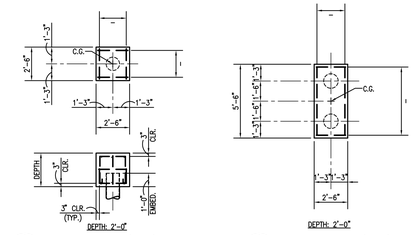
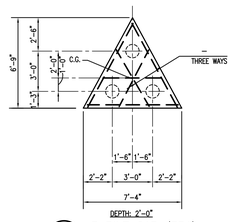
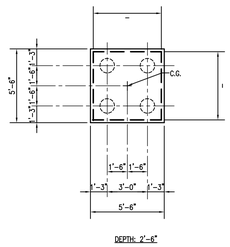


1 S001 1 PILE CAP (1PA) 3/4"=1'-0"

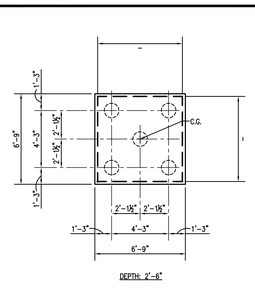
2 S001 2 PILE CAP (2PA) 3/4"=1'-0"



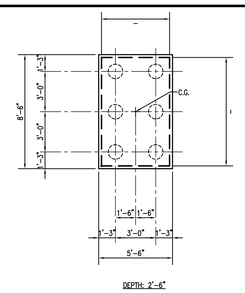
3 S001 3 PILE CAP (3PA) 3/4"=1'-0"



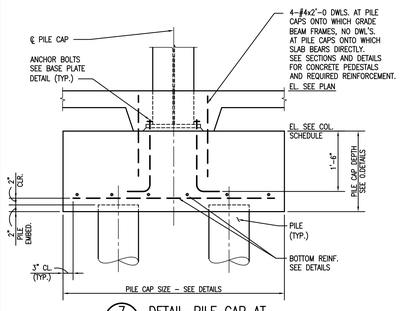
4 S001 4 PILE CAP (4PA) 3/4"=1'-0"



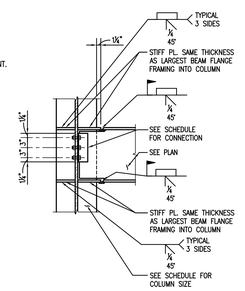
5 S1.0 5 PILE CAP (5PA) 3/4"=1'-0"



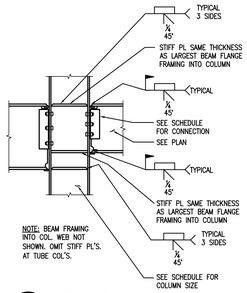
6 S001 6 PILE CAP (6PA) 3/4"=1'-0"



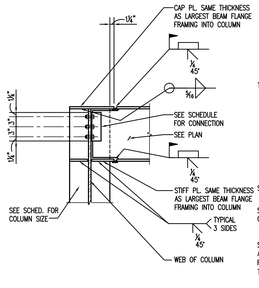
7 S001 DETAIL–PILE CAP AT STEEL COLUMN 3/4"=1'-0"



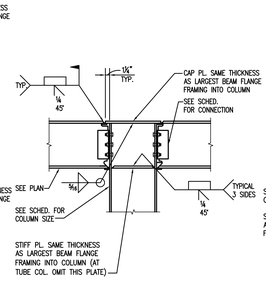
8 S001 DETAIL–TYPICAL MOMENT CONNECTION 3/4"=1'-0"



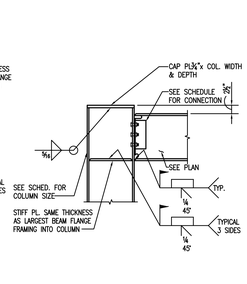
9 S001 DETAIL–TYPICAL MOMENT CONNECTION 3/4"=1'-0"



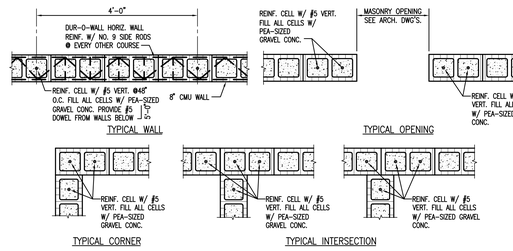
10 S001 DETAIL–TYPICAL MOMENT CONNECTION 3/4"=1'-0"



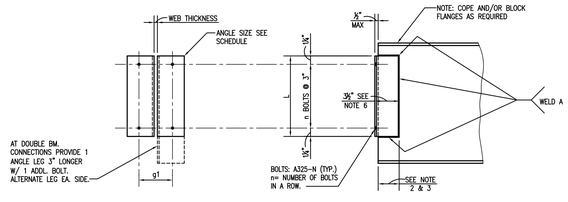
11 S001 DETAIL–TYPICAL MOMENT CONNECTION 3/4"=1'-0"



12 S001 DETAIL–TYPICAL MOMENT CONNECTION 3/4"=1'-0"



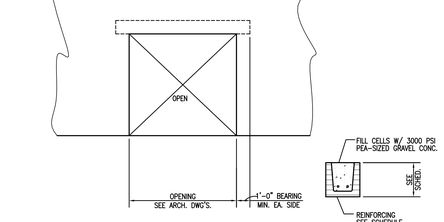
13 S001 TYPICAL DETAIL – CMU VERTICAL WALL REINFORCEMENT 3/4"=1'-0"



14 S001 TYPICAL BEAM CONNECTION DETAIL AND SCHEDULE

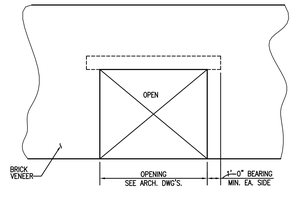
ANGLES 0° TO 33.7°

BEAM SIZE	ANGLE SIZE	L in.	n	g1	WELD A (TYP.)	BOLT (TYP.)
W8's,W10's,C10,C8	L4x3½x5/16	5½	2	5¼	3/16	¾"
W12's,C12	L4x3½x5/16	8½	3	5¼	3/16	¾"
W14's	L4x3½x5/16	11½	4	5¼	3/16	¾"
W16's	L4x3½x5/16	14½	5	5¼	3/16	¾"
W18's	L4x3½x5/16	14½	5	5¼	3/16	¾"
W21's	L4x3½x5/16	17½	6	5¼	3/16	¾"
W24's	L4x3½x5/16	20½	7	5½	¼	¾"
W27's	L4x3½x5/16	23½	8	5½	¼	¾"
W30's	L4x3½x⅝	26½	9	5½	¼	¾"
W33's	L4x3½x⅝	31½	10	5¼	5/16	⅞"



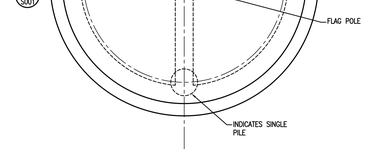
16 S001 LINTEL SCHEDULE–CONCRETE MASONRY UNITS N.T.S.

LINTEL SCHEDULE–CONCRETE MASONRY UNITS	
OPENING SIZE (UP TO & INCLUDING)	LINTEL "U"–BLOCK SIZE AND REINFORCING
4'–0"	8" "U"–BLOCK 2–#4 CONT. BOT.
6'–0"	8" "U"–BLOCK 2–#5 CONT. BOT.
8'–0"	16" "U"–BLOCK 2–#5 CONT. BOT.
12'–0"	2–16" "U" BLOCK 2–#5 CONT. BOT. EA. BLOCK

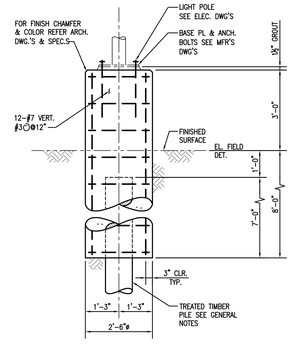


15 S001 LINTEL SCHEDULE–STEEL ANGLES BRICK VENEER OPENINGS N.T.S.

LINTEL SCHEDULE–STEEL ANGLES	
OPENING SIZE (UP TO & INCLUDING)	SIZE OF LINTEL
4'–0"	L3x3x5/16
7'–0"	L4x3x5/16
9'–0"	L5x3x⅜
12'–0"	L7x4x½
16'–0"	L9x4x½



17A S001 / 17B S001 DETAIL – FLAG POLE FOUNDATION 3/8"=1'-0"



18 S001 DETAIL – MAIN LIGHT POLE FOUNDATION 3/8"=1'-0"

17 S001 GENERAL NOTES

1. PILES:
   WOOD–CONCRETE COMPOSITE PILES
   A. SYMBOL ON PLAN: ●
   B. SYMBOL IN DETAILS: (-)
   C. TIP PENETRATION BELOW FIRST FLOOR ELEVATION: (–)60.0 FEET
   (ASSUMES EXIST. GRADE ELEVATION = 0.00 FEET)
   D. LOWER TIMBER SECTION:
   1. TYPE: UNTREATED TIMBER PILE, A.S.T.M. D25
   2. MINIMUM TIP DIAMETER: 7"
   3. MINIMUM DIAMETER 3'–0" FROM BUTT: 12"
   4. LENGTH: 48 FEET
   E. CONCRETE UPPER SECTION:
   1. TYPE: CAST–IN–PLACE CONCRETE
   2. STRENGTH: 4,000 P.S.I. AT 28 DAYS
   3. CASING: SEE SPECIFICATIONS
   4. CONNECTOR: SEE SPECIFICATIONS
   5. LENGTH: AS REQUIRED TO EXTEND FROM BUTT OF TIMBER LOWER SECTION TO CUT–OFF ELEVATIONS.
   6. REINFORCING: REFER TO DRAWINGS
   F. PREDRILL TO ELEVATION (–)10' WITH 6"ø BIT
   G. HAMMER: VULCAN NO. 1 (15,000 ft.–Lbs PER BLOW)
   H. EXPLORATORY PILES: 15 TOTAL – LAST 3 TO BE LOCATED AFTER FIRST 12 ARE DRIVEN.
   I. LOAD TEST : TWO – REFER TO SPECIFICATIONS
   J. DESIGN LOAD: 25 TONS
2. CONCRETE:
   A.C.I. 301–89 SPECIFICATIONS, NORMAL WEIGHT CONCRETE.
3. CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS:
   4,000 P.S.I.
4. REINFORCING STEEL:
   BARS – A.S.T.M. A615, GRADE 60
   WELDED WIRE MESH – A.S.T.M. A185
5. REINFORCING CLEARANCES REQUIRED ARE AS FOLLOWS:
   A. SLABS: ¾" CLEAR BOTTOM, ¾" CLEAR TOP.
   B. BEAMS: 1½" CLEAR BOTTOM FORMED, 3" CLEAR BOTTOM CAST ON EARTH, 1½" CLEAR SIDES AND TOP FORMED, 3" CLEAR SIDES EARTH FORMED, 1½" CLEAR TOP.
   C. COLUMNS: 1½" CLEAR, TYPICAL.
   D. WALLS: ¾" CLEAR, TYPICAL.
6. REINFORCING DETAILS:
   A.C.I. 315 STANDARDS; UNLESS SPECIFICALLY NOTED OR SHOWN ON THE DRAWINGS, BAR LAPS AND CONFIGURATIONS SHALL BE AS FOLLOWS:
   A. CONTINUOUS TOP BARS: HOOK AT NON–CONTINUOUS ENDS; LAP 30 DIAS. AT MID–SPAN.
   B. CONTINUOUS BOTTOM BARS: LAP 6" AT CENTER OF SUPPORT.
   C. TEMPERATURE BARS IN SLAB AND INTERMEDIATE HORIZONTAL BARS IN WALLS AND BEAMS: TENSION LAP SPLICES. SEE TENSION LAP SPLICE TABLE BELOW.
   D. CORNER BARS: PROVIDE CORNER BARS AT EACH OUTSIDE CORNER FOR EACH HORIZONTAL BAR IN WALLS AND BEAMS, CORNER BARS SHALL LAP WITH HORIZONTAL BARS.
   LAP #3 TO #6 BARS 30" EACH WAY AND
   LAP #7 TO #11 BARS 48" EACH WAY. HOOK INSIDE BARS IN WALLS AT ENDS.
   E. TENSION LAP SPLICE TABLE:

LAP SPLICES (INCHES)		
SIZE	LOCATION	
	TOP	OTHER
#3	24	19
#4	32	25
#5	40	31
#6	48	37
#7	70	54
#8	80	62
#9	91	70
#10	102	79
#11	113	87

7. STRUCTURAL STEEL:
   A.I.S.C. SPECIFICATIONS; WIDE FLANGE SECTIONS A.S.T.M. A572, GRADE 50 STEEL, ALL OTHER A.S.T.M. A–36 STEEL, EXCEPT TUBES A.S.T.M. A–500 GRADE B; A–325 BOLTS (¾"ø MIN.); E–70 ELECTRODES, EXCEPT AS NOTED, ALL BEAM CONNECTIONS SHALL BE A.I.S.C. STANDARD FRAMED CONNECTIONS, BOLTED OR WELDED.
   CONNECTIONS TO BE DESIGNED FOR ½ UNIFORM LOAD BEAM CAPACITY FOR PROPER BEAM SPAN UNLESS OTHERWISE INDICATED. CONNECTIONS NOT DETAILED ON THE DRAWINGS, INCLUDING A.I.S.C. STANDARD FRAMED CONNECTIONS, SHALL BE DESIGNED BY A LOUISIANA REGISTERED CIVIL ENGINEER COMMISSIONED BY THE CONTRACTOR AND SHOP DRAWINGS MUST BEAR THAT ENGINEER'S SEAL.
8. METAL DECK FOR FLOORS WITH CONCRETE SLAB:
   A. GALVANIZED STEEL COMPOSITE TYPE DECK, CONTINUOUS OVER 3 OR MORE SPANS; FIELD WELDED TO SUPPORTS AND/OR FASTENED WITH AUTOMATIC END WELDED SHEAR CONNECTORS WHERE INDICATED.
   B. DEPTH: 1½ INCHES
   C. GAGE: 20
   D. MIN. SECTION MODULUS (POSITIVE): 0.222 INxINxIN PER FT.
   E. MIN. SECTION MODULUS (NEGATIVE): 0.234 INxINxIN PER FT.
   F. MIN. MOMENT OF INERTIA: 0.209 INxINxINxIN PER FT.
   G. DESIGN THICKNESS: 0.358 IN.
   H. ALLOWABLE STRESS: 50 KSI
9. METAL ROOF DECK (TYPICAL U.N.O.)
   A. GALVANIZED METAL ROOF DECK, TYPE "B", CONTINUOUS OVER 3 OR MORE SPANS; FIELD WELD TO SUPPORTS.
   B. DEPTH: 1½"
   C. CAGE: 20
   D. MIN. SECTION MODULUS (POSITIVE): 0..234 INxINxIN PER FT.
   E. MIN. SECTION MODULUS (NEGATIVE): 0..247 INxINxIN PER FT.
   F. MIN. MOMENT OF INERTIA: 0.212 INxINxINxIN PER FT.
   G. ALLOWABLE STRESS: 33 KSI
   H. DECK UPLIFT DESIGN: DECK AND ITS FASTENINGS TO ITS SUPPORTING FRAMING SHALL BE DESIGNED (AS A MINIMUM) FOR A NET UPLIFT PRESSURE APPLIED TO THE HORIZONTAL PROJECTED ROOF AREA AS FOLLOWS:
   75 POUNDS PER SQUARE FOOT FOR 10 FOOT BY 10 FOOT AT EACH CORNER;
   45 POUNDS PER SQUARE FOOT FOR A 10 FEET WIDE EDGE STRIP AROUND THE PERIMETER OF ALL ROOFS AND 30 POUNDS PER SQUARE FOOT IN THE REMAINING INTERIOR ROOF AREAS. THIS PRESSURE IS UNFACTORED AND CONSEQUENTLY THE 1/3 INCREASE IN ALLOWABLE STRESS CAN BE APPLIED.
   THE MANUFACTURER SHALL SUBMIT A NOTARIZED AFFIDAVIT STATING FULL DESIGN COMPLIANCE WITH THE UPLIFT CRITERION.
10. OPEN WEB STEEL JOISTS:
    A. S.J.I. SPECIFICATIONS AND CODE OF STANDARD PRACTICE; (1994) FIELD WELDED IN PLACE, MANUFACTURER MUST BE A MEMBER OF STEEL JOIST INSTITUTE.
    B. JOIST UPLIFT DESIGN: JOISTS AND BRIDGING AT ROOF SHALL BE DESIGNED FOR A NET UPLIFT APPLIED TO THE HORIZONTAL PROJECTED ROOF AREA AS FOLLOWS:
    40 POUNDS PER SQUARE FOOT FOR 10 FEET WIDE EDGE STRIP AROUND THE PERIMETER OF ALL ROOFS AND 30 POUNDS PER SQUARE FOOT IN THE REMAINING INTERIOR AREAS. THIS PRESSURE IS UNFACTORED AND CONSEQUENTLY THE 1/3 INCREASE IN ALLOWABLE STRESS CAN BE APPLIED.
    PROVIDE ALL BRIDGING AND BRACING (INCLUDING THE BRIDGING AND BRACING AT THE FIRST BOTTOM CHORD PANEL POINT AT EACH END OF THE JOIST REQUIRED FOR UPLIFT DESIGN) AS REQUIRED BY SJI SPECIFICATIONS. ALL BRIDGING AND BRACING SHALL BE FIELD WELDED. ALL CONTINUOUS BRIDGING AND BRACING MUST BE CONNECTED TO ALL AND ANY BEAMS AND WALLS WHICH ARE PARALLEL TO JOISTS. THE MANUFACTURER SHALL SUBMIT A NOTARIZED AFFIDAVIT STATING FULL DESIGN COMPLIANCE WITH THE UPLIFT CRITERION.
11. OTHER WORK:
    COORDINATE ALL OTHER WORK WITH STRUCTURAL; UNLESS DETAILED OR SPECIFIED, THE ARCHITECT IS TO APPROVE ALL OPENINGS, SLEEVES, EMBEDDED ITEMS ETC. INVOLVED IN STRUCTURAL WORK PRIOR TO THEIR BEING SET. DO NOT CUT OR DRILL HOLES IN STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ARCHITECT. ALL SUCH ITEMS SHALL NOT IMPAIR THE STRUCTURAL INTEGRITY OF THE MEMBER.
12. CAMBER:
    ALL BEAMS SHALL BE CAMBERED UPWARD THE DESIGNATED AMOUNTS SHOWN ON THE PLANS. BEAMS WITHOUT A SPECIFIED CAMBER SHALL BE ORIENTED SUCH THAT ANY INCIDENTAL MILL CAMBER IS UPWARD.
13. AUTOMATICALLY END WELDED SHEAR CONNECTORS (A.E.W.S.C.); AS MANUFACTURED BY NELSON STUDS OR APPROVED EQUAL.
14. DESIGN LOADS:
    A. BUILDING CODE:
    INTERNATIONAL BUILDING CODE 2003
    B. FLOOR LIVE LOAD
    OFFICE: 50 PSF
    ASSEMBLY: 100 PSF
    MECHANICAL/STORAGE: 150 PSF
    C. ROOF LIVE LOAD
    TYPICAL: 20 PSF
    D. ROOF SNOW LOAD
    GROUND SNOW LOAD: 0 PSF
    E. WIND LOAD
    BASIC WIND SPEED: 130 MPH
    WIND IMPORTANCE FACTOR: 1.15
    BUILDING CATAGORY:
    WIND EXPOSURE: B
    INTERNAL PRESSURE COEFFICIENT: 0.18

COMPONENTS AND CLADDING DESIGN PRESSURES (PSF)									
EWA (FT²)	ZONE	1		2		3		4	5
10		26	-42	26	-72	26	-107	45 -49	45 -61
20		25	-40	26	-67	26	-103	44 -47	44 -57
50		21	-39	26	-59	26	-92	42 -45	42 -51
100		18	-38	26	-53	26	-84	38 -44	38 -47

SEE FIGURE 1606.4(2), INTERNATIONAL BUILDING CODE (2003)

    F. EARTHQUAKE DESIGN DATA
    SEISMIC USE GROUP: III
    SPECTRAL RESPONSE COEFFICIENTS
    Sds: 0.083
    Sm: 0.117
    SITE CLASS: E
    BASIC SEISMIC–FORCE–RESISTING SYSTEM: ORDINARY STEEL MOMENT FRAME
    ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
    G. FLOOD LOAD
    LOWEST FLOOR ELEVATION: SEE SITE PLAN
    DRY FLOOD PROOFED ELEVATION: SEE ARCHITECTURAL PLANS
    LOWEST HORIZONTAL STRUCTURAL MEMBER ELEVATION: SEE PLANS
15. DESIGN OF THE BUILDING IS IN ACCORDANCE WITH: UFC 4–023–03, 25 JANUARY, 2005 & FUTURE ADDITIONS FOR PROGRESSIVE COLLAPSE RESISTANCE. UFC 4–010–01, 8 OCTOBER, 2006 & FUTURE ADDITIONS FOR PROGRESSIVE COLLAPSE RESISTANCE.
16. LEVEL OF PROTECTION DESIGN REQUIREMENT : LOW LEVEL OF PROTECTION.
17. METHOD OF PROGRESSIVE COLLAPSE RESISTANCE : TIE FORCE

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LOUISIANA ARMY NATIONAL GUARD
141st FIELD ARTILLERY BATTALION
READINESS CENTER
JACKSON BARRACKS NEW ORLEANS, LOUISIANA

MARK	DATE	DESCRIPTION
1	2/22/06	PROG. REVIEW #1
2	3/10/06	PROG. REVIEW #2
3	3/27/06	PROG. REVIEW #3
4	4/10/06	PROG. REVIEW #4
5	5/03/06	PRELIM. DESIGN

DRAWING ISSUED: 05/03/2006

NGB PROJECT NO.: 220027

PRELIMINARY DESIGN

DRAWING TITLE:
GENERAL NOTES, SCHEDULES AND DETAILS

MORPHY MAKOFSKY INCORPORATED CONSULTING ENGINEERS (504)488-1317 JOB NO. 06018

DRAWING NUMBER:
S001
SHEET 13 OF 106