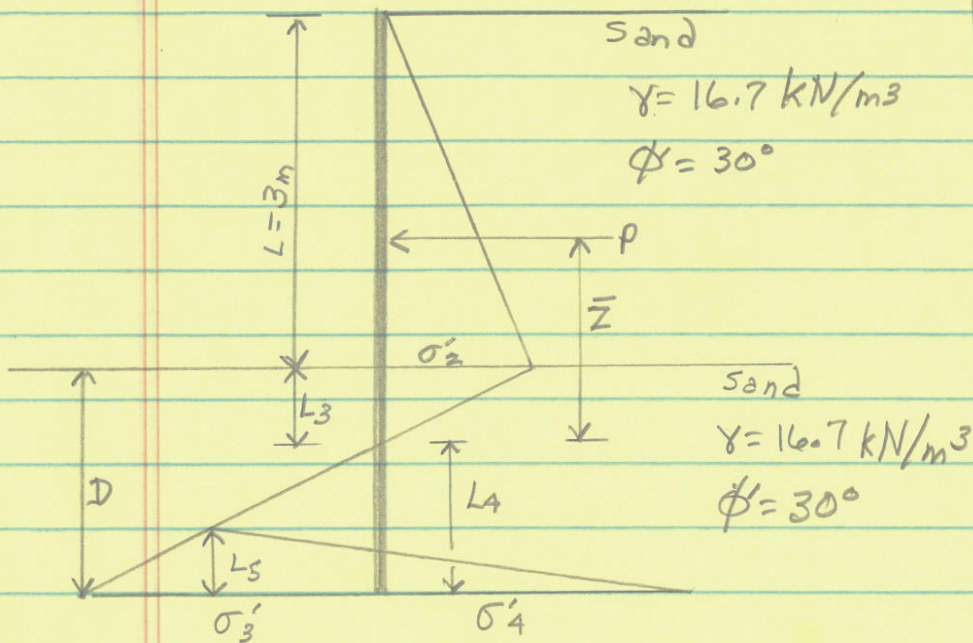


Chapter 9

Pg 1 of 9

#9.3 Given:

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Required: Calculate the theoretical depth of penetration D & the maximum moment.

$$a) K_a = \tan^2\left(45 - \frac{\phi'}{2}\right) = \tan^2\left(45 - \frac{30}{2}\right) = 0.33$$

$$K_p = \tan^2\left(45 + \frac{\phi'}{2}\right) = \tan^2\left(45 + \frac{30}{2}\right) = 3.00$$

$$[9.24] \sigma'_2 = \gamma L K_a = (16.7)(3)(0.33) = 16.53 \text{ kN/m}^2$$

$$[9.28] L_3 = \frac{L K_a}{K_p - K_a} = \frac{(3)(0.33)}{3 - 0.33} = 0.371 \text{ m}$$

$$[9.27] \sigma'_5 = \gamma L K_p + \gamma L_3 (K_p - K_a)$$

$$= (16.7)(3)(3) + (16.7)(0.371)(3 - 0.33)$$

$$= \sigma'_5 = 166.84 \text{ kN/m}^2$$

$$[9.29] P = \frac{1}{2} \sigma'_2 L + \frac{1}{2} \sigma'_5 L_3$$

$$P = \frac{1}{2} (16.53)(3) + \frac{1}{2} (166.84)(0.371)$$

$$P = 27.86 \text{ kN/m}$$

Next \curvearrowright

#9.3 Cont.

$$[9.30] \quad \bar{z} = \frac{L(2k_a + k_p)}{3(k_p - k_a)} = \frac{3[2(0.33) + 3]}{3(3 - 0.33)} = 1.37 \text{ m}$$

$$[9.32] \quad A'_1 = \frac{\sigma'_s}{\gamma(k_p - k_a)} = \frac{166.84}{(16.7)(3 - 0.33)} = 3.74 \text{ m}^2$$

$$[9.33] \quad A'_2 = \frac{8P}{\gamma(k_p - k_a)} = \frac{(8)(27.86)}{(16.7)(3 - 0.33)} = 5.00 \text{ m}^2$$

$$[9.34] \quad A'_3 = \frac{6P[2\bar{z}\gamma(k_p - k_a) + \sigma'_s]}{\gamma^2(k_p - k_a)^2}$$

$$= \frac{6(27.86)[2(1.37)(16.7)(3 - 0.33) + 166.84]}{(16.7)^2(3 - 0.33)^2}$$

$$A'_3 = 23.75 \text{ m}^2$$

$$[9.35] \quad A'_4 = \frac{P(6\bar{z}\sigma'_s + 4P)}{\gamma^2(k_p - k_a)^2}$$

$$= \frac{(27.86)[6(1.37)(166.84) + 4(27.86)]}{16.7^2(3 - 0.33)^2}$$

$$= 20.83 \text{ m}^2$$

$$[9.31] \quad L_4: L_4^4 + A'_1 L_4^3 - A'_2 L_4^2 - A'_3 L_4 - A'_4 = 0$$

$$L_4^4 + 3.74 L_4^3 - 5 L_4^2 - 23.75 L_4 - 20.83 = 0$$

$$L_4 = 2.65 \text{ m}$$

$$\rightarrow D_{\text{theory}} = L_3 + L_4 = 0.371 + 2.65 = \boxed{3.02 \text{ m}}$$

$$z' = \sqrt{\frac{2P}{\gamma(k_p - k_a)}} = \sqrt{\frac{2(27.86)}{(16.7)(3 - 0.33)}} = 1.12 \text{ m}$$

Next \downarrow

#9.3 Continued

$$\begin{aligned}M_{\max} &= P(\bar{z} + z') - \left[\frac{1}{2} \gamma z'^2 (K_p - K_a) \right] \frac{1}{3} z' \\ &= (27.86)(1.37 + 1.12) - \left[\frac{1}{2} (16.7) (1.12)^2 (3 - 0.33) \right] \frac{1}{3} (1.12) \\ &= 58.81 \text{ kN-m/m}\end{aligned}$$

#9.7 assume $D_{\text{actual}} = 1.3 D_{\text{theory}}$

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#9.6 An anchored sheet pile

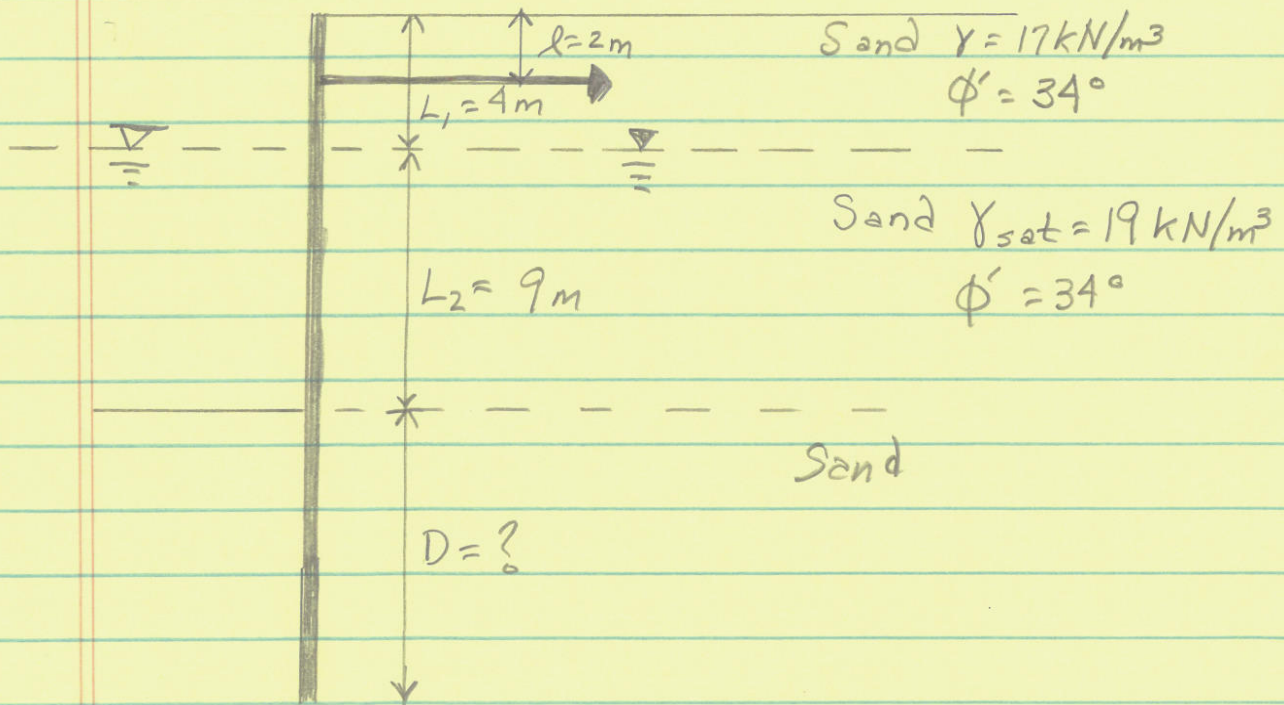
bulk head is shown below $L_1 = 4\text{m}$, $L_2 = 9\text{m}$, $l = 2\text{m}$,
 $\gamma = 17\text{ kN/m}^3$, $\gamma_{\text{sat}} = 19\text{ kN/m}^3$ & $\phi' = 34^\circ$

a) calculate D_{theory}

#9.7

b) Determine the theoretical maximum moment

c) Use Rowe's moment reduction technique,
 Choose a sheet pile section. Take $E = 210 \times 10^3 \text{ MN/m}^2$
 & $\sigma_{\text{all}} = 210,000 \text{ kN/m}^2$



#9.7 Cont.

$$k_a = \tan^2\left(45 - \frac{\phi'}{2}\right) = \tan^2\left(45 - \frac{34}{2}\right) = 0.283$$

$$k_p = \tan^2\left(45 + \frac{\phi'}{2}\right) = \tan^2\left(45 + \frac{34}{2}\right) = 3.537$$

$$k_p - k_a = 3.537 - 0.283 = 3.254$$

$$\gamma' = \gamma_{sat} - \gamma_w = 19 - 9.81 = 9.19 \text{ kN/m}^3$$

$$[9.1] \sigma_1' = \gamma L_1 k_a = (17)(4)(0.283) = 19.24 \text{ kN/m}^2$$

$$[9.2] \sigma_2' = (\gamma L_1 + \gamma' L_2) k_a = [(17 \times 4) + (9.19 \times 9)] 0.283 = 42.65 \text{ kN/m}^2$$

$$[9.6] L_3 = \frac{\sigma_2'}{\gamma' (k_p - k_a)} = \frac{42.65}{9.19(3.254)} = 1.42 \text{ m}$$

$$P = \frac{1}{2} \sigma_1' L_1 + \sigma_2' L_2 + \frac{1}{2} (\sigma_2' - \sigma_1') L_2 + \frac{1}{2} \sigma_2' L_3$$

$$= \frac{1}{2} (19.24)(4 \text{ m}) + (42.65)(9) + \frac{1}{2} (42.65 - 19.24)(9) + \frac{1}{2} (42.65)(1.42)$$

$$P = 38.48 + 383.85 + 105.35 + 30.28 = 557.96$$

$$\bar{z} = \frac{\sum M E}{P} = \left[38.48 \left(1.42 + 9 + \frac{4}{3}\right) + 383.85 \left(1.42 + \frac{9}{2}\right) + 105.35 \left(1.42 + \frac{9}{3}\right) + 30.28 \left(\frac{2 \times 1.42}{3}\right) \right] \times \frac{1}{557.96}$$

$$\bar{z} = \frac{452.268 + 2272.392 + 465.647 + 28.665}{557.96}$$

$$\bar{z} = 5.77 \text{ m}$$

$$[9.67] L_4: L_4^3 + 1.5 L_4^2 (L_2 + L_2 + L_3) - 3P \left[(L_1 + L_2 + L_3) - (\bar{z} + L_1) \right] = 0$$

$$\frac{\gamma' (k_p - k_a)}{\gamma' (k_p - k_a)}$$

$$L_4^3 + 1.5 L_4^2 (2 + 9 + 1.42) - 3P \left[(4 + 9 + 1.42) - (5.77 + 4) \right] = 0$$

$$\frac{9.19 (3.254)}{9.19 (3.254)}$$

$$L_4 = 4.05 \text{ m}$$

Next ↴

9.7 Cont

$$D_{\text{theory}} = L_3 + L_4 = 1.42 + 4.05 = \boxed{5.47} \text{ m}$$

$$b) D_{\text{ACTUAL}} = 1.3 D_{\text{theory}} = 1.3 (5.47) = \boxed{7.11} \text{ m}$$

c) Anchor Force

$$F = P - \frac{1}{2} \gamma' (K_p - K_a) L_4^2$$

$$= 577.96 - \frac{1}{2} (9.19) (3.254) (4.05)^2 = 332.71 \text{ kN/m}$$

Max moment

$$[9.69] \quad \frac{1}{2} \sigma'_1 L_1 - F + \sigma'_1 (z - L_1) + \frac{1}{2} K_a \gamma' (z - L_1)^2 = 0$$

$$\text{Let } (z - L_1) = x \therefore$$

$$\frac{1}{2} \sigma'_1 L_1 - F + \sigma'_1 (x) + \frac{1}{2} K_a \gamma' x^2 = 0$$

$$\frac{1}{2} (19.24) (4) - 332.71 + (19.24 x) + \frac{1}{2} (0.283) (9.19) x^2 = 0$$

$$1.30 x^2 + 19.24 x - 313.47 = 0$$

$$x = 9.80$$

$$z = x + L_1 = 9.80 + 4 = 13.80 \text{ m}$$

$$M_{\text{max}} = \frac{1}{2} \sigma'_1 L_1 \left(x + \frac{L_1}{3} \right) + F(x + L_2) - \sigma'_1 \frac{x^2}{2} - \frac{1}{2} K_a \gamma' x^2 \left(\frac{x}{3} \right)$$

$$= \frac{1}{2} (19.24) (4) \left(9.8 + \frac{4}{3} \right) + 332.71 (9.8 + 2)$$

$$- (19.24) \frac{9.8^2}{2} - \frac{1}{2} (0.283) (9.19) (9.8)^2 \left(\frac{9.8}{3} \right)$$

$$= 428.41 + 3925.98 - 923.9 - 407.97$$

$$M_{\text{max}} = 3022.52 \text{ kN}\cdot\text{m/m}$$

9.7 Cont

c) Using Free Earth Support Method

$$\text{Fig 9.19} \quad \frac{l_1}{L_1 + L_2} = \frac{2\text{m}}{13\text{m}} = 0.154 \quad \phi' = 34^\circ$$

$$\boxed{GF \approx 0.06}$$

$$\text{Fig 9.22} \quad \frac{L_1}{L_1 + L_2} = \frac{4}{3} = 0.31 \quad \boxed{CFL \approx 1.68}$$

$$[9.73] \quad \gamma_a = \frac{\gamma L_1^2 + \gamma' L_2^2 + 2\gamma L_1 L_2}{(L_1 + L_2)^2}$$

$$\gamma_a = \frac{(17)4^2 + (9.19)9^2 + 2(17)(4)(9)}{(4+9)^2} = \boxed{13.26 \text{ kN/m}^3}$$

$$[9.71] \quad F = \gamma_a (L_1 + L_2)^2 (GF) (CFL)$$

$$F = (13.3)(4+9)^2 (0.06)(1.68) = \boxed{226.6 \text{ kN/m}}$$

Max moment

$$\text{Fig 9.20} \quad \frac{l_1}{L_1 + L_2} = 0.154 \quad \phi' = 34^\circ \quad GM = 0.018$$

$$\text{Fig 9.23} \quad \frac{L_1}{L_1 + L_2} = 0.31 \quad CML = 1.08$$

$$[9.72] \quad M_{\max} = \gamma_a (L_1 + L_2)^3 (GM) (CML)$$

$$= (13.26)(4+9)^3 (0.018)(1.08)$$

$$= \boxed{566.33 \text{ kN}\cdot\text{m/m}}$$

9.7 Cont

Use Rowe's moment reduction diagram

Fig 9.24 $E = 210 \times 10^3 \text{ MN/m}^2$

$\& \sigma_{\text{all}} = 210,000 \text{ kN/m}^2$

$H' = L_1 + L_2 + D_{\text{actual}} = 4 + 9 + 7.11 = 20.11 \text{ m}$

$M_{\text{max}} = 566.33 \text{ kN}\cdot\text{m/m}$

[9.74a] $e = 10.91 \times 10^{-7} \left(\frac{H'}{EI} \right)^4$

Section	I	H'	e	log e	S	Md	$\frac{Md}{M_{\text{max}}}$
PZ 27	251.5×10^{-6}	20.11	3.378×10^{-3}	-2.47	162.3×10^{-5}	340.8	0.601
PZ 35	493.4×10^{-6}	20.11	1.722×10^{-3}	-2.76	260.5×10^{-5}	547.1	0.966

Use PZ 35

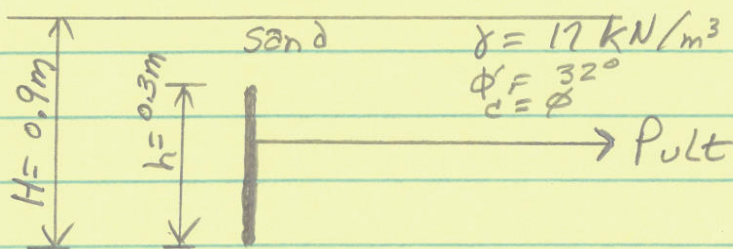
9.12

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A single anchor slab shown below. $H = 0.9\text{m}$, $h = 0.3\text{m}$, $\gamma = 17\text{ kN/m}^3$, $\phi' = 32^\circ$. Calculate the ultimate holding capacity of the anchor slab if the width B is

a) 0.3m b) 0.6m c) 0.9m

(Note center to center spacing $S' = \infty$. Use empirical correlation [9.94])



[9.94] a)
$$P_{ult} = \frac{5.4}{\tan \phi'} \left(\frac{H^2}{A} \right)^{0.28} \gamma A H$$

$$= \frac{5.4}{\tan 32^\circ} \left(\frac{0.9^2}{0.09} \right)^{0.28} (17)(0.09)(0.9)$$

$$P_{ult} = 22.01 \text{ kN}$$

$B = 0.3\text{m}$
 $A = Bh = (0.3)(0.3)$
 $A = 0.09 \text{ m}^2$

b)
$$P_{ult} = \frac{5.4}{\tan 32^\circ} \left(\frac{0.9^2}{0.18} \right)^{0.28} (17)(0.18)(0.9)$$

$$P_{ult} = 36.26 \text{ kN}$$

$B = 0.6\text{m}$
 $A = 0.3 \times 0.6 = 0.18$

c)
$$P_{ult} = \frac{5.4}{\tan 32^\circ} \left(\frac{0.9^2}{0.27} \right)^{0.28} (17)(0.27)(0.9)$$

$$P_{ult} = 48.56 \text{ kN}$$

$B = 0.9$
 $A = (0.3)(0.9) = 0.27$