

PROB # 4.28

$$\text{Beam wt} = \left(\frac{bd}{144}\right)(150) = 1.04167 bd \text{ \#/ft}$$

Using ACI Load Factor Equation 9-1

$$w_u = (1.4)(1.04167)(bd) = 1.4583 bd \text{ \#/ft}$$

$$M_u = \frac{(1.4583)(200)^2 bd}{8} = 7291.67 bd \text{ ft-lbs.}$$

Using $e = 0.0136$ (Table A.7)

$$\frac{M_u}{\phi bd^2} = 685.0 \text{ from Appendix Table A.12}$$

$$bd^2 = \frac{(12)(7291.67 bd)}{(0.90)(685)} = 141.9 bd$$

$$d = 141.9 \text{ in.} = \boxed{11.83 \text{ ft}}$$

✓ g cm^c

PROB # 4.29

$$\text{Beam wt} = \left(\frac{bd}{144}\right)(150) = 1.04167 bd \text{ \#/ft}$$

neglect beam wt. below steel

Using ACI Load Factor Equation 9-1

$$w_u = (1.4)(1.04167 bd) = 1.4583 bd \text{ \#/ft}$$

$$M_u = \frac{(1.4583 bd)(100)^2}{8} = 1822.9 bd$$

$$\text{using } e = \frac{1}{2} e_b = \left(\frac{1}{2}\right)(0.0285) = 0.01425$$

$$\frac{M_u}{\phi bd^2} \text{ Appendix Table A.13} = 747.15$$

$$bd^2 = \frac{(12)(1822.9 bd)}{(0.90)(747.15)} = 32.53 bd$$

$$d = 32.53 \text{ in.} = \boxed{2.71 \text{ ft}}$$

✓ g cm^c

PROB #5.6

Using 4 #10 bars (5.06 in.²)

$$e = \frac{A_s}{b_w d} = \frac{5.06}{16(24)} = 0.0132$$

> $e_{min} = 0.0033$ from Appendix Table A.13

$$T = A_s F_y = (5.06)(60) = 303.6 \text{ k}$$

$$A_c = \frac{T}{0.85 f'_c} = \frac{303.6}{(0.85)(4)} = 89.29 \text{ in.}^2$$

$$< \text{flange area} = (3)(36) = 108 \text{ in.}^2$$

∴ N.A. is in flange

$$a = \frac{A_c}{b_f} = \frac{89.29}{36} = 2.48 \text{ in.}$$

$$c = \frac{a}{\beta_1} = \frac{2.48}{0.85} = 2.92 \text{ in.}$$

$$e_t = \left(\frac{d-c}{c}\right)(0.003) = \left(\frac{24-2.92}{2.92}\right)(0.003)$$

$$= 0.0216 > 0.0050$$

∴ Section is ductile and $\phi = 0.90$

$$\begin{aligned} \phi M_m &= \phi T \left(d - \frac{a}{2}\right) = (0.90)(303.6) \left(24 - \frac{2.48}{2}\right) \\ &= 6219 \text{ in.-k} = \boxed{518.2 \text{ ft-k}} \end{aligned}$$

✓ O.C.M.E

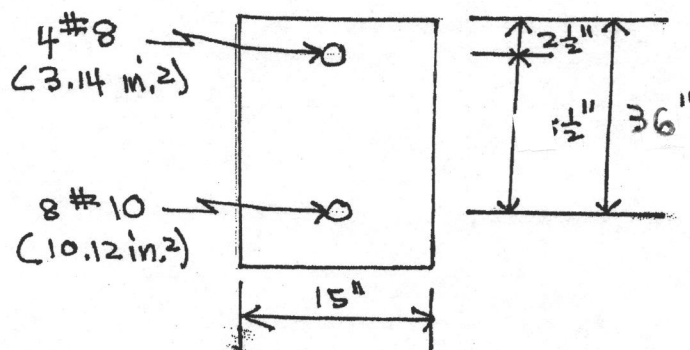
PROB# 5.26

Assuming compression steel yields

$$A_{s2} = 3.14 \text{ in.}^2 = A'_s$$

$$A_{s1} = 10.12 - 3.14 = 6.98 \text{ in.}^2$$

$$a = \frac{A_{s1} f_y}{0.85 f'_c b} = \frac{(6.98)(60)}{(0.85)(4)(15)} = 8.21 \text{ in.}$$



Locating neutral axis and checking strain in comp. steel

$$c = \frac{a}{\beta_1} = \frac{8.21}{0.85} = 9.66 \text{ in.}, \quad a = 0.85(9.66) = 8.21 \text{ in.}$$

$$\epsilon'_s = \left(\frac{9.66 - 2.50}{9.66} \right) (0.003) = 0.00222 > 0.00207$$

∴ Compression steel yields

Design strength of member

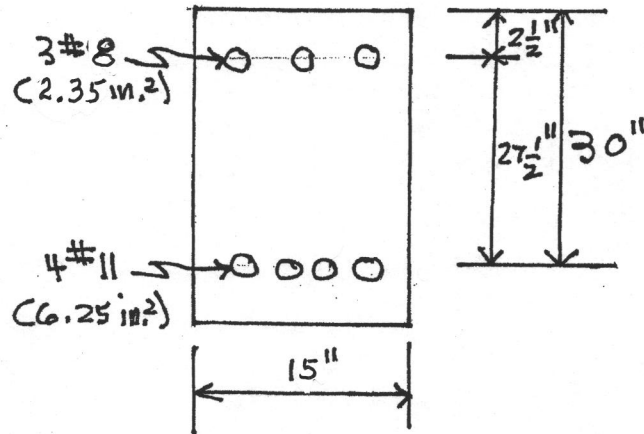
$$\begin{aligned} \phi M_m &= \phi A_{s1} f_y \left(d - \frac{a}{2} \right) + \phi A_{s2} f_y (d - d') \\ &= (0.9)(6.98)(60) \left(36 - \frac{8.21}{2} \right) + (0.9)(3.14)(60)(36 - 2.5) \\ &= 17,702 \text{ in.-lb} = \boxed{1475 \text{ ft.-lb}} \quad \text{v.g.c.m.} \end{aligned}$$

Checking Tensile Steel Strain

$$\begin{aligned} \epsilon_t &= \left(\frac{d - c}{c} \right) (0.003) = \left(\frac{36 - 9.66}{9.66} \right) (0.003) \\ &= 0.0082 > 0.005 \quad \therefore \phi = 0.9 \end{aligned}$$

OK

PROB # 5.28 (1)



Assuming compression steel yields

$$A_{s2} = 2.35 \text{ in.}^2 = A'_s$$

$$A_{s1} = 6.25 - 2.35 = 3.90 \text{ in.}^2$$

$$a = \frac{A_{s1} f_y}{0.85 f'_c b} = \frac{(3.90)(60)}{(0.85)(4)(15)} = 4.59 \text{ in.}$$

Locating N.A. and checking strain in comp. steel

$$c = \frac{a}{\beta_1} = \frac{4.59}{0.85} = 5.40 \text{ in.}$$

$$\epsilon'_s = \frac{5.40 - 2.50}{5.40} (0.003) = 0.00161 < 0.00207$$

∴ Compression steel does not yield

Locating N.A. and computing f'_s

$$c + c' = T$$

$$(0.85)(4)(15)(0.85c) + 2.35 \left[(0.003)(29,000) \left(\frac{c-2.5}{c} \right) \right] = (6.25)(60)$$

$$43.35c + 204.45 - \frac{511.13}{c} = 375$$

PROB# 5.28 (2)

$$43.35c^2 + 204c - 511 = 375c$$

$$43.35c^2 - 171c = 511$$

$$c = 5.92 \text{ in.}$$

$$f'_s = \left(\frac{5.92 - 2.50}{5.92} \right) (0.003)(29,000) = 50.26 \text{ ksi}$$

Is section ductile?

$$\epsilon_t = \frac{d-c}{c} (0.003) = \left(\frac{30 - 5.92}{5.92} \right) (0.003) = 0.0122$$

> 0.0050 \therefore Its ductile and $\phi = 0.9$

Design strength of member

$$A'_s f'_s = A_{s2} f_y$$

$$A_{s2} = \frac{A'_s f'_s}{f_y} = \frac{(2.35)(50.26)}{60} = 1.97 \text{ in.}^2$$

$$A_{s1} = A_s - A_{s2} = 6.25 - 1.97 = 4.28 \text{ in.}^2$$

$$a = \beta_1 c = (0.85)(5.92) = 5.03 \text{ in.}$$

$$\phi M_m = \phi [A_{s1} f_y (d - \frac{a}{2}) + A_{s2} f_y (d - d')]$$

$$= 99 [(4.28)(60)(30 - \frac{5.03}{2}) + (1.97)(60)(30 - 2.5)]$$

$$= 9277 \text{ in.-k} = \boxed{773.1 \text{ ft-k}}$$