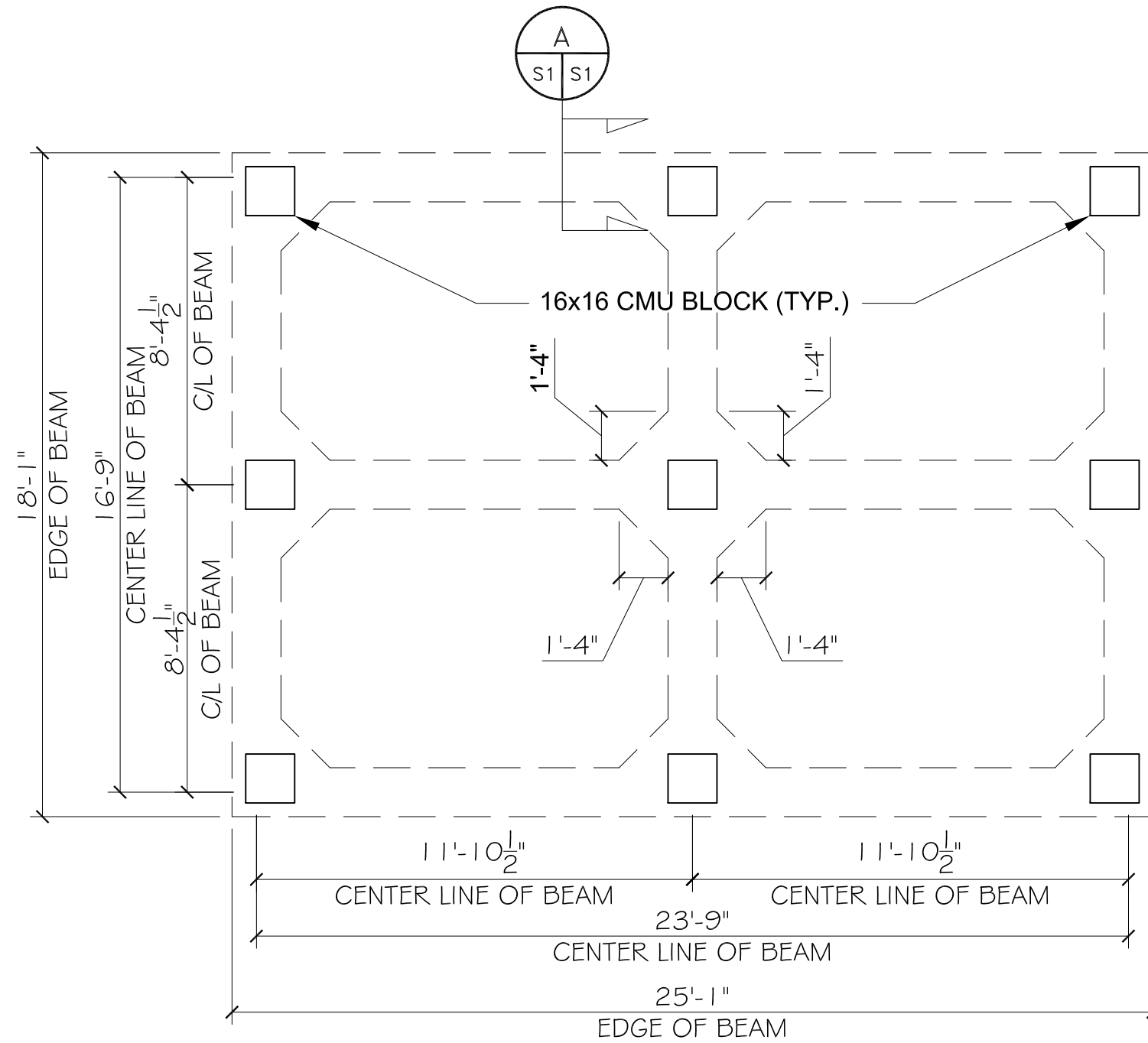


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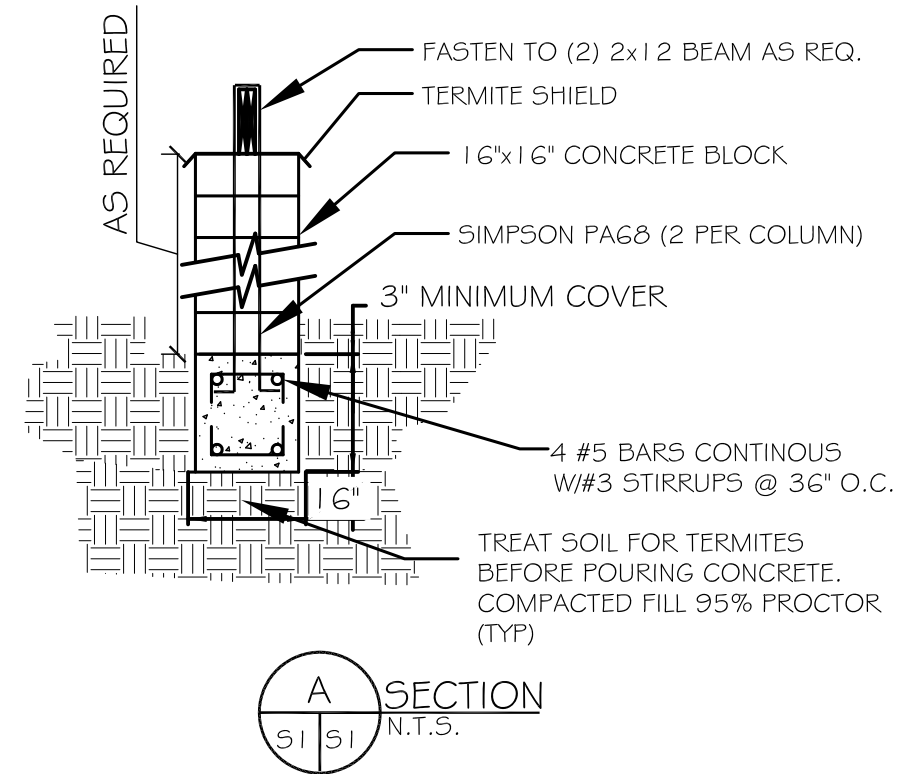


FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

GENERAL FOUNDATION NOTES

1. ALL DIMENSIONS ARE EDGE OF CONCRETE (EOC) TO EDGE OF CONCRETE (EOC) UNLESS NOTED OTHERWISE.
2. VERIFY ALL PLUMBING ROUGH-IN LOCATIONS ON ARCHITECTURAL DWGS.
3. CONCRETE MIX SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONCRETE MIX SHALL BE IN ACCORDANCE WITH ACI-318.
4. ALL CONVENTIONAL REINFORCING STEEL SHALL MEET ASTM-A615 (GRADE 60).
5. ALL REINFORCING STEEL AND MESH SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING CONCRETE PLACEMENT.
6. THE CONTRACTOR SHALL VERIFY ALL DROPS, OFFSETS, BRICK LEDGES, DIMENSIONS AND CONFIGURATIONS. CONTRACTOR MUST BE RESPONSIBLE FOR SAME.
7. GRADE BEAM DIMENSIONS MAY VARY BY -5%, +20%.
8. ALL SOIL BELOW SLAB SHALL RECEIVE TERMITE TREATMENT.



NOTES:

1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS & DEPRESSIONS PRIOR TO COMMENCING WORK.
2. CONTRACTOR SHALL REMOVE ANY EXPANSIVE SOIL OR DELETERIOUS MATERIAL IN THE AREA WHERE THE GRADE BEAMS ARE TO BE CONSTRUCTED. CONTRACTOR SHALL REPLACE SOILS WITH STRUCTURAL FILL IN 6" LIFTS AND COMPACTED TO 95% PROCTOR. SUITABLE FILL MATERIAL COULD CONSIST OF A RED CLAY-SAND TYPE MATERIAL HAVING LESS THAN 30 PERCENT FINES PASSING THE NO. 200 SIEVE.
3. TREAT SOIL WITHIN 4 FEET OF GRADE BEAM FOR TERMITES.
4. CONCRETE MIX SHALL BE 3000 PSI. MINIMUM.
5. REINFORCEMENT BARS SHALL BE GRADE 60.

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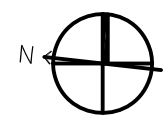
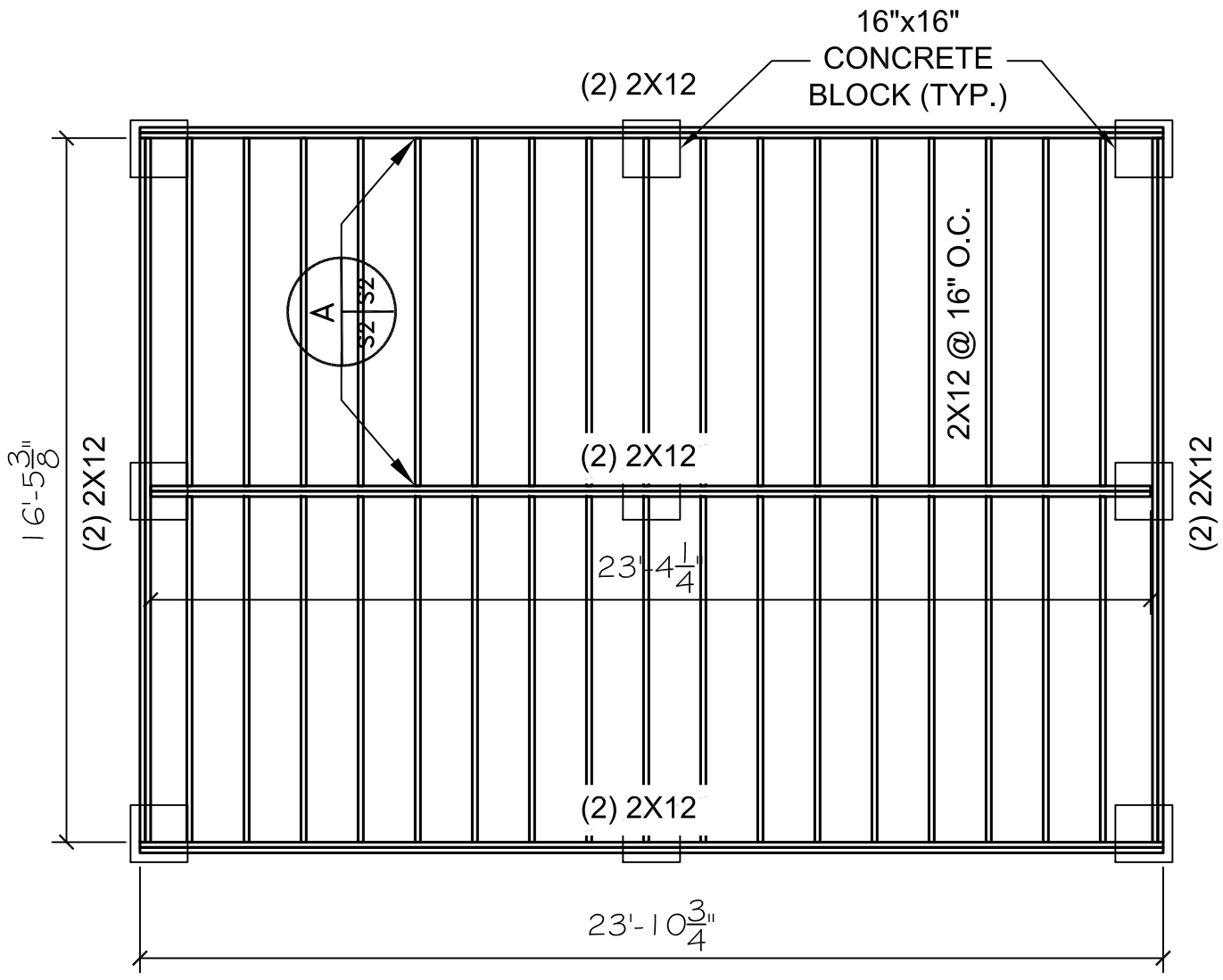
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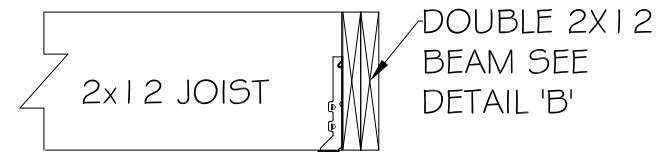
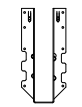
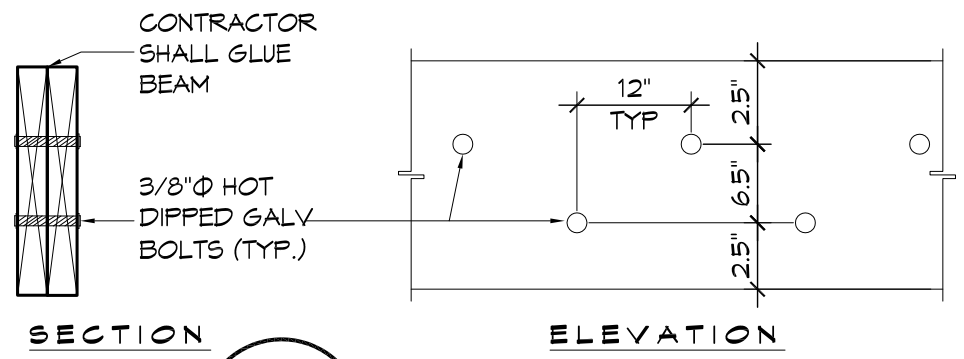
PAGE : 1 OF 3

EXISTING RESIDENCE THIS SIDE



SUB-FLOOR PLAN

SCALE: 1/4" = 1'-0"



LUS 210 SIMPSON JOIST HANGER



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JOB No: _____
 DATE: _____

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S-2
 PAGE : 2 OF 3

UPLIFT CONNECTIONS-130MPH WINDS EXPOSURE "C"

CONNECTION	FRAMING SPACING (in.)	ROOF SPAN (ft.)	U	L	S	NUM. OF 8d COM. NAILS OR 10d BOX NAILS IN EA. END OF 1-1/4"X20 GA. STRAP
ROOF ASSEMBLY TO WALL ASSEMBLY	16" O.C.	17	386	246	109R	4
WALL ASSEMBLY TO WALL ASSEMBLY	16" O.C.	17	386	246	109R	4
WALL ASSEMBLY TO FOUNDATION	16" O.C.	17	170	185	436	4

THERMAL COMPONENT CRITERIA (U-FACTOR AND R-VALUE)

MAX. GLAZING U-FACTOR	MINIMUM INSULATION R-VALUE				
	CEILINGS	WALLS	FLOORS	BASEMENT WALLS	CRAWL SPACE WALLS
.75	R-26	R-13	R-11	R-5	R-5

WINDBORNE DEBRIS PROTECTION FASTENING SCHEDULE FOR WOOD STRUCTURAL PANELS

FASTENER TYPE	FASTENER SPACING		
	PANEL SPAN ≤ 4 FOOT	4 FOOT PANEL SPAN ≤ 6 FOOT	6 FOOT PANEL SPAN ≤ 8 FOOT
2-1/2" #6 WOOD SCREWS	16"	12"	9"
2-1/2" #8 WOOD SCREWS	16"	16"	12"

WINDOWS IN BUILDINGS LOCATED IN WIND BORNE DEBRIS REGIONS SHALL HAVE GLAZED OPENINGS PROTECTED FROM WINDBORNE DEBRIS. WOOD STRUCTURAL PANELS WITH A MIN. THICKNESS OF 7/16" AND A MAX. SPAN OF 8 FEET SHALL BE PERMITTED FOR OPENING PROTECTION IN ONE AND TWO STORY BUILDINGS. PANELS SHALL BE PRECUT TO COVER THE GLAZED OPENINGS WITH ATTACHMENT HARDWARE PROVIDED.

JACK STUD REQUIREMENTS - INT LOADBEARING WALLS

HEADER SUPPORTING	HEADER SPAN (ft.)	ROOF SPAN (ft.)															
		12 FEET				24 FEET				36 FEET							
		3'	4.5'	5'	6.5'	3'	4.5'	5'	6.5'	3'	4.5'	5'	6.5'				
ROOF AND CEILING	2	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
	4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
	6	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
	8	1	1	1	1	2	1	1	1	2	2	2	2	1	1	1	1
	10	1	1	1	1	2	2	1	1	3	2	2	2	2	2	2	2
	12	1	1	1	1	2	2	2	1	3	2	2	2	2	2	2	2
	14	2	1	1	1	3	2	2	2	4	3	3	2	2	2	2	2
	16	2	1	1	1	3	2	2	2	4	3	3	2	2	2	2	2
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10	2	2	2	1	4	3	3	2	6	4	4	3	2	2	2	2	
12	3	2	2	2	5	3	3	3	7	5	4	4	3	2	2	2	
14	3	2	2	2	5	4	3	3	8	5	5	4	3	2	2	2	
16	4	3	2	2	6	4	4	3	9	6	5	5	4	3	2	2	

HEADER WIDTH-3' (2-2X), 4.5' (3-2X), 5' (3-2X), 6.5' (4-2X) EACH W/ 1/2" PLYWD. SPACER BETWEEN

SILL OR BOTTOM PLATE TO FND CONNECTIONS RESISTING UPLIFT LOADS - 130MPH WINDS EXP "B"

BOTTOM PLATE TO FND. ANCHOR BOLT CONNECTION RESISTING	FOUNDATION SUPPORTING	MAX. ANCHOR BOLT SPACING (in.)	
		8' END ZONES	INTERIOR ZONES
UPLIFT LOADS	1-3 STORIES	28	33

SILL OR BOTTOM PLATE TO FND CONNECTIONS RESISTING SHEAR LOADS-130MPH WINDS EXP "B"

BOTTOM PLATE TO FND. ANCHOR BOLT CONNECTION RESISTING	FOUNDATION SUPPORTING	MAX. ANCHOR BOLT SPACING (in.)	
		1/2" ANG. BOLTS	5/8" ANG. BOLTS
SHEAR LOADS	1-3 STORIES	30	45

HEADER SPANS - INTERIOR LOADBEARING WALLS

HEADER SUPPORTING	SIZE	BLDG. WIDTH (ft.)			
		12	24	36	
		SPANS (ft.-in.)			
ONE FLOOR (CENTER BEARING)	(2)2x4S	4'-4"	3'-1"	2'-6"	
	(2)2x6S	6'-5"	4'-6"	3'-8"	
	(2)2x8S	8'-11"	5'-9"	4'-8"	
	(2)2x10S	9'-11"	7'-0"	5'-9"	
	(2)2x12S	11'-6"	8'-11"	6'-7"	
	(3)2x8S	10'-2"	7'-2"	5'-10"	
	(3)2x10S	12'-5"	8'-9"	7'-2"	
	(3)2x12S	14'-4"	10'-2"	8'-3"	
	(4)2x8S	11'-6"	8'-3"	6'-9"	
	(4)2x10S	14'-4"	10'-1"	8'-3"	
2 FLOORS ONLY (CENTER BEARING)	(2)2x4S	2'-10"	2'-1"	1'-8"	
	(2)2x6S	4'-2"	3'-1"	2'-6"	
	(2)2x8S	5'-4"	3'-11"	3'-3"	
	(2)2x10S	6'-6"	4'-9"	3'-11"	
	(2)2x12S	7'-6"	5'-6"	4'-7"	
	(3)2x8S	6'-8"	4'-10"	4'-0"	
	(3)2x10S	8'-11"	6'-0"	4'-11"	
	(3)2x12S	9'-5"	6'-11"	5'-9"	
	(4)2x8S	7'-8"	5'-8"	4'-8"	
	(4)2x10S	9'-4"	6'-10"	5'-8"	
(4)2x12S	10'-10"	8'-0"	6'-7"		

* MAX. SPAN EXCEEDS 16' (SPANS LIM. TO 16')

NOTE: ALL HEADERS SHALL HAVE SOLID BLOCKING

ROOF SHEATH OR CLAD REQ - WIND LOAD EXP "C"

SHEATHING LOCATION	RAFTER/TRUSS SPAC	E		F	
		MAX. NAIL SPAC. FOR 8d COM. NAILS OR 10d BOX NAILS (INCHES, O.C.)		MAX. NAIL SPAC. FOR 8d COM. NAILS OR 10d BOX NAILS (INCHES, O.C.)	
INTERIOR ZONE	12" O.C.	6	12	6	12
	16" O.C.	6	12	6	12
	24" O.C.	6	12	6	12
PERIMETER EDGE ZONE	12" O.C.	6	12	6	12
	16" O.C.	6	12	6	12
	24" O.C.	6	12	6	12

130 MPH WINDS-EXPOSURE 'C' (TYP.)

HEADER SPANS-EXPOSURE C FOR EXTERIOR LOADBEARING WALLS

HEADER SIZE	SPAN	NUMBER FULL HEIGHT STUDS REQ AT EA END
(2)2x4S	4'-7"	2
(2)2x6S	5'-6"	2
(2)2x8S	6'-1"	3
(2)2x10S	6'-8"	3
(2)2x12S	7'-1"	3
(3)2x8S	7'-5"	3
(3)2x10S	8'-3"	3
(3)2x12S	8'-8"	3
(4)2x8S	8'-7"	3
(4)2x10S	9'-6"	3
(4)2x12S	10'-2"	4

130 MPH WINDS-EXPOSURE 'C' (TYP.) EACH W/ 1/2" PLYWD. SPACER BETWEEN

NOTE: 1. BLDG. WIDTH IS MEASURED PERPENDICULAR TO THE RIDGE. FOR WIDTHS BETWEEN THOSE SHOWN, SPANS ARE PERMITTED TO BE INTERPOLATED. 2. ALL HEADERS SHALL HAVE SOLID BLOCKING.

JACK STUD REQ - EXP "C" FOR EXT LOADBEARING WALLS

HEADER SUPPORTING	HEADER SPAN (ft.)	HEADER WIDTH			
		3'	4.5'	5'	6.5'
ROOF AND CEILING	2	1	1	1	1
	4	1	1	1	1
	6	2	1	2	2
	8	2	2	2	2
	10	3	2	2	2
	12	3	2	2	2
	14	4	3	2	2
	16	4	3	3	2
	2	1	1	1	1
	4	2	1	1	1
ROOF, CEILING, AND 1 CENTER BEARING WALL	6	2	2	2	1
	8	3	2	2	3
	10	4	3	2	2
	12	4	3	3	2
	14	5	3	3	3
	16	5	4	3	3

HEADER WIDTH-3' (2-2X), 4.5' (3-2X), 5' (3-2X), 6.5' (4-2X) EACH W/ 1/2" PLYWD. SPACER BETWEEN

WALL SHEATH OR CLAD REQ FOR WIND LOAD-EXPOSURE "C"

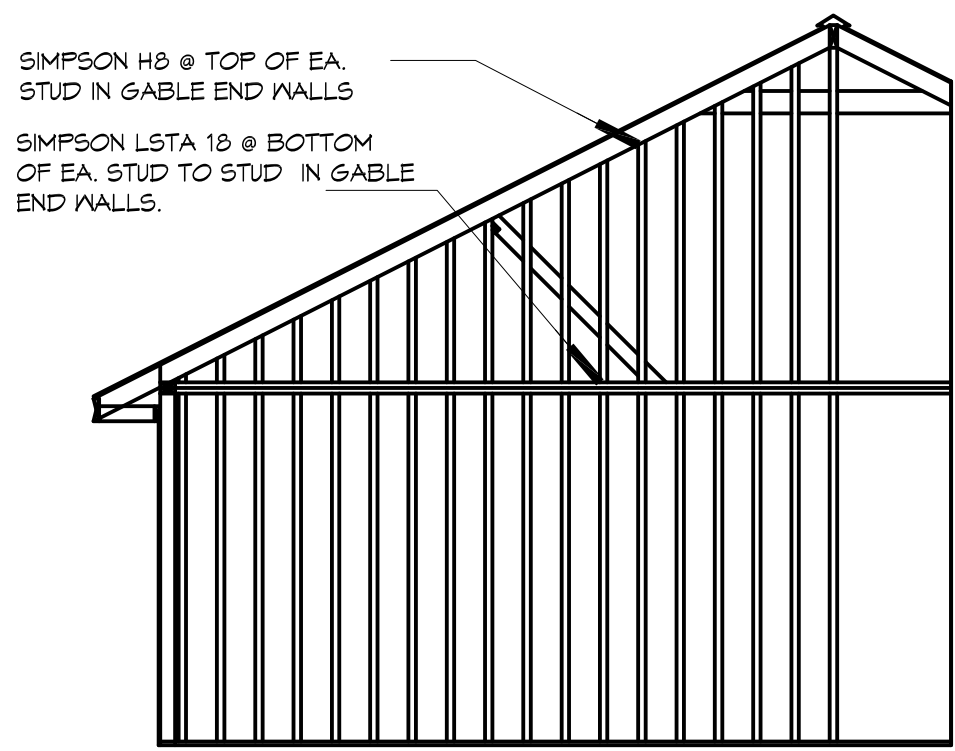
SHEATHING LOCATION	STUD SPACING	E		F	
		MAX NAIL SPACING FOR 8d COMMON NAILS OR 10d BOX NAILS (INCHES, O.C.)		MAX NAIL SPACING FOR 8d COMMON NAILS OR 10d BOX NAILS (INCHES, O.C.)	
INTERIOR ZONE	12" O.C.	6	12	6	12
	16" O.C.	6	12	6	12
	24" O.C.	6	12	6	12
PERIMETER EDGE ZONE	12" O.C.	6	12	6	12
	16" O.C.	6	12	6	12
	24" O.C.	6	12	6	12

130 MPH WINDS-EXPOSURE 'C' (TYP.)

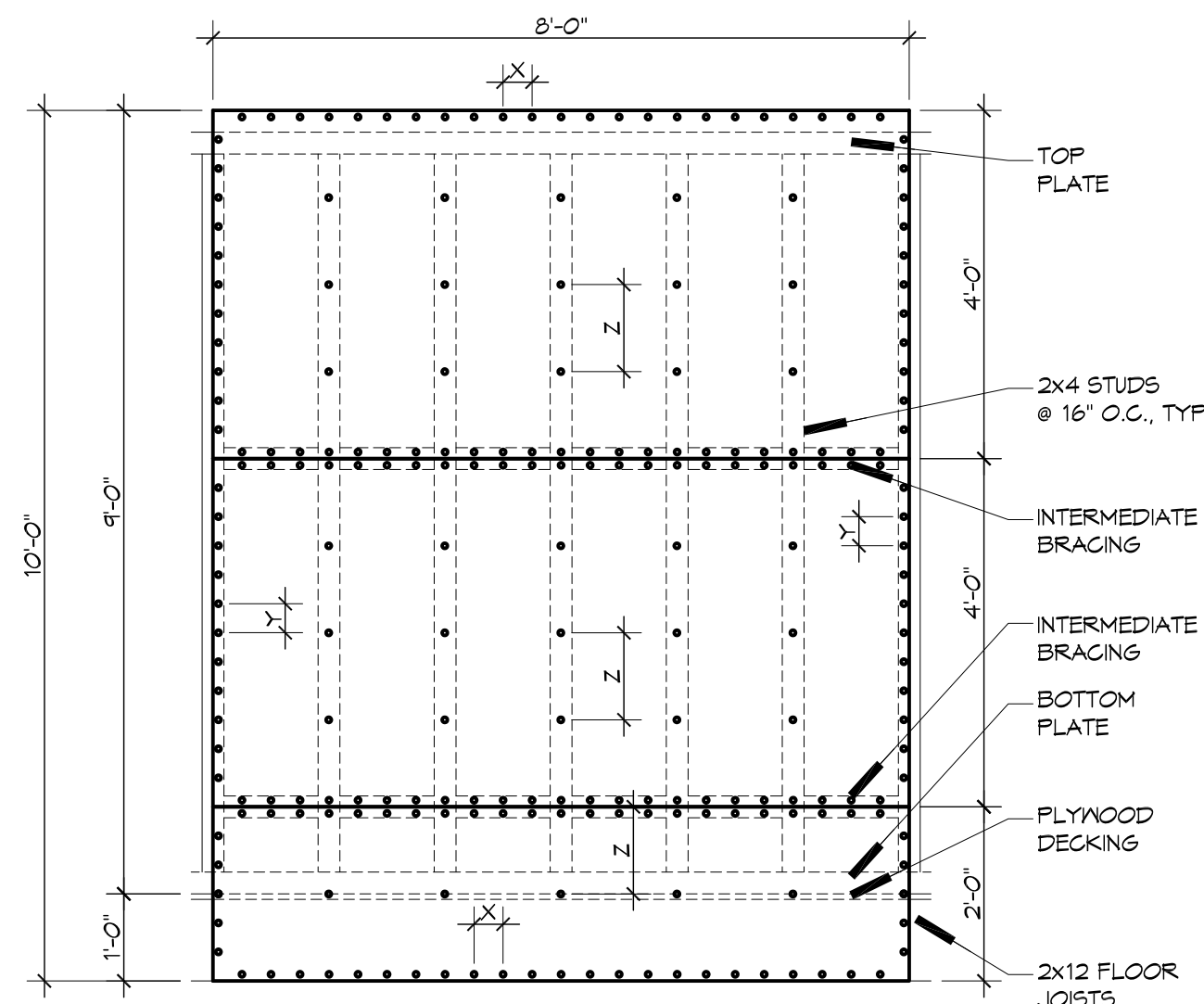
HEADER NAILING SCHEDULE

DESCRIPTION	NUM. OF COM. NAILS	NUM. OF BOX NAILS	SPACING
HEAD TO HEAD, (FACE-NAILED)	8d	10d	6" O.C. EDGES/ 12" O.C. FIELD

NOTE: ALL HEADERS SHALL HAVE SOLID BLOCKING



END WALL STRAPPING



DETAIL SCALE: NTS TYP. EXTERIOR SHEATHING NAILING PATTERN

ROOF UNDERLAYMENT APPLICATION

FOR ROOF SLOPES FROM TWO UNITS VERTICAL IN 12 UNITS HORIZONTAL (17-PERCENT SLOPE), UP TO FOUR UNITS VERTICAL IN 12 UNITS HORIZ. (33-PERCENT SLOPE), UNDERLAYMENT SHALL BE TWO LAYERS APPLIED IN THE FOLLOWING MANNER:

APPLY A 19 INCH STRIP OF UNDERLAYMENT FELT PARALLEL WITH AND STARTING AT THE EAVES, FASTENED SUFFICIENTLY TO HOLD IN PLACE. STARTING AT THE EAVE, APPLY 36 INCH WIDE SHEETS OF UNDERLAYMENT, OVERLAPPING SUCCESSIVE SHEETS 19 INCHES, AND FASTENED SUFFICIENTLY TO HOLD IN PLACE.

FOR ROOF SLOPES OF FOUR UNITS VERTICAL IN 12 UNITS HORIZONTAL (33-PERCENT SLOPE) OR GREATER, UNDERLAYMENT SHALL BE ONE LAYER APPLIED IN THE FOLLOWING MANNER:

UNDERLAYMENT SHALL BE APPLIED SHINGLE FASHION, PARALLEL TO AND STARTING FROM THE EAVE AND LAPPED 2 INCHES, FASTENED SUFFICIENTLY TO HOLD IN PLACE. END LAPS SHALL BE OFFSET BY 6 FEET.

SHINGLE APPLICATION/FASTENING

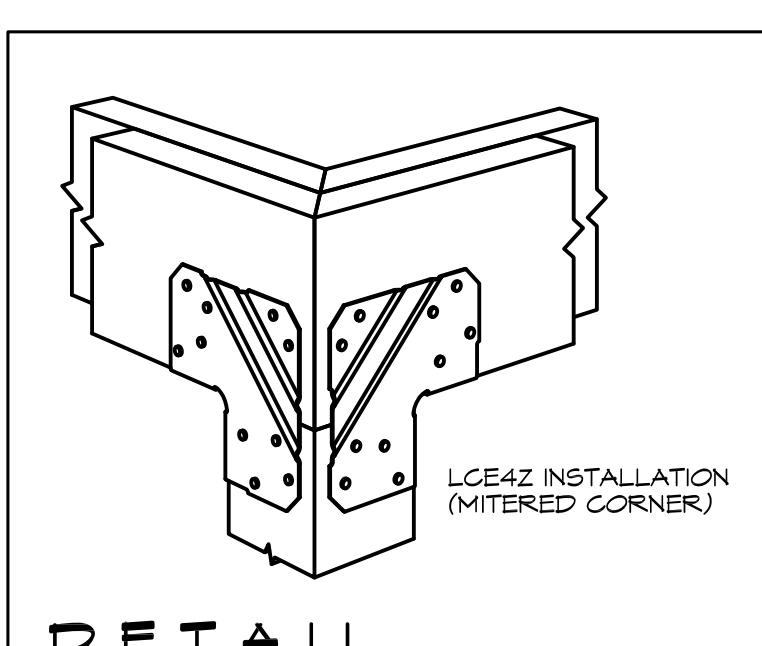
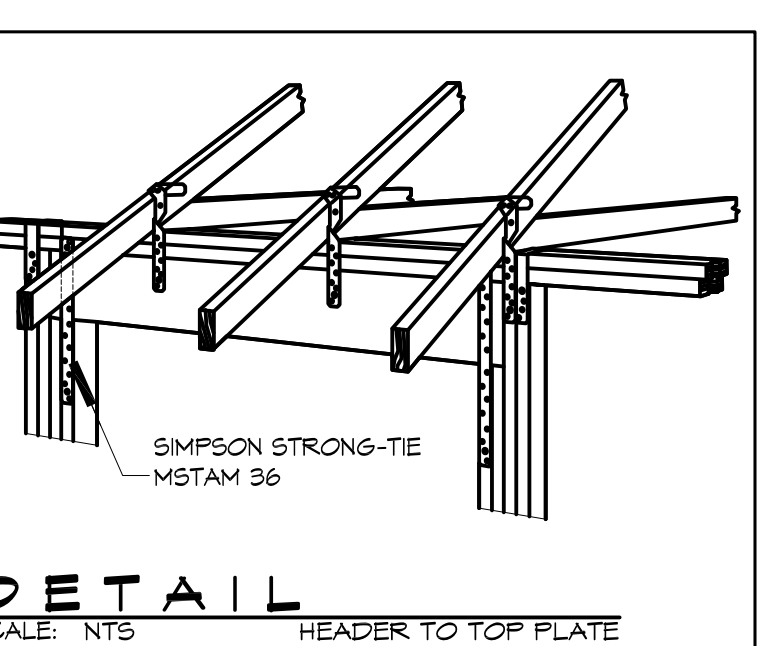
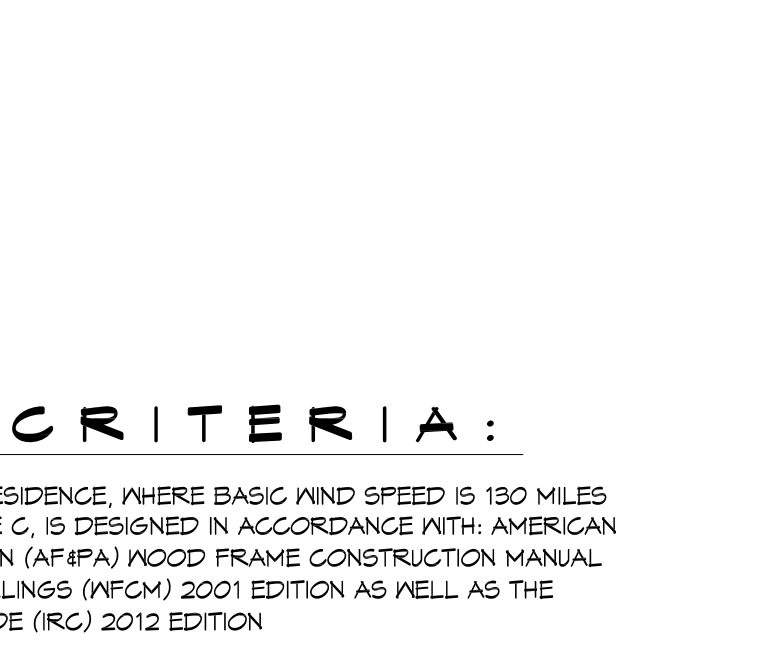
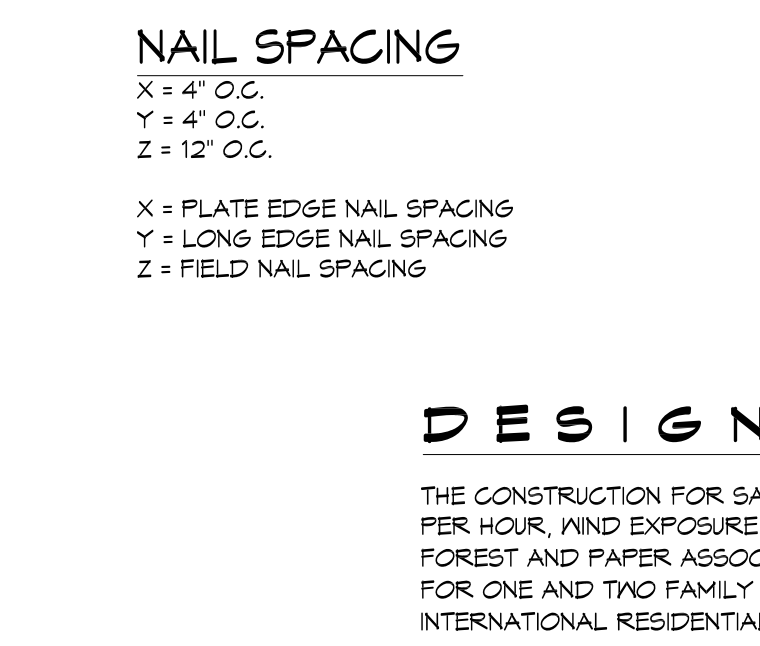
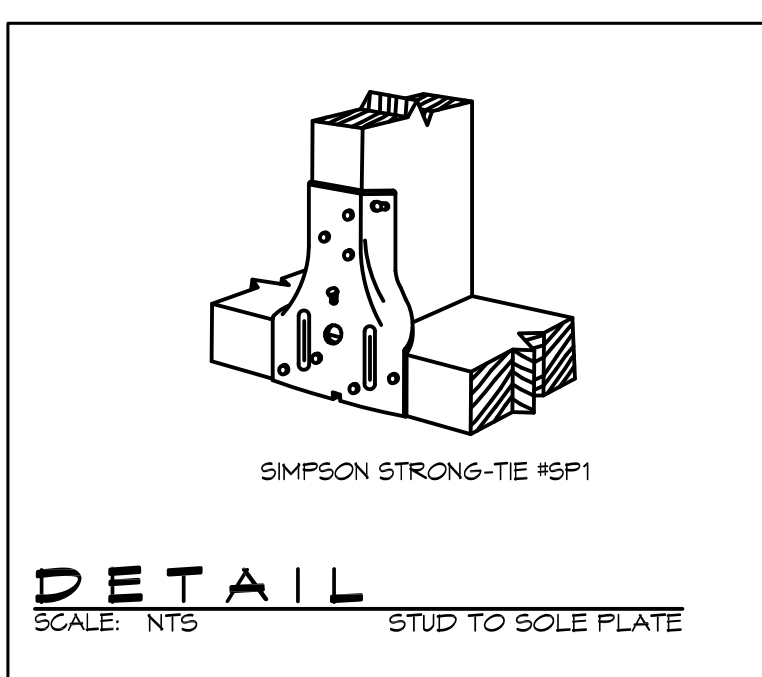
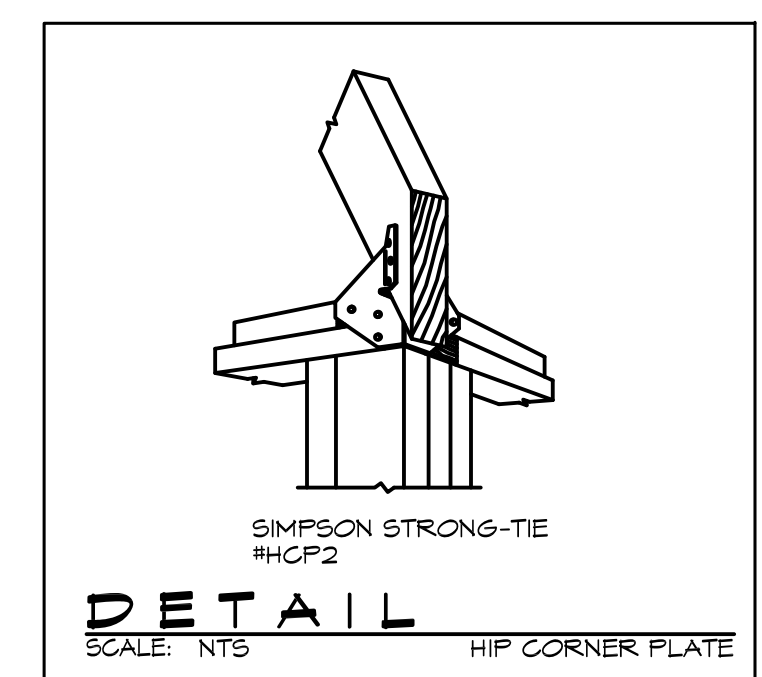
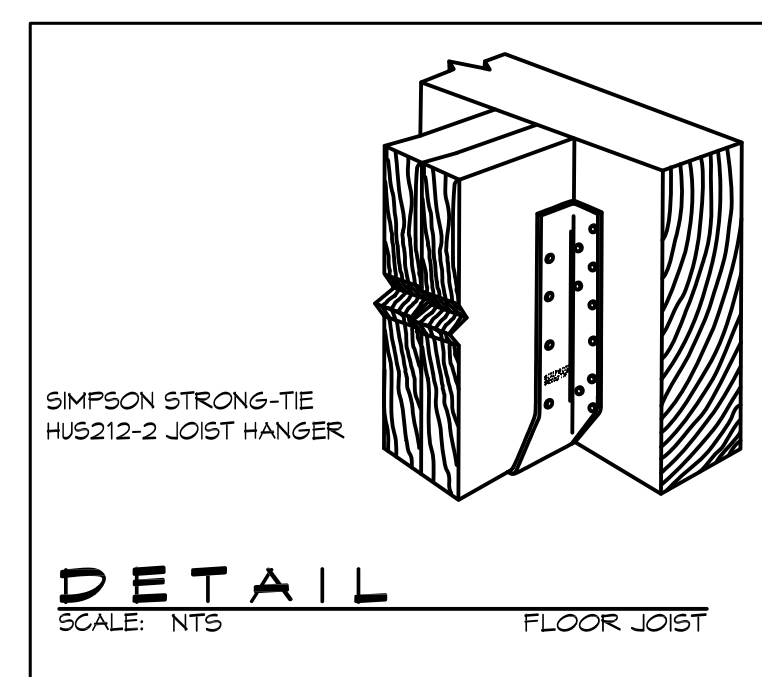
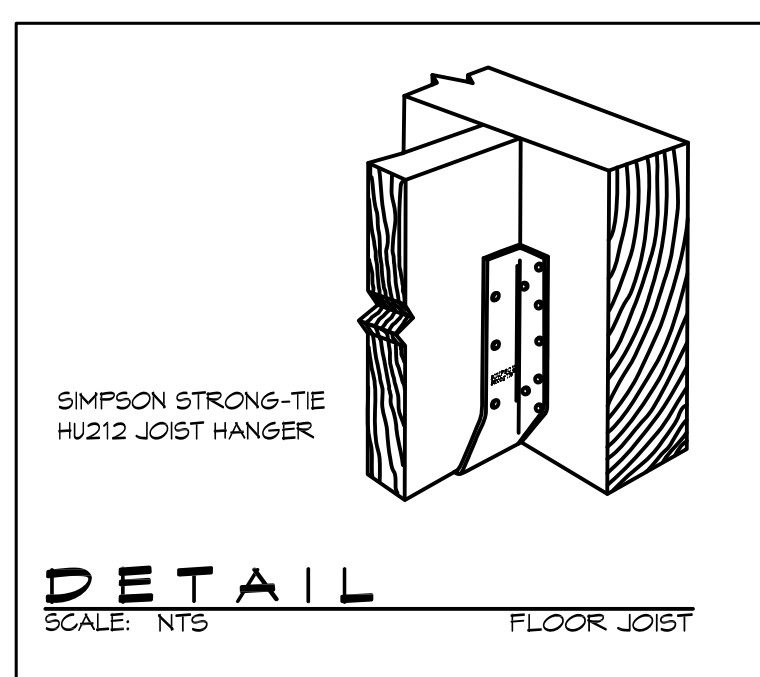
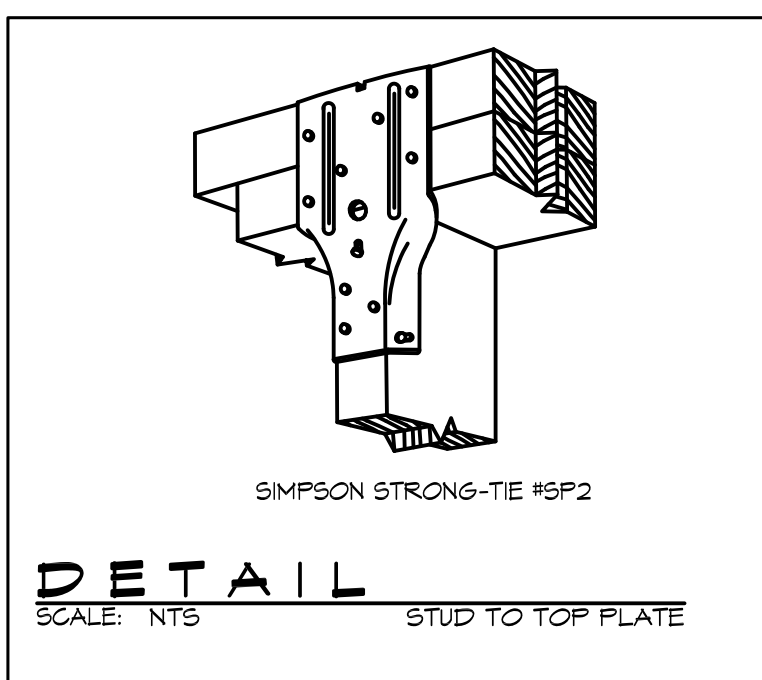
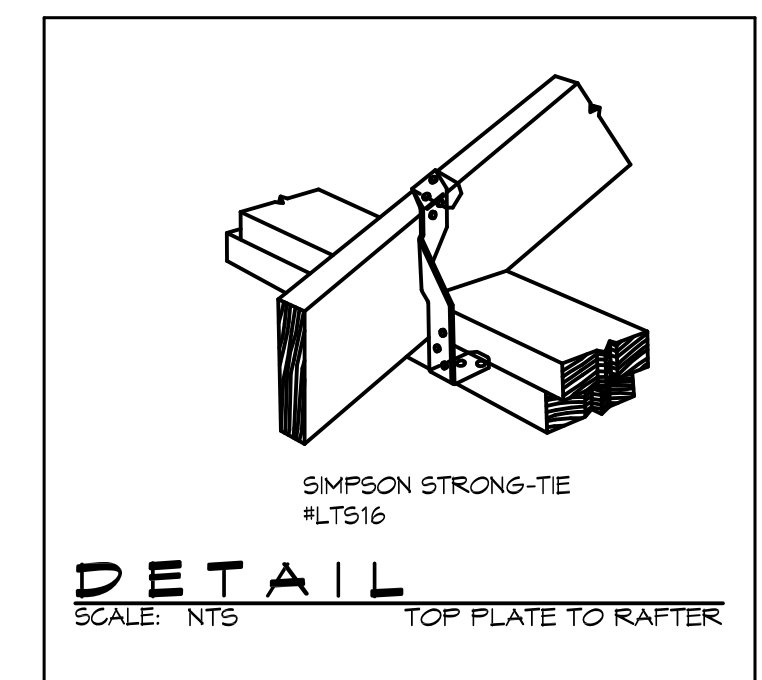
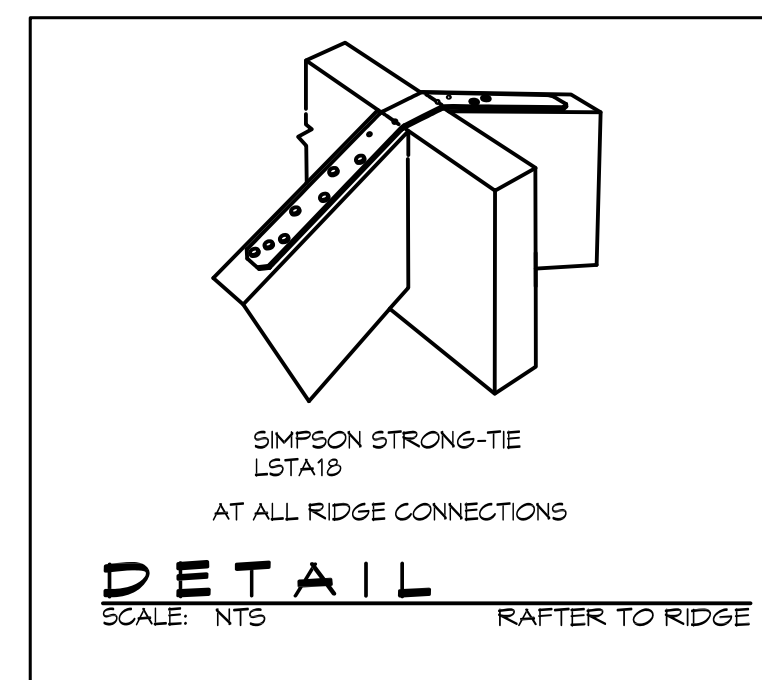
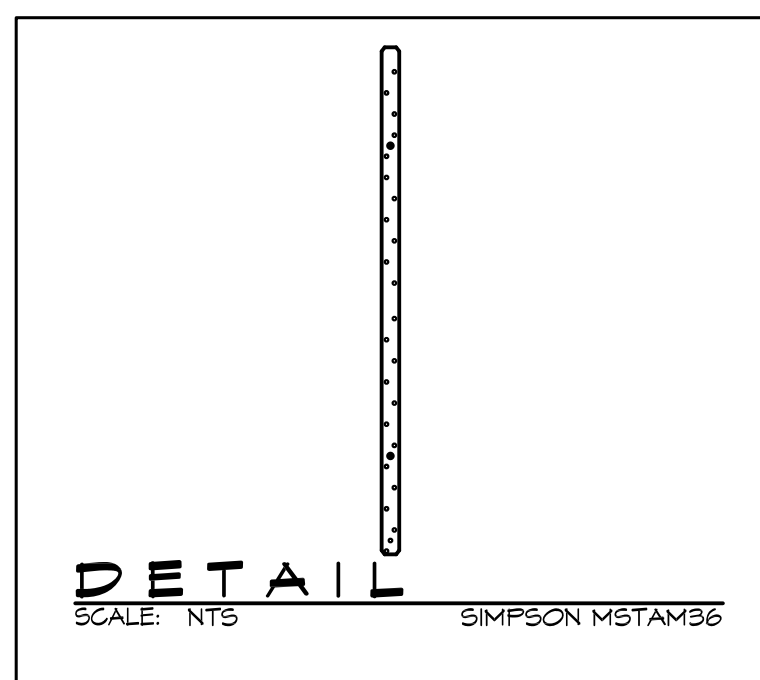
ASPHALT STRIP SHINGLES SHALL HAVE A MINIMUM OF SIX FASTENERS PER SHINGLE WHERE THE ROOF IS IN ONE OF THE FOLLOWING CATEGORIES:
1. THE BASIC WIND SPEED IS 110 MPH OR GREATER AND THE EAVE IS 20 FEET OR HIGHER ABOVE GRADE.
2. THE BASIC WIND SPEED IS 120 MPH OR GREATER.
3. SPECIAL WIND ZONES.

UPLIFT CONNECTIONS

ROOF ASSEMBLY TO WALL ASSEMBLY:
UPLIFT CONNECTIONS SHALL BE FROM RAFTER OR TRUSS TO WALL STUD. WHEN RAFTERS OR TRUSSES ARE NOT LOCATED DIRECTLY ABOVE STUDS, RAFTERS SHALL BE ATTACHED TO THE WALL PLATE AND THE WALL TOP PLATE SHALL BE ATTACHED TO THE WALL STUD WITH UPLIFT CONNECTIONS. UPLIFT CONNECTIONS SHALL BE IN ACCORDANCE WITH TABLE.

WALL ASSEMBLY TO WALL ASSEMBLY:
STORY TO STORY UPLIFT CONNECTIONS FROM UPPER STORY WALL STUD TO LOWER STORY WALL STUD. WHEN UPPER STORY WALL STUDS ARE NOT LOCATED DIRECTLY ABOVE LOWER WALL STUDS, THE STUDS SHALL BE ATTACHED TO A COMMON MEMBER IN THE FLOOR ASSEMBLY BY UPLIFT CONNECTIONS. UPLIFT CONNECTIONS SHALL BE IN ACCORDANCE WITH TABLE.

WALL ASSEMBLY TO FOUNDATION:
FIRST FLOOR WALL STUDS SHALL BE CONNECTED TO THE FOUNDATION, SILL PLATE, OR BOTTOM PLATE. A MINIMUM OF A 1-1/4" X 20 GA. ASTM A653 GRADE 33 STEEL STRAP SHALL BE NAILED TO THE WALL STUDS AND HAVE A MINIMUM EMBEDMENT OF 7 INCHES IN CONCRETE FOUNDATIONS AND SLABS-ON-GRADE. 15 INCHES IN MASONRY BLOCK FOUNDATIONS, OR BE LAPPED UNDER THE BOTTOM PLATE. 3 INCH SQUARE WASHERS SHALL BE USED ON THE ANCHOR BOLTS AND ANCHOR BOLT SPACINGS SHALL NOT EXCEED THE REQUIREMENTS. STEEL STRAPS EMBEDDED IN OR IN CONTACT WITH SLAB-ON-GRADE OR MASONRY BLOCK FOUNDATIONS SHALL BE HOT-DIPPED GALV. AFTER FABRICATION, OR MANUF. FROM G185 OR 2450 GALV. STL. CONNECTIONS SHALL BE IN ACCORDANCE WITH TABLE.



NAIL SPACING

X = 4" O.C.
Y = 4" O.C.
Z = 12" O.C.

X = PLATE EDGE NAIL SPACING
Y = LONG EDGE NAIL SPACING
Z = FIELD NAIL SPACING

DESIGN CRITERIA:

THE CONSTRUCTION FOR SAID RESIDENCE, WHERE BASIC WIND SPEED IS 130 MILES PER HOUR, WIND EXPOSURE ZONE C, IS DESIGNED IN ACCORDANCE WITH AMERICAN FOREST AND PAPER ASSOCIATION (AF&PA) WOOD FRAME CONSTRUCTION MANUAL FOR ONE AND TWO FAMILY DWELLINGS (WFCM) 2001 EDITION AS WELL AS THE INTERNATIONAL RESIDENTIAL CODE (IRC) 2012 EDITION

RESIDENTIAL FOUNDATION
SUE GILLY

4118 ST. LOUIS ST.
SLIDELL LA. 70461

JOB No: 2254 DATE: 8-22-2015

DRAWING NUMBER

STRAPPING AND CONNECTION DETAILS

SB

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PA Strap Tie Holdowns

PA strap tie holdowns are wood-to-concrete connectors that connect studs to the foundation to satisfy engineering and code requirements.

Material: 12 gauge

Finish: Galvanized or [ZMAX®](#) coating.

Installation:

- Use all specified fasteners. See [Holdown and Tension Tie General Notes](#).
- For additional length, an MST strap can be attached using 1/2" bolts through existing holes
- Refer to technical bulletin [T-PAUPLIFT](#) for additional information.



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[Drawings](#)

[Catalog Page](#)

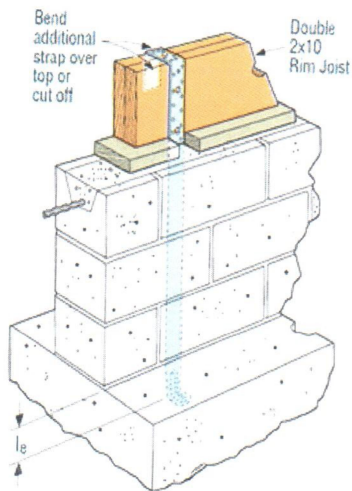
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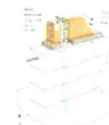
roll over images below to see larger image



PA51
(PA68
similar)



Typical PA connecting Stud to
Foundation



Typical PA51
Installation
(PA68 similar)

Load Table: See [code report listings](#) below

▲ top

These products are available with [additional corrosion protection](#). Additional products on this page may also be available with this option, [check with Simpson Strong-Tie](#) for details.

These models are approved for installation with the [Strong-Drive® SD Connector screw](#).

Wind and SDC A & B – Allowable Tension Loads

Model No.	Strap Length, L (in.)	le (in.)	Non-Cracked Concrete		Cracked Concrete	
			Required Nails	Tension	Required Nails	Tension
PA51	51	4	10 -10d Common	2025	10 -10d Common	2025
PA68	70	4	10 -10d Common	2025	10 -10d Common	2025

SDC C-F – Allowable Tension Loads

Model No.	Strap Length, L (in.)	le (in.)	Non-Cracked Concrete		Cracked Concrete	
			Required Nails	Tension	Required Nails	Tension
PA51	51	4	10 -10d Common	2025	10 -10d Common	1980
PA68	70	4	10 -10d Common	2025	10 -10d Common	1980

1. Allowable loads have been increased for earthquake or wind load durations with no further increases allowed.
2. Concrete shall have a minimum concrete strength, f'c of 2500 psi.
3. Strong-Drive® SD9x1 1/2 (0.131"x1 1/2") screws may be substituted for nails with no reduction.
4. Nails: 10d = 0.148" dia. x 3" long. See [other nail sizes and information](#).

Code Reports (PDFs):

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LEGACY REPORTS

IAPMO UES
ER

ICC-ES ESR

CITY OF LOS ANGELES

STATE OF FLORIDA

ICC-ES NER

ICC-ES ER

ICC-ES ES

PA

See specific model numbers for code listings.

PA51

[ESR-2920](#) / [ESR-2523](#) *

[FL13904](#)

PA68

[ESR-2920](#) / [ESR-2523](#) *

[FL13904](#)

* ESR-2523 is an Index of many of Simpson Strong-Tie Stamped and Welded Cold-formed Steel Products for Wood or Cold-formed Steel Construction

Drawings: To download drawings, right-click or Ctrl-click on the link, then choose "Save Target As..."

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Download the [Simpson Strong-Tie® AutoCAD® Menu](#), which allows you to insert Ortho views directly into your AutoCAD drawing.

ORTHOGRAPHIC

PERSPECTIVE

PA

None for this model

PA & HPA: [DWG](#) | [DXF](#)

PA Stud to Foundation Connection: [DWG](#) | [DXF](#)

PA51

PA51: [DWG](#) | [DXF](#)

PA51 front view: [DWG](#) | [DXF](#)

PA51 left view: [DWG](#) | [DXF](#)

PA51 right view: [DWG](#) | [DXF](#)

High Wind-Resistant Construction D61: Stemwall/Crawlspace: [DWG](#) | [DXF](#)

High Wind-Resistant Construction D62: Stemwall/Crawlspace: [DWG](#) | [DXF](#)

Girder/Truss to Concrete Connections 1: [DWG](#) | [DXF](#)

Lateral Load Connectors: [DWG](#) | [DXF](#)

PA51: [DWG](#) | [DXF](#)

PA68

PA68: [DWG](#) | [DXF](#)

PA68 front view: [DWG](#) | [DXF](#)

PA68 left view: [DWG](#) | [DXF](#)

PA68 right view: [DWG](#) | [DXF](#)

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