

ENCE 3318
Fluid Mechanics for Civil Engineering
TEST 4

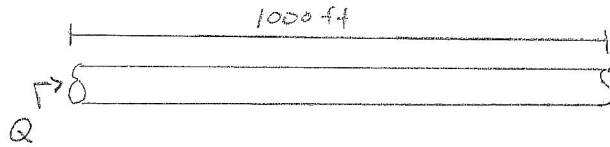
1. A 6 inch cast iron pipe 1000 feet long carries 400 gallons per minute of water. Calculate the head loss when the water is at (a) 40°F, (b) 200°F.

$$V_g = 0.13368 \text{ ft}^3$$

$$A = \frac{\pi (.5 \text{ ft})^2}{4} = .1963 \text{ ft}^2$$

$$V = \frac{Q}{A} = \frac{.8912 \text{ ft}^3/\text{s}}{.1963 \text{ ft}^2}$$

$$V = 4.54 \text{ ft/s}$$



$$Q = \left(\frac{400 \text{ gal}}{60 \text{ s}} \right) \left(\frac{.13368 \text{ ft}^3}{1 \text{ gal}} \right) = .8912 \text{ ft}^3/\text{s}$$

a) h_f @ $T = 40^\circ\text{F}$

$$R_n = \frac{Vd}{\nu} = \frac{(4.54)(.5)}{1.664 \times 10^{-5}} = 136.48$$

$$h_f = \frac{f L V^2}{d 2g}$$

$$\frac{f}{d} = \frac{.00085}{.5} = .0017 \rightarrow (\text{Moody diagram}) = f = .023$$

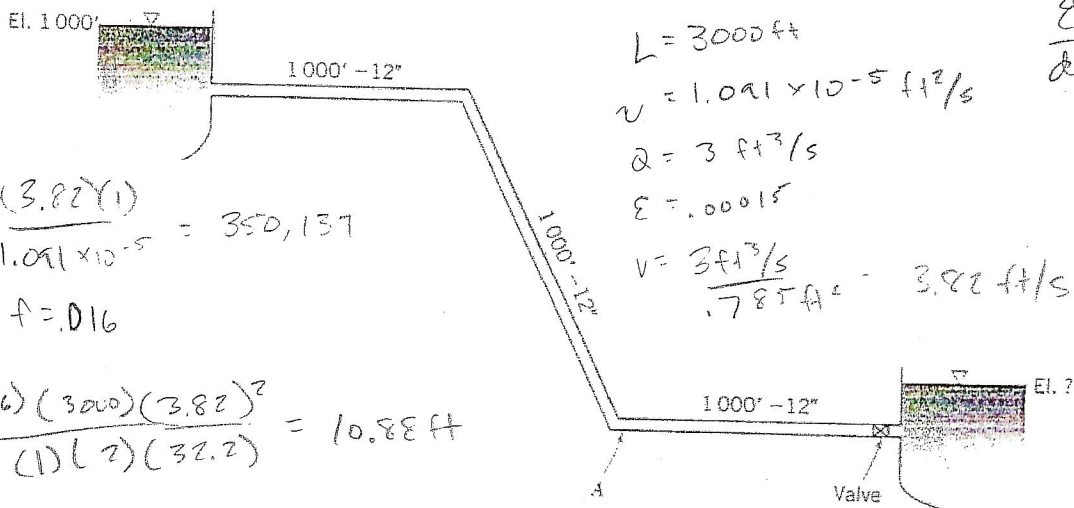
$$h_f = \frac{(.023)(1000)(4.54)^2}{(.5)(2)(32.2)} = 14.72 \text{ ft}$$

b) h_f @ 200°F

$$R_n = \frac{Vd}{\nu} = \frac{(4.54)(.5)}{.341 \times 10^{-5}} = 665.689$$

$$h_f = \frac{f L V^2}{d 2g} = \frac{(.022)(1000)(4.54)^2}{(.5)(2)(32.2)} = 14.08 \text{ ft}$$

2. A wrought iron pipe 3000 feet long conveys water (68°F) between two reservoirs. If the discharge is 3.0 ft³/s, find the elevation of the lower reservoir. A fully open valve is near the lower reservoir. The entrance loss coefficient is 0.04. There are two pipe bends each with a radius of 2 feet. The diameter of the pipe is 12 inches.



$$R_n = \frac{(3.82)(1)}{1.091 \times 10^{-5}} = 350,137$$

$$f = .016$$

$$h_f = \frac{(0.16)(3000)(3.82)^2}{(1)(2)(32.2)} = 10.88 \text{ ft}$$

$$\frac{r}{d} = \frac{2}{1} = 2 \quad k_b = .19 \quad h = \frac{k_b v^2}{2g} \Rightarrow \frac{(.19)(3.82)^2}{(2)(32.2)} = .0431 \text{ ft} \quad (2)(.0431) = .0862 \text{ ft}$$

$$k_v = .15 \quad h_v = \frac{k_v (v^2)}{2g} = \frac{.15(3.82)^2}{(2)(32.2)} = .034$$

$$h_e = \frac{(.04)(3.82)^2}{64.4} = .0091$$

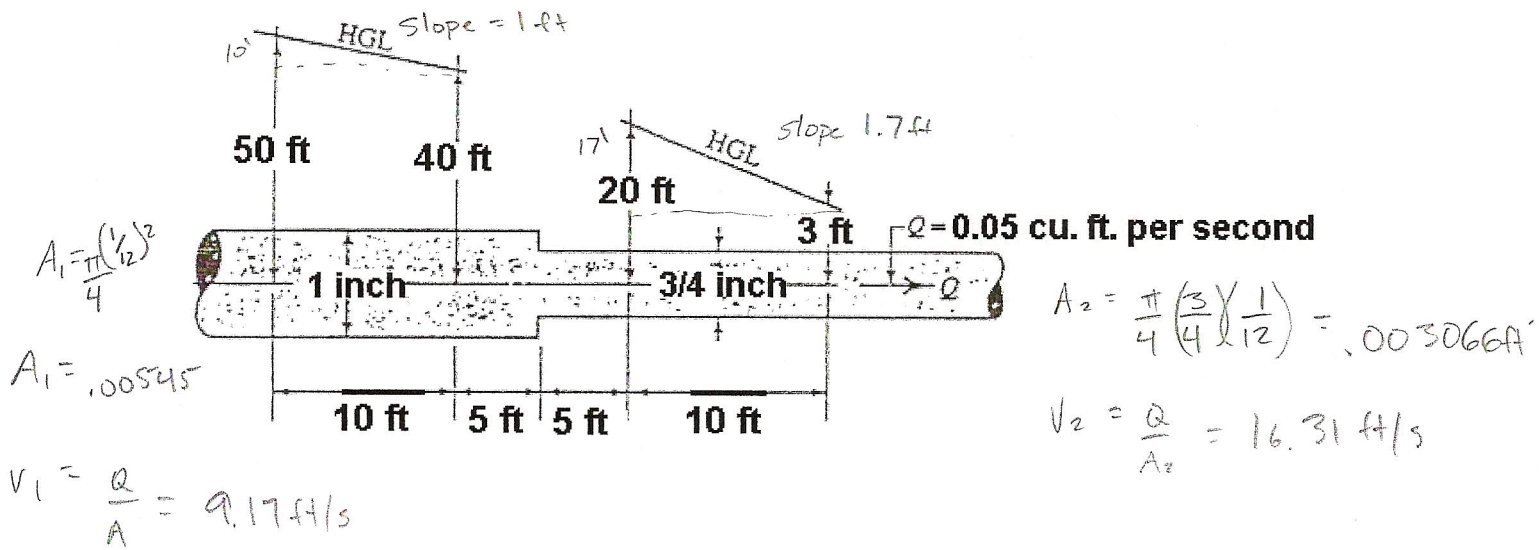
$$\frac{P_1}{\gamma} + \frac{V_1^2}{2g} + z_1 = \frac{P_2}{\gamma} + \frac{V_2^2}{2g} + z_2$$

$$1000 = \frac{3.82^2}{64.4} + z_2$$

$$999.77 - 10.88 - .0431 - .034 - .0091 = z_2$$

$$z_2 = 988.8 \text{ ft}$$

3. Calculate the head loss coefficient caused by this restricted contraction.



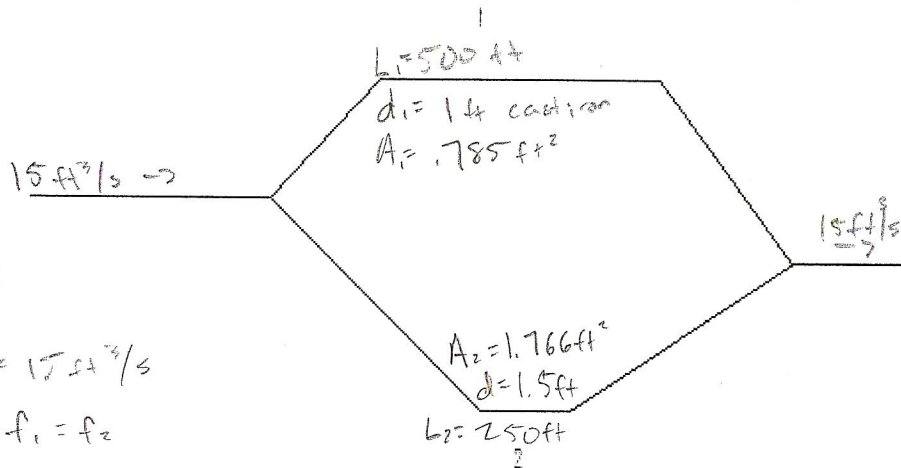
$$h_{f1} = 50 - 40 = 10 \text{ ft}$$

$$h_{f2} = 20 - 3 = 17 \text{ ft}$$

$$\frac{d_2}{d_1} = \frac{.75}{1} = .75 \Rightarrow k_c = .215 \rightarrow \text{interpolated}$$

$$h_c = k_c \frac{V_2^2}{2g} \Rightarrow .215 \left(\frac{(16.31)^2}{(2)(32.2)} \right) = .989 \text{ ft}$$

4. Two pipes are parallel (and horizontal) as shown below. Pipe 1 is cast iron (new), 12 inches in diameter, and 500 feet long. Pipe 2 is riveted steel (few rivets), 18 inches in diameter, and 250 feet long. If the total discharge is 15 ft³/s and the water temperature is 180°F, what will be the discharge in each line?



assume $h_{f1} = h_{f2}$

$$Q = Q_1 + Q_2 = 15 \text{ ft}^3/\text{s}$$

assume $f = .02 = f_1 = f_2$

$$\frac{f/L_1 \cdot v_1^2}{d_1 \cdot 2g} = \frac{f/L_2 \cdot v_2^2}{d_2 \cdot 2g} \Rightarrow \frac{500 v_1^2}{1} = \frac{250 v_2^2}{1.5} \Rightarrow v_1^2 = .333 v_2^2$$

$$v_1 = .577 v_2 \quad 15 = .577 v_2 (.785) + 1.766 (v_2)$$

$$15 = 2.219 v_2 \Rightarrow v_2 = 6.76 \quad v_1 = 3.90$$

$$R_1 = \frac{v_1 d_1}{\nu} = \frac{3.90(1)}{.385 \times 10^{-5}} = 1.01 \times 10^6 \quad \frac{\epsilon}{d} = \frac{.00085}{1} = .00085$$

$$f_1 = .019 \quad R_2 = \frac{v_2 d_2}{\nu} = \frac{6.76(1.5)}{.385 \times 10^{-5}} = 2.63 \times 10^6 \quad \frac{\epsilon}{d} = \frac{.003}{1.5} = .002$$

$$f_2 = .024$$

$$.019 \left(\frac{500}{1} \right) \frac{v_1^2}{2g} = .024 \left(\frac{250}{1.5} \right) \frac{v_2^2}{2g} = v_1 = .649 v_2$$

$$15 = .649 v_2 (.785) + 1.766 v_2 \Rightarrow v_2 = 6.59 \text{ ft/s} \quad v_1 = 4.28 \text{ ft/s}$$

$$Q_1 = A_1 v_1 \Rightarrow (.785)(4.28) = 3.36 \text{ ft}^3/\text{s}$$

$$Q_2 = A_2 v_2 = (1.766)(6.59) = 11.64 \text{ ft}^3/\text{s}$$