES: 1 2 3 4 5 6



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame Line	Col Line	Load H	Load V	Wind H	Wind V	q ₁	q ₂	q ₃	q ₄	q ₅	q ₆	Beam Projct'n	Beam Length	Beam Thick	Grout
1*	C	2	6.3	14.6	3	-4.3	-8.8	-8.1	4	0.750	8.000	11.00	0.390	0.0	0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia	Type	Prod
0	24	3/4"	A307	3.00

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
2. THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
3. ALL ANCHOR BOLTS SHALL BE ASTM A307 OR EQUIVALENT TO CONTRACT SPECIFICATIONS. MANUFACTURER DESIGN ASSUMPTIONS BASED ON A MANUAL OF STEEL CONSTRUCTION.
4. ANCHOR BOLTS TO BE SUPPLIED BY OTHERS TO BE DESIGNED BY OTHERS.
5. ANCHOR BOLT EMBEDMENT LENGTH IN CONCRETE TO BE DESIGNATED BY OTHERS.
6. ANCHOR BOLT PROJECTION ABOVE CONCRETE NOTED BY FOUNDATION DESIGNER.
7. ANCHOR BOLTS SHALL BE CORROSION RESISTANT TO A TOLERANCE OF 4% IN ELEVATION SET TO AND LOCATION.
8. UNDER FIELD MARK OF STRUCTURAL, SECONDARY AND PANEL/JUNCTION ITEMS MAY BE NECESSARY TO A NORMAL PART OF FIELD BUILDING ERECTION FIELD SHALL NOT HONOR BACKWARDS FOR MAJOR FIELD MARK.
9. THESE DRAWINGS ARE NOT TO SCALE.
10. FINISHED FLOOR ELEVATION = 100'-0"

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	1	2	3	4	5	6
Form Column	0.8	0.9	2.1	0.9	2.2	0.8
Dead	0.5	0.6	1.5	0.6	1.6	0.5
Live	0.3	0.3	0.6	0.3	0.6	0.3
Wind Left	-0.2	-0.2	-0.2	-0.2	-0.2	-0.2
Wind Right	0.0	0.0	0.0	0.0	0.0	0.0

NOTES FOR REACTIONS

1. All loading conditions are combined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Foundation loads are shown in the table. Foundation loads are in opposite direction.
3. Bracing reactions are in the plane of the truss with the H pointing away from the braced bay. The vertical reaction is downward.
4. Bracing reactions are based on the following building data:
 - Span Length (ft) = 120.0
 - Clear Height (ft) = 18.0
 - Roof Slope (in/12) = 12.0/12.0
 - Dead Load (psf) = 2.0
 - Roof Live Load (psf) = 20.0
 - Roof Snow Load (psf) = 20.0
 - Form Wind Load (psf) = 12.0
 - Wind Speed (mph) = 120.0
 - Wind Code = RBC 21
 - Clearance = 0
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = B
 - Seismic Coef (Cross) = 0.15
5. Loading conditions are:
 - 1 Dead+Coll+Wind+Live
 - 2 Dead+Coll+Wind+0.75Live+0.45Wind+Longtr
 - 3 Dead+Coll+Wind+Live
 - 4 Dead+Coll+0.5Wind+Live

BUILDING BRACING REACTIONS

Bay	Col Line	Reaction	Frame Line	Wind	Dir	Note
1-2	SW	1	1	W	S	
2-3	SW	1	2	W	S	
3-4	SW	1	3	W	S	
4-5	SW	1	4	W	S	
5-6	SW	1	5	W	S	



11-16-2023

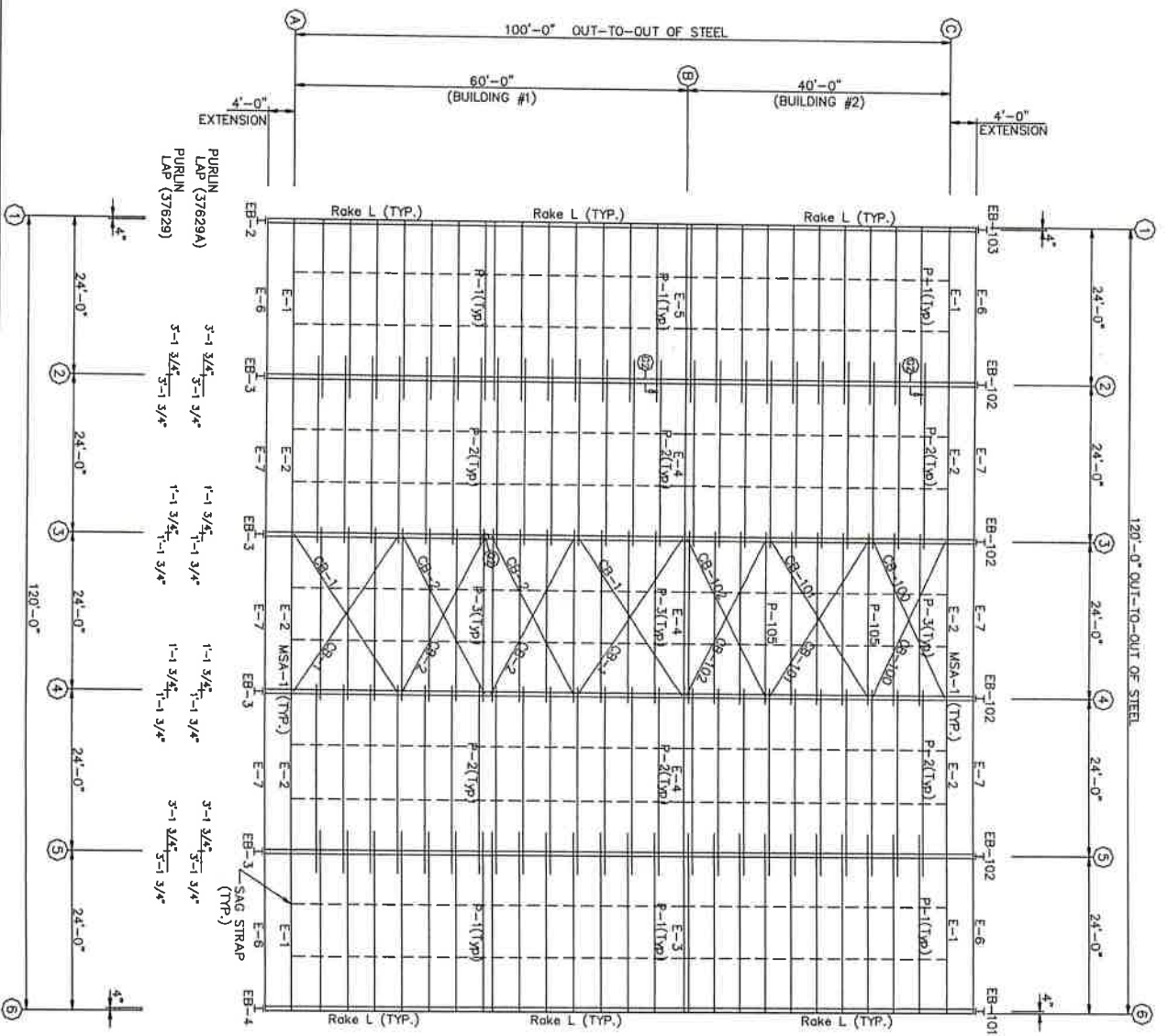
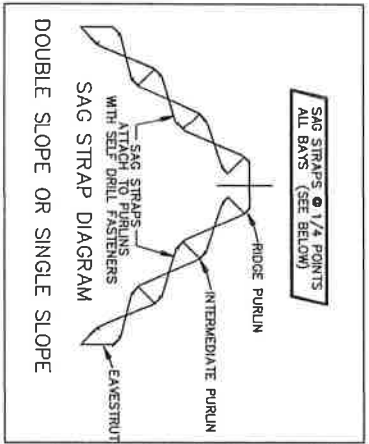
BUILDING #2 REACTIONS

ADCO Metals	
PROJECT	Melinda Nunez
ID	37629PR
PROJECT	24516 Hemphill Rd
ADDRESS	Bush, LA 70431
ANCHOR BOLT REACTIONS	
DESIGN: JM	DRAFT: GK
DATE: 10/3/23	SHEET: A4 OF 4

EXTENSION/CANOPY BOLTS			
MARK	QUANTITY	DIA	LENGTH
EB-2	4	3/8"	3/4"
EB-3	4	A325	3/8"
EB-101	4	A325	5/8"
EB-102	4	A325	5/8"
EB-103	4	A325	5/8"

MEMBER TABLE		
BUILDING #1	ROOF PLAN PART	LENGTH
EB-2	WBX10	5'-0" 11/16"
EB-3	WBX10	5'-0" 11/16"
EB-4	WBX10	5'-0" 11/16"
P-1	BX252/4	26'-3" 1/2"
P-2	BX252/6	26'-3" 1/2"
P-3	BES141	22'-11" 1/2"
E-1	BES141	23'-3" 1/2"
E-2	BES141	23'-11" 1/2"
E-3	BES141	23'-11" 1/2"
E-4	BES141	23'-11" 1/2"
E-5	BX35C12	23'-0" 1/2"
CB-1	BX35C12	23'-0" 1/2"
CB-2	0.55 CBL	27'-0"

BUILDING #2	ROOF PLAN PART	LENGTH
EB-101	WBX10	5'-0" 11/16"
EB-102	WBX10	5'-0" 11/16"
EB-103	WBX10	5'-0" 11/16"
CB-100	0.38 CBL	26'-8" 1/2"
CB-101	0.38 CBL	29'-2" 1/2"
CB-102	0.35 CBL	27'-0"



- GENERAL NOTES:**
1. INSTALL ALL PURLIN AND FLANGE BRACES AS SHOWN.
 2. ROOF PANEL PROVIDES STRUCTURAL STABILITY AND BRACING TO THE BUILDING.
 3. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
 4. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
 5. METAL SHAVINGS CAUSED BY DRILLING.

ROOF FRAMING PLAN

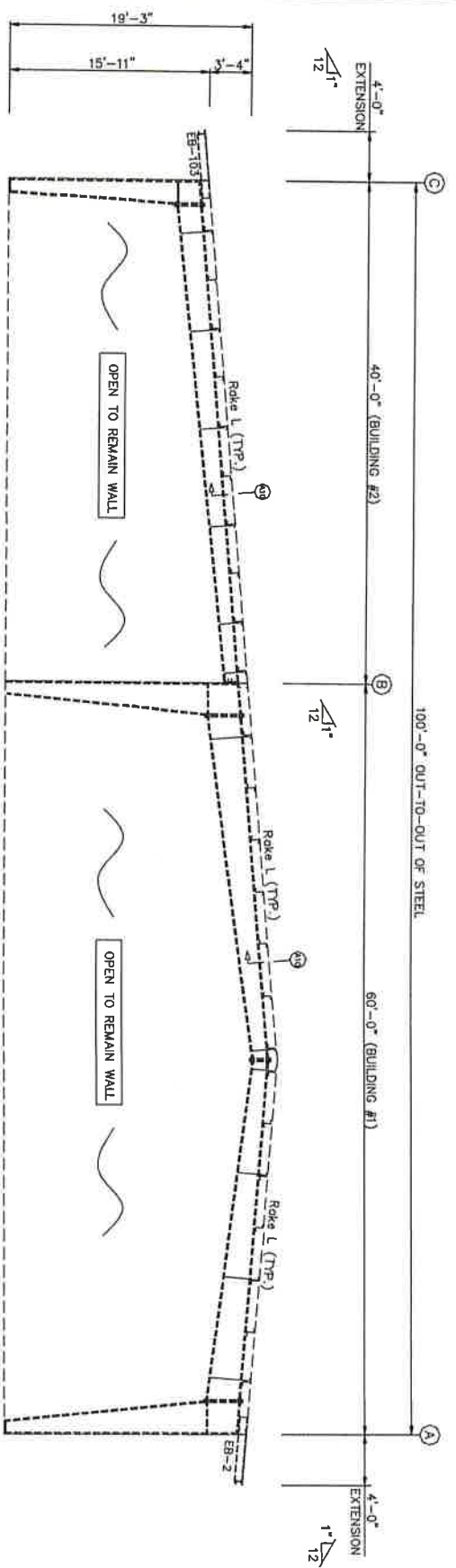
GENERAL NOTES: TYP ALL PAGES
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACK CHARGES FOR MINOR FIELD WORK.

11-16-2023

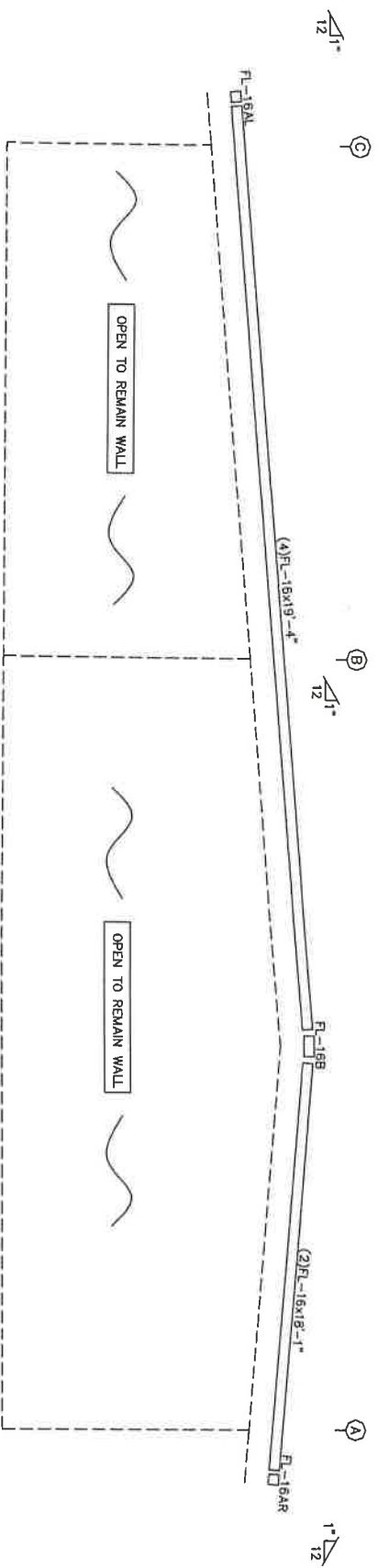


PROJECT		Meiinda Nunez	
ID	37629PR	ROOF FRAMING PLAN	
PROJECT	24516 Hemphill Rd	DESIGN: JM	DRAWN: GK
ADDRESS	Bush, LA 70431	DATE: 11/13/23	CHECK: LP
		SHEET	E2 OF E8

ADCO Metois



ENDWALL FRAMING: FRAME LINE 1

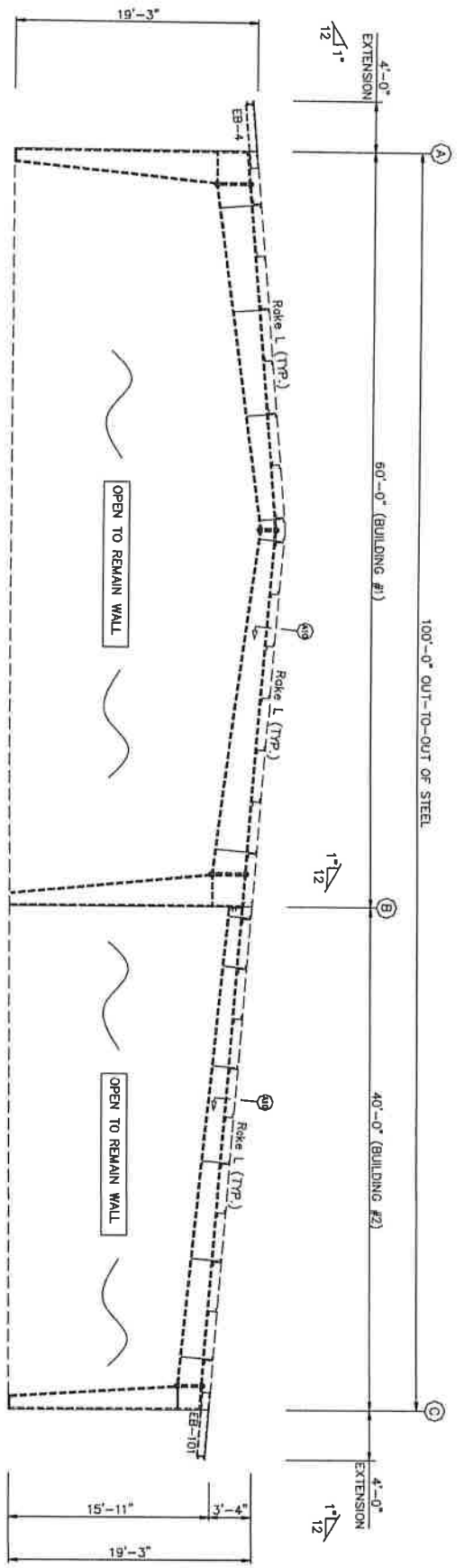


ENDWALL TRIM: FRAME LINE 1

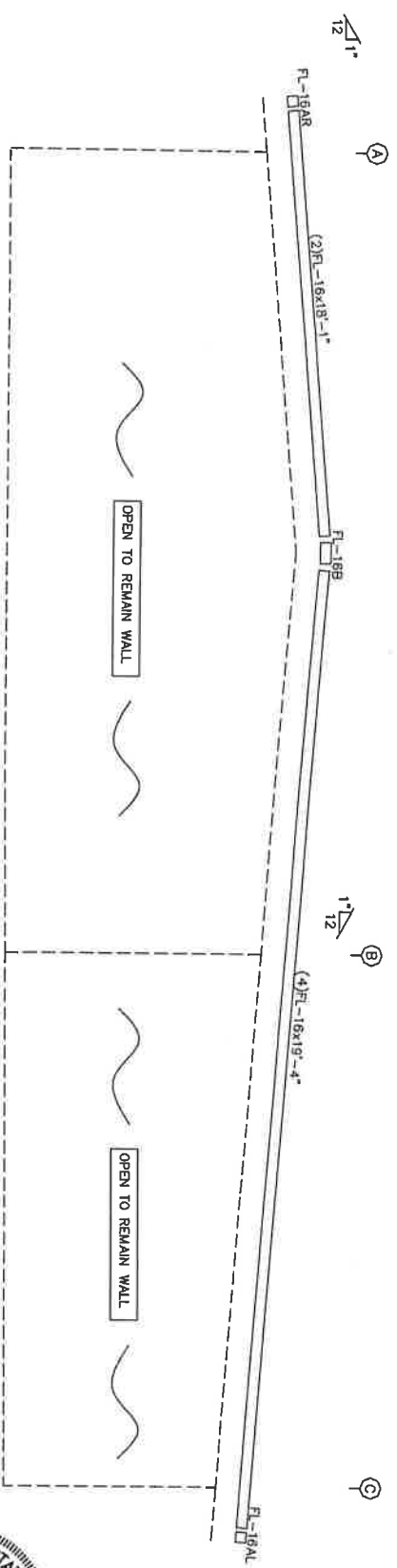
11-16-2023



PROJECT		Maindo Nunez		ADCO Metois	
ID	37629PR	ENDWALL FRAMING & SHEETING			
PROJECT	24516 Hemphill Rd	DESIGN: JM	DRAFT: GK	CHECK: LP	
ADDRESS	Bush, LA 70431	DATE: 11/13/23	SHEET	E4 OF E8	

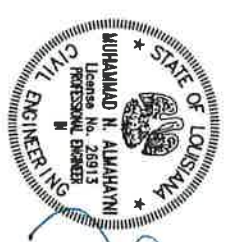


ENDWALL FRAMING: FRAME LINE 6



ENDWALL TRIM: FRAME LINE 6

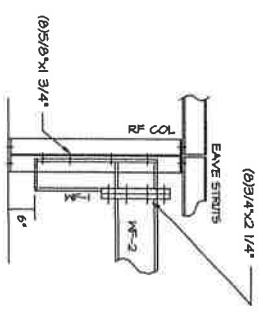
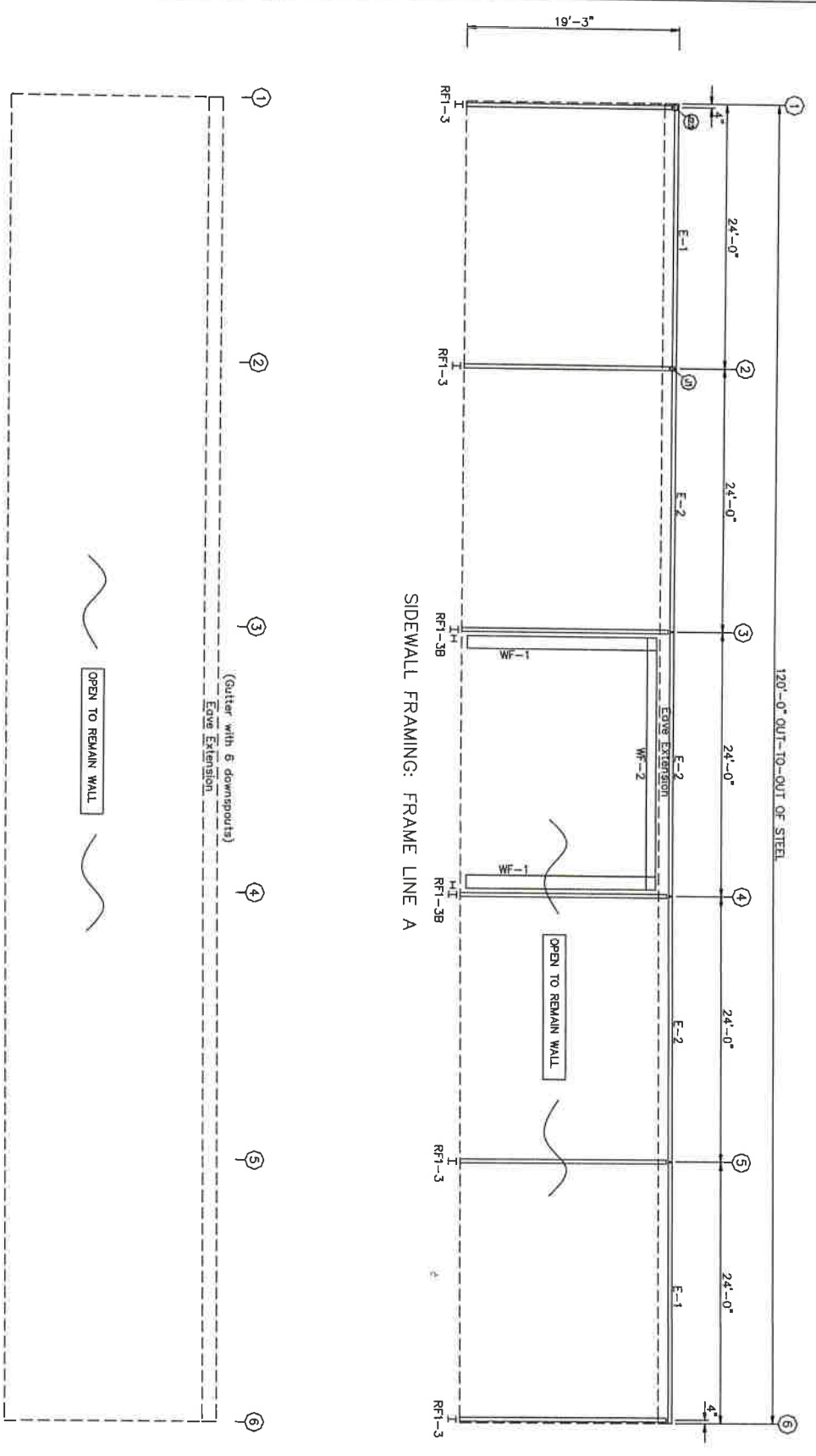
11-16-2023



ADCO Metals

PROJECT	Melindo Nunez	ENDWALL FRAMING & SHEETING
ID	37629PR	DESIGN: JM DRAFT: OK CHECK: LP
PROJECT	24516 Hemphill Rd	DATE: 11/13/23 SHEET: E5 OF E8
ADDRESS	Bush, LA 70431	

MEMBER TABLE			
FRAME LINE A			
MARK	PART	LENGTH	
WF-1	W1494	17'-7"	
WF-2	W10651	21'-7"	
E-1	BS141	22'-11 1/2"	
E-2	BS141	23'-3 1/2"	



DETAIL @ PORTAL FRAME

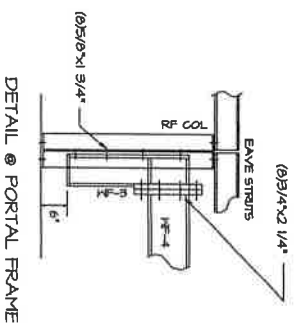
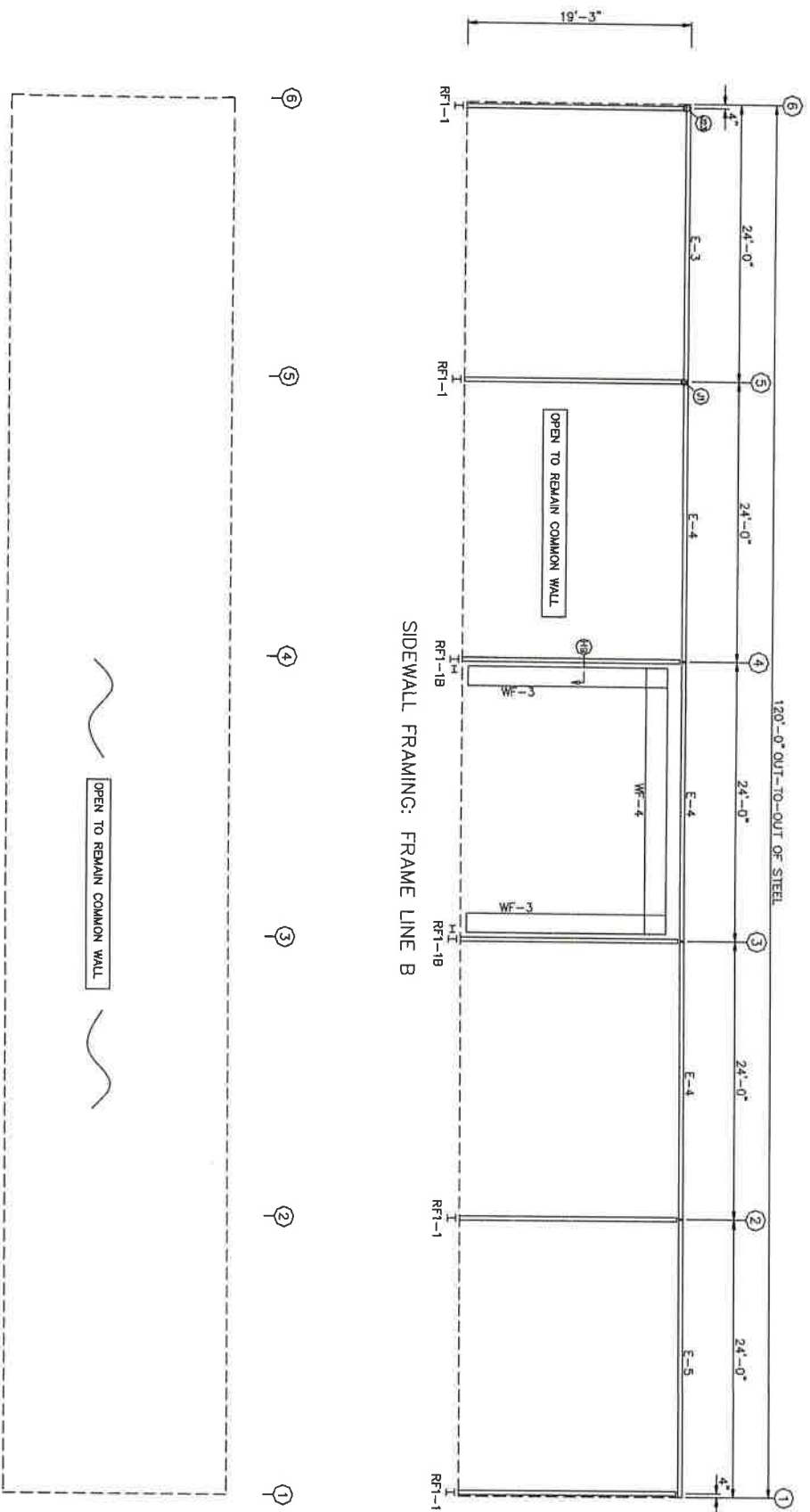
SIDEWALL TRIM: FRAME LINE A



11-16-2023

PROJECT		Maïrinda Nunez		ADCO Metals	
ID	37629PR	DESIGN: JM		DRAFT: GK CHECK: LP	
PROJECT	24516 Hemphill Rd	DATE: 11/13/23	SHEET	E6	OF E8
ADDRESS	Bush, LA 70431				

MEMBER TABLE		
MARK	PART	LENGTH
WF-3	W20082	12'-7 1/8"
WF-4	BE22032	20'-11 1/2"
E-2	RES141	23'-11 1/2"
E-5	RES141	23'-11 1/2"

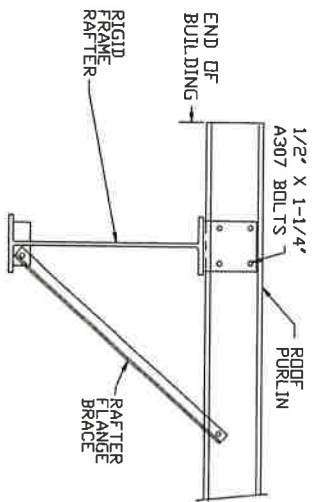


DETAIL @ PORTAL FRAME

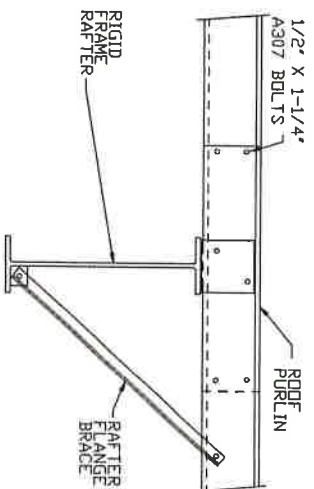


11-16-2023

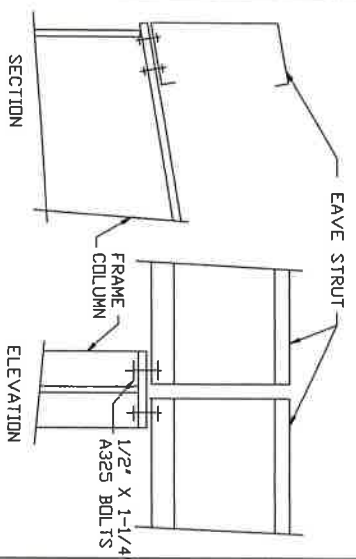
ADCO Metals	
PROJECT	Melinda Nunez
ID	37629PR
PROJECT	24516 Hemphill Rd
ADDRESS	Bush, LA 70431
SIDEWALL FRAMING & SHEETING	
DESIGN:	JM DRAFT: GK CHECK: LP
DATE:	11/13/23 SHEET E7 OF E8



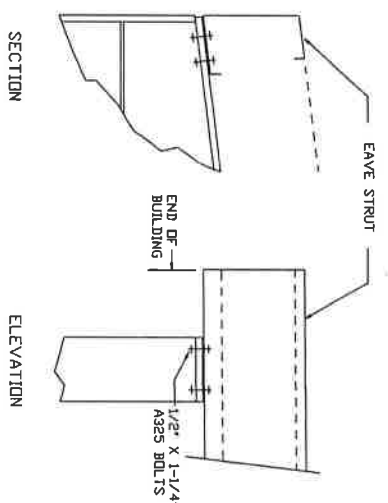
A10 ROOF PURLIN TO EXPANDABLE ENDWALL RIGID FRAME



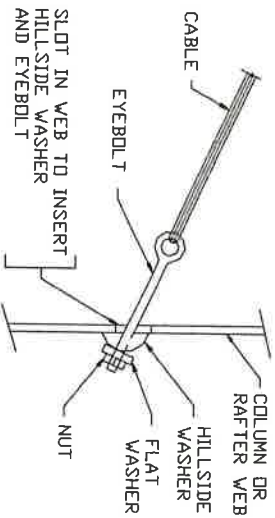
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



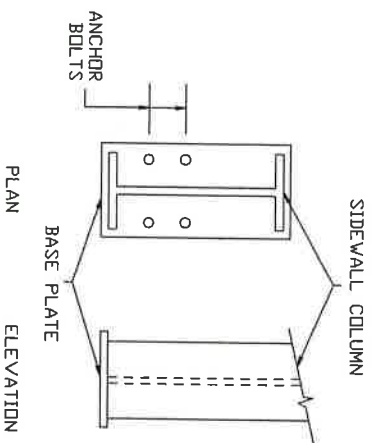
J1 EAVE STRUT TO RIGID FRAME



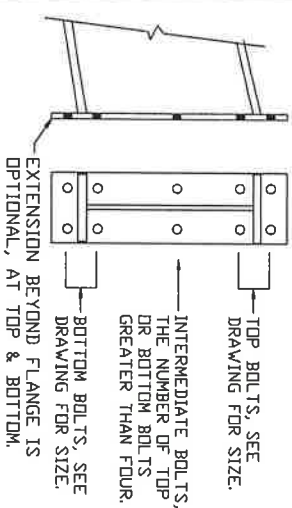
J23 EAVE STRUT TO RIGID FRAME



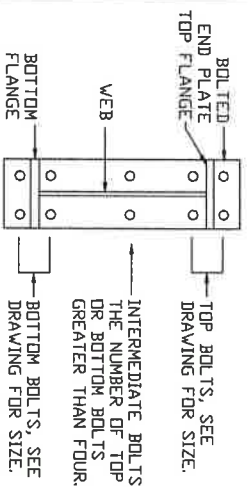
Q2 DIAGONAL CABLE, EYEBOLT END



R2 ANCHOR BOLTS AT SIDEWALL COLUMN



U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK

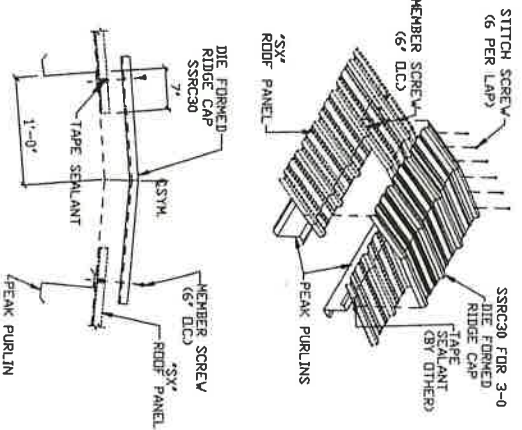


U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

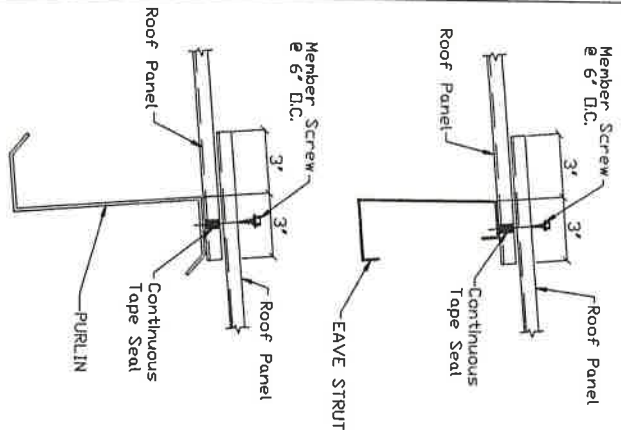


11-16-2023

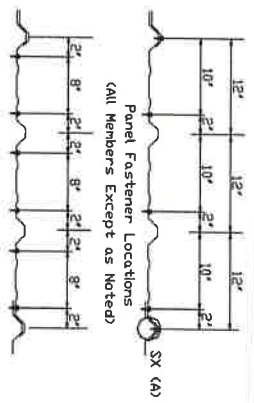
ADCO Metals	
PROJECT	Melinda Nunez
ID	37629PR
PROJECT	24516 Hemphill Rd
ADDRESS	Bush, LA 70431
STRUCTURAL DETAILS	
DESIGN: JM	DRAFT: GK
DATE: 11/13/23	CHECK: LP
	SHEET D1 OF D4



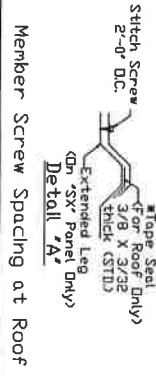
DIE FORMED RIDGE DETAIL - 'SX'



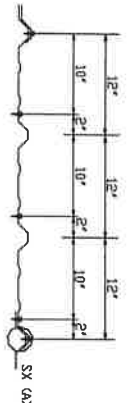
Section Thru Panel End Laps



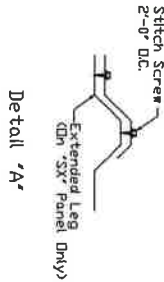
Panel Fastener Locations
(All Members Except as Noted)



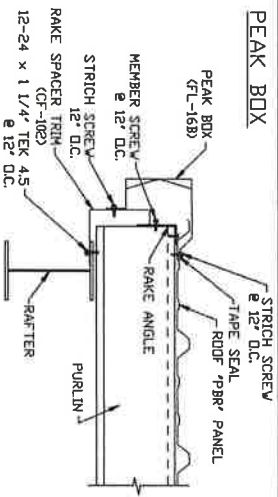
Member Screw Spacing at Roof



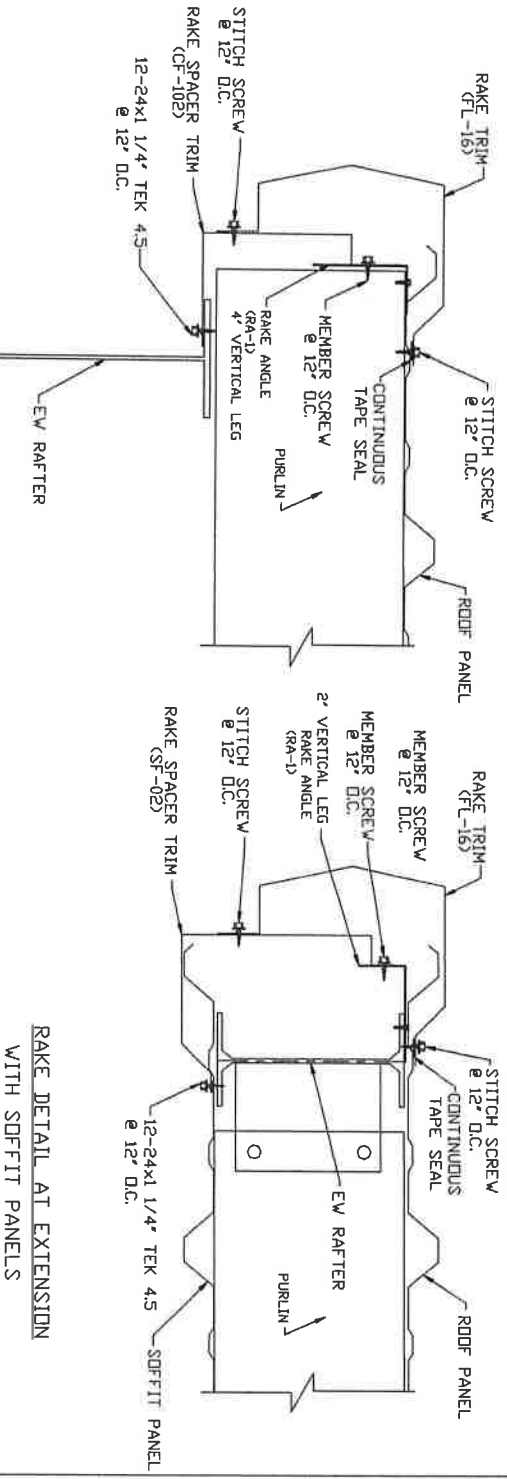
Panel Fastener Locations
(All Members Except as Noted)



Member Screw Spacing at Soffit



PEAK BOX

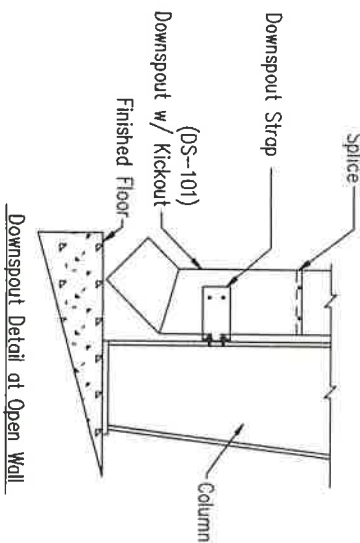
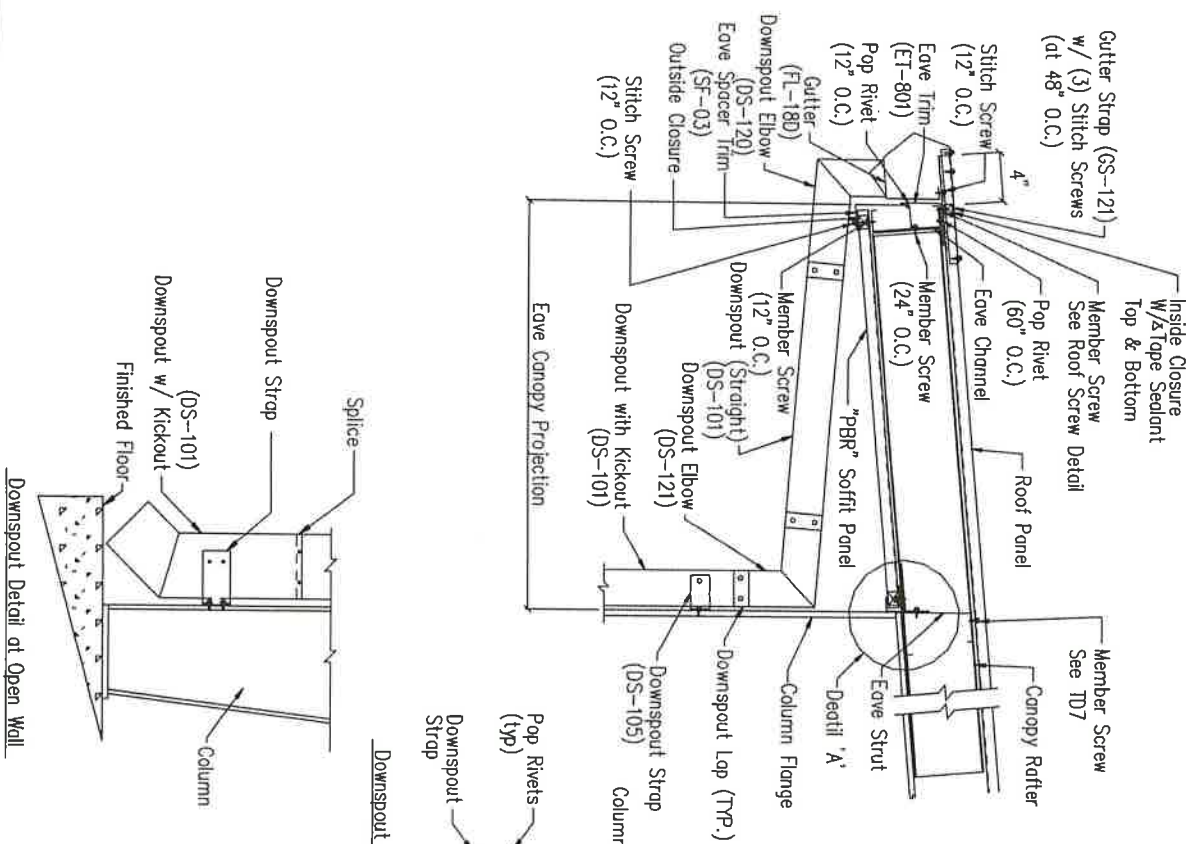
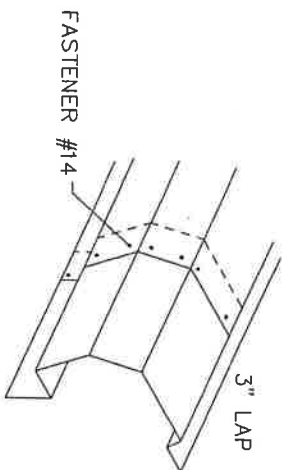


RAKE DETAIL AT EXTENSION WITH SOFFIT PANELS

ADCO Metals			
PROJECT	Melinda Nunez	STRUCTURAL DETAILS	
ID	37629PR	DESIGN: JM	DRAFT: GK
PROJECT	24516 Hemphill Rd	DATE: 11/13/23	CHECK: LP
ADDRESS	Bush, LA 70431		SHEET D2 OF D3

11-16-2023





NOTE:
REFER TO ROOF SHEETING PLAN
FOR DOWNSPOUT LOCATIONS.

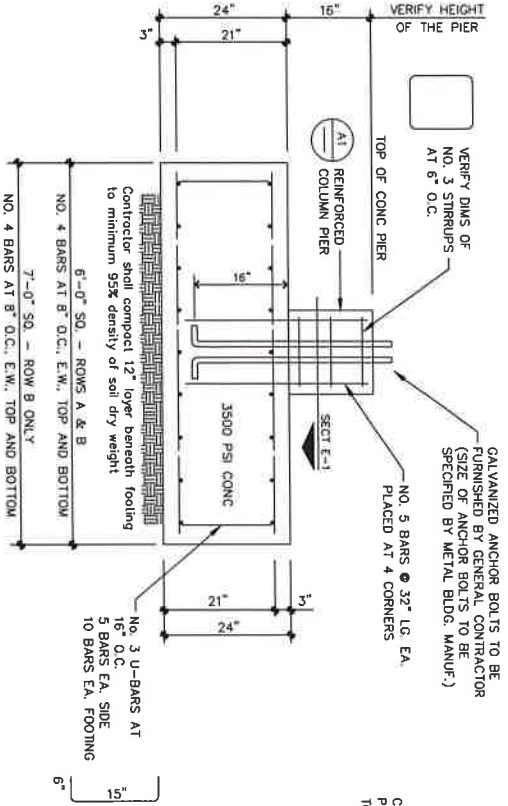
11-16-2023



ADCO Metals	
PROJECT	Mainda Nunez
ID	37629PR
PROJECT	24516 Hemphill Rd
ADDRESS	Bush, LA 70431
STRUCTURAL DETAILS	
DESIGN: JM	DRAFT: GK
DATE: 11/13/23	CHECK: LP
	SHEET 03 OF 03

PIER HEIGHT/DIMENSIONS

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE HEIGHT AND DIMENSIONS OF THE COLUMN PIERS TO PROVIDE FOR FUTURE SLAB AND FOOTINGS POUR.



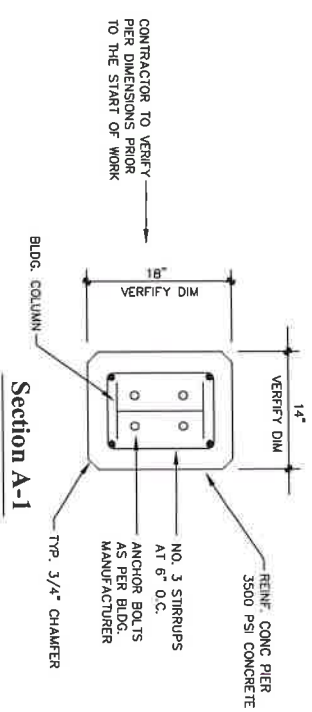
REINFORCED CONCRETE SPREAD FOOTING DETAIL

SCALE: 3/4" = 1'-0"

1. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES OF CONSTRUCTION AND THE CONTRACTOR SHALL BE RESPONSIBLE TO SEE THAT THE FINISHED WORK COMPLES ACCURATELY WITH THE PROJECT DRAWINGS.
2. A 10-MIL. THK. VAPOR BARRIER OF POLYETHYLENE SHALL BE PLACED UNDER ALL CONCRETE, UNLESS OTHERWISE NOTED.
3. CURING COMPOUND WILL BE PLACED ON SLAB IMMEDIATELY AFTER FINISHING TO CONTROL SHRINKAGE CRACKING.
4. ALL CONCRETE SHALL HAVE A "MINIMUM" COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS. SUBMIT MIX DESIGN TO ENGINEER.
5. COORDINATE STRUCTURAL DRAWINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL OPENINGS, INSERTS AND ALL OTHER RELATED ITEMS. ALL REQUIRED FORM WORK, INCLUDING BRICK LEDGES, DROP FORMS, BLOCKOUTS AND DEPRESSION FORMS, ETC., SHALL BE COMPLETED PRIOR TO SYSTEM INSTALLATIONS.

GENERAL CONSTRUCTION NOTES

6. FORMS ARE TO BE STRIPPED NO LATER THAN 5 DAYS AFTER PLACEMENT OF CONCRETE.
7. PROVIDE MINIMUM 2" OF COVER OVER ALL REINFORCING STEEL.
8. PROVIDE MINIMUM 24" SPLICE FOR ALL MAIN REINFORCING STEEL. STAGGER SPLICES WHERE POSSIBLE.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LAYING OUT THE WORK AND PROVIDING THE NECESSARY SURVEY CONTROL DURING THE COURSE OF CONSTRUCTION.
10. ALL DIMENSIONS SHOWN ON THE DRAWINGS SHOULD BE VERIFIED BY THE CONTRACTOR FOR CONFORMITY WITH THE FLOOR PLANS PRIOR TO FORMING WORK AND PLACEMENT OF THE ANCHOR BOLTS.
11. NO FIELD SUPERVISION OR INSPECTION PROVIDED UNDER THIS SEAL UNLESS NOTED OTHERWISE.
12. ALL REINFORCING STEEL TO BE GRADE 60 UNLESS NOTED OTHERWISE ON THE DRAWINGS OR DIRECTED BY THE ENGINEER.



STATE OF LOUISIANA
 PROFESSIONAL ENGINEER
 HARRY J. DUPRE
 REG. NO. 19208
 IN CIVIL ENGINEERING

DATE: 8 NOV 2023

DISCLAIMER

THE ENGINEER HAS PREPARED THIS PRINT BASED ON HIS KNOWLEDGE, EXPERIENCE, AND INSTRUCTIONS PROVIDED BY THE OWNER BASED ON GENERAL SITE CONDITIONS. THERE IS NO WARRANTY, HOWEVE, WHETHER EXPRESSED OR IMPLIED, WITH RESPECT TO THE CONTENT OF THIS PRINT AND THE INFORMATION CONTAINED HEREIN, INCLUDING BUT NOT LIMITED TO THE DESIGN, CONSTRUCTION, AND PERFORMANCE OF THE WORK. THE GENERAL CONTRACTOR, AT THE SITE IN BISHOP, LOUISIANA.

NEKO SOLUTIONS, LLC
 576 KINGS HILLS BLVD.
 PIGEON FORGE, TN 37863
 PHONE (985) 870-7172

FILE NAME	M NUMBER - 3	SPREAD FOOTING SECTION	SHEET NO.
JOB NO.	2023	GENERAL CONSTRUCTION NOTES	2
DATE	8 NOV 2023	PROPOSED 100' x 120' METAL CANOPY	OF
PLLOT SCALE	1/4" = 1'-0"	MELINDA NUÑEZ	2
DRAWN BY	LJD	24516 HEMPHILL RD.	
APPROVED	LJD	BUSH, LA. 70431	
IMP NO.		ST. TAMMANY PARISH	

BUILDING SPECIFICATIONS

The manufacturer is not responsible for the concrete foundation design. The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The manufacturer will assume no responsibility for any loads not indicated.

This manufactured building is designed with the manufacturer's standard design practices which are based on pertinent procedures and recommendations of the following organizations and codes:

- American Institute of Steel Construction "Specification for the design fabrication and erection of structural steel for buildings" 9th edition.
- American Iron and Steel Institute "Specification for the design of cold formed steel structural members" 1996 edition.
- American Welding Society "Structural Welding Code" AWS D1.1-98 Chapter 2 relating to prequalified complete joint penetration groove welds and chapter 8 part B relation to allowable stresses.
- Metal Building Manufacturers Association "Specification for the design fabrication and erection of the structural system" most current edition.

Material properties of steel plate and sheet used in fabrication of primary rigid frames and all primary structural framing members (other than cold-formed sections) conform to ASTM A-529 or A-570 or A-572 all with a minimum yield point of 50 KSI.

Material properties of cold formed light gage steel members conform to the requirements of ASTM A-570, with a minimum yield point of 55 KSI.

High strength bolts and their installation shall conform to ASTM specification A-325 and are designed as bearing type connections with threads included in the shear plane. Tightening of these bolts is recommended by the Turn-of-the-nut method, per "AISC Specification for Structural Joints."

All primary structural members except bolts and fasteners shall receive one coat of Iron Oxide Inhibitive primer.

Shop and field inspections and associated fees are the responsibility of the builder, unless stipulated otherwise.

BUILDER RESPONSIBILITIES

The builder must secure all required approvals and permits from the appropriate agency as required.

Approval of the manufacturer's drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract drawings and specifications. (Sec. 4.2.1 AISC Code of Standard Practice, 9th edition.)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Sec. 3.3 AISC Code of Standard Practice, 9th edition.)

Design considerations of any materials in the structure which are not furnished by the manufacturer, are the responsibility of the builder and engineers other than the manufacturer's engineering, unless specifically indicated. The builder is responsible for all erection of steel and associated work in compliance with the manufacturer's "For Construction" drawings.

Temporary supports, such as guys, braces, flashwork or other elements required for the erection will be determined and furnished and installed by the erector. (Sec. 7.9.1 AISC Code of Standard Practice, 9th edition.)

It is the builders responsibility to apply or observe all pertinent safety rules and regulations, as per OSHA standards as applicable. The builder is responsible for the verification of all shipments received. Any "external" damage or shortages must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim. NO EXCEPTIONS!!!

ADCO Metals

BUILDING LOADS

This structure is designed utilizing the loads indicated and applied by the:

IBC 201

It is the builder's responsibility to confirm that these loads comply with the requirements of the local building department.

- 2000 PSF Dead Load (ROOF PANELS & PURLINS)
 - 2000 PSF Live Load
 - 35 PSF Roof Snow Load
 - 133 MPH Wind Load Exposure B
 - 3 PSF Collateral Load
 - 0.147 SEISMIC COEFFICIENTS
- IMPORTANT FACTORS:
 $I_s = 1.0$ $I_w = 1.00$ $I_e = 1.00$
 (Snow) (Wind) (Seismic)

GENERAL NOTES

INSTALL AND ERECT THIS BUILDING AND ALL ITS PARTS AND COMPONENTS PER THESE DRAWINGS. NO CHANGES SHOULD BE MADE TO THIS BUILDING SYSTEM UNLESS APPROVED IN WRITING BY THE MANUFACTURER'S ENGINEERING DEPARTMENT. UNAPPROVED CHANGES COULD RESULT IN AN UNSAFE BUILDING DESIGN AND COULD ENDANGER PUBLIC SAFETY.

THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LIDGERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES.

Design Data

SEISMIC USE GROUP 1, SITE CLASS D
 ANALYSIS PROCEDURE IS SIMPLIFIED ANALYSIS PROCEDURE.
 BASIC STRUCTURAL SYSTEM = ORDINARY MOMENT
 $I_s = 1.0$ $C_e = 1.0$ $C_t = 1.0$
 $I_s = 1.0$ BUILDING WITH LESS THAN 250 OCCUPANTS AND NOT CRITICAL TO NATIONAL EMERGENCIES, SUCH AS HOSPITALS, FIRE STATIONS, POLICE STATIONS, ETC.
 $C_t = 1.0$ BUILDING IS HEATED.
 $C_e = 1.0$ BUILDING IS PARTIALLY EXPOSED TO WIND, NOT SURROUNDED BY HIGHER STRUCTURES. TREES OR ANY OBSTRUCTIONS ON ALL SIDES WITHIN 10 TIMES THE BUILDING HEIGHT.
 IF THIS BUILDING DOES NOT MEET THE ABOVE CONDITIONS, PLEASE CONTACT US IMMEDIATELY TO ENSURE PROPER STRUCTURE DESIGN.

DRAWING INDEX

- Cover Sheet
- Anchor Bolt Plan
- Anchor Bolt Details & Reactions
- Roof Framing Plan
- Roof Sheeting Plan
- Rigid Frame Elevation
- Endwall Framing & Sheeting
- Sidewall Framing & Sheeting
- Detail Drawings

- These Drawings are for:
- Construction
 - Not for Construction
 - Permit Only
 - Approval *
 - Other :

* Approval orders at the time of release for fabrication, cost, might be adjusted to reflect only cost increases incurred by the manufacturer.
 Special attention should be given in approving dimensions and/or details. Please verify requested dimensions by indicating "OK".

These Drawings and Documents are Copyright protected. Use or reproduction is prohibited without our written consent.

Engineering Seal

THE UNDERSIGNED PROFESSIONAL ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT.

STRUCTURAL DESIGN CONCEPTS, INC.
 COA 5402
 5200 Village Creek Dr. STE 101
 Plano, TX 75093 Ph:214-501-8000



DSN: JM
 DET: GK
 DWN: LP
 CHK: LP
 REV: REVISIONS
 NO. DATE
 11-16-2023
 PROFESSIONAL ENGINEER

DRAWINGS COVER SHEET

Melinda Nunez
 Bush, LA 70431
 60 x 120 x 19.25
 40 x 120 x 15.92
 ADCCO Metals
 64101 Hwy 434
 Lacombe, LA 70445

SCALE: NOT TO SCALE

DATE: 11/13/23

JOB NO: 37629PR

SHT. NO: CS