

# Design of Eccentrically Loaded Columns Using Interaction Diagrams

11/8/10

If we prepared them for all combinations there would be too many.

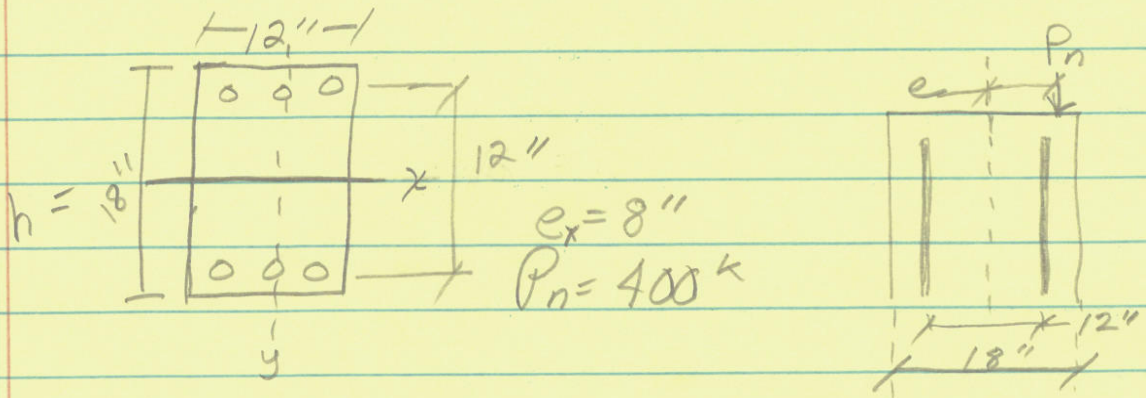
Column Design  
Using Interaction  
Diagrams  
Examples

$$x\text{-axis: } \frac{P_n e}{f'_c A_g h} = R_n$$

$$y\text{-axis: } \frac{P_n}{f'_c A_g} = k_n$$

Warning: Be sure the picture agrees with the column being consider'd

Example: Use the interaction curves in Appendix A to select reinforcing  $f'_c = 4 \text{ ksi}$   $f_y = 60 \text{ ksi}$



① Structural Analysis (done already)

$$P_n = 400 \text{ k}$$

$$M_{n_x} = P_n \cdot e_x = (400 \text{ k})(8'') = 3200 \text{ in-k}_x$$

② Select column cross section  
- already done

### ③ Analyze Column

$\frac{b}{h}$  about bending axis

Left side of chart

$$\gamma = \text{ratio} = \frac{12''}{18''} = 0.667$$

$$K_n = \frac{P_n}{f'_c A_g} = \frac{400k}{4 \text{ ksi } (12'' \times 18'')} = 0.463$$

Bottom of chart

$$R_n = \frac{P_n e}{f'_c A_g h} = \frac{(400k)(8'')}{(4 \text{ ksi})(12'' \times 18'')(18'')} = 0.206$$

from chart 0.60

interpolate

from chart 0.70

$\gamma$	0.600	0.667	0.70
$e_g$	0.03	0.026	0.024

$$A_g = \rho_g b h = 0.026(12)(18)$$

$$A_g = 5.62 \text{ in}^2 \quad \text{use 6 \#9 } (A_g = 6.0 \text{ in}^2)$$

Check fit of bars [7.6.3]

Clear (min. 1.5 db or 1 1/2" [7.6.3])

Distance (4/3 max agg. size [3.3.2])

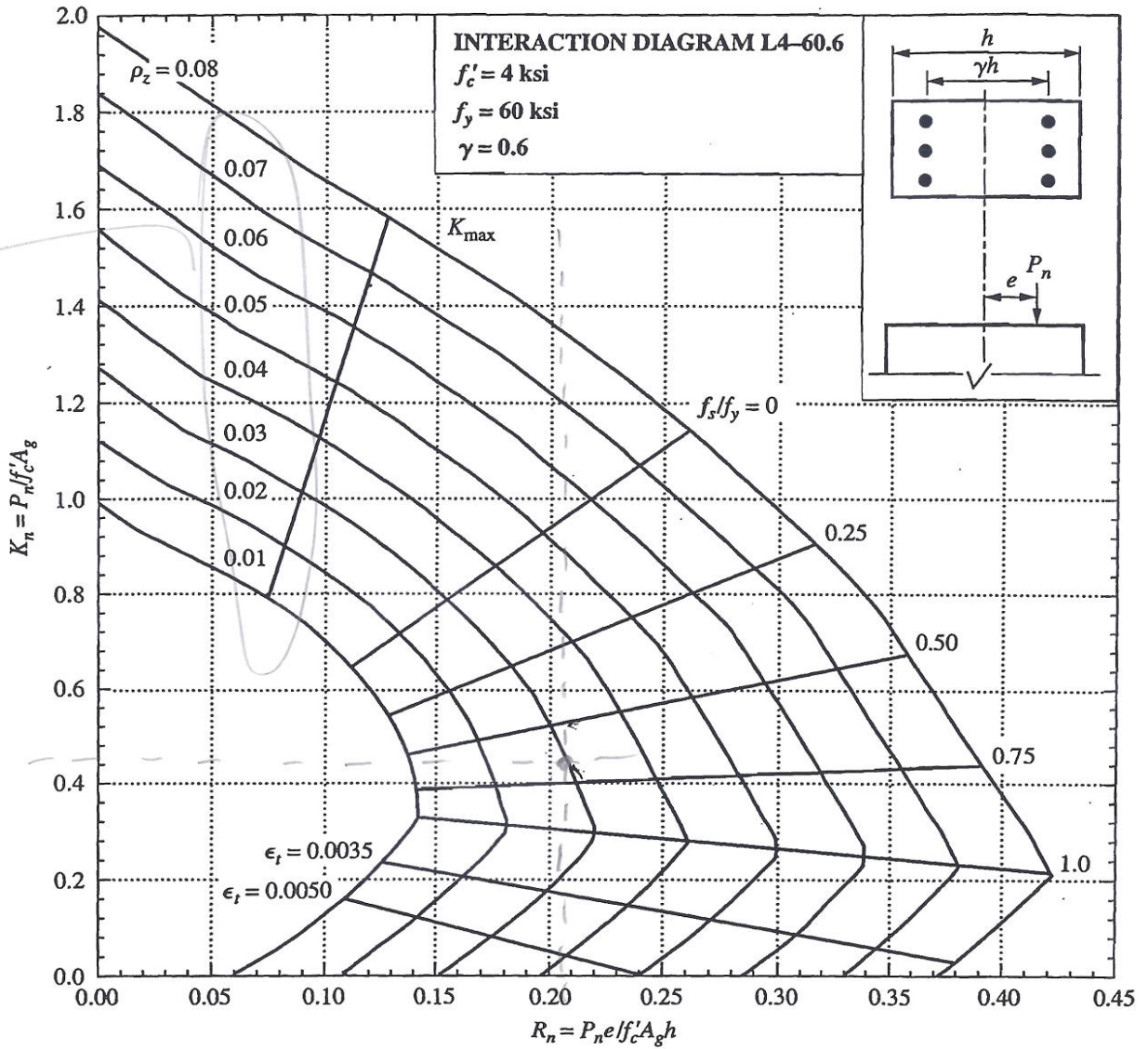
$$1.5(1.25) = 1.6875$$

$$1.5 \longrightarrow 1.5''$$

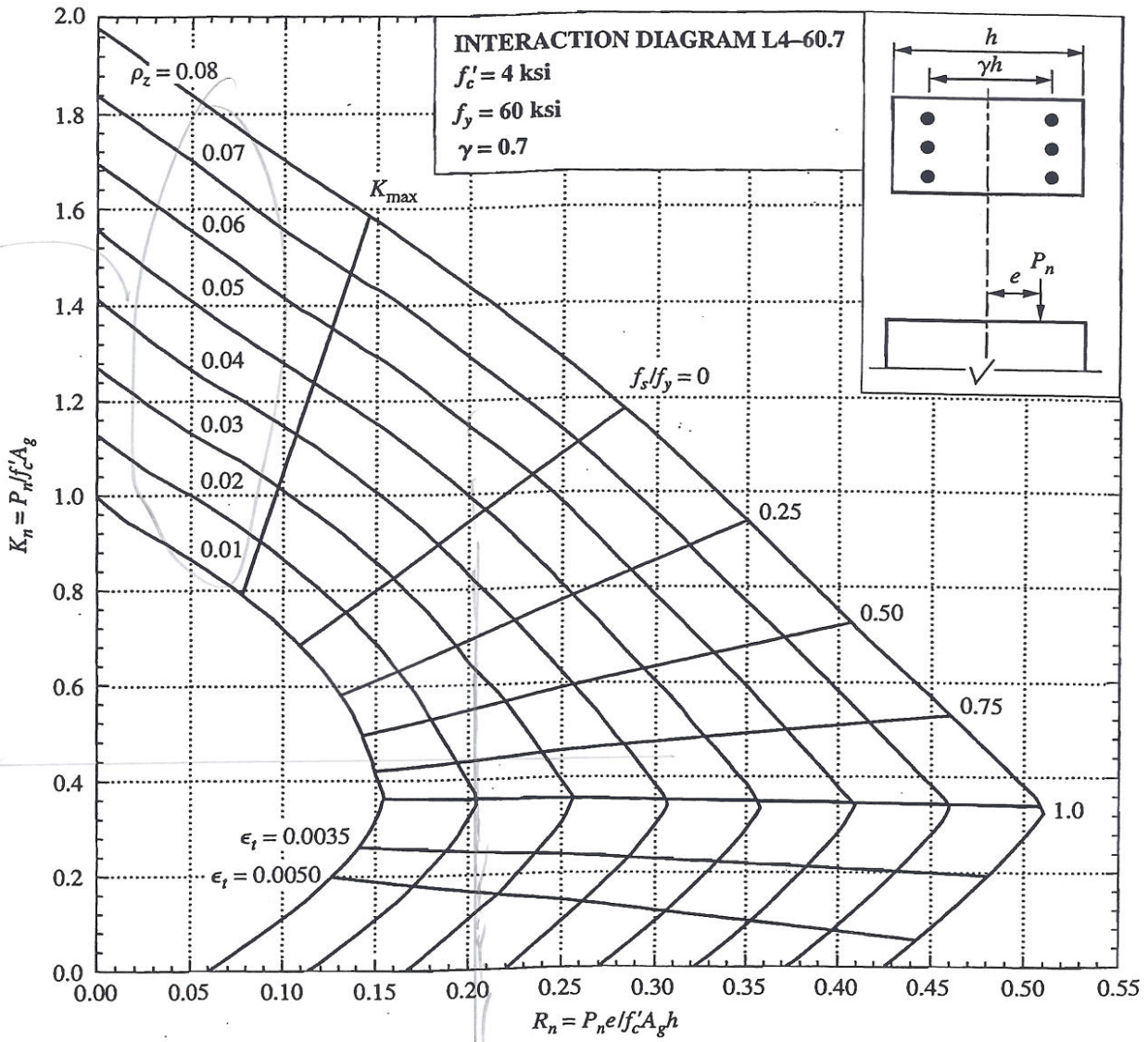
$$4/3(1'') = 1.25$$

$$b_{\text{req}} = \underbrace{3(1.25)}_{\text{bars}} + \underbrace{2(3/8)}_{\text{ties}} + \underbrace{2(1.5)}_{\text{Spacing between bars}} + \underbrace{2(1)}_{\text{cover}}$$

$$= 9.125 < 12'' \quad \text{OK}$$



**Graph 2** Column interaction diagrams for rectangular tied columns with bars on end faces only. (Graphs 2 through 13 are published with the permission of the American Concrete Institute.)



**Graph 3** Column interaction diagrams for rectangular tied columns with bars on end faces only.

Ties  
#3 bars for #9 longitudinal bars

$$Tie S_{\max} = \min \begin{cases} 16 d_{\text{bar}} = 16(1.125) = 18'' \\ 48 d_{\text{tie}} = 48(\frac{3}{8}'') = 18'' \\ b = 12'' \end{cases}$$

$$S_{\max} = 12''$$

Check Shear

$$V_c = 2 \left[ 1 + \frac{N_u}{2000 A_g} \right] \sqrt{f'_c} b_w d \quad [eq 11-4]$$

\*

\* Accounts for increased capacity due to aggregate inter lock

