



Standard Test Method for Pullout Resistance of Ties and Anchors Embedded in Masonry Mortar Joints¹

This standard is issued under the fixed designation E 754; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

ε¹ NOTE—Keywords were added to this standard in April 2000.

INTRODUCTION

The attachment of masonry walls to building exteriors has been based largely on experience and professional judgment. Codes and standards requirements relating to the attachment of masonry veneer and nonload-bearing masonry walls generally specify the size and spacing of the fasteners to be provided (ties or anchors) rather than the forces to be resisted. In addition, there are very limited data available on the structural performance of most types of fasteners currently used in masonry construction.

This test method is recommended for determining conservative ultimate pullout values of masonry fasteners under conditions that approach those usually found in the upper courses of masonry wall construction, which experience little or practically no vertical load restraint. Its use is recommended until more comprehensive methods are developed which can evaluate, economically, the capacity of masonry ties and anchors to resist pullout loads, giving due consideration to service parameters such as expanse of surrounding wall area and in-plane dead loads.

1. Scope

1.1 This test method provides procedures for determining the ability of individual masonry ties and anchors to resist extraction from a masonry mortar joint.

1.2 Two laboratory testing procedures are provided for use with test specimens which consist of a masonry tie or anchor embedded in mortar between twin stack-bonded masonry units.

1.2.1 *Procedure A*—For use with small (brick-size) masonry units.

1.2.2 *Procedure B*—For use with large (block-size) masonry units.

1.3 *This standard does not purport to address the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.*

2. Referenced Documents

2.1 ASTM Standards:

C 67 Test Methods for Sampling and Testing Brick and Structural Clay Tile²

C 109 Test Method for Compressive Strength of Hydraulic Cement Mortars (Using 2-in. or 50-mm Cube Specimens)³

C 140 Test Method of Sampling and Testing Concrete Masonry Units²

C 144 Specification for Aggregate for Masonry Mortar²

C 270 Specification for Mortar for Unit Masonry²

E 4 Practices for Force Verification of Testing Machines⁴

E 6 Terminology Relating to Methods of Mechanical Testing⁴

E 171 Specification for Standard Atmospheres for Conditioning and Testing Materials⁵

E 575 Practice for Reporting Data from Structural Tests of Building Constructions, Elements, Connections, and Assemblies⁶

¹ This test method is under the jurisdiction of ASTM Committee E-6 on Performance of Buildings and is the direct responsibility of Subcommittee E06.13 on Structural Performance of Connections in Building Constructions.

Current edition approved Aug. 1, 1980. Published January 1981.

² *Annual Book of ASTM Standards*, Vol 04.05.

³ *Annual Book of ASTM Standards*, Vol 04.01.

⁴ *Annual Book of ASTM Standards*, Vol 03.01.

⁵ *Annual Book of ASTM Standards*, Vol 15.09.

⁶ *Annual Book of ASTM Standards*, Vol 04.11.

3. Terminology

3.1 *Definitions*—For definitions of terms used in this test method, refer to Terminology E 6.

4. Summary of Test Method

4.1 These procedures provide for a static pullout test of a tie or anchor embedded in a hardened mortar joint between two masonry units.

5. Significance and Use

5.1 This test method is intended to provide a simple inexpensive means of generating conservative, comparative data on pullout strengths of various ties and anchors used with different types of masonry units and mortars. This test method is recommended for such use until economical, improved methods can be developed to simulate service conditions more inclusively.

6. Apparatus

6.1 *Procedure A (Using Brick-Size Masonry Units):*

6.1.1 The testing machine shall conform to the requirements of Practices E 4.

6.1.2 Auxiliary pulling apparatus is shown in Fig. 1 and Fig. 2. These fixtures are provided with swivel joints and roller bearings to eliminate lateral restraint and bending when applying the pullout loads. The apparatus shall be designed to have enough strength and stiffness to prevent its yielding (in order to minimize apparatus deformation) and to maintain uniform distribution of the axially applied test loads until failure of the specimen occurs.

6.2 *Procedure B (Using Block-Size Masonry Units)*—Apparatus for this procedure is shown in Fig. 3 and Fig. 4. Its special purpose is to avoid handling an unwieldy specimen in apparatus such as that used in Procedure A by applying pullout loads to the specimen placed on a convenient work surface. The apparatus shall be of sufficient strength and stiffness to minimize bending deformations that would tend to promote a flexural bond failure of the mortar joint; and it shall have the capacity to develop the pullout failure load of the fastener without yielding.

6.3 For both Procedures A and B, the apparatus is made from common rolled metal sections and other stock items. The knockdown jig used for pulling the end of the fastener protruding from the masonry also serves as a reusable mold for casting a gypsum head on that end of the fastener. The mold/jig is reassembled around the gypsum pulling head when testing.

6.4 *Instrumentation*—Displacement of the fastener relative to the masonry shall be measured by a dial gage or by an electronic displacement transducer (for example, a linear variable differential transformer (LVDT)). The instrument used shall be capable of reading 0.001 in. (0.025 mm) and be accurate to 1 %.

7. Test Specimen

7.1 *Sampling of Fasteners (Masonry Ties or Anchors)*—Test at least five specimens of a given type of fastener for each given combination of masonry constituent materials. More specimens may be needed depending on the variability of the masonry constituents. If necessary, determine suitable statistical parameters to evaluate the composite variability and to

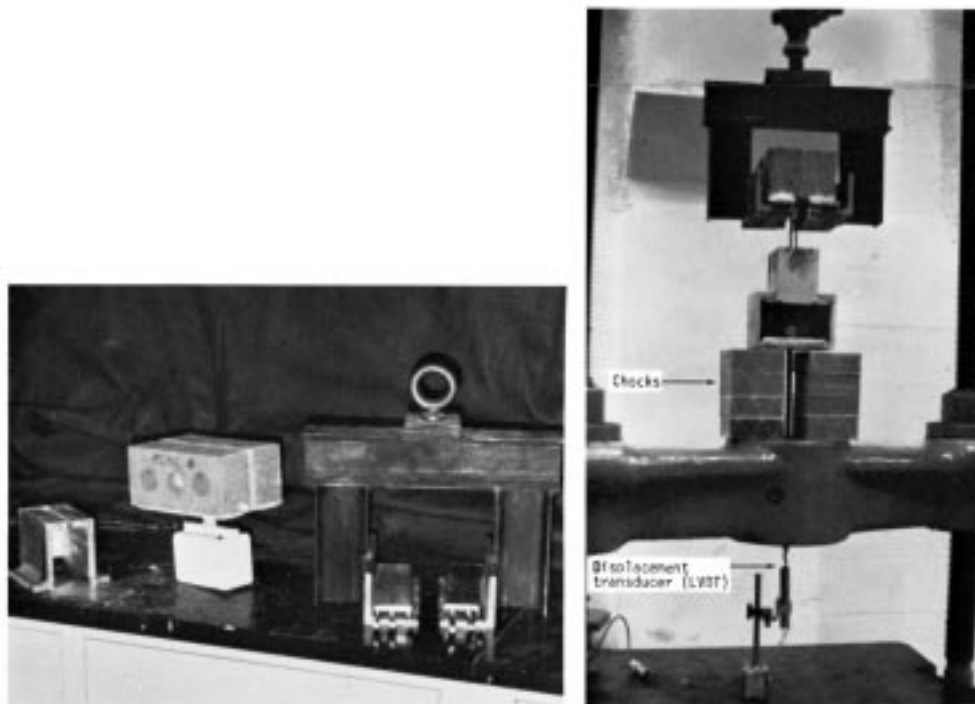
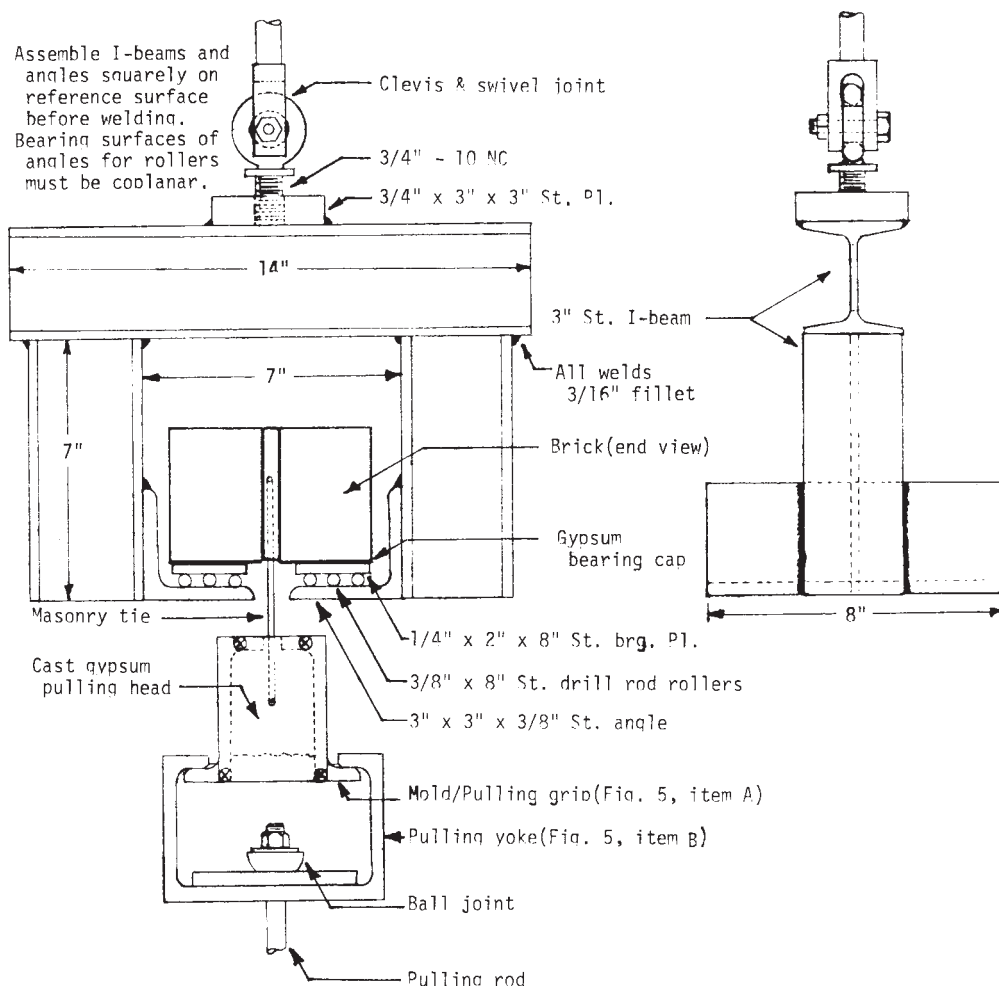


FIG. 1 Procedure A Apparatus and Setup



NOTE 1—1 in. = 25.4 mm.

FIG. 2 Procedure A Apparatus and Setup

select the number of specimens needed for obtaining a given minimum precision in determining the capacity of the fastener.

7.2 Sampling and Testing of Masonry Units—Select representative masonry units and test according to applicable standard test methods (Methods C 67 and C 140) to determine basic properties.

7.3 Sampling and Testing of Mortar—Use mortar that is representative of that specified for the intended construction; or, for comparative research data, select one of the types of mortar specified in Specification C 270. Determine the compressive strength of the mortar in accordance with Test Method C 109 except that the sand shall conform to Specification C 144. Allow the mortar to cure undisturbed for 24 h in the molds. Release the cubes and store them in the same atmosphere as the pullout test specimens, as specified in 7.5. Determine and record the 28-day compressive strength of the mortar (at least three cubes for each set of five pullout specimens), and the sieve analysis of the sand used.

7.4 Fabrication—Construct at least five specimens for a given combination of variables. Make each specimen a singlewythe (one-tier), stack-bonded, two-unit prism containing one tie or anchor. Unless otherwise specified, make the single mortar joint $\frac{3}{8}$ in. (10 mm) thick. Embed the fastener as

required for its proper use at the midlength of the masonry units. (Unless otherwise specified, use a full bed of mortar on solid masonry units (without furrowing the mortar bed), and fully bed the face shells of hollow units.) Strike the mortar joint flush with the faces of the masonry units without tooling. Mark the lower masonry unit for future reference (8.4.2).

7.5 Curing and Handling:

7.5.1 Cure all pullout specimens and corresponding mortar cube specimens for 28 days. Unless other conditions are specified, cure and test all specimens in laboratory air maintained at 73°F (23°C) and 50 % relative humidity as provided by Specification E 171.

7.5.2 To prevent accidental failure of mortar bond in the joints, do not disturb the pullout specimens for at least 14 days. After that time they may be handled with care in order to complete capping operations before testing at a specimen age of 28 days.

8. Procedure

8.1 Gypsum Capping Test Specimens—To provide a smooth bearing surface on the fastener side of the fabricated specimen, apply a gypsum plaster cap. Taking care to prevent the gypsum paste from bridging the mortar joint, cap the

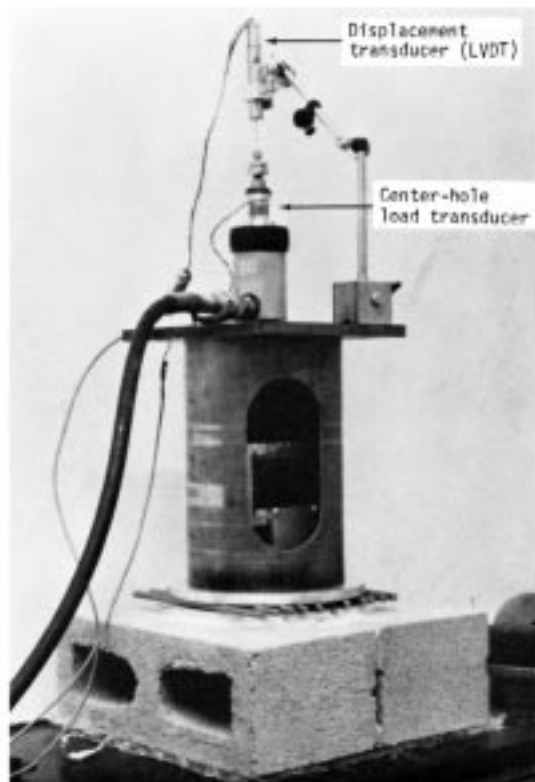


FIG. 3 Procedure B Apparatus and Setup

masonry unit faces beyond which the fastener protrudes, in the same general way described under “Gypsum Plaster Capping” of Methods C 67 or C 140, except:

8.1.1 Procedure A Specimens—For a casting surface on which to spread the gypsum plaster, use the notched metal capping plate (Fig. 5) elevated on supports that are high enough to accommodate the protruding fastener in a hanging position.

8.1.2 Procedure B Specimens—Cap specimens made of block-size units in an inverted position with the fastener side up. To cap the test load-bearing surface of these specimens, bed the half-ring plates (two halves held coplanar by attachment to a whole-ring plate: Fig. 6) in a thin coat of gypsum paste that has been spread on the masonry unit faces on the side toward the fastener.

8.2 Casting the Gypsum Pulling Heads—In order to grip the end of the fastener with the pulling apparatus, cast a slug of high-strength gypsum around the protruding end in an upright position (for example, Fig. 6) using the four-piece mold shown in Fig. 7 (Part A). For casting, support the assembled mold so as to enclose the maximum available length of the fastener, but leaving exposed the necessary length to accommodate the pulling apparatus when testing (Fig. 2 and Fig. 4). The mold must be coaxial with the fastener. If necessary, mechanical anchoring of the fastener in the gypsum casting can be increased by bending the end of the fastener, or by attaching (for example, by welding) a small suitable auxiliary anchor. Coat the inner surfaces of the mold with a light film of oil, and cast the head using high-strength capping gypsum. Before

removing the mold after the gypsum has set, mark the head to identify its position relative to the mold; this will make possible a matching fit when reassembling the mold/jig on the gypsum pulling head for testing. Casting must be completed in time to allow the gypsum to attain sufficient strength for testing the pullout specimens at an age of 28 days.

8.3 Pullout Testing:

8.3.1 Pulling-Head Grip—In both Procedures A and B, reassemble the mold/pulling jig on the gypsum head of the test specimen oriented so that originally matching surfaces are in contact.

8.3.2 Procedure A (Small, Masonry-Unit Specimens)—Center the specimen in the auxiliary pulling apparatus and universal testing machine as shown in Fig. 1 and Fig. 2. The specimen must also rest symmetrically on the bearing plates and rollers.

8.3.3 Procedure B (Large, Masonry-Unit Specimens)—Place the specimen on a sponge rubber mat and assemble the special pulling apparatus centrally on the specimen, positioning the roller bearings parallel to the mortar joint (Fig. 3 and Fig. 4).

NOTE 1—Detach the two half-ring plates which were used for capping from the whole-ring plate and reinstall on the plaster caps; these three plates serve as bearing plates for the rollers between them (Fig. 3 and Fig. 4).

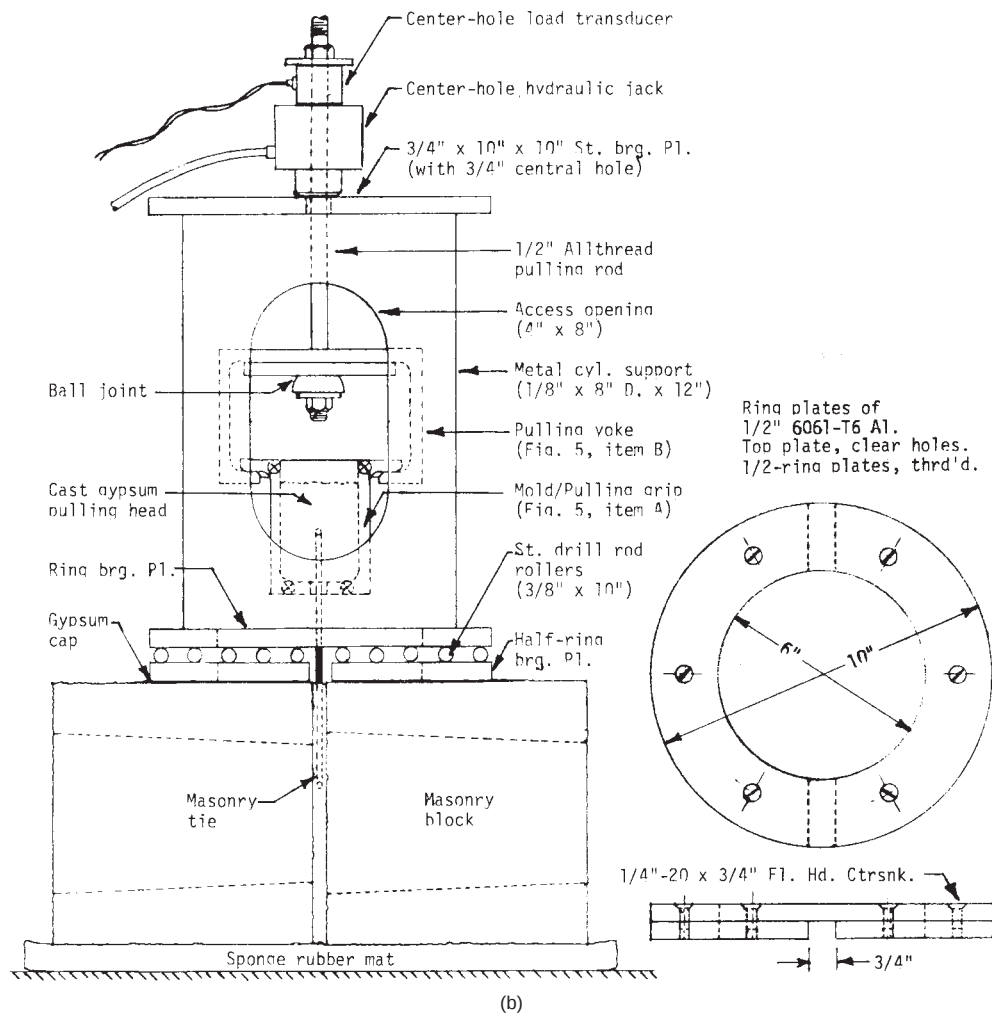
8.3.4 Gage Mounting—Mount the displacement measuring gage to detect the movement of the end of the pulling rod relative to the masonry (Figs. 1-4). This movement is taken to be equal to the displacement of the fastener under load, relative to the surrounding masonry.

NOTE 2—For the magnitude of the loads and displacements accompanying these test conditions, it is not likely that elongation of the apparatus will be significant. However, if necessary, such measurements can be corrected by calibration-test data for an equivalent setup using a rigid mock-up (steel block and rod) in place of the masonry and fastener specimen.

8.3.5 Loading—Apply an initial load to the specimen of about 5 % of the fastener’s estimated maximum pullout capacity in order to bring all elements of the pulling apparatus into tensile contact. The initial load value may be adjusted for subsequent tests of replicate specimens, depending on the results of preceding tests. Set the displacement measuring gage to zero while maintaining the initial load constant. Continue to apply increasing load at the rate of 20 % of the estimated fastener maximum capacity per minute until failure occurs. Record at least eight intermediate load/displacement measurements in addition to the initial and maximum values at approximately equal load intervals.

NOTE 3—A convenient way of recording the entire test load/displacement history is by means of an electronic X-Y plotter used with an LVDT displacement transducer and an electronic (a) center-hole load transducer in line with the pulling apparatus, or (b) pressure transducer tapped into the hydraulic system of the loading apparatus (if such is used). See Fig. 8.

8.4 Observation of Failure—Note and record whether the failure was:



NOTE 1—1 in. = 25.4 mm.

FIG. 4 Procedure B Apparatus and Setup

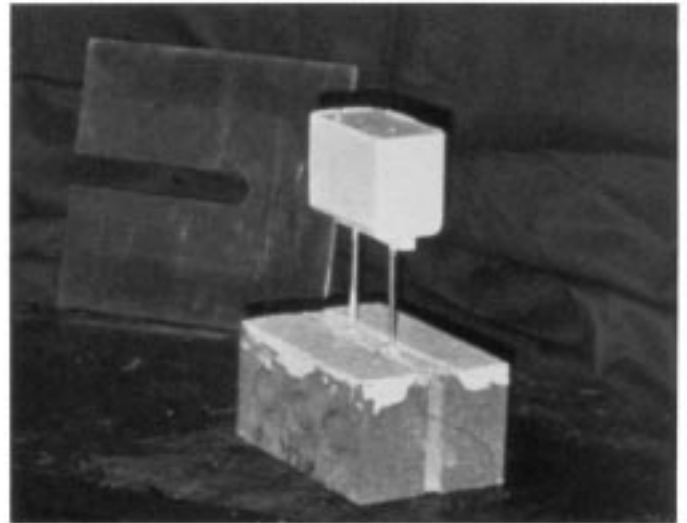
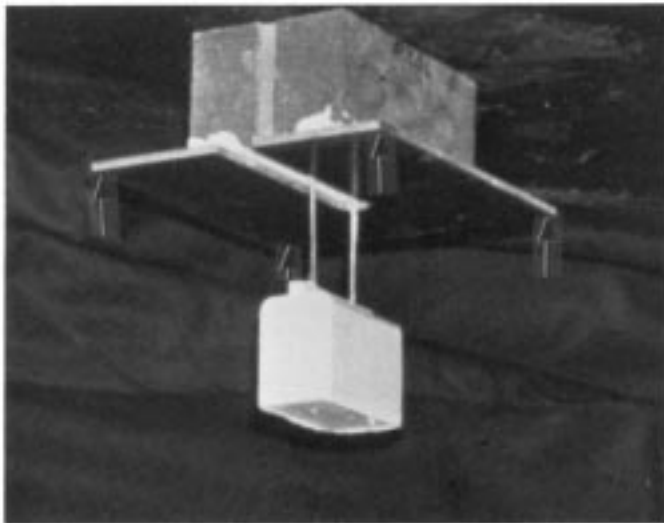


FIG. 5 Notched Metal Plate Used for Capping Specimens in Procedure A

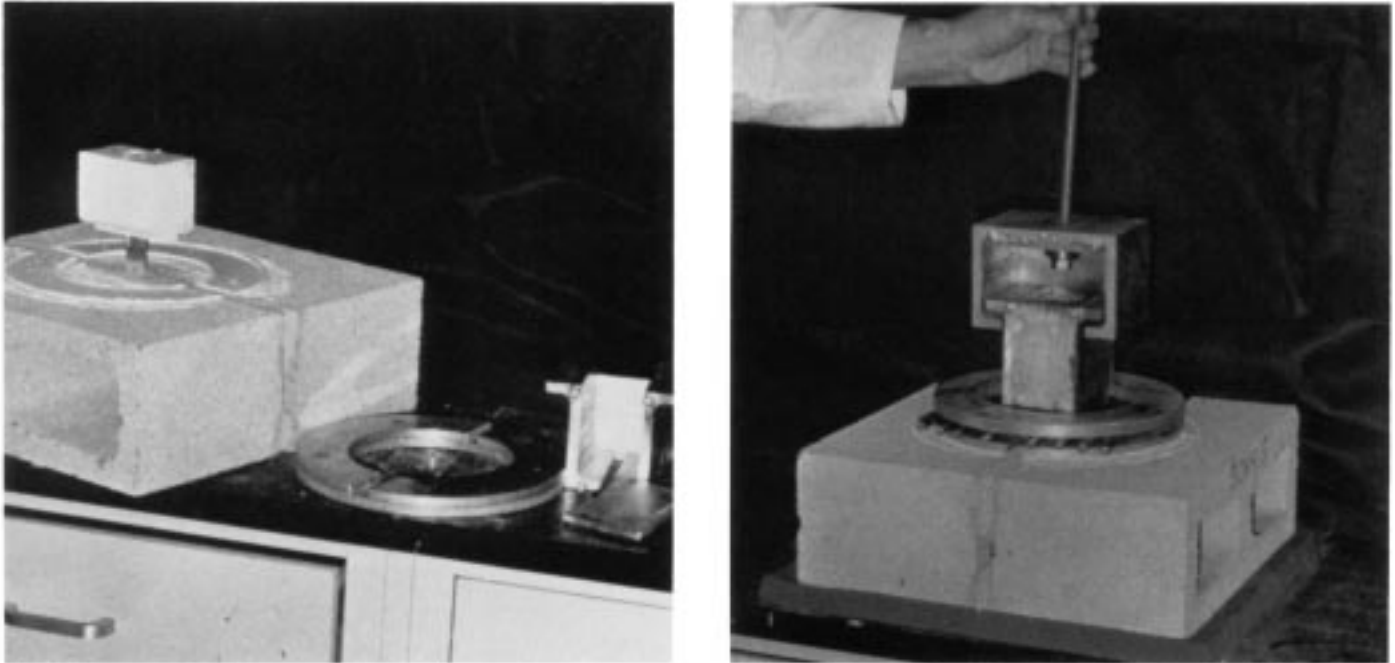
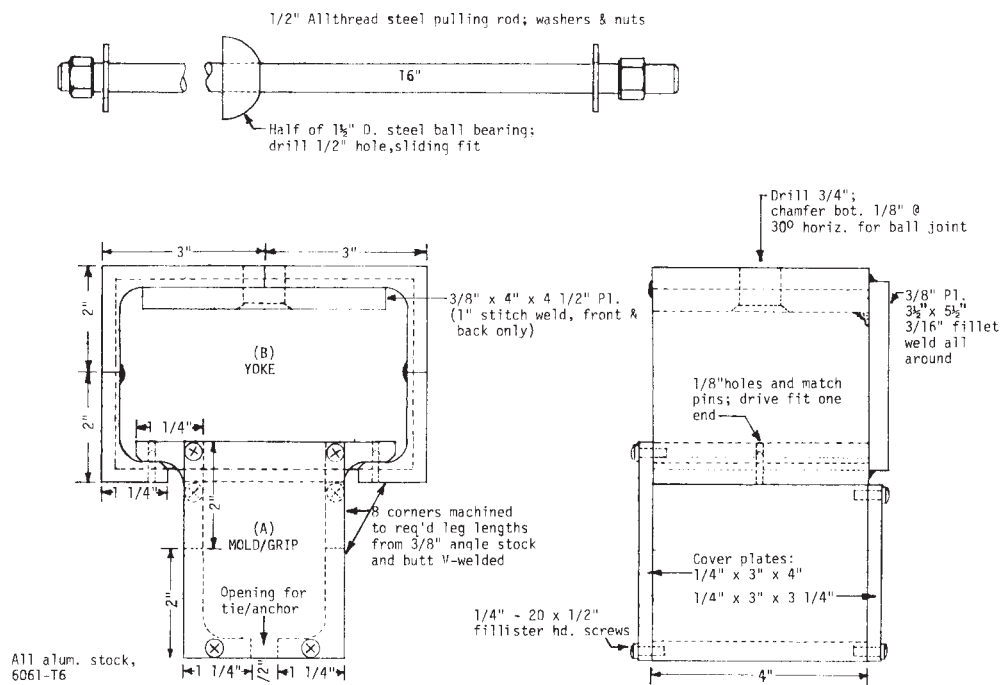


FIG. 6 Half-Ring and Whole-Ring Plates Used for Capping and Load Bearing in Procedure B



NOTE 1—1 in. = 25.4 mm.

FIG. 7 Mold/Grip (A) and Yoke (B) Used in Procedures A and B

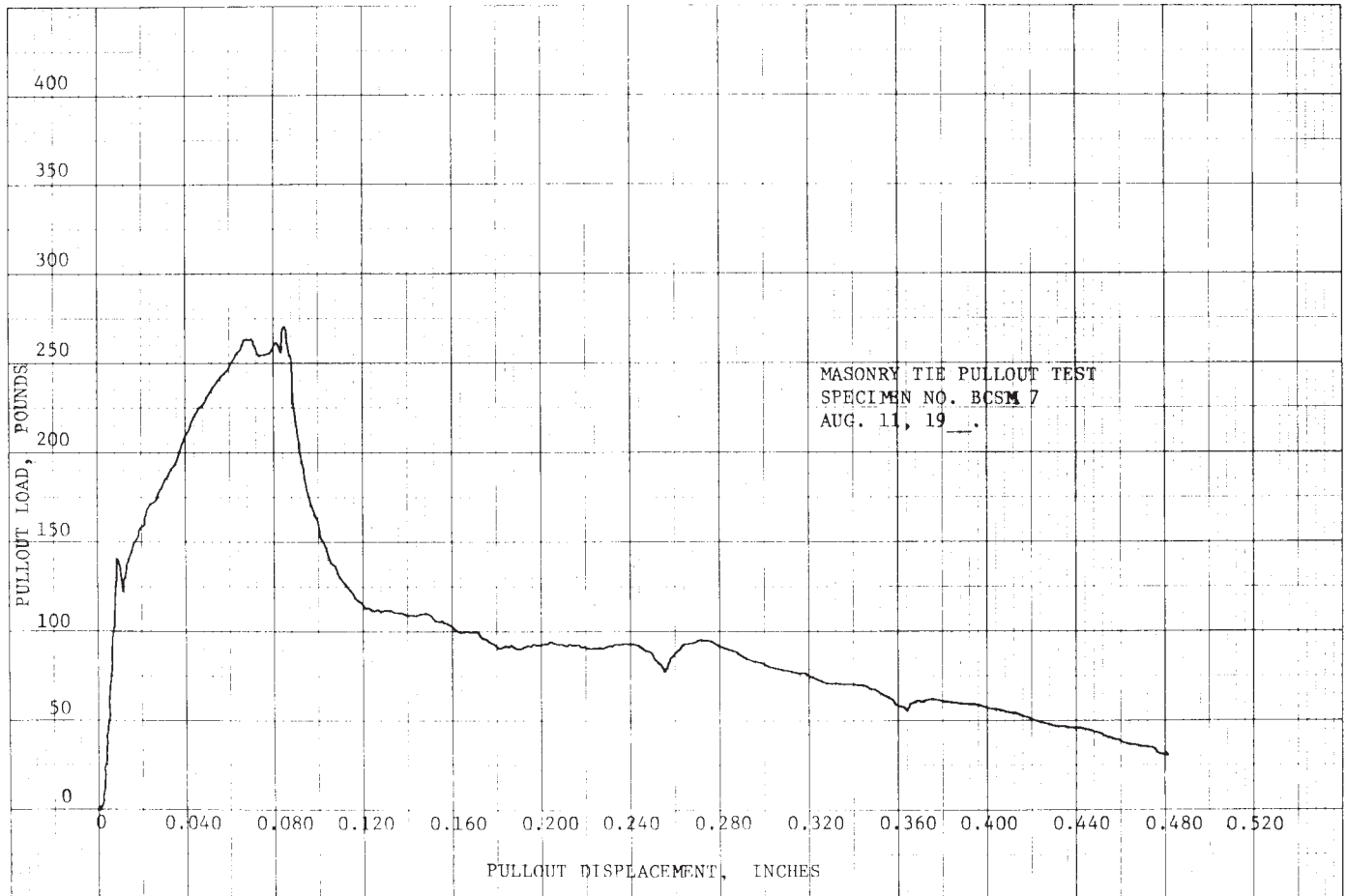
8.4.1 Failure of the bond between the tie or anchor and the mortar, resulting in a pullout of the fastener without separation of the masonry units;

8.4.2 Failure of the bond between a masonry unit and the mortar (note also, relative to the fastener, which of the two interfaces of the joint separated) resulting in collapse of the test specimen; or

8.4.3 Yielding or fracture of the fastener.

9. Calculation

9.1 Determine the adjusted displacement measurements by plotting observed displacement measurements (8.3.5) in function of applied loads and extrapolating a smooth curve through the data points back to an adjusted origin at zero load.



NOTE 1—1 in. = 25.4 mm.

FIG. 8 Autographic Record of Pullout Test

Determine the adjusted displacement at any load from the plot relative to the adjusted origin.

9.2 For each set of five specimens (or more if specified: 7.1) calculate the arithmetic means of the maximum loads and of the corresponding displacements.

10. Report

10.1 The report shall follow the general outline of Practice E 575 and shall specifically include the following information in addition to that which is mandatory:

- 10.1.1 Test sponsor and test agency,
- 10.1.2 General description of fastener and its use in service,
- 10.1.3 Identification of the masonry units: manufacturer, type, material, dimensions, specified physical properties, and photograph or drawing,

10.1.4 Compressive strength and other specified physical properties of the mortar; sand sieve-analysis and other specified physical properties of the constituent materials together with manufacturer's identification and material designation,

10.1.5 Brief description of specimen fabrication, including placement of fastener and its depth of embedment, method of bonding, joint thickness, and specimen dimensions,

10.1.6 Age of specimen at time of test,

10.1.7 Individual and average maximum pullout loads,

10.1.8 Individual and average maximum adjusted pullout displacements,

10.1.9 Summary of findings, and

10.1.10 Names of observers and signatures and if required, the professional certificate or seal of responsible persons.

11. Precision and Bias

11.1 The within-laboratory and between-laboratory precision and bias of this test method have not been established.

12. Keywords

12.1 anchors; masonry; mortar joints; ties

The American Society for Testing and Materials takes no position respecting the validity of any patent rights asserted in connection with any item mentioned in this standard. Users of this standard are expressly advised that determination of the validity of any such patent rights, and the risk of infringement of such rights, are entirely their own responsibility.

This standard is subject to revision at any time by the responsible technical committee and must be reviewed every five years and if not revised, either reapproved or withdrawn. Your comments are invited either for revision of this standard or for additional standards and should be addressed to ASTM Headquarters. Your comments will receive careful consideration at a meeting of the responsible technical committee, which you may attend. If you feel that your comments have not received a fair hearing you should make your views known to the ASTM Committee on Standards, at the address shown below.

This standard is copyrighted by ASTM, 100 Barr Harbor Drive, PO Box C700, West Conshohocken, PA 19428-2959, United States. Individual reprints (single or multiple copies) of this standard may be obtained by contacting ASTM at the above address or at 610-832-9585 (phone), 610-832-9555 (fax), or service@astm.org (e-mail); or through the ASTM website (www.astm.org).