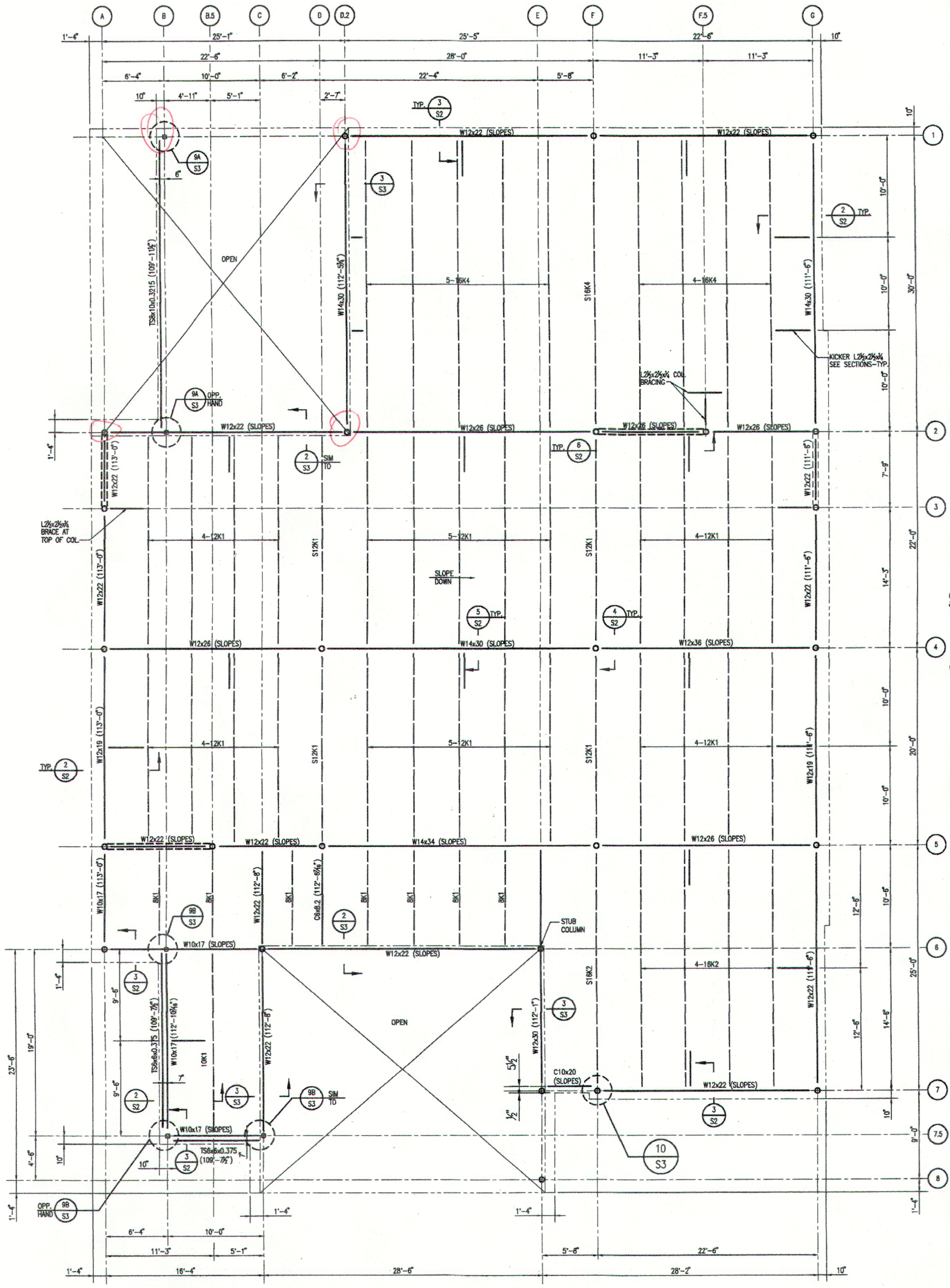
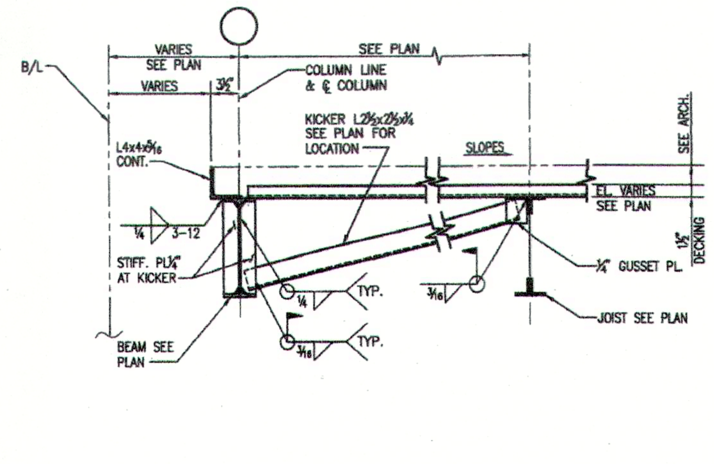
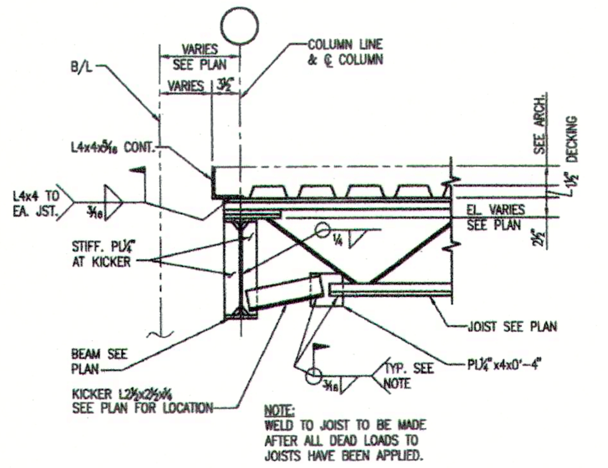


1 / S2 — ROOF PLAN 3/16" = 1'-0"

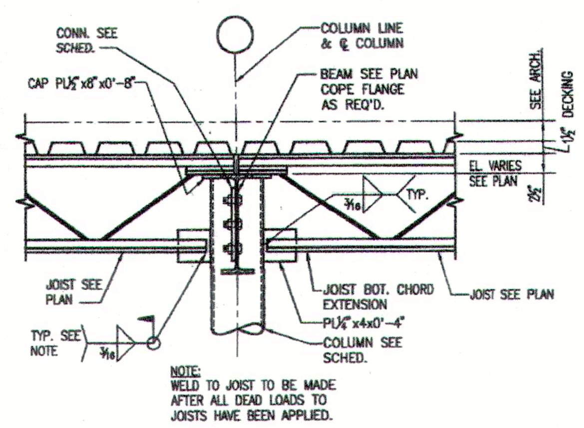
PLAN NOTES: UNLESS NOTED OTHERWISE:
1. TOP OF STRUCTURAL STEEL EL. SEE PLAN.
2. ROOF SYSTEM TO CONSIST OF 1½" DEEP TYPE "F" 20 GAGE GALVANIZED METAL DECKING. SEE ARCH. DWG'S FOR INSULATION AND ROOFING ON TOP OF DECKING.
3. S12K1 – INDICATES SPECIAL JOIST CONNECTION TO COLUMN SEE DETAIL 4/S2.
4. (symbol) – INDICATES PIPE STRUT WIND BRACING SEE 6/S2.
5. EQUALLY SPACE JOISTS BETWEEN COLUMN LINES AS SHOWN.



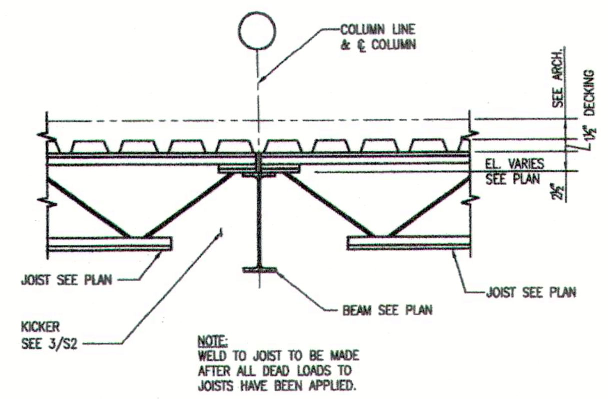
2 / S2 — SECTION 1" = 1'-0"



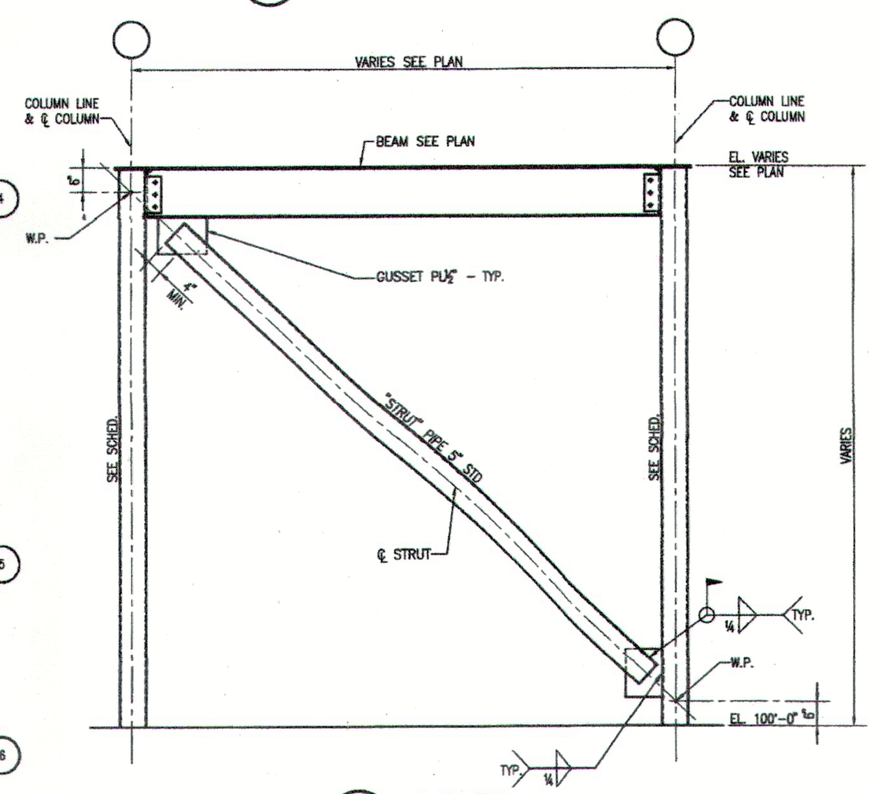
3 / S2 — SECTION 1" = 1'-0"



4 / S2 — SECTION 1" = 1'-0"



5 / S2 — SECTION 1" = 1'-0"

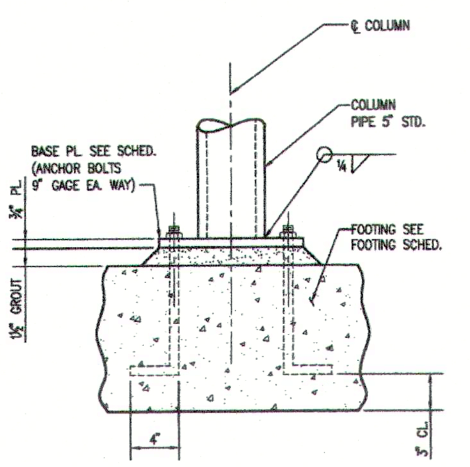


6 / S2 — ELEVATION – WIND BRACING 3/8" = 1'-0"

PROGRESS SET

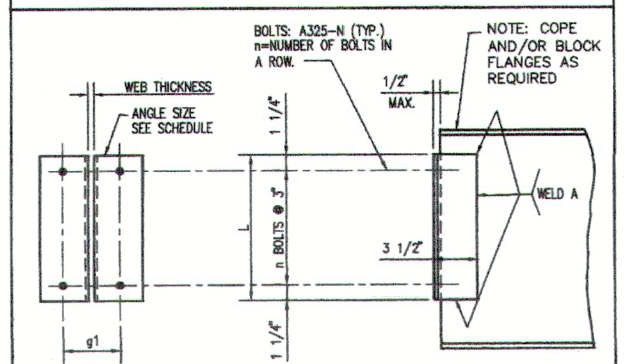
7 / S2 — COLUMN SCHEDULE

COLUMN DESIGNATION	A-2, B-1, D.2-1, D.2-2	C-6, E-7, C-7.5, E-8	A-3, A-4, A-5, A-6, B-6, B-7.5, B.5-5, D-4, D-5, F-1, F-2, F-4, F-5, F-7, F.5-2, G-1, G-2, G-3, G-4, G-5, G-7	B-2, B-6	E-6
HIGH ROOF EL. SEE PLAN					PL¾"x7"x0'-11" W/ 4-¾"ø BOLTS (8"x3½" GAGE)
ROOF EL. VARIES SEE PLAN	PIPE 5" STD. / TS 5x5x0.5 AT B-1	PIPE 5" STD. / TS 5x5x0.3125 AT C-7.5	PIPE 5" STD. / TS 5x5x0.3125 AT B-7.5	TS 5x5x0.3125	PIPE 5" STD.
1ST. FLOOR EL. 100'-0"					
TOP OF FTG. EL. SEE PLAN					
BASE PLATE SIZE	PL¾"x12"x1'-0" W/ 4-¾"ø A36 ANCHOR BOLTS	PL¾"x12"x1'-0" W/ 4-¾"ø A36 ANCHOR BOLTS	PL¾"x12"x1'-0" W/ 4-¾"ø A36 ANCHOR BOLTS	PL¾"x12"x1'-0" W/ 4-¾"ø A36 ANCHOR BOLTS	



8 / S2 — DETAIL – BASE PLATE 1½" = 1'-0"

9 / S2 — TYPICAL BEAM TO BEAM CONNECTION DETAIL AND SCHEDULE



NOTES:
1. ANGLES SHALL BE BENT OR ROTATED WHEN REQ'D BY GEOMETRY OF FRAMED BEAMS.
2. WHERE OPPOSING BEAMS CONNECT WITH DIFFERENT g1'S, LARGER g1 SHALL BE USED.
3. CLIP ANGLES SHALL BE LOWERED AS REQUIRED WHERE SUPPORT BEAM IS 2½" HIGHER.

BEAM SIZE	ANGLE SIZE	L in.	n	g1	WELD A	BOLT
C10, W10	L4x3½x5/16	5 1/2"	2	5¼	3/16	3/4"
W12	L4x3½x5/16	5 1/2"	2	5¼	3/16	3/4"

10 / S2 — GENERAL NOTES

1. FOOTINGS:
FOOTINGS MUST BEAR ON FIRM NATURAL UNDISTURBED SOIL. STRIP FOOTINGS ARE DESIGNED FOR A SOIL PRESSURE OF 1000PSF. SPOT FOOTINGS ARE DESIGNED FOR A SOIL PRESSURE OF 1200PSF. CONSULT THE ENGINEER IF UNSATISFACTORY SOIL CONDITIONS ARE ENCOUNTERED AT BOTTOM OF FOOTING.

2. CONCRETE:
A.C.I. 301-89 SPECIFICATIONS, NORMAL WEIGHT CONCRETE.

3. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
A. ALL CONCRETE SHALL BE NORMAL WEIGHT (150 LB/FT³) WITH F'c = 4,000 P.S.I.

4. REINFORCING STEEL:
BARS – A.S.T.M. A615, GRADE 60
WELDED WIRE MESH – A.S.T.M. A185

5. REINFORCING CLEARANCES REQUIRED ARE AS FOLLOWS:
A. SLABS: ¾" CLEAR TOP AND BOTTOM FORMED. 1" CLEAR BOTTOM ¾" CLEAR TOP, EARTH FORMED.
B. BEAMS: 1½" CLEAR BOTTOM FORMED, 3" CLEAR BOTTOM CAST ON EARTH, 1½" CLEAR SIDES AND TOP FORMED, 3" CLEAR SIDES EARTH FORMED, 1½" CLEAR TOP EARTH FORMED.
C. WALLS: 2" CLEAR SIDES FORMED THEN EXPOSED TO EARTH; ¾" CLEAR OTHERWISE.

6. REINFORCING DETAILS:
A.C.I. 315 STANDARDS; UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, BAR LAPS AND CONFIGURATIONS SHALL BE AS FOLLOWS:
A. CONTINUOUS TOP BARS: HOOK AT NON-CONTINUOUS ENDS; LAP 30 DIAS. AT MID-SPAN.
B. CONTINUOUS BOTTOM BARS; LAP 6" AT CENTER OF SUPPORT.
C. TEMPERATURE BARS IN SLAB AND INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS: TENSION LAP SPLICE, SEE TENSION LAP SPLICE TABLE BELOW.
D. CORNER BARS: PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS, CORNER BARS SHALL LAP WITH HORIZONTAL BARS, LAP 30 DIAS. EACH WAY IN BEAMS; SEE SPLICE TABLE BELOW. FOR WALLS, HOOK INSIDE BAR IN WALLS AT ENDS.
E. TENSION LAP SPLICE TABLE:

LAP SPLICES (INCHES)

BAR SIZE	LOCATION TOP	LOCATION OTHER	BAR SIZE	LOCATION TOP	LOCATION OTHER
#3	18	16	#8	48	37
#4	24	19	#9	55	42
#5	30	23	#10	65	50
#6	36	28	#11	80	62
#7	42	33			

7. STRUCTURAL STEEL:
A.I.S.C. SPECIFICATIONS; A.S.T.M. A-50 (WIDE FLANGE SECTIONS) ALL OTHER A.S.T.M. A-36 STEEL, EXCEPT PIPE A.S.T.M. A-53 GRADE B; A-325 BOLTS (¾" DIA MIN.); E-70 ELECTRODES. EXCEPT AS NOTED, ALL BEAM CONNECTIONS SHALL BE A.I.S.C. STANDARD FRAMED CONNECTIONS, BOLTED OR WELDED. CONNECTIONS TO BE DESIGNED FOR ½ UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN UNLESS OTHERWISE INDICATED. CONNECTIONS NOT DETAILED ON THE DRAWINGS, INCLUDING A.I.S.C. STANDARD FRAMED CONNECTIONS SHALL BE DESIGNED BY A LOUISIANA REGISTERED CIVIL ENGINEER COMMISSIONED BY THE CONTRACTOR, AND SHOP DRAWINGS MUST BEAR THAT ENGINEER'S SEAL.

8. METAL ROOF DECK (TYPICAL U.N.O.):
A. GALVANIZED METAL ROOF DECK, TYPE "F". CONTINUOUS OVER THREE OR MORE SPANS; FIELD WELD TO SUPPORTS.
B. DEPTH: 1½"
C. GAGE: 20
D. MINIMUM SECTION MODULUS (POSITIVE): 0.139 INxINxIN PER FT.
E. MINIMUM SECTION MODULUS (NEGATIVE): 0.148 INxINxIN PER FT.
F. MINIMUM MOMENT OF INERTIA: 0.151 INxINxINxIN PER FT.
G. ALLOWABLE STRESS: 33 KSI
H. DECK UPLIFT DESIGN: DECK AND ITS FASTENING TO ITS SUPPORTING FRAMING SHALL BE DESIGNED (AS A MINIMUM) FOR A NET UPLIFT PRESSURE APPLIED TO THE HORIZONTAL PROJECTED ROOF AREA AS FOLLOWS:
A. ALL ROOF AREAS:
65 PSF FOR A 6'-0" x 6'-0" CORNER ZONE, 35 PSF FOR A 6'-0" EDGE STRIP AROUND PERIMETER, & 25 PSF FOR THE REMAINING INTERIOR AREAS. THESE PRESSURES ARE FACTORED AND CONSEQUENTLY CANNOT BE REDUCED. THE MANUFACTURER SHALL SUBMIT A NOTORIZED AFFIDAVIT STATING FULL DESIGN COMPLIANCE WITH THE UPLIFT CRITERION.

9. OPEN WEB STEEL JOISTS:
A. S.J.I. SPECIFICATIONS AND CODE OF STANDARD PRACTICE; (1990) FIELD WELDED IN PLACE, MANUFACTURER MUST BE A MEMBER OF STEEL JOIST INSTITUTE.
B. JOIST UPLIFT DESIGN: JOISTS AND BRIDGING AT ROOF SHALL BE DESIGNED FOR A NET UPLIFT OF 30 P.S.F. APPLIED TO THE HORIZONTAL PROJECTED ROOF AREA. THIS PRESSURE IS FACTORED AND CONSEQUENTLY CANNOT BE REDUCED. PROVIDE ALL BRIDGING AND BRACING INCLUDING THE BRIDGING AND BRACING AT THE FIRST BOTTOM CORD PANEL POINT AT EACH END OF THE JOIST REQUIRED FOR UPLIFT DESIGN AS REQUIRED BY SJI. SPECIFICATIONS. ALL BRIDGING AND BRACING SHALL BE FIELD WELDED. ALL CONTINUOUS BRIDGING AND BRACING MUST BE CONNECTED TO ALL AND ANY BEAMS AND WALLS WHICH ARE PARALLEL TO JOISTS. THE MANUFACTURER SHALL SUBMIT A NOTORIZED AFIDAVIT STATING FULL DESIGN COMPLIANCE WITH THE UPLIFT CRITERION.
C. SEE PLAN FOR COLUMNS REQUIRING BOTTOM CHORD EXTENSION.

10. AUTOMATICALLY END WELDED SHEAR CONNECTORS (A.E.W.S.C.):
AS MANUFACTURED BY NELSON STUDS OR APPROVED EQUAL.

11. ANCHORS TO HARDENED CONCRETE:
WEDGE TYPE ANCHORS; "KWIK" BOLTS AS MANUFACTURED BY HILTI FASTENING SYSTEMS OR AN APPROVED EQUAL. IN THE INSTALLATION OF "KWIK" BOLTS, CARE SHALL BE EXERCISED SO AS NOT TO NICK OR CUT EXISTING REINFORCING. THE CONTRACTOR SHALL LOCATE REINFORCING BY MEANS OF SMALL DRILLED PILOT HOLES, THEN DETERMINE THE LOCATION OF THE "KWIK" BOLT. FABRICATE (FROM A FIELD TEMPLATE) THE STEEL TO BE FASTENED TO THE CONCRETE BY THE "KWIK" BOLTS AND COMPLETE THE INSTALLATION.

12. OTHER WORK:
COORDINATE ALL OTHER WORK WITH STRUCTURAL. UNLESS DETAILED OR SPECIFIED, THE ARCHITECT IS TO APPROVE ALL OPENINGS, SLEEVES EMBEDDED ITEMS ETC. INVOLVED IN STRUCTURAL WORK PRIOR TO BEING SET. ALL SUCH ITEMS SHALL NOT IMPAIR THE STRUCTURAL INTEGRITY OF THE MEMBER.

RECEIVED DEC 17 2002 GARCIA BRENNER STROMBERG

BID SET

MORPHY MAKOFSKY INCORPORATED (504)488-1317 JOB NO. 02104

DEC 23 2002

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REVISIONS
A PROJECT FOR:
SATNOH, LLC
130 W. RUELLE
MANDEVILLE, LA 70471
GARCIA BRENNER STROMBERG
ARCHITECTURE

SEAL

ROOF FRAMING PLAN

TULANE / HONTAS MEDICAL CENTER

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