

ETC, HERE IS THE BUDG 11B 69193-1

MFG. FOR NORTH LAKE MARINE.

CHUCK

FRAME REACTIONS REPORT

NORTH LAKE MARINE / SNOW WT

General Reaction Notes:

1. Vertical column reactions are positive in the upward direction.
2. Transverse horizontal column reactions are positive to the right on all columns.
3. Longitudinal horizontal reactions are perpendicular to the transverse horizontal reactions, and the positive direction is inward to the page.
4. Fixed base moments are positive in the counter-clockwise direction on all columns.
5. Reactions for a particular load combination may be obtained by summing-up individual load reactions that have been multiplied by their load factors.
6. Forces on the foundations will act in the opposite direction to the direction of the column reactions.

Explanation of Load Group Names Used in Load Combinations:

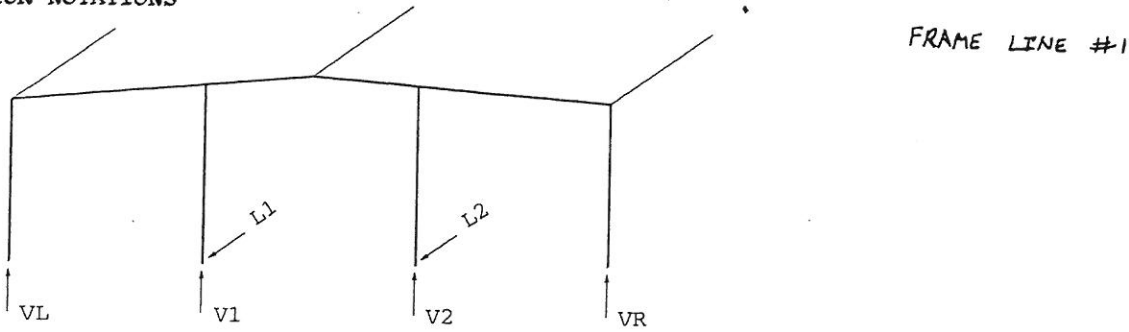
- $A_i D_j$  = Crane  $i$  Dead Loading  $j$
- $A_i D_j L$  = Crane  $i$  Dead Loading  $j$  Max. at Left
- $A_i D_j R$  = Crane  $i$  Dead Loading  $j$  Max. at Right
- $A_i H_j L$  = Crane  $i$  Horizontal Loading  $j$  Max. at Left
- $A_i H_j R$  = Crane  $i$  Horizontal Loading  $j$  Max. at Right
- $A_i V_j L$  = Crane  $i$  Vertical Loading  $j$  Max. at Left
- $A_i V_j R$  = Crane  $i$  Vertical Loading  $j$  Max. at Right
- $A_i V_j$  = Crane  $i$  Vertical Loading  $j$
- AD = Crane Dead Load
- ADW = Crane Dead Weight
- AL = Crane Live Load
- COLL = Roof Collateral Load
- $DI_n$  = Distributed Load (Pattern, Partial Roof or Non-Counteracting Roof Loads)
- DL = Roof Dead Load
- EQ = Lateral Seismic Load
- EQDW = Seismic Mass
- LEQ = Longitudinal Seismic Load
- LL = Roof Live Load
- LS = Unbalanced Left Roof Snow Load
- $LWB_n$  = Longitudinal Primary Wind Load (Base Plate, Anchor Bolts and Foundation)
- $LWL_n$  = Longitudinal Primary Wind Load
- MD = Mezzanine Dead Load
- ML = Mezzanine Live Load
- MW = Mezzanine Dead Weight
- RS = Unbalanced Right Roof Snow Load
- SNOW = Roof Snow Load
- $WL_n$  = Lateral Primary Wind Load

- 
- $i$  = Crane Number
  - $j$  = Crane Loading Number
  - $n$  = Loading Number

SUPPORT REACTIONS FOR EACH LOAD GROUP  
 NOTE: All reactions are in kips and kip-ft.

TIME:11:53:56

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

D C

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2		
BASE PLATE	0.0X0.0X0.0			0.0X0.0X0.0			8.0X8.0X0.375			8.0X8.0X0.375		
ANC. BOLTS	N/A			N/A			(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2
D	0.	0.	0.	0.	0.	0.	0.	0.625	0.	0.	0.625	0.
W+	0.	0.	0.	0.	0.	0.	0.	0.	7.298	0.	0.	7.298
W-	0.	0.	0.	0.	0.	0.	0.	0.	-7.853	0.	0.	-7.853
E+	0.	0.	0.	0.	0.	0.	0.	0.	0.883	0.	0.	0.883
E-	0.	0.	0.	0.	0.	0.	0.	0.	-0.883	0.	0.	-0.883

LOAD GROUP DESCRIPTION

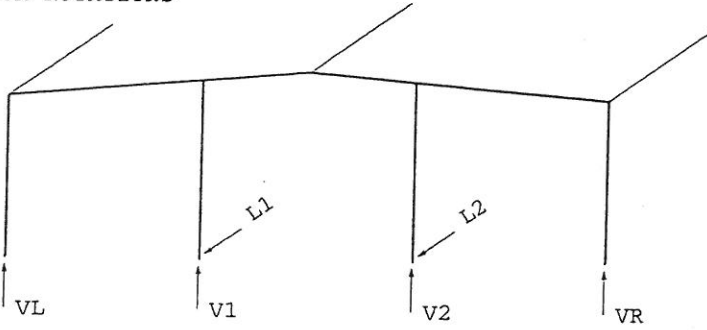
- D : DEAD LOAD
- W+ : WIND LOAD AS AN INWARD ACTING PRESSURE
- W- : WIND LOAD AS AN OUTWARD ACTING SUCTION
- E+ : EARTHQUAKE FORCE ACTING INWARD
- E- : EARTHQUAKE FORCE ACTING OUTWARD

**MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS**

NOTES: (1) All reactions are in kips and kip-ft.  
 (2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

TIME:11:53:56

**REACTION NOTATIONS**



**LOAD COMBINATION MAXIMUM REACTION TABLE**

D C

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2		
BASE PLATE	0.0X0.0X0.0			0.0X0.0X0.0			8.0X8.0X0.375			8.0X8.0X0.375		
ANC. BOLTS	N/A			N/A			(4)-3/4			(4)-3/4		
LOAD COMB	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2

**WIND LOAD COMBINATION**

1	0.	0.	0.	0.	0.	0.	0.	0.375	7.298	0.	0.375	7.298
2	0.	0.	0.	0.	0.	0.	0.	0.375	-7.853	0.	0.375	-7.853

**LONGITUDINAL EARTHQUAKE LOAD COMBINATION**

3	0.	0.	0.	0.	0.	0.	0.	0.544	0.883	0.	0.544	0.883
4	0.	0.	0.	0.	0.	0.	0.	0.544	-0.883	0.	0.544	-0.883

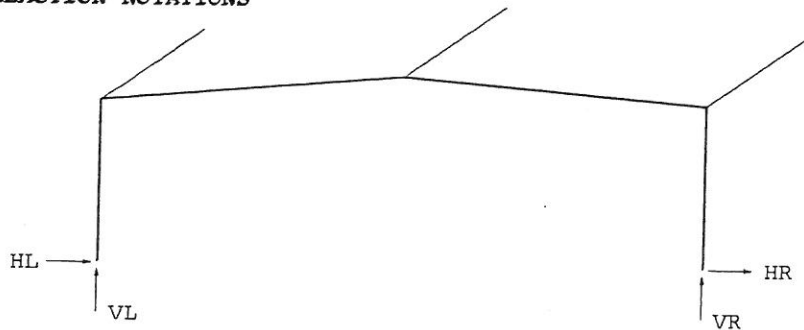
**LOAD COMBINATION DESCRIPTION**

- 1 : 0.60D + W+
- 2 : 0.60D + W-
- 3 : 0.87D + E+
- 4 : 0.87D + E-

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #06 LOCATION:frame lines 1  
 NOTE: All reactions are in kips and kip-ft.

TIME:13:50:23

**REACTION NOTATIONS**



**LOAD GROUP REACTION TABLE**

*E*

*B*

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.375			8.0X10.0X0.5		
ANC. BOLTS	(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	1.2	2.6	0.0	-1.2	3.1	0.0
COLL	1.5	3.0	0.0	-1.5	3.9	0.0
PAR1	0.5	1.3	0.0	-0.5	0.4	0.0
PAR2	0.6	0.4	0.0	-0.6	1.3	0.0
PLLL1	-0.1	2.4	0.0	0.0	0.0	0.0
PLL1	3.8	5.7	0.0	-3.8	5.7	0.0
PLLR1	-0.1	0.0	0.0	0.1	3.7	0.0
LL	3.7	8.0	0.0	-3.6	9.3	0.0
SNOW	1.1	2.3	0.0	-1.1	2.7	0.0
EQ	-0.5	-0.3	0.0	-0.5	0.3	0.0
WL1	-12.4	-20.9	0.0	5.5	-18.5	0.0
WL2	-1.1	-5.0	0.0	0.1	1.5	0.0
LWL1	-10.9	-19.5	0.0	7.7	-23.5	0.0
LWL2	-11.2	-18.6	0.0	7.3	-24.5	0.0
LWL3	-8.3	-13.9	0.0	5.3	-17.2	0.0
LWL4	-8.5	-13.5	0.0	5.1	-17.6	0.0
WL3	-9.0	-15.1	0.0	9.1	-26.2	0.0
WL4	2.2	0.7	0.0	3.7	-6.2	0.0
RS	1.1	1.1	0.0	-1.1	4.3	0.0
LS	1.2	2.4	0.0	-1.2	1.4	0.0

**LOAD GROUP DESCRIPTION**

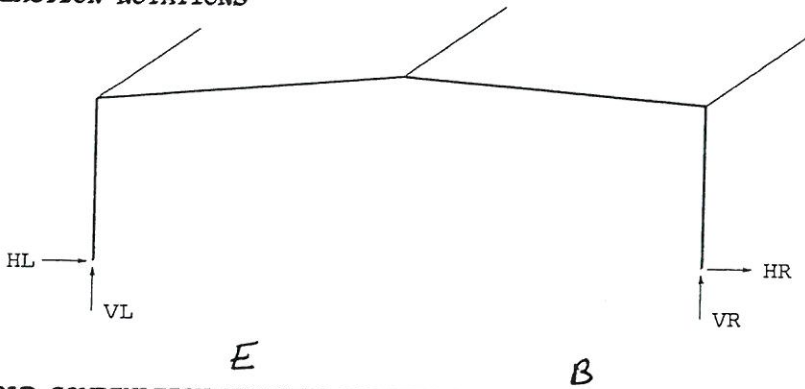
- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PAR1 : Partial Load [PARxx]
- PAR2 : Partial Load [PARxx]
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- PLL1 : Pattern Live Load [PLLxx]
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load
- LS : Unbalanced Left Roof Snow Load

R4

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #06 LOCATION:frame lines 1

NOTES:(1) All reactions are in kips and kip-ft. TIME:13:50:23  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS



*Handwritten red notes:*  
64 160  
2.6  
2.4  
150  
24000 lbs  
150  
900  
200

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X10.0X0.375			8.0X10.0X0.5		
ANC. BOLTS	(4)-3/4			(4)-3/4		

GRAVITY LOAD COMBINATION

6	6.5	11.3	0.0	-6.5	12.6	0.0
3	6.4	13.7	0.0	-6.4	12.6	0.0
4	6.5	11.2	0.0	-6.5	16.3	0.0

WIND LOAD COMBINATION

56	7.1	12.2	0.0	-2.6	9.3	0.0
33	-11.7	-19.3	0.0	4.8	-16.7	0.0
39	-8.3	-13.6	0.0	8.4	-24.4	0.0
50	4.6	7.8	0.0	-5.3	15.1	0.0

TRANSVERSE EARTHQUAKE LOAD COMBINATION

19	6.0	12.0	0.0	-5.0	13.9	0.0
18	5.1	11.5	0.0	-5.9	14.3	0.0

LOAD COMBINATION DESCRIPTION

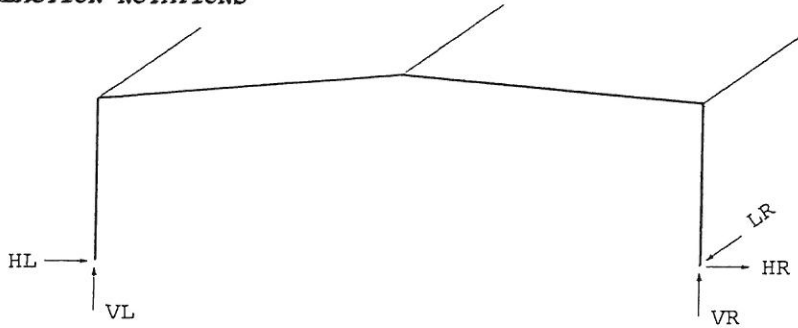
- 3 : DL +COLL +PLLL1 +PLL1
- 4 : DL +COLL +PLL1 +PLLR1
- 6 : DL +COLL +PLL1
- 18 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 19 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 33 : 0.6DL +WL1
- 39 : 0.6DL +WL3
- 50 : DL +0.75LL +COLL +0.75WL2
- 56 : DL +0.75LL +COLL +0.75WL4

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #05 LOCATION:frame lines 2

NOTES:(1) All reactions are in kips and kip-ft.  
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

TIME:13:50:01

REACTION NOTATIONS



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LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X11.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	2.0	3.9	0.0	-2.0	4.3	0.0
COLL	2.9	5.1	0.0	-2.9	6.0	0.0
PAR1	1.0	2.4	0.0	-1.0	0.8	0.0
PAR2	1.0	0.7	0.0	-1.0	2.4	0.0
PLLL1	-0.1	2.4	0.0	0.0	0.0	0.0
PLL1	7.1	10.6	0.0	-7.1	10.6	0.0
PLLR1	-0.1	0.0	0.0	0.1	3.7	0.0
LL	7.0	13.0	0.0	-6.9	14.3	0.0
SNOW	2.0	3.8	0.0	-2.0	4.2	0.0
LEQ	0.0	0.0	0.0	0.0	-2.2	-3.0
EQ	-0.7	-0.4	0.0	-0.7	0.4	0.0
WL1	-17.6	-29.1	0.0	10.7	-26.5	0.0
WL2	1.4	-1.5	0.0	-2.4	5.3	0.0
LWL1	-19.6	-32.5	0.0	16.4	-45.2	-11.4
LWL2	-19.9	-31.8	0.0	16.0	-46.1	-11.4
LWL3	-14.8	-23.6	0.0	11.7	-35.4	-11.4
LWL4	-14.9	-23.3	0.0	11.6	-35.8	-11.4
WL3	-14.3	-23.1	0.0	14.3	-34.4	0.0
WL4	4.7	4.5	0.0	1.2	-2.7	0.0
RS	2.1	2.0	0.0	-2.1	6.4	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PAR1 : Partial Load [PARxx]
- PAR2 : Partial Load [PARxx]
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- PLL1 : Pattern Live Load [PLLxx]
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- LEQ : Longitudinal Seismic Load [located in perp. plane]
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load

SUPPORT REACTIONS FOR EACH LOAD GROUP

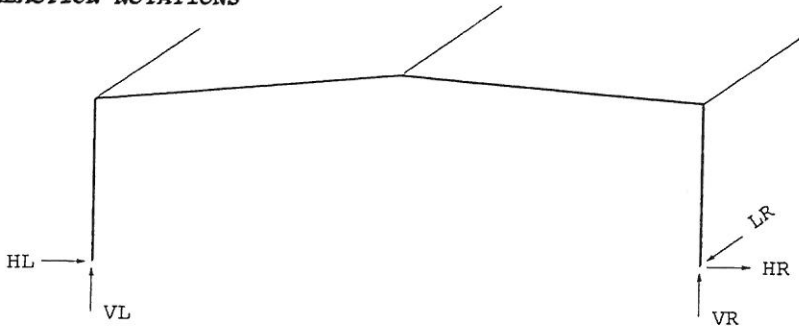
FRAME ID #05 LOCATION:frame lines 2

NOTES: (1) All reactions are in kips and kip-ft.

TIME:13:50:01

(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
 Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

REACTION NOTATIONS



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LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X11.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
LS	2.2	4.5	0.0	-2.2	2.4	0.0

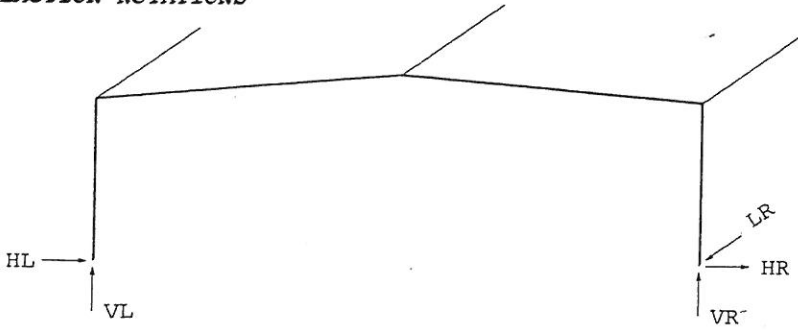
LOAD GROUP DESCRIPTION

LS : Unbalanced Left Roof Snow Load

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #05 LOCATION:frame lines 2

NOTES:(1) All reactions are in kips and kip-ft. TIME:13:50:01  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.  
(3) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions. Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

REACTION NOTATIONS



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LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X11.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD COMB	HL	VL	LL	HR	VR	LR

GRAVITY LOAD COMBINATION

6	12.0	19.6	0.0	-11.9	20.9	0.0
3	11.9	22.0	0.0	-11.9	20.9	0.0
4	11.9	19.6	0.0	-11.8	24.6	0.0

WIND LOAD COMBINATION

66	13.6	22.1	0.0	-9.2	19.0	0.0
46	-18.7	-29.5	0.0	14.9	-43.5	-11.4
45	-18.4	-30.1	0.0	15.2	-42.6	-11.4
60	11.2	17.6	0.0	-11.9	25.0	0.0
37	-17.6	-28.6	0.0	14.4	-40.9	-11.4

TRANSVERSE EARTHQUAKE LOAD COMBINATION

25	10.9	19.3	0.0	-9.5	20.9	0.0
24	9.6	18.6	0.0	-10.8	21.5	0.0

LONGITUDINAL EARTHQUAKE LOAD COMBINATION

13	10.3	19.0	0.0	-10.1	19.5	-2.3
14	10.2	18.9	0.0	-10.2	19.6	-2.3

"BASE-ONLY" EARTHQUAKE LOAD COMBINATION

33	1.8	3.4	0.0	-1.7	-2.9	-9.1
34	1.7	3.4	0.0	-1.8	-2.9	-9.1

LOAD COMBINATION DESCRIPTION

- 3 : DL +COLL +PLLL1 +PLL1
- 4 : DL +COLL +PLL1 +PLLR1
- 6 : DL +COLL +PLL1
- 13 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 14 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 24 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 25 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 33 : 0.8693DL +3.LEQ
- 34 : 0.8693DL +3.LEQ
- 37 : DL +LWL1
- 45 : 0.6DL +LWL1
- 46 : 0.6DL +LWL2
- 60 : DL +0.75LL +COLL +0.75WL2
- 66 : DL +0.75LL +COLL +0.75WL4

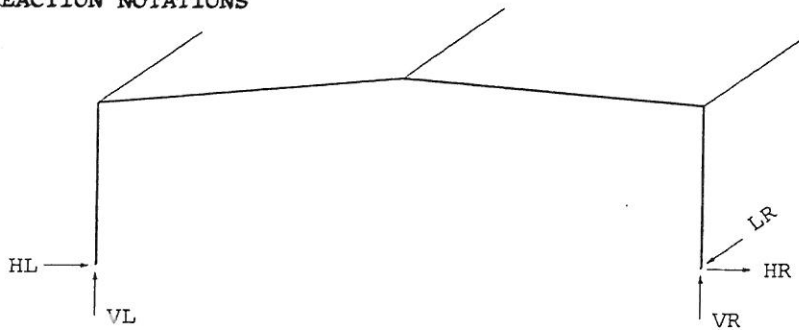
R8

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #09 LOCATION:frame lines 3-5

NOTES:(1) All reactions are in kips and kip-ft.  
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

TIME:13:51:20

REACTION NOTATIONS



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LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	2.0	4.7	0.0	-2.0	4.5	0.0
COLL	2.9	6.3	0.0	-2.9	6.1	0.0
PAR1	1.0	2.4	0.0	-1.0	0.8	0.0
PAR2	1.1	0.8	0.0	-1.1	2.4	0.0
PLLL1	-0.2	4.2	0.0	0.1	0.0	0.0
PLL1	7.2	10.9	0.0	-7.2	10.9	0.0
PLLR1	0.0	0.0	0.0	0.1	3.7	0.0
LL	7.0	15.0	0.0	-7.0	14.6	0.0
SNOW	2.0	4.9	0.0	-2.0	4.2	0.0
LEQ	0.0	0.0	0.0	0.0	-2.2	-3.0
EQ	-0.9	-0.4	0.0	-0.8	0.4	0.0
WL1	-17.1	-34.2	0.0	11.3	-27.2	0.0
WL2	1.9	-4.0	0.0	-2.2	5.3	0.0
LWL1	-19.2	-37.5	0.0	16.9	-46.2	-11.7
LWL2	-19.6	-37.2	0.0	16.5	-47.1	-11.7
LWL3	-14.6	-26.9	0.0	12.1	-36.1	-11.7
LWL4	-14.8	-26.8	0.0	11.9	-36.5	-11.7
WL3	-14.2	-26.6	0.0	14.7	-35.2	0.0
WL4	4.8	3.6	0.0	1.2	-2.7	0.0
RS	2.2	2.1	0.0	-2.1	6.5	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PAR1 : Partial Load [PARxx]
- PAR2 : Partial Load [PARxx]
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- PLL1 : Pattern Live Load [PLLxx]
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- LEQ : Longitudinal Seismic Load [located in perp. plane]
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load

R9

SUPPORT REACTIONS FOR EACH LOAD GROUP

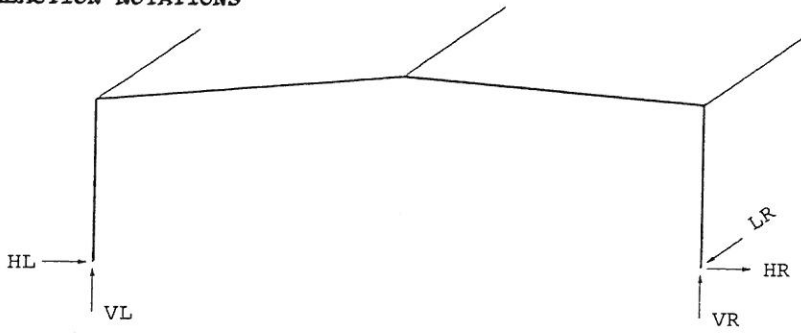
FRAME ID #09 LOCATION:frame lines 3-5

NOTES:(1) All reactions are in kips and kip-ft.

(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
 Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

TIME:13:51:20

**REACTION NOTATIONS**



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**LOAD GROUP REACTION TABLE**

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
LS	2.2	4.6	0.0	-2.2	2.4	0.0

**LOAD GROUP DESCRIPTION**

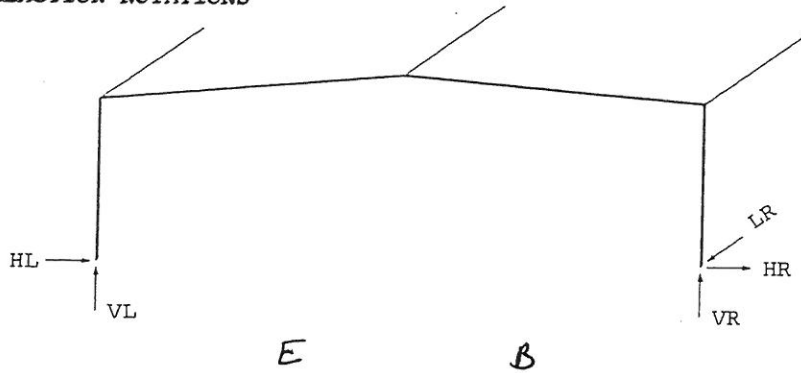
LS : Unbalanced Left Roof Snow Load

R10

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #09 LOCATION:frame lines 3-5

NOTES:(1) All reactions are in kips and kip-ft. TIME:13:51:20  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.  
(3) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions. Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

REACTION NOTATIONS



LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD COMB	HL	VL	LL	HR	VR	LR

GRAVITY LOAD COMBINATION

6	12.1	21.9	0.0	-12.1	21.5	0.0
3	11.9	26.0	0.0	-12.0	21.5	0.0
4	12.0	21.8	0.0	-11.9	25.2	0.0

WIND LOAD COMBINATION

66	13.7	24.9	0.0	-9.2	19.5	0.0
46	-18.4	-34.4	0.0	15.3	-44.4	-11.7
45	-18.0	-34.7	0.0	15.7	-43.5	-11.7
60	11.5	19.2	0.0	-11.8	25.5	0.0
37	-17.2	-32.8	0.0	14.9	-41.7	-11.7

TRANSVERSE EARTHQUAKE LOAD COMBINATION

25	11.0	22.9	0.0	-9.6	21.4	0.0
24	9.5	22.1	0.0	-11.0	22.1	0.0

LONGITUDINAL EARTHQUAKE LOAD COMBINATION

13	10.3	22.5	0.0	-10.2	20.1	-2.3
14	10.2	22.4	0.0	-10.3	20.1	-2.3

"BASE-ONLY" EARTHQUAKE LOAD COMBINATION

33	1.8	4.1	0.0	-1.8	-2.7	-9.1
34	1.8	4.1	0.0	-1.8	-2.7	-9.1

LOAD COMBINATION DESCRIPTION

- 3 : DL +COLL +PLLL1 +PLL1
- 4 : DL +COLL +PLL1 +PLLR1
- 6 : DL +COLL +PLL1
- 13 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 14 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 24 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 25 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 33 : 0.8693DL +3.LEQ
- 34 : 0.8693DL +3.LEQ
- 37 : DL +LWL1
- 45 : 0.6DL +LWL1
- 46 : 0.6DL +LWL2
- 60 : DL +0.75LL +COLL +0.75WL2
- 66 : DL +0.75LL +COLL +0.75WL4

SUPPORT REACTIONS FOR EACH LOAD GROUP

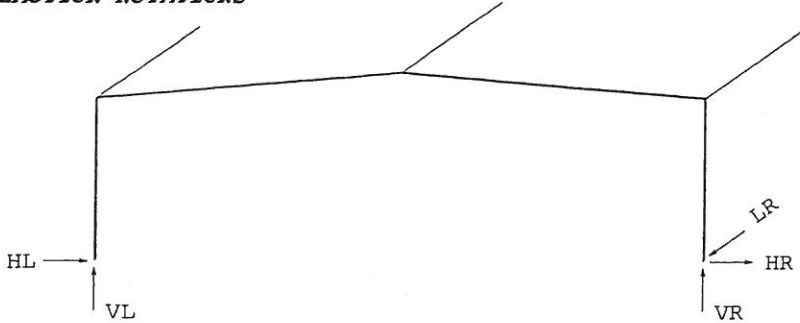
FRAME ID #07 LOCATION:frame lines 6

NOTES:(1) All reactions are in kips and kip-ft.

TIME:13:50:40

(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

REACTION NOTATIONS



E B

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	1.9	4.5	0.0	-1.9	4.4	0.0
COLL	2.8	6.1	0.0	-2.8	6.0	0.0
PAR1	1.0	2.4	0.0	-1.0	0.8	0.0
PAR2	1.0	0.7	0.0	-1.0	2.4	0.0
PLLL1	-0.2	4.2	0.0	0.1	0.0	0.0
PLL1	7.0	10.6	0.0	-7.0	10.6	0.0
PLLR1	-0.1	0.0	0.0	0.1	3.7	0.0
LL	6.7	14.8	0.0	-6.8	14.3	0.0
SNOW	1.9	4.8	0.0	-2.0	4.2	0.0
LEQ	0.0	0.0	0.0	0.0	-2.2	-3.0
EQ	-0.8	-0.4	0.0	-0.8	0.4	0.0
WL1	-16.0	-33.4	0.0	11.0	-26.7	0.0
WL2	-2.1	-4.8	0.0	-3.5	5.9	0.0
LWL1	-14.5	-35.9	0.0	17.7	-46.3	-11.7
LWL2	-14.9	-35.6	0.0	17.3	-47.2	-11.7
LWL3	-10.1	-25.6	0.0	13.0	-36.4	-11.7
LWL4	-10.2	-25.5	0.0	12.8	-36.8	-11.7
WL3	-10.2	-25.3	0.0	15.3	-35.2	0.0
WL4	3.7	3.3	0.0	0.9	-2.5	0.0
RS	2.1	2.0	0.0	-2.0	6.4	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PAR1 : Partial Load [PARxx]
- PAR2 : Partial Load [PARxx]
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- PLL1 : Pattern Live Load [PLLxx]
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- LEQ : Longitudinal Seismic Load [located in perp. plane]
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load

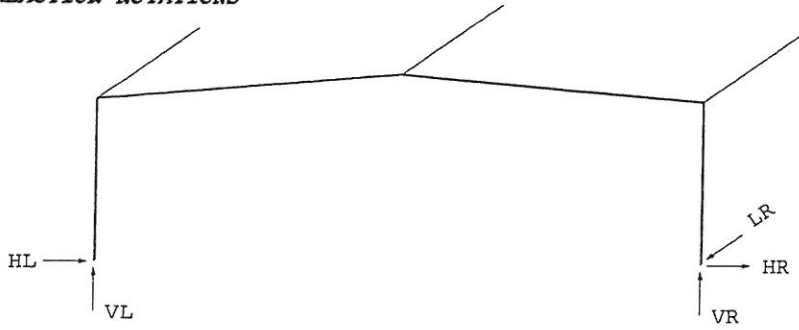
R12

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #07 LOCATION:frame lines 6

NOTES:(1) All reactions are in kips and kip-ft.  
 (2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.  
 Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

TIME:13:50:40

**REACTION NOTATIONS**



E B

**LOAD GROUP REACTION TABLE**

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LL	HR	VR	LR
LS	2.1	4.5	0.0	-2.1	2.4	0.0

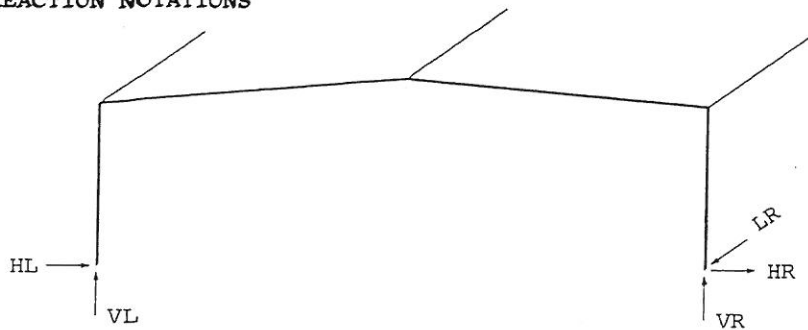
**LOAD GROUP DESCRIPTION**

LS : Unbalanced Left Roof Snow Load

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #07 LOCATION:frame lines 6

NOTES:(1) All reactions are in kips and kip-ft. TIME:13:50:40  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.  
(3) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions. Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 3.000

**REACTION NOTATIONS**



E B

**LOAD COMBINATION MAXIMUM REACTION TABLE**

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X10.0X0.5			8.0X10.0X0.625		
ANC. BOLTS	(4)-1			(4)-1		
LOAD COMB	HL	VL	LL	HR	VR	LR

**GRAVITY LOAD COMBINATION**

6	11.7	21.3	0.0	-11.7	21.0	0.0
3	11.5	25.5	0.0	-11.6	20.9	0.0
4	11.6	21.3	0.0	-11.6	24.7	0.0

**WIND LOAD COMBINATION**

66	12.6	24.2	0.0	-9.1	19.2	0.0
43	-14.9	-30.7	0.0	9.8	-24.1	0.0
45	-13.4	-33.2	0.0	16.5	-43.6	-11.7
60	8.2	18.1	0.0	-12.4	25.5	0.0
46	-13.7	-32.9	0.0	16.1	-44.5	-11.7
37	-12.6	-31.4	0.0	15.8	-41.9	-11.7

**TRANSVERSE EARTHQUAKE LOAD COMBINATION**

25	10.6	22.4	0.0	-9.2	21.0	0.0
24	9.2	21.6	0.0	-10.6	21.6	0.0

**LONGITUDINAL EARTHQUAKE LOAD COMBINATION**

13	10.0	22.0	0.0	-9.9	19.6	-2.3
14	9.8	22.0	0.0	-10.0	19.7	-2.3

**"BASE-ONLY" EARTHQUAKE LOAD COMBINATION**

33	1.7	4.0	0.0	-1.7	-2.9	-9.1
34	1.7	4.0	0.0	-1.7	-2.9	-9.1

**LOAD COMBINATION DESCRIPTION**

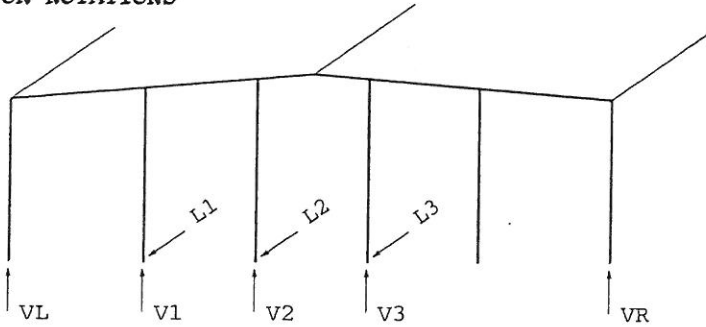
- 3 : DL +COLL +PLLL1 +PLL1
- 4 : DL +COLL +PLL1 +PLLR1
- 6 : DL +COLL +PLLL1
- 13 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 14 : 1.023DL +0.75LL +1.023COLL +0.75LEQ
- 24 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 25 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 33 : 0.8693DL +3.LEQ
- 34 : 0.8693DL +3.LEQ
- 37 : DL +LWL1
- 43 : 0.6DL +WL1
- 45 : 0.6DL +LWL1
- 46 : 0.6DL +LWL2
- 60 : DL +0.75LL +COLL +0.75WL2
- 66 : DL +0.75LL +COLL +0.75WL4

SUPPORT REACTIONS FOR EACH LOAD GROUP  
 NOTE: All reactions are in kips and kip-ft.

@ Fl#6

TIME:11:56:05

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

B.6 C.2 C.8

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2			INTERIOR COLUMN 3		
BASE PLATE	0.0X0.0X0.0			0.0X0.0X0.0			8.0X8.25X0.375			8.0X8.25X0.375			8.0X8.25X0.375		
ANC. BOLTS	N/A			N/A			(4)-3/4			(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3
D	0.	0.	0.	0.	0.	0.	0.	0.352	0.	0.	0.372	0.	0.	0.372	0.
W+	0.	0.	0.	0.	0.	0.	0.	0.	4.386	0.	0.	4.232	0.	0.	4.232
W-	0.	0.	0.	0.	0.	0.	0.	0.	-4.710	0.	0.	-4.543	0.	0.	-4.543

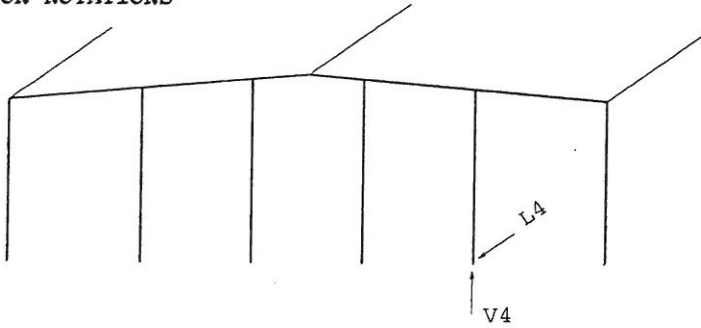
LOAD GROUP DESCRIPTION

- D : DEAD LOAD
- W+ : WIND LOAD AS AN INWARD ACTING PRESSURE
- W- : WIND LOAD AS AN OUTWARD ACTING SUCTION

SUPPORT REACTIONS FOR EACH LOAD GROUP  
 NOTE: All reactions are in kips and kip-ft.

TIME:11:56:05

**REACTION NOTATIONS**



D.2

**LOAD GROUP REACTION TABLE**

COLUMN	INTERIOR COLUMN 4		
BASE PLATE	8.0X8.25X0.375		
ANC. BOLTS	(4)-3/4		
LOAD GROUP	H4	V4	L4
D	0.	0.352	0.
W+	0.	0.	4.386
W-	0.	0.	-4.710

**LOAD GROUP DESCRIPTION**

- D : DEAD LOAD
- W+ : WIND LOAD AS AN INWARD ACTING PRESSURE
- W- : WIND LOAD AS AN OUTWARD ACTING SUCTION

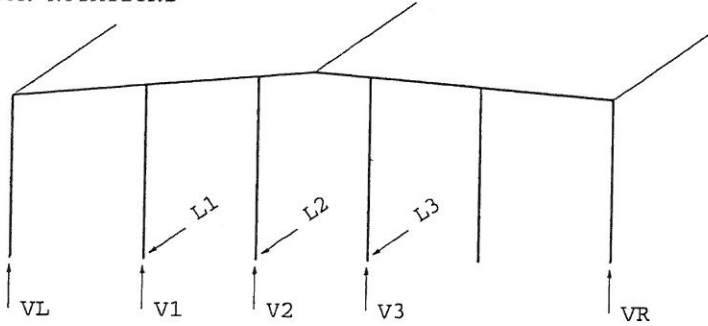
MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS

NOTES: (1) All reactions are in kips and kip-ft.

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

TIME:11:56:05

REACTION NOTATIONS



LOAD COMBINATION MAXIMUM REACTION TABLE

B.6 C.2 C.8

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2			INTERIOR COLUMN 3		
BASE PLATE	0.0X0.0X0.0			0.0X0.0X0.0			8.0X8.25X0.375			8.0X8.25X0.375			8.0X8.25X0.375		
ANC. BOLTS	N/A			N/A			(4)-3/4			(4)-3/4			(4)-3/4		
LOAD COMB	HL	VL	LL	HR	VR	LR	H1	V1	L1	H2	V2	L2	H3	V3	L3

WIND LOAD COMBINATION

1	0.	0.	0.	0.	0.	0.	0.	0.211	4.386	0.	0.223	4.232	0.	0.223	4.232
2	0.	0.	0.	0.	0.	0.	0.	0.211	-4.710	0.	0.223	-4.543	0.	0.223	-4.543

LOAD COMBINATION DESCRIPTION

- 1 : 0.60D + W+
- 2 : 0.60D + W-

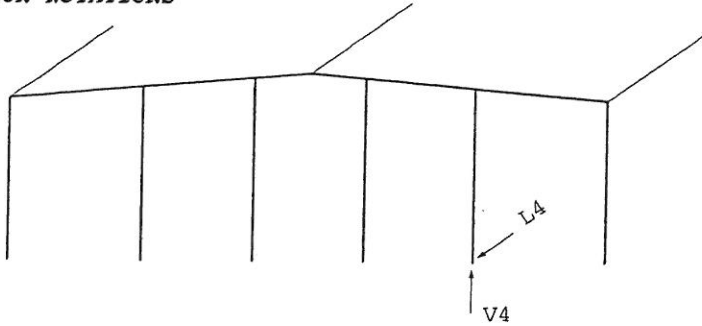
MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS

NOTES: (1) All reactions are in kips and kip-ft.

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

TIME:11:56:05

REACTION NOTATIONS



D.2

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	INTERIOR COLUMN 4		
BASE PLATE	8.0X8.25X0.375		
ANC. BOLTS	(4)-3/4		
LOAD COMB	H4	V4	L4

WIND LOAD COMBINATION

1	0.	0.211	4.386
2	0.	0.211	-4.710

LOAD COMBINATION DESCRIPTION

- 1 : 0.60D + W+  
 2 : 0.60D + W-

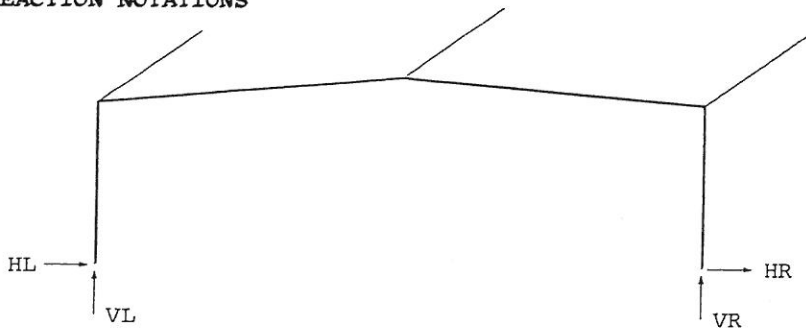
SUPPORT REACTIONS FOR EACH LOAD GROUP

FRAME ID #08 LOCATION:frame lines 7

NOTE: All reactions are in kips and kip-ft.

TIME:13:50:57

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X10.0X0.375			8.0X10.0X0.375		
ANC. BOLTS	(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	1.3	2.5	0.0	-1.2	3.4	0.0
COLL	1.6	2.5	0.0	-1.6	4.0	0.0
PAR1	0.5	1.3	0.0	-0.5	0.4	0.0
PAR2	0.5	0.4	0.0	-0.5	1.3	0.0
PLL1	3.6	5.7	0.0	-3.6	5.7	0.0
PLLR1	-0.1	0.0	0.0	0.1	3.7	0.0
LL	3.6	5.6	0.0	-3.5	9.4	0.0
SNOW	1.0	1.6	0.0	-1.0	2.7	0.0
EQ	-0.4	-0.2	0.0	-0.5	0.2	0.0
WL1	-13.3	-19.6	0.0	7.1	-20.0	0.0
WL2	-6.6	-5.2	0.0	1.3	0.4	0.0
LWL1	-8.3	-17.7	0.0	7.2	-23.4	0.0
LWL2	-8.3	-16.2	0.0	6.6	-24.3	0.0
LWL3	-5.5	-12.8	0.0	4.8	-17.0	0.0
LWL4	-5.5	-12.1	0.0	4.6	-17.4	0.0
WL3	-5.6	-13.4	0.0	8.9	-26.3	0.0
WL4	1.2	1.0	0.0	3.1	-5.9	0.0
RS	1.1	1.1	0.0	-1.0	4.3	0.0
LS	1.1	2.4	0.0	-1.1	1.4	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PAR1 : Partial Load [PARxx]
- PAR2 : Partial Load [PARxx]
- PLL1 : Pattern Live Load [PLLxx]
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load
- LS : Unbalanced Left Roof Snow Load

R19

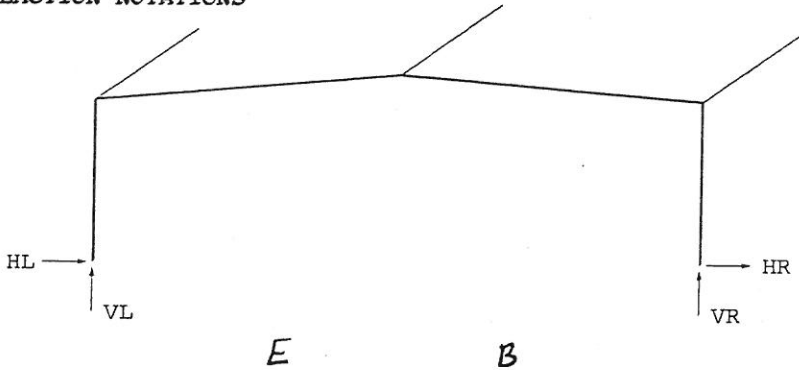
MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #08 LOCATION:frame lines 7

NOTES:(1) All reactions are in kips and kip-ft.

TIME:13:50:57

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS



LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X10.0X0.375			8.0X10.0X0.375		
ANC. BOLTS	(4)-3/4			(4)-3/4		

GRAVITY LOAD COMBINATION

3	6.5	10.6	0.0	-6.5	13.1	0.0
5	6.5	10.6	0.0	-6.3	16.8	0.0

WIND LOAD COMBINATION

54	6.5	9.9	0.0	-3.1	10.1	0.0
31	-12.6	-18.2	0.0	6.3	-18.0	0.0
37	-4.8	-12.0	0.0	8.1	-24.2	0.0
48	0.6	5.3	0.0	-4.5	14.8	0.0

TRANSVERSE EARTHQUAKE LOAD COMBINATION

17	6.0	9.5	0.0	-5.1	14.5	0.0
16	5.3	9.1	0.0	-5.9	14.8	0.0

LOAD COMBINATION DESCRIPTION

- 3 : DL +COLL +PLL1
- 5 : DL +LL +COLL
- 16 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 17 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 31 : 0.6DL +WL1
- 37 : 0.6DL +WL3
- 48 : DL +0.75LL +COLL +0.75WL2
- 54 : DL +0.75LL +COLL +0.75WL4

R20

SUPPORT REACTIONS FOR EACH LOAD GROUP

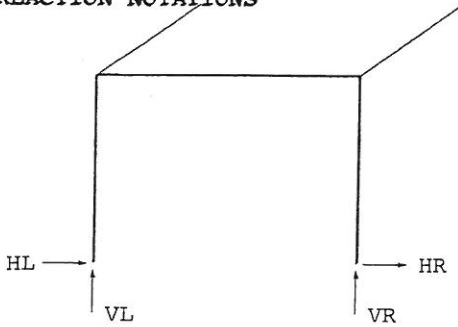
FRAME ID #10 LOCATION:bays

NOTE: All reactions are in kips and kip-ft.

TIME:13:51:38

REACTION NOTATIONS

PORTAL FRAME ALONE  
 GRID LINE "E"



5 4

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	8.0X30.375X0.375			8.0X30.375X0.375		
ANC. BOLTS	(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	0.1	1.3	0.0	-0.1	1.3	0.0
EQ	-2.7	-4.5	0.0	-2.9	4.5	0.0
WL1	-10.5	-17.7	0.0	-11.5	17.7	0.0
WL2	11.5	17.7	0.0	10.5	-17.7	0.0

LOAD GROUP DESCRIPTION

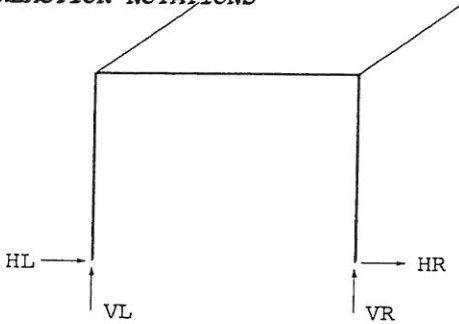
- DL : Roof Dead Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load

R21

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #10 LOCATION:bays

NOTES: (1) All reactions are in kips and kip-ft. TIME:13:51:38  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS



LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X30.375X0.375			8.0X30.375X0.375		
ANC. BOLTS	(4)-3/4			(4)-3/4		

GRAVITY LOAD COMBINATION

1	0.1	1.3	0.0	-0.1	1.3	0.0
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WIND LOAD COMBINATION

15	11.6	19.1	0.0	10.4	-16.4	0.0
16	-10.5	-16.9	0.0	-11.6	18.5	0.0
17	11.6	18.5	0.0	10.5	-16.9	0.0
14	-10.4	-16.4	0.0	-11.6	19.1	0.0

TRANSVERSE EARTHQUAKE LOAD COMBINATION

12	2.0	4.5	0.0	2.0	-1.8	0.0
7	-1.8	-2.4	0.0	-2.1	3.9	0.0
8	1.9	3.9	0.0	2.0	-2.4	0.0
11	-1.8	-1.8	0.0	-2.1	4.5	0.0

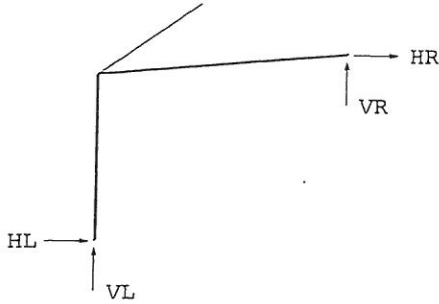
LOAD COMBINATION DESCRIPTION

- 1 : DL
- 7 : 0.5785DL +0.7EQ
- 8 : 0.5785DL -0.7EQ
- 11 : 1.023DL +0.7EQ
- 12 : 1.023DL -0.7EQ
- 14 : DL +WL1
- 15 : DL +WL2
- 16 : 0.6DL +WL1
- 17 : 0.6DL +WL2

SUPPORT REACTIONS FOR EACH LOAD GROUP FRAME ID #03 LOCATION:frame lines 1-7  
 NOTE: All reactions are in kips and kip-ft.

TIME:13:49:36

**REACTION NOTATIONS**



A

**LOAD GROUP REACTION TABLE**

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
BASE PLATE	8.0X10.25X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	0.0	1.4	0.0	0.0	1.0	0.0
COLL	0.0	1.7	0.0	0.0	1.5	0.0
PLLR1	0.1	4.0	0.0	-0.1	3.6	0.0
LL	0.1	4.1	0.0	-0.1	3.7	0.0
SNOW	0.0	1.2	0.0	0.0	1.1	0.0
EQ	0.0	0.0	0.0	-0.2	0.0	0.0
WL1	4.4	-8.7	0.0	6.4	-6.7	0.0
WL2	-1.4	1.7	0.0	-1.8	1.3	0.0
LWL1	5.2	-11.6	0.0	7.9	-9.1	0.0
LWL2	5.1	-14.6	0.0	8.3	-10.2	0.0
LWL3	5.2	-8.7	0.0	7.4	-6.7	0.0
LWL4	5.2	-10.0	0.0	7.5	-7.1	0.0
WL3	0.6	-11.2	0.0	2.8	-9.6	0.0
WL4	-5.2	-0.8	0.0	-5.4	-1.6	0.0
RS	0.0	2.2	0.0	0.0	1.9	0.0
LS	0.0	0.4	0.0	0.0	0.3	0.0

**LOAD GROUP DESCRIPTION**

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PLLR1 : Pattern Live Load Right Leanto/Canopy [PLLRxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load
- LS : Unbalanced Left Roof Snow Load

R23

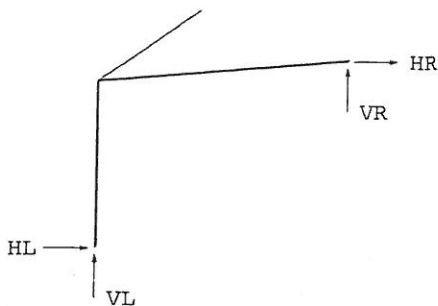
MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #03 LOCATION:frame lines 1-7

NOTES:(1) All reactions are in kips and kip-ft.

TIME:13:49:36

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS



A

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X10.25X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		

GRAVITY LOAD COMBINATION

2	0.2	7.2	0.0	-0.2	6.1	0.0
3	0.2	7.3	0.0	-0.2	6.1	0.0

WIND LOAD COMBINATION

33	5.3	-5.6	0.0	7.3	-4.2	0.0
28	-5.1	0.1	0.0	-5.4	-1.0	0.0
38	-0.9	7.6	0.0	-1.5	6.2	0.0
24	5.1	-13.7	0.0	8.3	-9.6	0.0
36	-5.1	2.4	0.0	-5.5	0.9	0.0

TRANSVERSE EARTHQUAKE LOAD COMBINATION

9	0.1	6.3	0.0	-0.3	5.3	0.0
12	0.0	0.8	0.0	0.1	0.6	0.0
10	0.1	6.3	0.0	0.0	5.3	0.0

LOAD COMBINATION DESCRIPTION

- 2 : DL +COLL +PLLR1
- 3 : DL +LL +COLL
- 9 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 10 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 12 : 0.5785DL -0.7EQ
- 24 : 0.6DL +LWL2
- 28 : 0.6DL +WL4
- 33 : DL +COLL +LWL3
- 36 : DL +COLL +WL4
- 38 : DL +0.75LL +COLL +0.75WL2

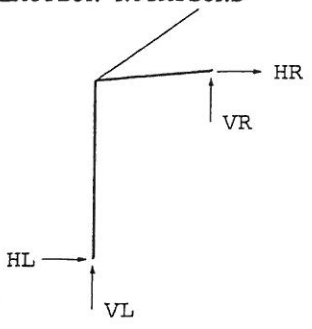
SUPPORT REACTIONS FOR EACH LOAD GROUP  
NOTE: All reactions are in kips and kip-ft.

FRAME ID #01 LOCATION: frame lines 1-2

TIME:11:17:19

**BLDG #4**

**REACTION NOTATIONS**



**F**

**LOAD GROUP REACTION TABLE**

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
BASE PLATE	8.0X9.875X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	0.0	0.7	0.0	0.0	0.4	0.0
COLL	0.0	0.9	0.0	0.0	0.7	0.0
PLLL1	0.1	2.9	0.0	-0.1	2.4	0.0
LL	0.0	3.1	0.0	0.0	2.4	0.0
SNOW	0.0	0.9	0.0	0.0	0.7	0.0
EQ	0.0	0.0	0.0	-0.1	0.0	0.0
WL1	-1.3	-3.8	0.0	-0.7	-3.1	0.0
WL2	-3.3	-2.0	0.0	-3.2	-2.0	0.0
LWL1	3.5	-5.9	0.0	4.7	-3.9	0.0
LWL2	3.5	-4.2	0.0	4.4	-2.6	0.0
LWL3	3.5	-3.5	0.0	4.3	-2.1	0.0
LWL4	3.5	-2.8	0.0	4.2	-1.5	0.0
WL3	2.6	-2.7	0.0	3.2	-1.6	0.0
WL4	0.6	-0.9	0.0	0.8	-0.6	0.0

**LOAD GROUP DESCRIPTION**

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load

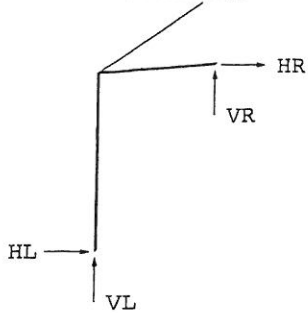
**R25**

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS FRAME ID #01 LOCATION frame lines 1-2

NOTES: (1) All reactions are in kips and kip-ft. TIME:11:17:19  
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS

BLOG #4



F

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X9.875X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		
<b>GRAVITY LOAD COMBINATION</b>						
1	0.1	4.4	0.0	-0.1	3.4	0.0
3	0.1	4.6	0.0	-0.1	3.4	0.0
<b>WIND LOAD COMBINATION</b>						
34	3.6	-1.2	0.0	4.2	-0.5	0.0
22	-3.3	-1.6	0.0	-3.2	-1.8	0.0
44	0.5	3.2	0.0	0.5	2.4	0.0
23	3.5	-5.5	0.0	4.7	-3.7	0.0
30	-3.3	-0.4	0.0	-3.2	-1.0	0.0
<b>TRANSVERSE EARTHQUAKE LOAD COMBINATION</b>						
10	0.1	3.9	0.0	0.0	2.9	0.0
9	0.1	3.9	0.0	-0.1	2.8	0.0
12	0.0	0.4	0.0	0.1	0.2	0.0

LOAD COMBINATION DESCRIPTION

- 1 : DL +COLL +PLLL1
- 3 : DL +LL +COLL
- 9 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 10 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 12 : 0.5785DL -0.7EQ
- 22 : 0.6DL +WL2
- 23 : 0.6DL +LWL1
- 30 : DL +COLL +WL2
- 34 : DL +COLL +LWL4
- 44 : DL +0.75LL +COLL +0.75WL4

R26

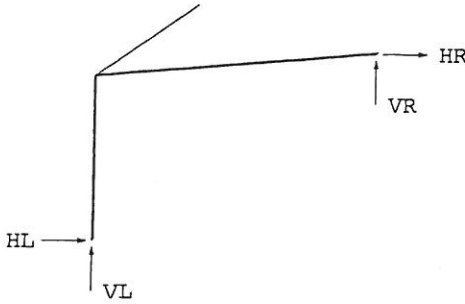
SUPPORT REACTIONS FOR EACH LOAD GROUP  
NOTE: All reactions are in kips and kip-ft.

FRAME ID #05 LOCATION frame lines 3-6

TIME:11:01:55

REACTION NOTATIONS

BLOG #3



G

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
BASE PLATE	8.0X9.875X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		
LOAD GROUP	HL	VL	LL	HR	VR	LR
DL	0.0	1.3	0.0	0.0	1.1	0.0
COLL	0.0	1.9	0.0	0.0	1.7	0.0
PLLL1	0.1	4.5	0.0	-0.1	4.1	0.0
LL	0.1	4.6	0.0	-0.1	4.1	0.0
SNOW	0.0	1.9	0.0	0.0	1.7	0.0
EQ	0.0	0.0	0.0	-0.2	0.0	0.0
WL1	-1.4	-8.6	0.0	0.3	-7.7	0.0
WL2	-3.2	-4.8	0.0	-2.5	-4.8	0.0
LWL1	3.1	-12.1	0.0	5.8	-8.4	0.0
LWL2	3.2	-9.0	0.0	5.2	-7.3	0.0
LWL3	3.2	-7.1	0.0	4.8	-5.0	0.0
LWL4	3.2	-5.8	0.0	4.6	-4.5	0.0
WL3	2.4	-5.7	0.0	3.7	-4.6	0.0
WL4	0.5	-1.9	0.0	1.0	-1.6	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- COLL : Roof Collateral Load
- PLLL1 : Pattern Live Load Left Leanto/Canopy [PLLLxx]
- LL : Roof Live Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS

FRAME ID #05 LOCATION frame lines 3-6

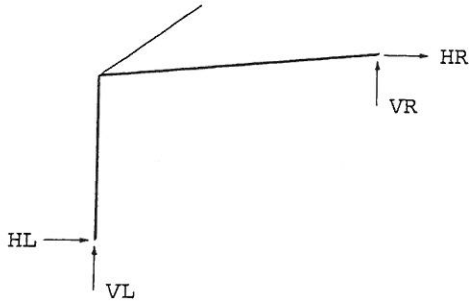
TIME: 11:01:55

NOTES: (1) All reactions are in kips and kip-ft.

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

REACTION NOTATIONS

BLOG #3



G

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT SUPPORT		
	HL	VL	LL	HR	VR	LR
BASE PLATE	8.0X9.875X0.375			N/A		
ANC. BOLTS	(4)-3/4			N/A		
<b>GRAVITY LOAD COMBINATION</b>						
1	0.2	7.7	0.0	-0.2	6.9	0.0
3	0.2	7.9	0.0	-0.2	6.9	0.0
<b>WIND LOAD COMBINATION</b>						
34	3.3	-2.5	0.0	4.6	-1.7	0.0
22	-3.2	-4.0	0.0	-2.5	-4.1	0.0
44	0.6	5.3	0.0	0.6	4.7	0.0
23	3.1	-11.3	0.0	5.6	-7.7	0.0
30	-3.1	-1.5	0.0	-2.5	-2.0	0.0
<b>TRANSVERSE EARTHQUAKE LOAD COMBINATION</b>						
9	0.1	6.8	0.0	-0.3	5.9	0.0
12	0.0	0.8	0.0	0.1	0.6	0.0
10	0.1	6.8	0.0	0.0	5.9	0.0

LOAD COMBINATION DESCRIPTION

- 1 : DL +COLL +PLLL1
- 3 : DL +LL +COLL
- 9 : 1.023DL +0.75LL +1.023COLL +0.75EQ
- 10 : 1.023DL +0.75LL +1.023COLL -0.75EQ
- 12 : 0.5785DL -0.7EQ
- 22 : 0.6DL +WL2
- 23 : 0.6DL +LWL1
- 30 : DL +COLL +WL2
- 34 : DL +COLL +LWL4
- 44 : DL +0.75LL +COLL +0.75WL4

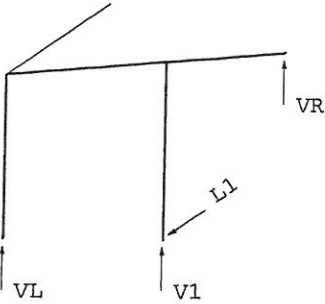
R28

SUPPORT REACTIONS FOR EACH LOAD GROUP  
 NOTE: All reactions are in kips and kip-ft.

TIME:09:52:10

REACTION NOTATIONS

Frame Line 3  
 BLDG#3



LOAD GROUP REACTION TABLE

F

COLUMN	LEFT COLUMN			RIGHT SUPPORT			INTERIOR COLUMN 1		
BASE PLATE	0.0X0.0X0.0			N/A			8.0X8.0X0.375		
ANC. BOLTS	N/A			N/A			(4)-3/4		
LOAD GROUP	HL	VL	LL	HR	VR	LR	H1	V1	L1
D	0.	0.	0.	0.	0.	0.	0.	0.173	0.
W+	0.	0.	0.	0.	0.	0.	0.	0.	3.292
W-	0.	0.	0.	0.	0.	0.	0.	0.	-3.643
E+	0.	0.	0.	0.	0.	0.	0.	0.	0.191
E-	0.	0.	0.	0.	0.	0.	0.	0.	-0.191

LOAD GROUP DESCRIPTION

- D : DEAD LOAD
- W+ : WIND LOAD AS AN INWARD ACTING PRESSURE
- W- : WIND LOAD AS AN OUTWARD ACTING SUCTION
- E+ : EARTHQUAKE FORCE ACTING INWARD
- E- : EARTHQUAKE FORCE ACTING OUTWARD

R29

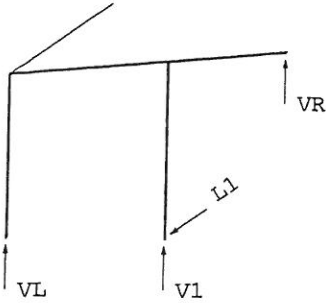
MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS

NOTES: (1) All reactions are in kips and kip-ft.

(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.

TIME:09:52:10

REACTION NOTATIONS



Frame Line 3

BLDG #3

LOAD COMBINATION MAXIMUM REACTION TABLE

F

COLUMN	LEFT COLUMN			RIGHT SUPPORT			INTERIOR COLUMN 1		
BASE PLATE	0.0X0.0X0.0			N/A			8.0X8.0X0.375		
ANC. BOLTS	N/A			N/A			(4) -3/4		
LOAD COMB	HL	VL	LL	HR	VR	LR	H1	V1	L1

WIND LOAD COMBINATION

1	0.	0.	0.	0.	0.	0.	0.	0.104	3.292
2	0.	0.	0.	0.	0.	0.	0.	0.104	-3.643

LONGITUDINAL EARTHQUAKE LOAD COMBINATION

3	0.	0.	0.	0.	0.	0.	0.	0.151	0.191
4	0.	0.	0.	0.	0.	0.	0.	0.151	-0.191

LOAD COMBINATION DESCRIPTION

- 1 : 0.60D + W+
- 2 : 0.60D + W-
- 3 : 0.87D + E+
- 4 : 0.87D + E-

R30