3

DESIGN PACKAGE

BUILDER: **Casey Civil, LLC**
CUSTOMER: **Pinnacle Casino - Manhattan Construction**
JOB NUMBER: **12-B-63841**

TABLE OF CONTENTS

	Page
Design Criteria	1
Notes on Drawings	2
Deflection Criteria	NA
Project Layout	3
Building A	4-16
Building B	17-24
Building C	NA
Special Details	25-26

Original Design Completed thru Change Order # 0
Revision History

Rev #	Update Reactions ?	Reason for Revision	Pages Revised	Date Revised	Eng.
3	Y	BLDB B GEOMETRY CHANGE, BLDG A FO CHANGE	3,4,6,14-15,17-24	6/27/11	AK

Project Engineer: ANU KHANNA (Oklahoma City)
Checking Engineer: BRIAN CARMICHAEL (Oklahoma City)
Signing Engineer: BRIAN CARMICHAEL (Oklahoma City)

Star Building Systems

P.O. Box 94910
Oklahoma City, OK 73143
(405) 636-2010
1-800-879-7827
FAX (405) 636-2419

June 29, 2011

Casey Civil, LLC
1080 Old Spanish Trail, STE 8
Slidell, LA 70458

12-B-63841 (REVISED)
Pinnacle Casino - Manhattan Construction
Baton Rouge, LA
MULTIPLE BUILDINGS

To Whom It May Concern:

This is to certify that materials for the subject structure have been designed in accordance with the order documents, specifically as shown per the attached Engineering Design Criteria Sheet.

Aspects of code compliance as related to use or occupancy, such as sprinkler requirements, are not addressed by these documents.

These materials, when properly erected on an adequate foundation in accordance with the erection drawings as supplied and using the components as furnished, will meet the attached loading requirements.

This certification does not cover field modifications or the design of materials not furnished by Star Building Systems.

The attached design criteria and calculations are to remain with and form part of this Letter of Certification. This letter voids and supersedes the previous Letter of Certification, dated May 11, 2011.

The calculations and the metal building they represent are the product of Star Building Systems or a division of its affiliate NCI Building Systems. The engineer whose seal appears hereon is employed by either Star Building Systems or a division of its affiliate NCI Building Systems and is not the engineer of record for this project.

Cordially,

Star Building Systems
Materials for Metal Buildings
An NCI Company

Brian Carmichael, P.E.
Regional Engineer

STATE OF LOUISIANA
BRIAN A CARMICHAEL
License No. 33110
PROFESSIONAL ENGINEER
CIVIL ENGINEERING

Brian Carmichael
06/29/11

ACCREDITED
AC472

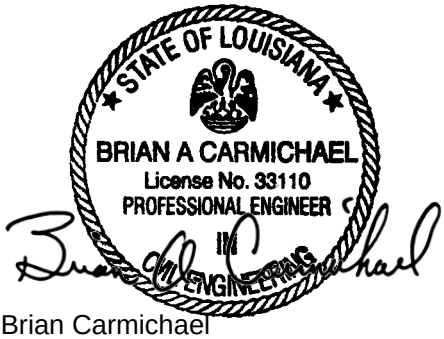
Star Building Systems

P.O. Box 94910
Oklahoma City, OK 73143
(405) 636-2010
1-800-879-7827
FAX (405) 636-2419

```
Building Code .....................   2006 IBC
Occupancy Category ................   Normal (Category II)
Roof Dead Load
     Superimposed .................   2.87 psf (Bldg A)
                                      2.27 psf (Bldg B)
     Collateral ...................   0.50 psf
     (0.50 psf Other)
Roof Live Load ....................   20.00 psf reduction allowed

Wind
     Basic Wind Speed .............   110.00 mph
     Wind Importance Factor (I) ...   1.00
     Wind Exposure Category .......   C
     Internal Pressure Coef (GCpi)    0.18/-0.18
     Loads for components not provided by building manufacturer
     Bldg A
     =======
     Corner Areas (within 9.07' of corner) 26.33 psf pressure -35.10 psf suction
     Other Areas                           26.33 psf pressure -28.52 psf suction
     Bldg B
     ======
     Corner Areas (within 3.40' of corner) 25.53 psf pressure -34.04 psf suction
     Other Areas                           25.53 psf pressure -27.66 psf suction
     These values are the maximum values required based on a 10 sq ft area.
     Components with larger areas may have lower wind loads.

Seismic
     Seismic Importance Factor (Ie)   1.00
     Seismic Design Category ......   B
     Soil Site Class ..............   D Stiff Soil
     Ss ...........................   0.121 g         Sds ..... 0.129 g
     S1 ...........................   0.053 g         Sd1 ..... 0.085 g
     Analysis Procedure ...........   Equivalent Lateral Force
      Bldg A
     =======
     Column Line                           All
     Basic Force Resisting System          H
     Response Modification Coefficient (R) 3.00
     Seismic Response Coefficient (Cs)     0.04
     Design Base Shear in kips (V)         5.03
     Bldg B
     =======
     Column Line                           All
     Basic Force Resisting System          H
     Response Modification Coefficient (R) 3.00
     Seismic Response Coefficient (Cs)     0.04
     Design Base Shear in kips (V)         0.88
     Basic Structural System (from ASCE 7-05 Table 12.1-1)
       H - Steel System not Specifically Detailed for Seismic Resistance
```



Brian Carmichael
06/29/11

ACCREDITED
AC472

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1101 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, other than flange braces, conform to ASTM 36 minimum. Hollow structural shapes conform to ASTM A500 grade B, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold-formed light gage steel members conform to grade 55, with a minimum yield point of 55 ksi. For Canada, material properties confirm to CAN/CSA G40.20/G40.21 or equivalent.

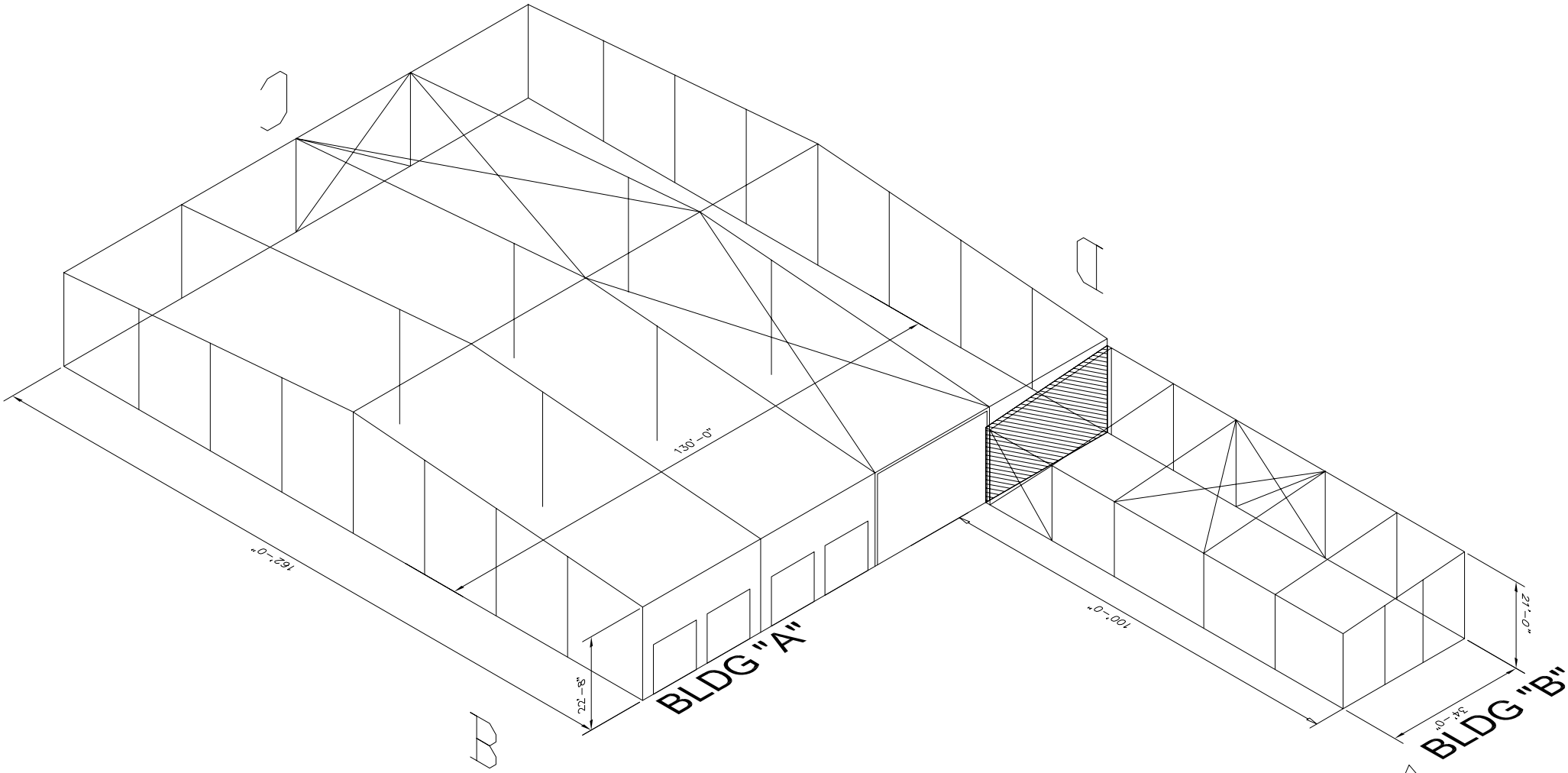
All bolted joints with A325M-09 Type 1 bolts are specified as snug-tightened joints in accordance with the "Specification for Structural Joints Using ASTM A325 or A490 Bolts, June 30, 2004". Pretensioning methods, including turn-of-nut and calibrated wrench are NOT required.

Using standard gutter with 4 x 5 downspouts, the roof drainage system for Bldg A has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 7.4 feet with the first downspout from both ends of the gutter run within 3.7 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 10 in/hr rainfall intensity as it corresponds to a 5 year recurrence interval.

Using standard gutter with 4 x 5 downspouts, the roof drainage system for Bldg B has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 24.6 feet with the first downspout from both ends of the gutter run within 12.3 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 10 in/hr rainfall intensity as it corresponds to a 5 year recurrence interval.

The cutting or removal of girts shown on the erection drawings due to the addition of open areas, framed openings, or doors not shown may void all warranties and certifications supplied by manufacturer as they apply to this condition.

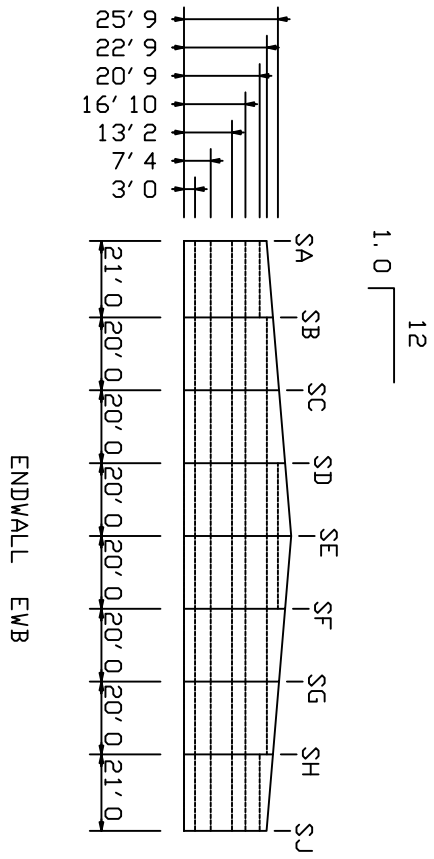
Field located framed openings shall be located in the bay and elevation as documented in the order documents and as shown on the erection drawings. Installation of the framed openings at different locations may void the warranties and certifications as they apply to the materials supplied by building manufacturer.

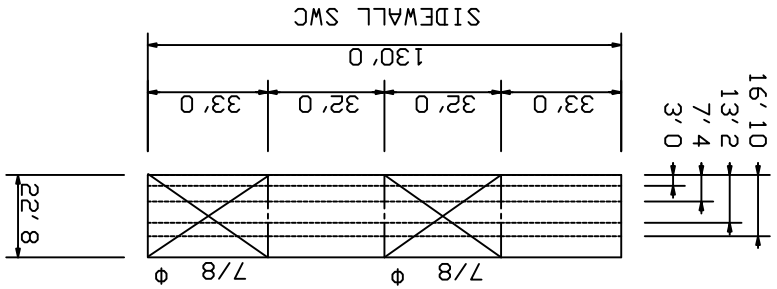


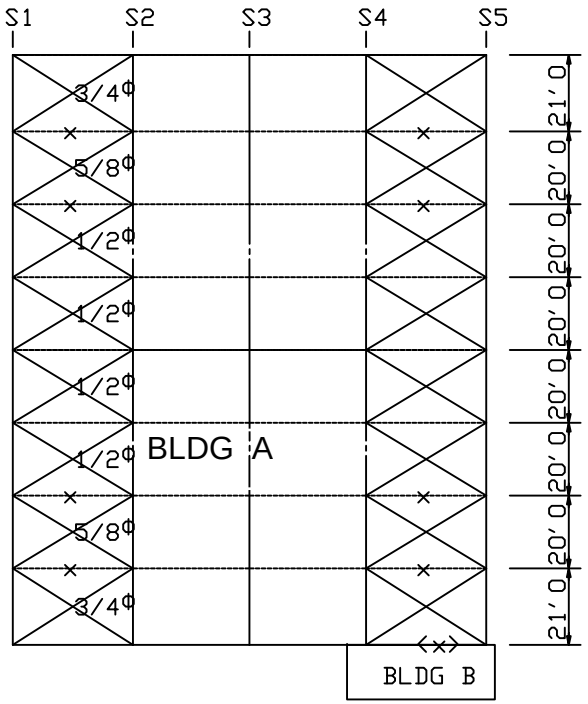
3
05/09/2011

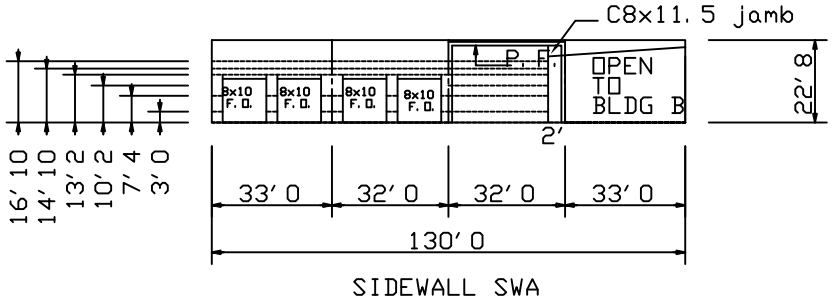
key Strut: x=double Z,
xx=triple Z,
o=pipe(WM)

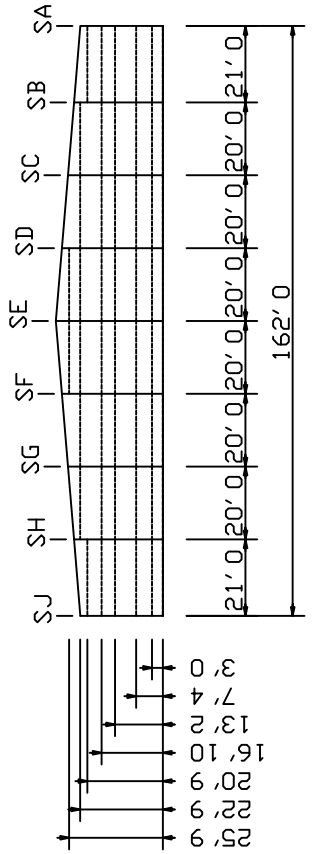
<x> CLIPPED DOUBLE Z











Owner :
Pinnacle Casino - Manhat
Baton Rouge LA 70810

Builder :
Casey Civil, LLC
Job No: 63841A run01
Version: ver01-axkhanna
Wed May 4 08:56:35 2011

3

M A I N B U I L D I N G A D E S I G N S U M M A R Y R E P O R T

ROOF PLANE ------- RPA
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AroofRPA_01.edf BUILDING A

Panel ..... SDR26 ROOF (DURA-RIB)
Purlins ..... 55.0 ksi Yield Strength
Eave Struts ..... 55.0 ksi Yield Strength

PURLIN SPACING : 2@3.4881 19@3'10 1.1911

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	33.000	8.5Z105	3 points	S 0.000	2.583 C
2	32.000	8.5Z85	None	C 1.083	1.083 C
3	32.000	8.5Z85	None	C 1.083	1.083 C
4	33.000	8.5Z105	3 points	C 2.583	0.000 S

Purlin @ Level 2-3: Washers Req`d @ Frame:2,4

Purlin Anchorage Hdwe @ Level 8,15,22 @ Frame Lines:1-5

All cold-formed connections use A325N bolts.

ROOF PLANE ------- RPC
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AroofRPC_01.edf

Panel ..... SDR26 ROOF (DURA-RIB)
Purlins ..... 55.0 ksi Yield Strength
Eave Struts ..... 55.0 ksi Yield Strength

PURLIN SPACING : 2@3.4881 19@3'10 1.1911

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	33.000	8.5Z105	3 points	S 0.000	2.583 C
2	32.000	8.5Z85	None	C 1.083	1.083 C
3	32.000	8.5Z85	None	C 1.083	1.083 C
4	33.000	8.5Z105	3 points	C 2.583	0.000 S

Purlin @ Level 2-3: Washers Req`d @ Frame:2,4

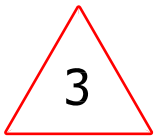
Purlin Anchorage Hdwe @ Level 8,15,22 @ Frame Lines:1-5

All cold-formed connections use A325N bolts.

Note: ** See Knock-In Bridging Detail

SWC Eave Strut @ 22.667 (ft) : 8.5E105 @ Bays 1-4;
SWA Eave Strut @ 22.667 (ft) : 8.5E105 @ Bays 1-2; 8.5E105(D) @ Bays 4;
Double purlin strut at eave located at SWA uses (4)-bolt clip(s) @ frame line 4-5

BRACING ---- Roof: 2 bays rods
Plane SWA :Portal Frame
Plane SWC : 2 bays rods
Plane EWB :Diaphragm
Plane EWD :Diaphragm



SIDEWALL PLANE SWA -- (8.500" Inset columns)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AwallSWA_01.edf

Panel ..... SDW26 WALL (DURA-RIB)
Girts ..... 55.0 ksi Yield Strength

GIRTS SPACINGS : 3'0 4'4 2'10 3'0 1'8 2'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'0	33.000	8.5Z59	F.O.	S 0.000	0.188 S
2	3'0	32.000	8.5Z59	F.O.	S 0.188	0.188 S
3	3'0	32.000	8.5Z105	4 points	S 0.188	0.188 S
1	7'4	33.000	8.5Z59	F.O.	S 0.000	0.188 S
2	7'4	32.000	8.5Z59	F.O.	S 0.188	0.188 S
3	7'4	32.000	8.5Z105	4 points	S 0.188	0.188 S
3	10'2	32.000	8.5Z85	4 points	S 0.188	0.188 S
1	13'2	33.000	**8.5Z105	5 points	S 0.000	0.188 S
2	13'2	32.000	**8.5Z105	5 points	S 0.188	0.188 S
3	13'2	32.000	8.5Z70	4 points	S 0.188	0.188 S
			**8.5Z105	REINFORCING GIRT		
1	14'10	33.000	8.5Z59	5 points	S 0.000	1.083 C
2	14'10	32.000	8.5Z59	5 points	C 1.083	1.083 C
3	14'10	32.000	8.5Z70	4 points	C 1.083	0.188 S
1	16'10	33.000	8.5Z85	5 points	S 0.000	2.083 C
2	16'10	32.000	8.5Z85	5 points	C 1.083	1.083 C
3	16'10	32.000	8.5Z85	4 points	C 1.083	0.188 S

Girt @ Level 13'2: Washers Req`d @ Frame:1-2

All cold-formed connections use A325N bolts.

Note : Maximum Distance To Extend Girt From Adjacent Bay is 12.00 inches.

FRAMED OPENINGS:

Qty	Size	Jamb & Header	Wall	Bay	Distance
1	8'0 x 10'0	8.5C105	SWA	1	3'0
1	8'0 x 10'0	8.5C105	SWA	1	18'0
1	8'0 x 10'0	8.5C105	SWA	2	3'0
1	8'0 x 10'0	8.5C105	SWA	2	18'0

OPEN AREAS:

Size	Wall	Distance
33'0 x 18'2	SWA	97'0

SIDEWALL PLANE SWC -- (8.500" Inset columns)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AwallSWC_01.edf

Panel ..... SDW26 WALL (DURA-RIB)
Girts ..... 55.0 ksi Yield Strength

GIRTS SPACINGS : 3'0 4'4 5'10 3'8

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'0	33.000	8.5Z105	4 points	S 0.000	3.188 C
2	3'0	32.000	8.5Z85	4 points	C 1.583	1.583 C
3	3'0	32.000	8.5Z85	4 points	C 1.583	1.583 C
4	3'0	33.000	8.5Z105	4 points	C 3.188	0.000 S
1	7'4	33.000	8.5Z105	4 points	S 0.000	2.083 C
2	7'4	32.000	8.5Z105	4 points	C 2.083	2.083 C
3	7'4	32.000	8.5Z105	4 points	C 2.083	2.083 C
4	7'4	33.000	8.5Z105	4 points	C 2.083	0.000 S
1	13'2	33.000	8.5Z105	4 points	S 0.000	3.188 C
2	13'2	32.000	8.5Z85	4 points	C 1.583	1.583 C
3	13'2	32.000	8.5Z85	4 points	C 1.583	1.583 C
4	13'2	33.000	8.5Z105	4 points	C 3.188	0.000 S
1	16'10	33.000	8.5Z105	4 points	S 0.000	3.188 C
2	16'10	32.000	8.5Z85	4 points	C 1.583	1.583 C
3	16'10	32.000	8.5Z85	4 points	C 1.583	1.583 C
4	16'10	33.000	8.5Z105	4 points	C 3.188	0.000 S

Girt @ Level 7'4,13'2,16'10: Washers Req`d @ Frame:2,4

All cold-formed connections use A325N bolts.

Note : Maximum Distance To Extend Girt From Adjacent Bay is 12.00 inches.

Endwall Plane EWB Design ..... Bearing Frame (BF)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AwallEWB_01.edf

Panel ..... SDW26 WALL (DURA-RIB)

RAFTERS -----

Mem #	Description Member Size Identification	Length (ft)	Start (ft)	End (ft)
1	W10X12 50.0 ksi	42.940	0.000	42.940
	connections... Left: Type-V SEP		Right: Type-II MEP	
2	W10X12 50.0 ksi	37.128	42.940	80.068
	connections... Left: Type-II MEP		Right: Type-III SEP	
3	W10X12 50.0 ksi	37.128	80.068	117.197
	connections... Left: Type-III SEP		Right: Type-II MEP	
4	W10X12 50.0 ksi	42.940	117.197	160.137
	connections... Left: Type-II MEP		Right: Type-V SEP	

Type-II MEP = (4)-1/2" A325N bolts w/ 3/8" Moment End Plate
Type-V SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate
Type-III SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWA: 10.809, 26.143,33.809,45.309,56.809,64.476,72.142 FB=C-TYPE
PLANE SWC: 10.809, 26.143,33.809,45.309,56.809,64.476,72.142 FB=C-TYPE

Girts ..... 55.0 ksi Yield Strength

Girts Spacings : 3'0 4'4 5'10 3'8 3'11 2'0 3'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'0	21.000	8.5Z85	2 points	S 0.000	0.000 S
2	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
7	3'0	20.000	8.5Z70	2 points	S 0.000	0.000 S
8	3'0	21.000	8.5Z85	2 points	S 0.000	0.000 S
1	7'4	21.000	8.5Z85	2 points	S 0.000	0.000 S
2	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
7	7'4	20.000	8.5Z70	2 points	S 0.000	0.000 S
8	7'4	21.000	8.5Z85	2 points	S 0.000	0.000 S
1	13'2	21.000	8.5Z85	2 points	S 0.000	0.000 S
2	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
7	13'2	20.000	8.5Z70	2 points	S 0.000	0.000 S
8	13'2	21.000	8.5Z85	2 points	S 0.000	0.000 S

EWB CONTD..

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	16'10	21.000	8.5Z59	2 points	S 0.000	0.000 S
2	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
7	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
8	16'10	21.000	8.5Z59	2 points	S 0.000	0.000 S
1	20'9	21.000	8.5Z59	2 points	S 0.000	0.000 S
8	20'9	21.000	8.5Z59	2 points	S 0.000	0.000 S
2	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	22'9	20.000	8.5Z85	2 points	S 0.000	0.000 S
4	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	22'9	20.000	8.5Z85	2 points	S 0.000	0.000 S
7	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	25'9	20.000	8.5Z59	2 points	S 0.000	0.000 S
5	25'9	20.000	8.5Z59	2 points	S 0.000	0.000 S

All cold-formed connections use A325N bolts.

EWB CONTD..

COLUMNS ----- (1.000" Inset columns)

Col #	Dist. from left	Description Member Size	Ident.	Base Elev (ft)	Base plate design information Thickness & bolts
SA	0.000'	W10X22	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SB	21.000'	W12X14	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SC	41.000'	W10X17	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SD	61.000'	W12X19	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 22.75 elev. FB=C-TYPE			
SE	81.000'	W12X19	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 25.75 elev. FB=C-TYPE			
SF	101.000'	W12X19	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 22.75 elev. FB=C-TYPE			
SG	121.000'	W10X17	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SH	141.000'	W12X14	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SJ	162.000'	W10X17	50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307

ENDWALL COLUMN TO BRIDGE CHANNEL CONNECTIONS:

COL. NO.	STRUT-TO-COLUMN CLIP ENDWALL FRAME LINE 1
PLANE SWC	
SB	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SC	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SD	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SE	AT PEAK, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 8.5C105 BRIDGE CHANNEL
SF	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SG	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SH	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL

PLANE SWA:

Endwall Plane EWD Design ..... Bearing Frame (BF)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-A\run01\AwallEWD_01.edf

Panel ..... SDW26 WALL (DURA-RIB)

RAFTERS -----

Mem #	Description Member Size Identification	Length (ft)	Start (ft)	End (ft)
1	W10X12 50.0 ksi	42.940	0.000	42.940
	connections... Left: Type-V SEP		Right: Type-II MEP	
2	W10X12 50.0 ksi	37.128	42.940	80.068
	connections... Left: Type-II MEP		Right: Type-III SEP	
3	W10X12 50.0 ksi	37.128	80.068	117.197
	connections... Left: Type-III SEP		Right: Type-II MEP	
4	W10X12 50.0 ksi	42.940	117.197	160.137
	connections... Left: Type-II MEP		Right: Type-V SEP	

Type-II MEP = (4)-1/2" A325N bolts w/ 3/8" Moment End Plate
Type-V SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate
Type-III SEP = (4)-1/2" A325N bolts w/ 3/8" Shear End Plate

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWA: 10.809, 26.143,33.809,45.309,56.809,64.476,72.142 FB=C-TYPE
PLANE SWC: 10.809, 26.143,33.809,45.309,56.809,64.476,72.142 FB=C-TYPE

Girts ..... 55.0 ksi Yield Strength

Girts Spacings : 3'0 4'4 5'10 3'8 3'11 2'0 3'0

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten	
1	3'0	21.000	8.5Z59	2 points	S 0.000	0.000	S
2	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
3	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
4	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
5	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
6	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
7	3'0	20.000	8.5Z59	2 points	S 0.000	0.000	S
8	3'0	21.000	8.5Z59	2 points	S 0.000	0.000	S
1	7'4	21.000	8.5Z85	2 points	S 0.000	0.000	S
2	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
3	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
4	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
5	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
6	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
7	7'4	20.000	8.5Z70	2 points	S 0.000	0.000	S
8	7'4	21.000	8.5Z85	2 points	S 0.000	0.000	S
1	13'2	21.000	8.5Z70	2 points	S 0.000	0.000	S
2	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
3	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
4	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
5	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
6	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
7	13'2	20.000	8.5Z70	2 points	S 0.000	0.000	S
8	13'2	21.000	8.5Z70	2 points	S 0.000	0.000	S

EWD CONTD..

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	16'10	21.000	8.5Z59	2 points	S 0.000	0.000 S
2	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
7	16'10	20.000	8.5Z70	2 points	S 0.000	0.000 S
8	16'10	21.000	8.5Z59	2 points	S 0.000	0.000 S
1	20'9	21.000	8.5Z59	2 points	S 0.000	0.000 S
8	20'9	21.000	8.5Z59	2 points	S 0.000	0.000 S
2	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
3	22'9	20.000	8.5Z85	2 points	S 0.000	0.000 S
4	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
5	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
6	22'9	20.000	8.5Z85	2 points	S 0.000	0.000 S
7	22'9	20.000	8.5Z70	2 points	S 0.000	0.000 S
4	25'9	20.000	8.5Z59	2 points	S 0.000	0.000 S
5	25'9	20.000	8.5Z59	2 points	S 0.000	0.000 S

All cold-formed connections use A325N bolts.

COLUMNS ----- (1.000" Inset columns)

Col #	Dist. from left	Description Member Size Ident.	Base Elev (ft)	Base plate design information Thickness & bolts
SJ	0.000'	W12X26 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SH	21.000'	W12X14 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SG	41.000'	W10X17 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SF	61.000'	W12X19 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 22.75 elev. FB=C-TYPE		
SE	81.000'	W12X19 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 25.75 elev. FB=C-TYPE		
SD	101.000'	W12X19 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
		Flange Brace @ 7.3, 13.17, 16.83, 22.75 elev. FB=C-TYPE		
SC	121.000'	W10X17 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SB	141.000'	W12X14 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
SA	162.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307

EWD CONTD..

ENDWALL COLUMN TO BRIDGE CHANNEL CONNECTIONS:

COL. NO.	STRUT-TO-COLUMN CLIP ENDWALL FRAME LINE 5
PLANE SWA:	
SH	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SG	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SF	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SE	AT PEAK, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 8.5C105 BRIDGE CHANNEL
SD	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SC	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
SB	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N NO COLUMN EXTENSION 10C105 BRIDGE CHANNEL
PLANE SWC:	

FRAMES -----	Type	Span	Live	Wind	Eave	Trib	Frame Lines
	SMT	162.000	20.00/110.00		22.67/ 32.00		S2-S4

Note: Use square anchor bolt layout.

Star Building Systems
8600 S. I-35, Oklahoma City, OK 73149

FRAME DESCRIPTION: smt 162./22.667/32. 20./110./0.

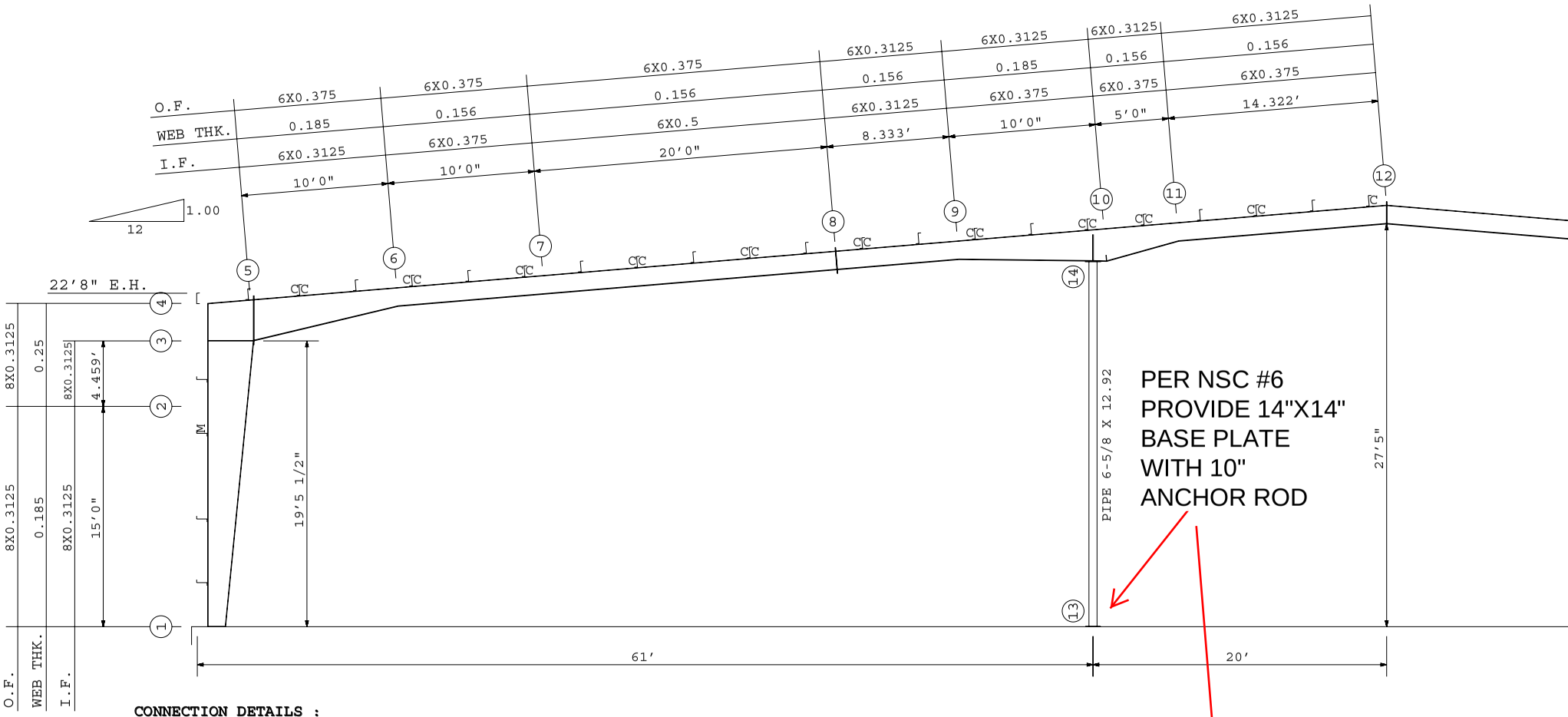
USER NAME: axkhanna **JOB NAME:** 63841A

DATE: 5/ 9/11 **TIME:** 15:14:43 **PAGE:** 2-1
FILE: fra234.fra

DESIGN AND ESTIMATION INFORMATION
WEIGHT: 6886 lbs
BOLTS:A325 SNUG TIGHT
PURLINS(horz. from eave) :8.5"-Z 2@3'5 7/8",19@3'10"
GIRTS (vert. from floor):8.5"-Z 3',4'4",5'10",3'8"(BYPASS)

FRAME ID #02 **LOCATION:** frame lines s2,s3,s4
DETAIL FILE: \ENG\12-B-63841\ver01-axkhanna\Bldg-A\Drftg\x02L

STANDARD NOTES:
(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.



CONNECTION DETAILS :

Location	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Web Dep.	14.0	31.7294	37.0	N/A	33.0	14.5	14.5	14.5	14.5	26.0	14.5	14.5	6.625	6.625
Type	BASE	SPLICE	HORZ STF	CAP (EXT)	4E/4E	SPLICE	SPLICE	3E/3E	SPLICE	PINCH	SPLICE	2E/2E	BASE	CAP/STF
Plate(DN)	8.0X0.5	N/A	3.75X0.3125	8.0X0.375	8.0X0.5	N/A	N/A	6.0X0.5	N/A	2.75X0.25	N/A	6.0X0.5	12.0X0.75	8.0X0.5
Plate(UP)	N/A	N/A	N/A	N/A	6.0X0.5	N/A	N/A	6.0X0.5	N/A	N/A	N/A	6.0X0.5	N/A	2.75X0.375
Bolts	(6)-3/4	N/A	N/A	N/A	(16)-3/4	N/A	N/A	(12)-3/4	N/A	N/A	N/A	(8)-3/4	(4)-3/4	(4)-1/2

3

Star Building Systems
8600 S. I-35, Oklahoma City, OK 73149

FRAME DESCRIPTION: smt 162./22.667/32. 20./110./0.

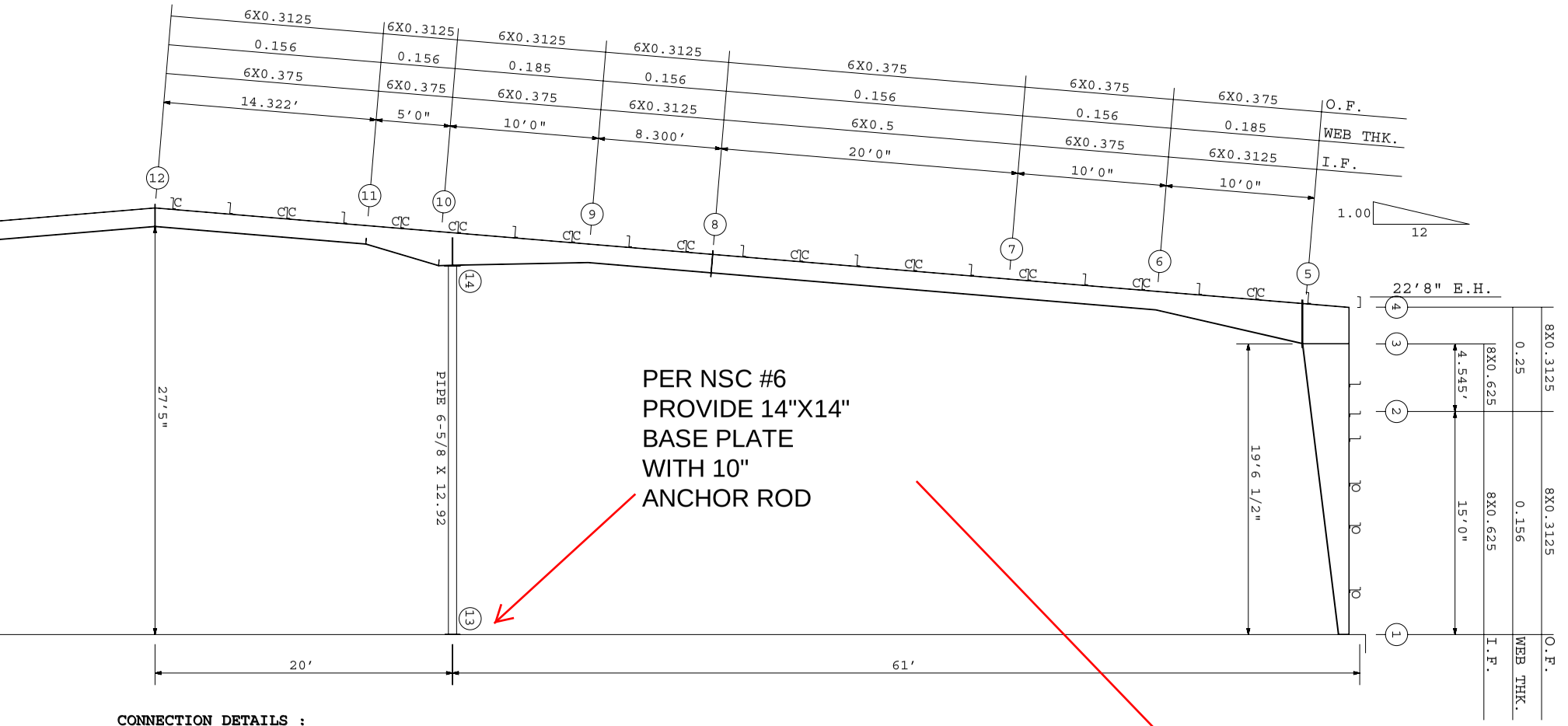
USER NAME: axkhanna **JOB NAME:** 63841A

DATE: 5/ 9/11 **TIME:** 15:14:43 **PAGE:** 2-2
FILE: fra234.fra

DESIGN AND ESTIMATION INFORMATION
WEIGHT: 6886 lbs
BOLTS:A325 SNUG TIGHT
PURLINS(horz. from eave) :8.5"-Z 2@3'5 7/8",19@3'10"
GIRTS (vert. from floor):8.5"-Z 3',4'4",2'10",3',1'8",2'(BYPASS)

FRAME ID #02 **LOCATION:** frame lines s2,s3,s4
DETAIL FILE: \ENG\12-B-63841\ver01-axkhanna\Bldg-A\Drftg\x02R

STANDARD NOTES:
(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.



CONNECTION DETAILS :

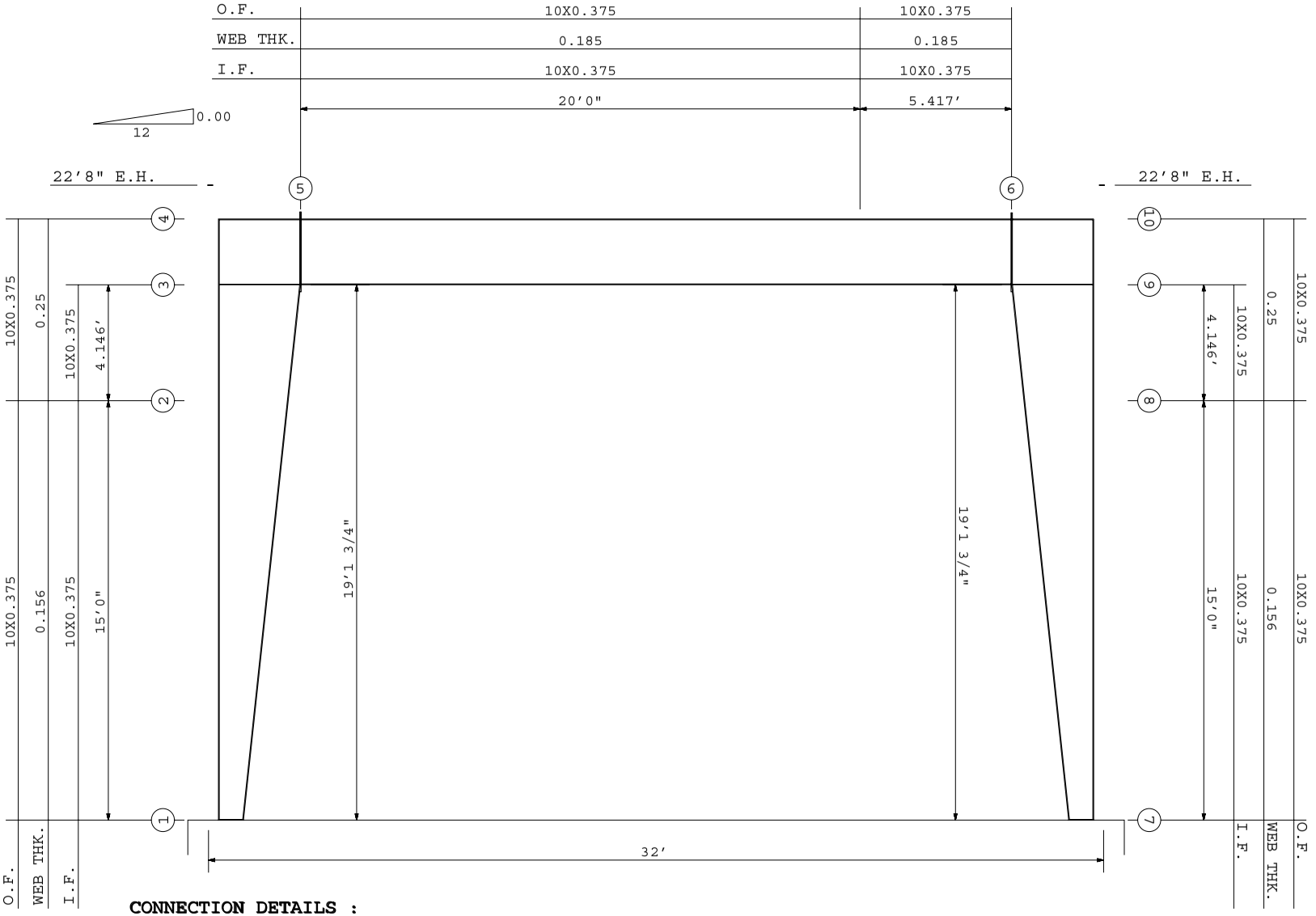
Location	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Web Dep.	8.0	30.2563	37.0	N/A	32.0	14.5	14.5	14.5	14.5	27.0	14.5	14.5	6.625	6.625
Type	BASE	SPLICE	HORZ STF	CAP (EXT)	4E/4E	SPLICE	SPLICE	3E/3E	SPLICE	PINCH	PINCH	2E/2E	BASE	CAP/STF
Plate(DN)	8.0X0.375	N/A	3.75X0.3125	8.0X0.375	8.0X0.625	N/A	N/A	6.0X0.5	N/A	2.75X0.25	2.75X0.25	6.0X0.5	12.0X0.75	8.0X0.5
Plate(UP)	N/A	N/A	N/A	N/A	6.0X0.5	N/A	N/A	6.0X0.5	N/A	N/A	N/A	6.0X0.5	N/A	2.75X0.375
Bolts	(4)-3/4	N/A	N/A	N/A	(16)-3/4	N/A	N/A	(12)-3/4	N/A	N/A	N/A	(8)-3/4	(4)-3/4	(4)-1/2

3

DESIGN AND ESTIMATION INFORMATION
WEIGHT: 3159 lbs
BOLTS:A325 SNUG TIGHT
Rafter Offset : [14"]
Column Offsets : [4.25"]- LEFT COLUMN [4.25"]- RIGHT COLUMN

FRAME ID #04 **LOCATION:** bays 3-(plane SWA)
DETAIL FILE: \ENG\12-B-63841\ver01-axkhanna\Bldg-A\Drftg\x04L

STANDARD NOTES:
(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.



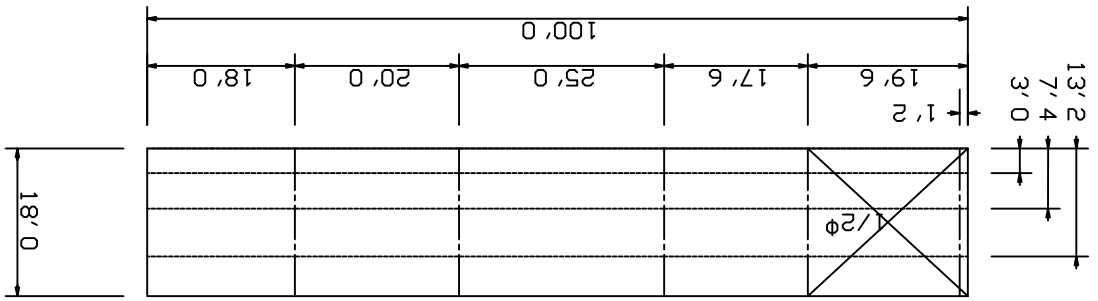
CONNECTION DETAILS :

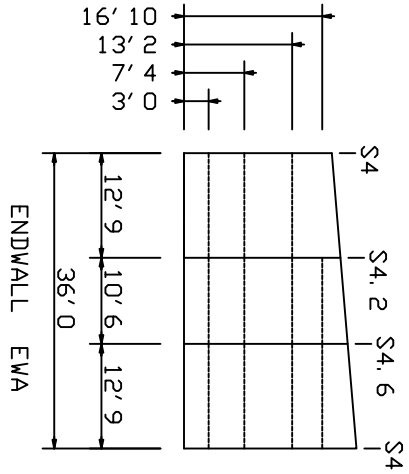
Location	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)
Web Dep.	10.0	29.1944	34.5	N/A	27.5	27.5	10.0	29.1944	34.5	N/A
Type	BASE	SPLICE	HORZ STF	CAP (EXT)	2E/2E	2E/2E	BASE	SPLICE	HORZ STF	CAP (EXT)
Plate(DN)	10.0X0.375	N/A	4.75X0.375	10.0X0.375	10.0X0.5 C	10.0X0.5 C	10.0X0.375	N/A	4.75X0.375	10.0X0.375
Plate(UP)	N/A	N/A	N/A	N/A	10.0X0.5 R	10.0X0.5 R	N/A	N/A	N/A	N/A
Bolts	(4)-3/4	N/A	N/A	N/A	(8)-3/4	(8)-3/4	(4)-3/4	N/A	N/A	N/A

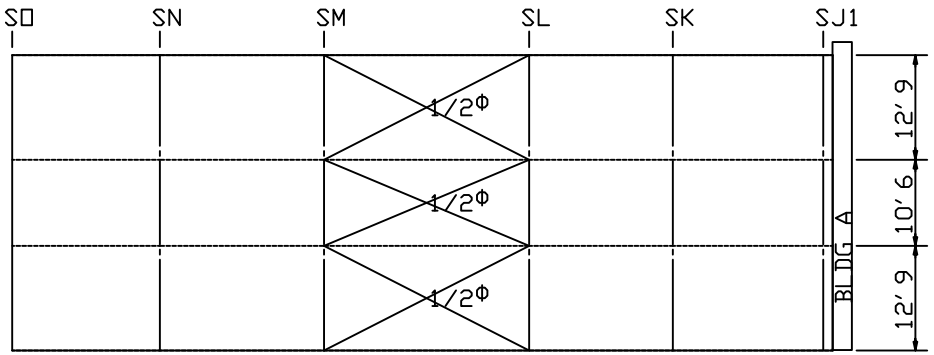
key Strut: x=double Z,
xx=triple Z,
o=pipe(WM)

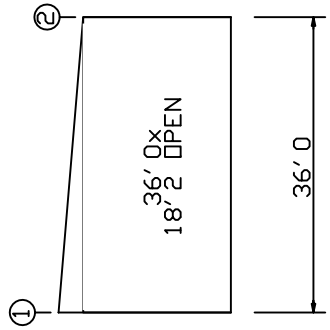
Builder :
Casey Civil, LLC
Job No: 63841B run01
Version: ver01-axkhanna
Wed Jun 22 15:07:54 2011

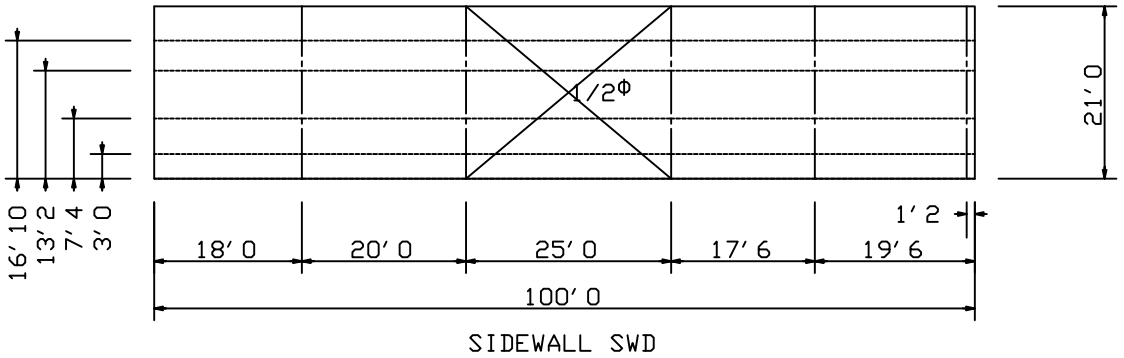
3



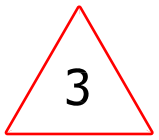








Owner :
Pinnacle Casino - Manhat
Baton Rouge LA 70810



M A I N B U I L D I N G B D E S I G N S U M M A R Y R E P O R T

ROOF PLANE ------- RPB
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-B\run01\BroofRPB_01.edf

BUILDING B

Panel ..... SDR26 ROOF (DURA-RIB)
Purlins ..... 55.0 ksi Yield Strength
Eave Struts ..... 55.0 ksi Yield Strength

PURLIN SPACING : 7@4'9 2'9

NOTE: Provide 5-7-5 screw pattern at all roof panel to purlin attachment locations

Bay #	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	19.500	8.5Z59	2 points	S 0.000	1.083 C
2	17.500	8.5Z59	None	C 1.083	1.583 C
3	25.000	8.5Z59	2 points	C 1.583	2.583 C
4	20.000	8.5Z59	None	C 2.583	1.083 C
5	18.000	8.5Z59	None	C 1.083	0.000 S

Purlin @ Level 2: Washers Req`d @ Frame:1-5
Purlin @ Level 3-8: Washers Req`d @ Frame:1-6

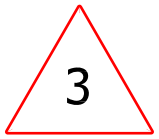
Purlin Anchorage Hdwe @ Level 8 @ Frame Lines:1-6

All cold-formed connections use A325N bolts.

** See Knock-In Bridging Detail

SWB Eave Strut @ 18.000 (ft) : 8.5E70 @ Bays 1-5;
SWD Eave Strut @ 21.000 (ft) : 8.5E70 @ Bays 1-5;

BRACING ---- Roof: 1 bays rods
Plane SWD : 1 bays rods
Plane SWB : 1 bays rods
Plane EWA :Diaphragm
Plane EWC :End Frame



SIDEWALL PLANE SWD -- (8.500" Inset columns)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-B\run01\BwallSWD_01.edf

Panel ..... SDW26 WALL (DURA-RIB)
Girts ..... 55.0 ksi Yield Strength

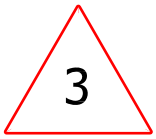
GIRTS SPACINGS : 3'0 4'4 5'10 3'8

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'0	18.000	8.5Z59	None	S 0.000	1.083 C
2	3'0	20.000	8.5Z59	None	C 1.083	1.083 C
3	3'0	25.000	8.5Z59	2 points	C 1.083	1.083 C
4	3'0	17.500	8.5Z59	None	C 1.083	1.083 C
5	3'0	19.500	8.5Z59	None	C 1.083	0.000 S
1	7'4	18.000	8.5Z59	None	S 0.000	1.083 C
2	7'4	20.000	8.5Z59	None	C 1.083	1.583 C
3	7'4	25.000	8.5Z59	2 points	C 1.583	1.083 C
4	7'4	17.500	8.5Z59	None	C 1.083	1.083 C
5	7'4	19.500	8.5Z59	None	C 1.083	0.000 S
1	13'2	18.000	8.5Z59	None	S 0.000	1.083 C
2	13'2	20.000	8.5Z59	None	C 1.083	1.083 C
3	13'2	25.000	8.5Z59	2 points	C 1.083	1.083 C
4	13'2	17.500	8.5Z59	None	C 1.083	1.083 C
5	13'2	19.500	8.5Z59	None	C 1.083	0.000 S
1	16'10	18.000	8.5Z59	None	S 0.000	1.083 C
2	16'10	20.000	8.5Z59	None	C 1.083	1.083 C
3	16'10	25.000	8.5Z59	2 points	C 1.083	1.083 C
4	16'10	17.500	8.5Z59	None	C 1.083	1.083 C
5	16'10	19.500	8.5Z59	None	C 1.083	0.000 S

Girt @ Level 7'4,13'2: Washers Req`d @ Frame:3-4

All cold-formed connections use A325N bolts.

Note : Maximum Distance To Extend Girt From Adjacent Bay is 12.00 inches.



SIDEWALL PLANE SWB -- (8.500" Inset columns)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-B\run01\BwallSWB_01.edf

Panel ..... SDW26 WALL (DURA-RIB)
Girts ..... 55.0 ksi Yield Strength

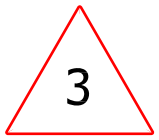
GIRTS SPACINGS : 3'0 4'4 5'10

Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten		R Lap Exten	
1	3'0	19.500	8.5Z59	None	S	0.000	1.083	C
2	3'0	17.500	8.5Z59	None	C	1.083	1.083	C
3	3'0	25.000	8.5Z59	2 points	C	1.083	1.083	C
4	3'0	20.000	8.5Z59	None	C	1.083	1.083	C
5	3'0	18.000	8.5Z59	None	C	1.083	0.000	S
1	7'4	19.500	8.5Z59	None	S	0.000	1.083	C
2	7'4	17.500	8.5Z59	None	C	1.083	1.083	C
3	7'4	25.000	8.5Z59	2 points	C	1.083	1.583	C
4	7'4	20.000	8.5Z59	None	C	1.583	1.083	C
5	7'4	18.000	8.5Z59	None	C	1.083	0.000	S
1	13'2	19.500	8.5Z59	None	S	0.000	1.083	C
2	13'2	17.500	8.5Z59	None	C	1.083	1.083	C
3	13'2	25.000	8.5Z59	2 points	C	1.083	1.083	C
4	13'2	20.000	8.5Z59	None	C	1.083	1.083	C
5	13'2	18.000	8.5Z59	None	C	1.083	0.000	S

Girt @ Level 7'4: Washers Req`d @ Frame:3-4
Girt @ Level 13'2: Washers Req`d @ Frame:2-5

All cold-formed connections use A325N bolts.

Note : Maximum Distance To Extend Girt From Adjacent Bay is 12.00 inches.



Endwall Plane EWA Design ..... Bearing Frame (BF)
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-B\run01\BwallEWA_01.edf

Panel ..... SDW26 WALL (DURA-RIB)

RAFTERS -----

Mem #	Description Member Size Identification	Length (ft)	Start (ft)	End (ft)
1	W8X10 50.0 ksi	33.700	0.000	33.700
	connections... Left: Type-IV SEP		Right: Type-IV SEP	

Type-IV SEP = (2)-1/2" A325N bolts w/ 3/8" Shear End Plate

Flange Braces at following purlins (horizontal distance from eave) :
PLANE SWB: 14.250 FB=C-TYPE

Girts ..... 55.0 ksi Yield Strength

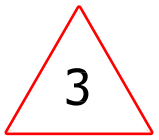
Girts Spacings : 3'0 4'4 5'10 3'8

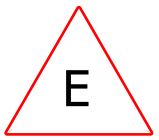
Bay #	Elev. (ft-in)	Length (ft)	Member Size Identification	Brace Locations	L Lap Exten	R Lap Exten
1	3'0	12.750	8.5Z59	None	S 0.000	1.083 C
2	3'0	10.500	8.5Z59	None	C 1.083	1.083 C
3	3'0	12.750	8.5Z59	None	C 1.083	0.000 S
1	7'4	12.750	8.5Z59	None	S 0.000	1.083 C
2	7'4	10.500	8.5Z59	None	C 1.083	1.083 C
3	7'4	12.750	8.5Z59	None	C 1.083	0.000 S
1	13'2	12.750	8.5Z59	None	S 0.000	1.083 C
2	13'2	10.500	8.5Z59	None	C 1.083	1.083 C
3	13'2	12.750	8.5Z59	None	C 1.083	0.000 S
2	16'10	10.500	8.5Z59	None	S 0.188	1.083 C
3	16'10	12.750	8.5Z59	None	C 1.083	0.000 S

All cold-formed connections use A325N bolts.

Note : Maximum distance to extend girt from 'adjacent bay is 12.00 inches.

EWA CONTD..





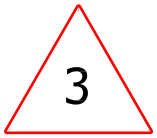
COLUMNS ----- (8.500" Inset columns)

Col #	Dist. from left	Description Member Size Ident.	Base Elev (ft)	Base plate design information Thickness & bolts
S4	0.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307
S4.2	12.750'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.75" A307
		Flange Brace @ 7.3, 16.83 elev.		
S4.6	23.250'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.75" A307
		Flange Brace @ 7.3, 16.83 elev.		
S4	36.000'	W10X12 50.0 ksi	0.0000'	0.375" BP thk w/(4)-0.750" A307

ENDWALL COLUMN TO BRIDGE CHANNEL CONNECTIONS:

COL. NO.	STRUT-TO-COLUMN CLIP ENDWALL FRAME LINE 1
PLANE SWB:	
S4.2	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N W8X10 COLUMN EXTENSION w/ 12.000 " LAP LENGTH; 8.5C105 BRIDGE CHANNEL
S4.6	BETWEEN PURLINS IN ZONE E, TYPE 1 CONN., 2:12 AND UNDER PC64 (0.2500") (4)-1/2" A325N W8X10 COLUMN EXTENSION w/ 12.000 " LAP LENGTH; 8.5C105 BRIDGE CHANNEL
PLANE SWD:	

Endwall Plane EWC Design ..... Non-Expandable Frame (SSB   )
R:\Jobs\Active\ENG\12-B-63841\ver01-axkhanna\Bldg-B\run01\BwallEWC_01.edf



OPEN TO BUILDING A

OPEN AREAS:

Size	Wall	Distance
36'0 x 18'2	EWC	0'0

FRAMES ----- Type	Span	Live	Wind	Eave	Trib	Frame Lines
SSB	36.000	20.00/110.00		21.00/	22.50	SN,SM,SL,SK,SJ1

Note: Use square anchor bolt layout.

Star Building Systems
8600 S. I-35, Oklahoma City, OK 73149

FRAME DESCRIPTION: ssb 36./18./22.5 20./110./0.

USER NAME: axkhanna **JOB NAME:** 63841B

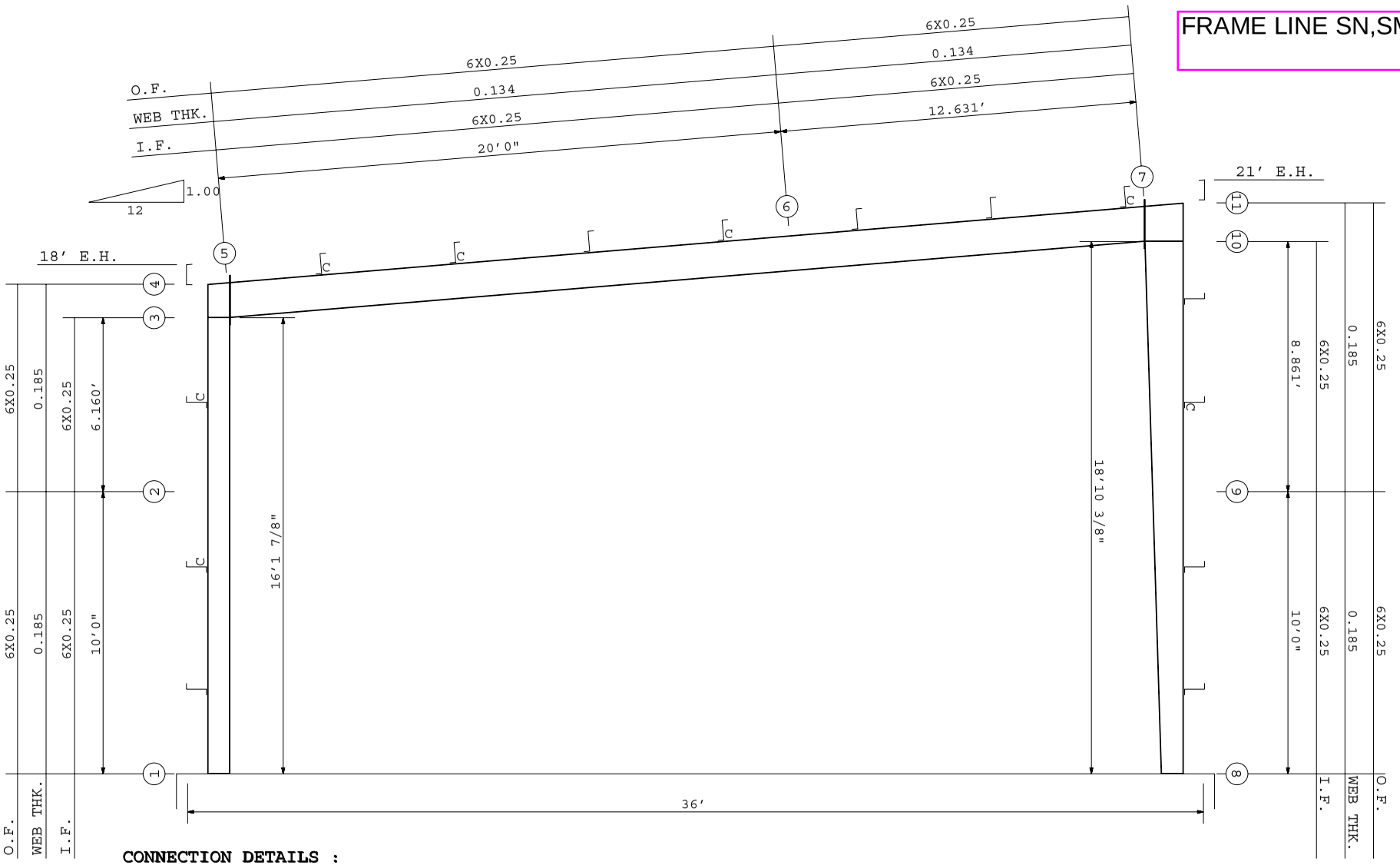
DATE: 6/22/11 **TIME:** 15:40:27 **PAGE:** 2-1
FILE: fra_nmlk.fra

DESIGN AND ESTIMATION INFORMATION
WEIGHT: 1318 lbs
BOLTS:A325 SNUG TIGHT
Rafter Offset :8.5"-Z 7@4'9"(BYPASS)
Column Offsets :8.5"-Z 3',4'4",5'10"(BYPASS) - LEFT COLUMN 8.5"-Z 3',4'4",5'10",3'8"(BYPASS) - RIGHT COLUMN

FRAME ID #02 **LOCATION:** frame lines SN,SM,SL,SK,SJ1
DETAIL FILE: \ENG\12-B-63841\ver01-axkhanna\Bldg-B\Drftg\x02L

STANDARD NOTES:
(1) All sectional dimensions are in inches.
(2) All Flange lengths are measured along outer flange.

FRAME LINE SN,SM,SL,SK,SJ1



CONNECTION DETAILS :

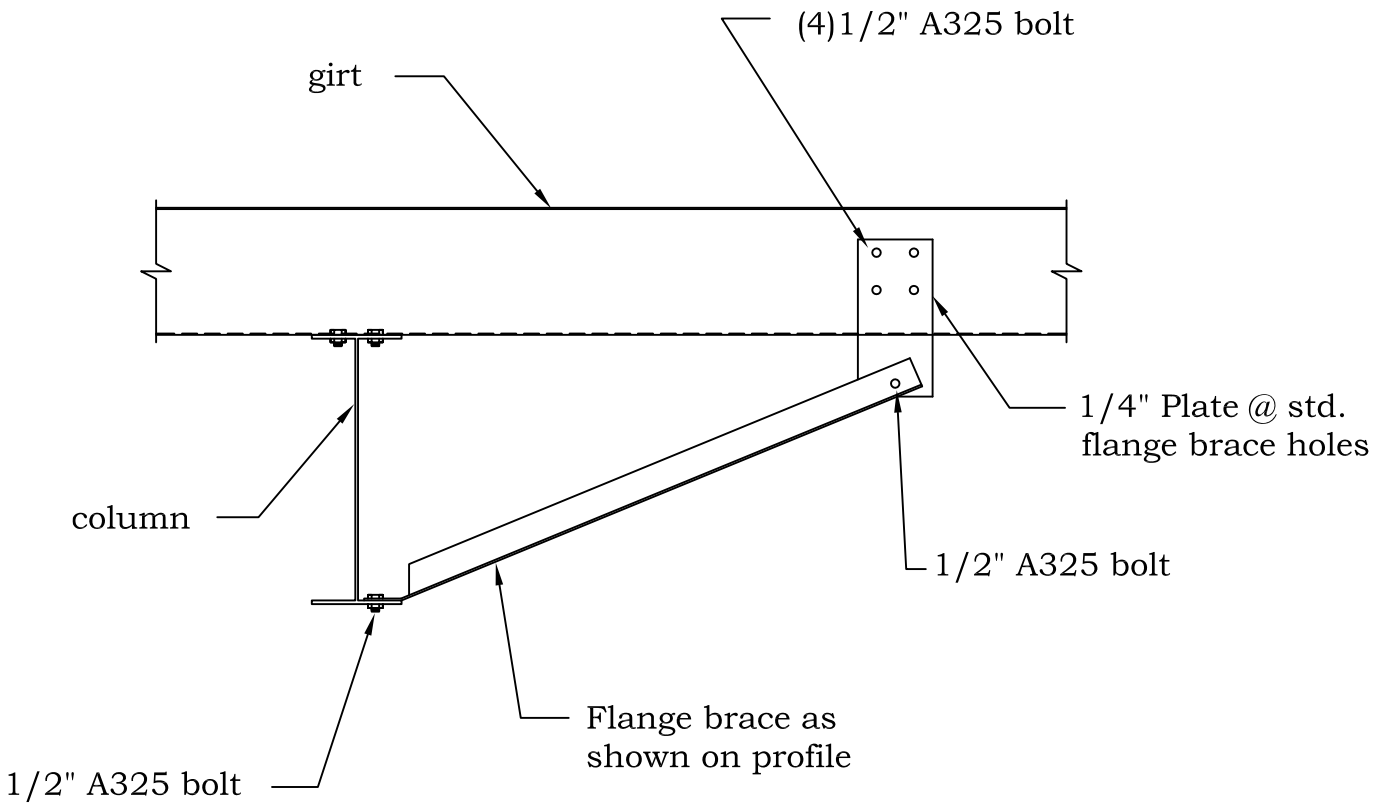
Location	1	2	3	4	5	6	7	8	9	10	11
Web Dep.	9.0	9.0	9.0	N/A	14.5	14.5	14.5	9.0	12.7113	16.0	N/A
Type	BASE	SPLICE	HORZ STF	CAP (EXT)	2E/2E	SPLICE	2E/2E	BASE	SPLICE	HORZ STF	CAP (EXT)
Plate(DN)	6.0X0.375	N/A	2.75X0.25	6.0X0.25	6.0X0.375	N/A	6.0X0.5	6.0X0.375	N/A	2.75X0.25	6.0X0.25
Plate(UP)	N/A	N/A	N/A	N/A	6.0X0.375	N/A	6.0X0.5	N/A	N/A	N/A	N/A
Bolts	(4)-3/4	N/A	N/A	N/A	(8)-3/4	N/A	(8)-3/4	(4)-3/4	N/A	N/A	N/A

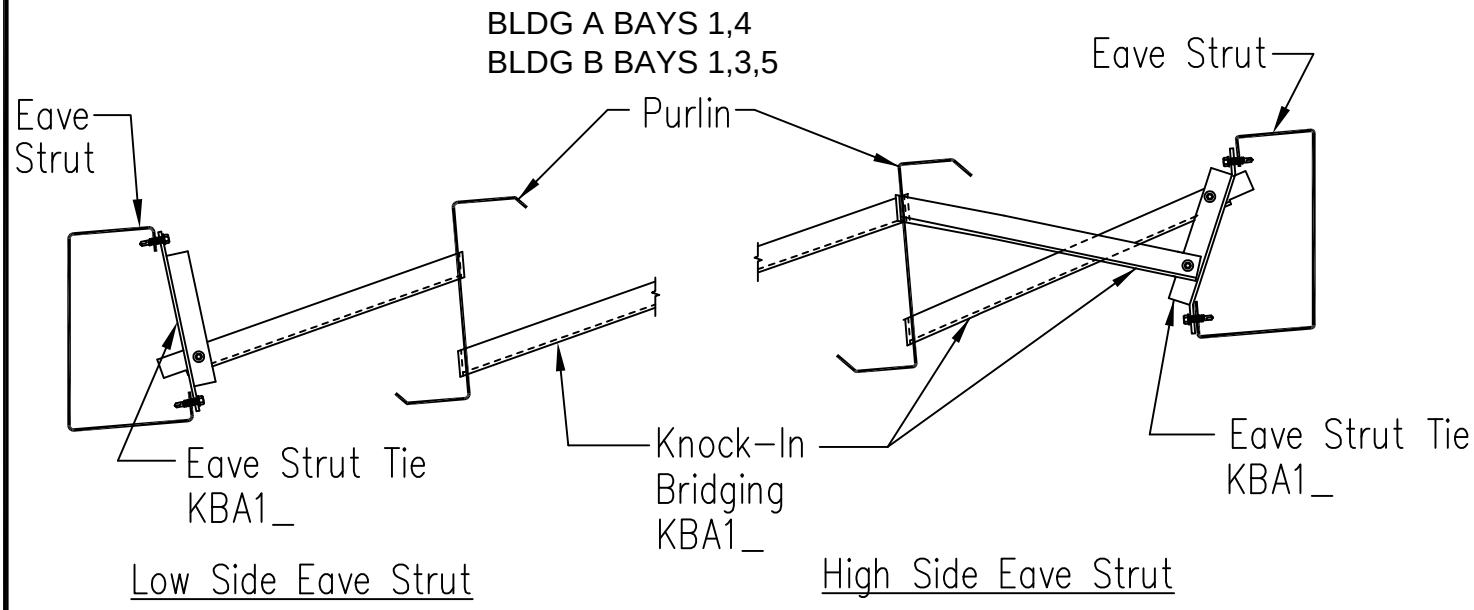
3

ALTERNATE FLANGE BRACE
ATTACHMENT PLATE
BOLTED TO WEB

12-B-63841

BUILDING A,B




BLDG A BAYS 1,4
BLDG B BAYS 1,3,5
Eave Strut
Purlin
Eave Strut
Eave Strut Tie KBA1_
Knock–In Bridging KBA1_
Eave Strut Tie KBA1_
Low Side Eave Strut
High Side Eave Strut

REACTIONS

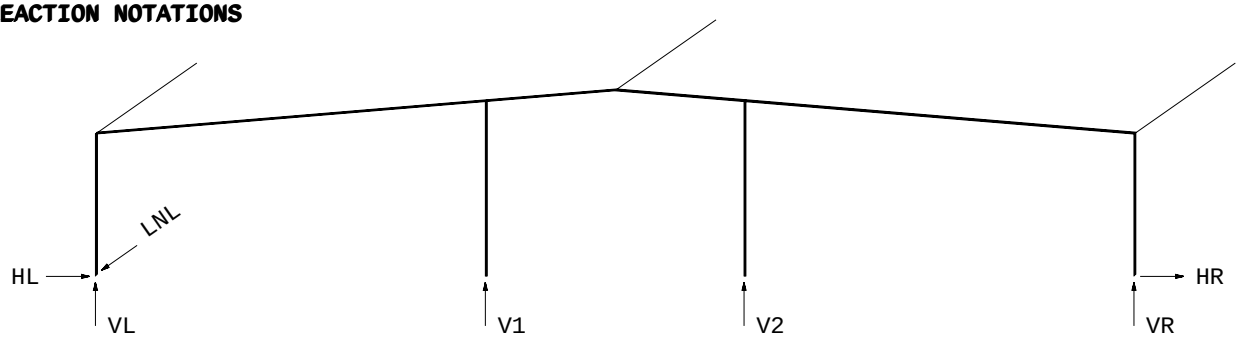
BUILDER: **Casey Civil, LLC**
CUSTOMER: **Pinnacle Casino - Manhattan Construction**
JOB NUMBER: **12-B-63841**

Notes

1) The reactions provided are based on the Order Documents at the time of mailing. Any changes to building loads or dimensions may change the reactions. The reactions will be superseded and voided by any future mailing.
2) The reactions provided have been created with the following layout (unless noted otherwise).
   a) A reaction table is provided with the reactions for each load group.
   b) Rigid Frames
      (1) Gabled Buildings
         (a) Left and Right columns are determined as if viewing the left side of the building, as shown on the anchor rod drawing, from the outside of the building.
         (b) Interior columns are spaced from left side to right side.
      (2) Single Slope Buildings
         (a) Left column is the low side column.
         (b) Right column is the high side column.
         (c) Interior columns are spaced from low side to high side.
   c) Endwalls
      (1) Left and Right columns are determined as if viewing the wall from the outside.
      (2) Interior columns are spaced from left to right.
   d) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer.
   e) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise on the anchor rod layout drawing.
   f) X-Bracing
      (1) Rod Bracing reactions have been included in values shown in the reaction tables.
      (2) For IBC and UBC based building codes, when x-bracing is present in the sidewall, individual longitudinal seismic loads (RBUPEQ and RBDWEQ) do not include the amplification factor, Ω_0.
      (3) For IBC and UBC based building codes, when x-bracing is present in the endwall, individual transverse seismic loads (EQ) do not include the amplification factor, Ω_0.
3) Reactions are provided as un-factored for each load group applied to the column. The foundation engineer will apply the appropriate load factors and combine the reactions in accordance with the building code and design specifications to determine bearing pressures and concrete design. The factors applied to load groups for the steel column design may be different than the factors used in the foundation design.
   Maximum reactions are <u>not</u> provided by the manufacturer to allow the foundation engineer to determine the correct values for his design procedures and allow for an economical foundation design.

SUPPORT REACTIONS FOR EACH LOAD GROUP **FRAME ID #**02 **LOCATION:**frame lines s2,s3,s4
NOTES:(1) All reactions are in kips and kip-ft. **TIME:**15:14:43
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



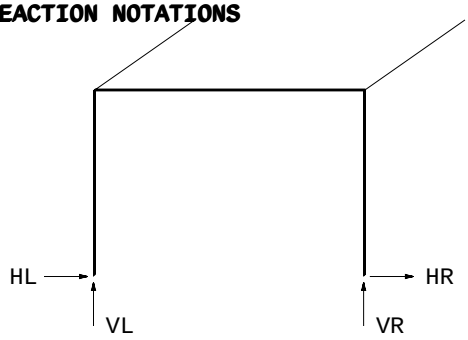
LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN			INTERIOR COLUMN 1			INTERIOR COLUMN 2		
BASE PLATE	8.0X15.0X0.5			8.0X10.0X0.375			12.0X12.0X0.75			12.0X12.0X0.75		
ANC. RODS	(6)-3/4			(4)-3/4			(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR	H1	V1	LN1	H2	V2	LN2
DL	2.3	5.0	0.0	-2.3	5.1	0.0	0.0	5.5	0.0	0.0	5.5	0.0
COLL	0.3	0.6	0.0	-0.3	0.6	0.0	0.0	0.7	0.0	0.0	0.7	0.0
PLL1	4.3	12.3	0.0	-4.3	2.8	0.0	0.0	14.8	0.0	0.0	-6.5	0.0
PLL2	-1.0	-1.0	0.0	1.0	-1.0	0.0	0.0	8.7	0.0	0.0	8.7	0.0
PLL3	4.3	2.8	0.0	-4.3	12.3	0.0	0.0	-6.5	0.0	0.0	14.9	0.0
LL	7.6	14.1	0.0	-7.6	14.0	0.0	0.0	17.0	0.0	0.0	17.1	0.0
RBDWEQ	0.0	0.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
EQ	-0.6	-0.3	0.0	-0.6	0.3	0.0	0.0	0.6	0.0	0.0	-0.6	0.0
RBUPEQ	0.0	-0.9	-1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
WL1	-15.8	-25.2	0.0	4.8	-16.4	0.0	0.0	-23.9	0.0	0.0	-17.1	0.0
WL2	-13.0	-14.5	0.0	2.0	-5.7	0.0	0.0	-12.1	0.0	0.0	-5.2	0.0
WL3	-4.8	-16.4	0.0	15.8	-25.1	0.0	0.0	-16.9	0.0	0.0	-24.1	0.0
WL4	-2.1	-5.7	0.0	13.1	-14.4	0.0	0.0	-5.1	0.0	0.0	-12.1	0.0
LWL1	-9.2	-30.6	0.0	9.7	-26.2	0.0	0.0	-30.1	0.0	0.0	-27.9	0.0
RBUPLW	0.1	-7.0	-9.9	-0.1	0.0	0.0	0.0	0.0	0.0	0.0	-0.1	0.0
LWL2	-9.7	-26.3	0.0	9.2	-30.4	0.0	0.0	-27.7	0.0	0.0	-30.3	0.0
LWL3	-3.9	-18.5	0.0	4.1	-16.7	0.0	0.0	-18.5	0.0	0.0	-17.7	0.0
LWL4	-4.1	-16.8	0.0	3.9	-18.4	0.0	0.0	-17.5	0.0	0.0	-18.7	0.0
RBDWLW	0.0	7.0	0.0	0.0	0.0	0.0	0.0	-0.1	0.0	0.0	0.1	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
COLL : Roof Collateral Load
PLL1 : Pattern Live Load [PLLxx]
PLL2 : Pattern Live Load [PLLxx]
PLL3 : Pattern Live Load [PLLxx]
LL : Roof Live Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load
WL3 : Lateral Primary Wind Load
WL4 : Lateral Primary Wind Load
LWL1 : Longitudinal Primary Wind Load
RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind
LWL2 : Longitudinal Primary Wind Load
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	10.0X11.0X0.375			10.0X11.0X0.375		
ANC. RODS	(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.2	1.4	0.0	-0.2	1.4	0.0
EQ	-1.3	-1.9	0.0	-1.4	1.9	0.0
WL1	-9.5	-14.0	0.0	-10.3	14.0	0.0
WL2	10.3	14.0	0.0	9.5	-14.0	0.0

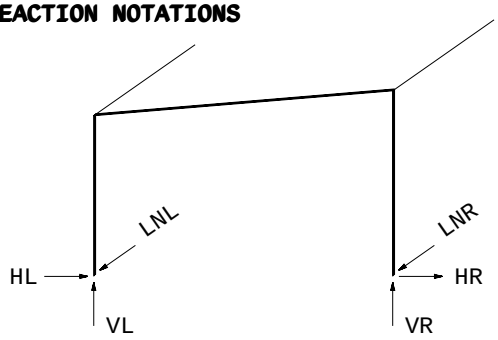
LOAD GROUP DESCRIPTION

DL : Roof Dead Load
EQ : Lateral Seismic Load [parallel to plane of frame]
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load

SUPPORT REACTIONS FOR EACH LOAD GROUP **FRAME ID #**02 **LOCATION:**frame lines SN,SM,SL,SK

NOTES:(1) All reactions are in kips and kip-ft.
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



FRAME LINE SN,SM,SL,SK,SJ1

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	6.0X9.5X0.375			6.0X10.0X0.375		
ANC. RODS	(4)-3/4			(4)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.2	1.4	0.0	-0.2	1.5	0.0
LL	0.7	4.6	0.0	-0.7	4.6	0.0
COLL	0.0	0.2	0.0	0.0	0.2	0.0
RBDWEQ	0.0	0.3	0.0	0.0	0.3	0.0
EQ	-0.1	-0.1	0.0	-0.1	0.1	0.0
RBUPEQ	0.0	-0.3	-0.4	0.0	-0.3	-0.4
WL1	-3.4	-8.8	0.0	-2.7	-3.8	0.0
WL2	-4.4	-5.5	0.0	-1.7	-0.7	0.0
WL3	3.3	-2.9	0.0	4.6	-9.7	0.0
WL4	2.3	0.4	0.0	5.6	-6.6	0.0
LWL1	1.6	-8.7	0.0	-1.1	-8.0	0.0
RBUPLW	0.0	-3.1	-3.2	0.0	-2.8	-3.4
LWL2	1.6	-7.6	0.0	-1.1	-9.2	0.0
LWL3	1.8	-5.5	0.0	-1.9	-4.8	0.0
LWL4	1.8	-5.0	0.0	-1.9	-5.3	0.0
RBDWLW	0.0	3.1	0.0	0.0	2.8	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LL : Roof Live Load
COLL : Roof Collateral Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load
WL3 : Lateral Primary Wind Load
WL4 : Lateral Primary Wind Load
LWL1 : Longitudinal Primary Wind Load
RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind
LWL2 : Longitudinal Primary Wind Load
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind