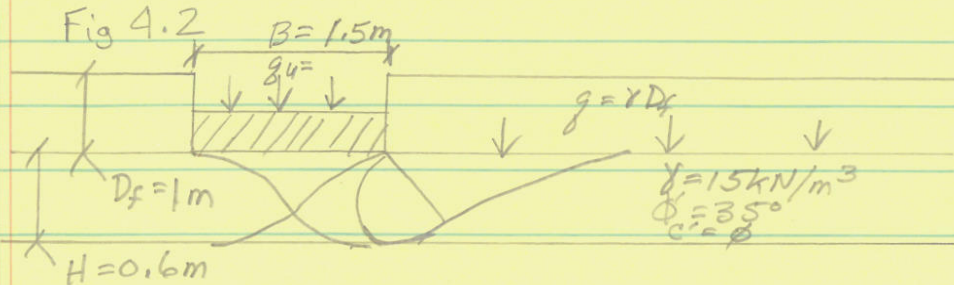


4.2
Pg 220

Square foundation, $B = 1.5\text{m}$, $L = 1.5\text{m}$, $D_f = 1\text{m}$,
 $H = 0.6\text{m}$, $\phi' = 35^\circ$, $c' = 0$ & $\gamma = 15\text{ kN/m}^3$; Use $FS = 3$



Determine gross allowable load. Use Eq [4.3]

$$[4.6] \quad q_u = q N_q^* F_{qs}^* + \frac{1}{2} \gamma B N_\gamma^* F_{\gamma s}^* \quad \frac{H}{B} = \frac{0.6\text{m}}{1.5\text{m}} = 0.4$$

Fig 4.4 $N_q^* = 325.0$

Fig 4.6 $m_1 = 0.579$

Fig 4.5 $N_\gamma^* = 131.6$

$m_2 = 0.586$

$$[4.4] \quad F_{qs}^* = 1 - m_1 \left(\frac{B}{L} \right) = 1 - 0.579 \left(\frac{1.5}{1.5} \right) = 0.4214$$

$$g = \gamma D_f = (15)(1) = 15\text{ kN/m}^2 \quad [4.5] \quad F_{\gamma s}^* = 1 - m_2 \left(\frac{B}{L} \right) = 1 - 0.586 \left(\frac{1.5}{1.5} \right) = 0.414$$

$$[4.6] \quad q_u = (15\text{ kN/m}^2)(325.0)(0.4214) + \frac{1}{2} (15)(1.5)(131.6)(0.414)$$

$$q_u = 2667.25\text{ kN}$$

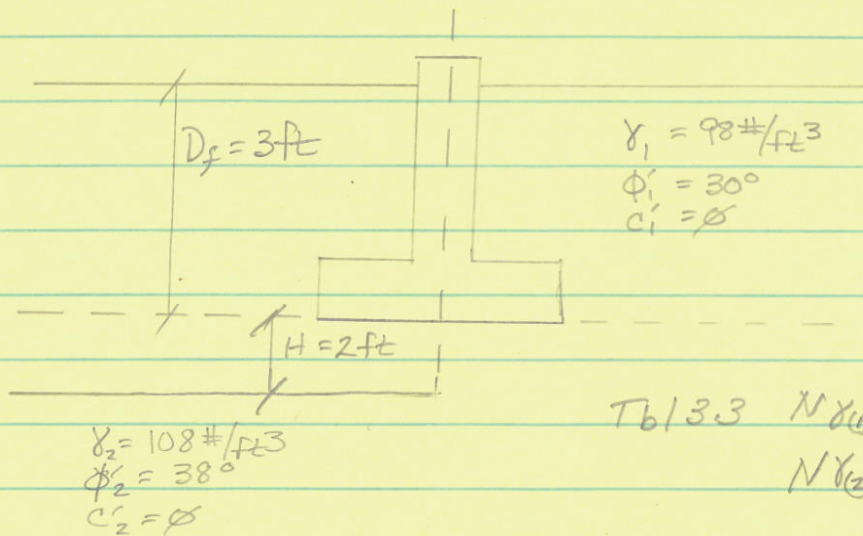
$$Q_{\text{ALL}} = \frac{q_u B^2}{FS} = \frac{(2667.25)(1.5)^2}{3} = \boxed{2000.1\text{ kN}}$$

David Demmon

4.8 Using Fig 4-11. $B = 4 \text{ ft}$, $L = 6 \text{ ft}$, $H = 2 \text{ ft}$, $D_f = 3 \text{ ft}$
 pg 220 $\gamma_1 = 98 \text{ #/ft}^3$, $\phi'_1 = 30^\circ$, $c'_1 = \emptyset$
 $\gamma_2 = 108 \text{ #/ft}^3$, $\phi'_2 = 38^\circ$, $c'_2 = \emptyset$

Use $FS = 4$

Determine gross allowable load.



Tbl 3.3 $N_{\gamma(1)} = 22.4$ $N_{q(1)} = 18.40$
 $N_{\gamma(2)} = 78.03$ $N_{q(2)} = 48.93$

[4.15] $g_1 = \frac{c'_1}{\emptyset} N_{c(1)} + \frac{1}{2} \gamma_1 B N_{\gamma(1)} = \frac{1}{2} (98)(4)(22.4) = 4390.4$

[4.16] $g_2 = \frac{c'_2}{\emptyset} N_{c(2)} + \frac{1}{2} \gamma_2 B N_{\gamma(2)} = \frac{1}{2} (108)(4)(78.03) = 16,854.48$

$\frac{g_2}{g_1} = \frac{16,854.5}{4390.4} = 3.84 > 1$ so Eq [4.32] applies

Tbl 3.4 $F_{gs(1)} = 1 + \left(\frac{B}{L}\right) \tan \phi' = 1 + \left(\frac{4}{6}\right) \tan(30^\circ) = 1.385$

$F_{gs(2)} = 1 + \left(\frac{B}{L}\right) \tan \phi' = 1 + \left(\frac{4}{6}\right) \tan(38^\circ) = 1.521$

$F_{\gamma sc} = 1 - 0.4 \left(\frac{B}{L}\right) = 1 - 0.4 \left(\frac{4}{6}\right) = 0.733$

[4.33] $g_{t1} = c_1 N_{c(1)} F_{cs(1)} + \gamma_1 D_f N_{q(1)} F_{qs(1)} + \frac{1}{2} \gamma_1 B N_{\gamma(1)} F_{\gamma s(1)}$

$g_{t1} = (98)(3)(18.4)(1.385) + \frac{1}{2} (98)(4)(22.4)(0.733) = 10,710.45$

$g_{b1} = (108)(3)(48.93)(1.521) + \frac{1}{2} (108)(4)(78.03)(0.733) = 36,467.23$

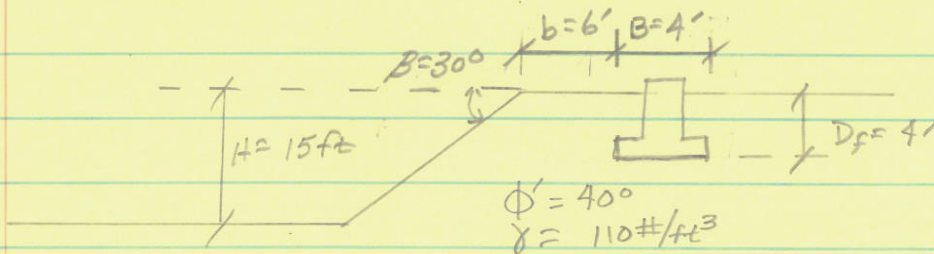
$D \approx B$

[4.32] $g_u = g_{t1} + (g_{b1} - g_{t1}) \left(\frac{H}{D}\right)^2 = 10,710 + (36,467 - 10,710) \left(\frac{2}{4}\right)^2 = 17,149 \text{ #/ft}^2$

David Dammon

4.11
Pg 220 A continuous foundation near a slope of granular soil (Fig 4.14). $B = 4\text{ft}$, $b = 6\text{ft}$, $H = 15\text{ft}$,
 $D_f = 4\text{ft}$, $\beta = 30^\circ$, $\phi' = 40^\circ$ & $\gamma = 110\text{#/ft}^3$

Estimate ultimate bearing capacity with Meyerhof's



Granular Soil Eq [4.38] Applies $q_u = \frac{1}{2} \gamma B N_{\gamma g}$ (pg 203)

Fig 4.15 $\frac{b}{B} = \frac{6}{4} = 1.5$ $\frac{D_f}{B} = \frac{4}{4} = 1 \therefore N_{\gamma g} = 53.57$

[4.38] $q_u = \frac{1}{2} (110) (4) (53.57) = 11,785.4\text{#/ft}^2$