



**Tiger Brand
Wire Rope
Engineering
Hand Book**



United States Steel

525 William Penn Place
Pittsburgh, Penna. 15230

I N T R O D U C T I O N

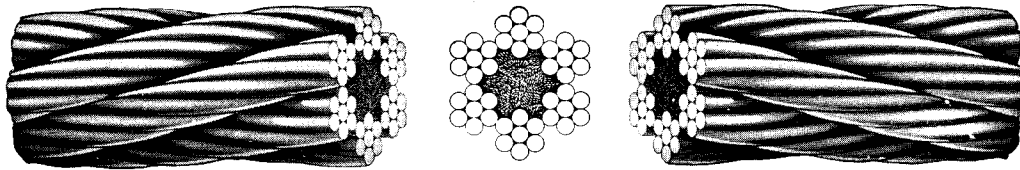
THE WIDESPREAD use of wire rope in almost every type of industry—and the many ramifications and variations of such service—requires constant and up-to-date knowledge of every technical advance pertaining to the construction of wire rope and to its application. ¶ To make available such information to engineers—and to others who have need for exact facts relating to the subject—is the purpose of this Handbook. ¶ It is our belief that you will find the contents not only of very real help—but that you will recognize in it a broadness of scope and a completeness that could only result from knowledge gained through many years of leadership in this important field. ¶ This leadership has been achieved because of steadfast adherence to unvarying standards of quality—and because of ability to provide a perfect answer to the many usual and unusual application problems that are constantly occurring. ¶ In the future—as in the past—the same exceptional degree of quality and service will be maintained—two vital factors that have made us the world's largest manufacturers of wire rope.

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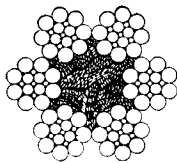
WIRE ROPE CONSTRUCTIONS

Left Lay

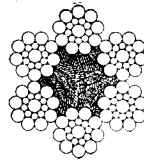
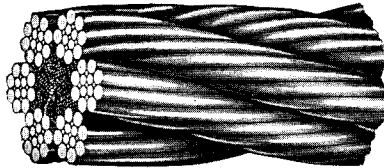
Right Lay



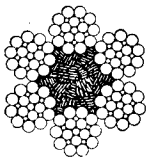
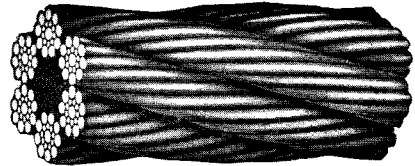
6 x 7—Regular Lay



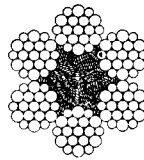
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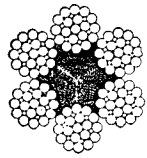
6 x 19—Seale Patent



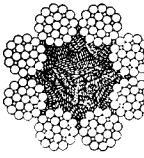
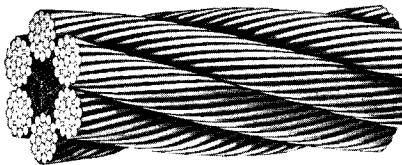
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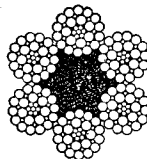
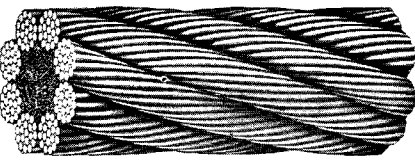
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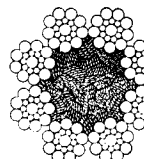
6 x 19—Type N



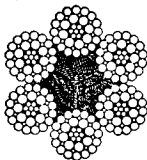
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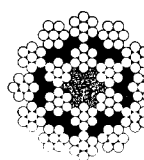
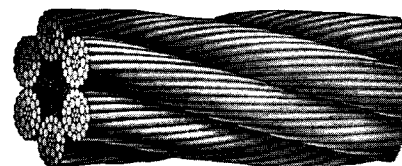
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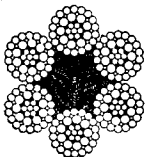
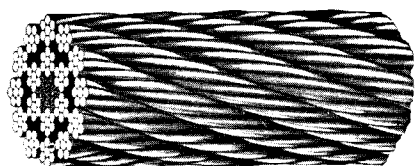
8 x 19—Seale Patent



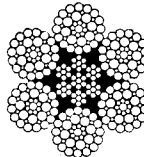
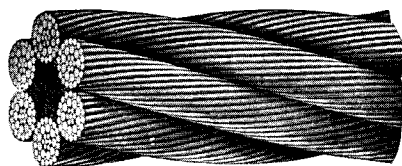
6 x 33—Type R



18 x 7—Non-Spinning



6 x 37—Type S



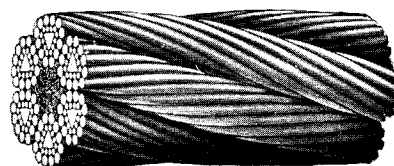
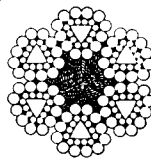
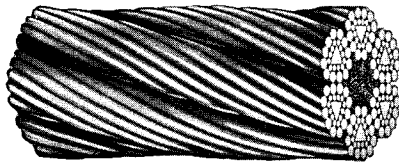
6 x 41—Type T—7 x 7 Independent Wire Rope Core



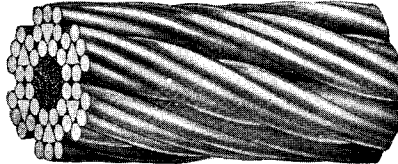
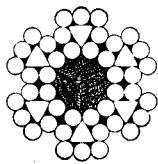
WIRE ROPE CONSTRUCTIONS

Lang Lay

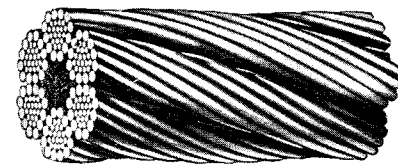
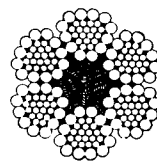
Reverse Lay



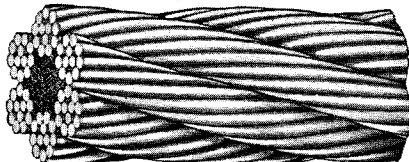
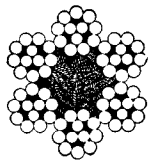
6 x 25—Type B—Flattened Strand



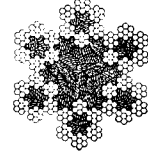
6 x 8—Type D—Flattened Strand



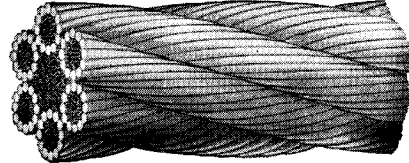
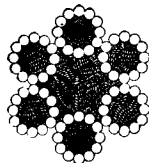
6 x 30—Type G—Flattened Strand



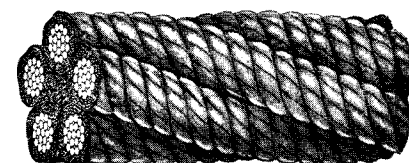
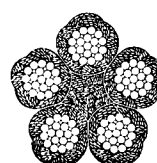
6 x 12—Galvanized Guy Rope



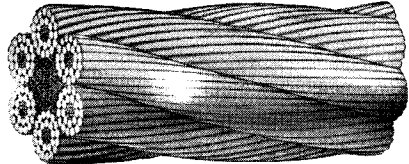
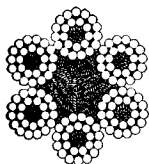
6 x 42—7 Fiber Cores—Tiller Rope



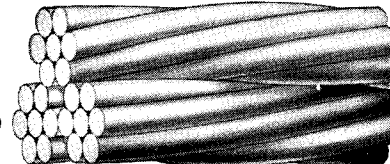
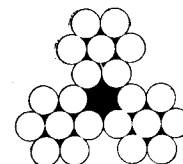
6 x 12—7 Fiber Cores—Galvanized Running Rope



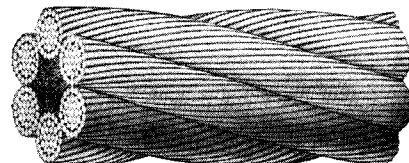
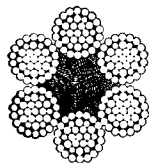
5 x 19—Marlin Clad



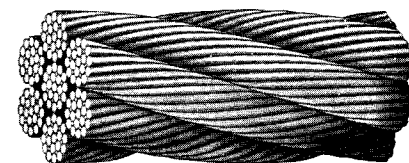
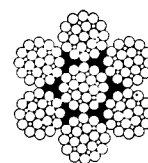
6 x 24—7 Fiber Cores—Galvanized Mooring Line



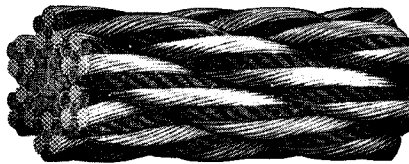
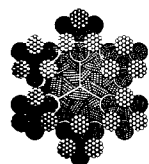
3 x 7—Galvanized Highway Guard Cable



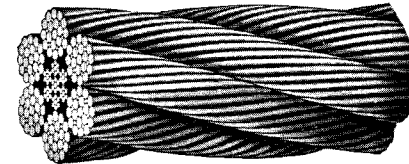
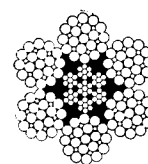
6 x 37—Galvanized Hawser



6 x 19—Type N—Wire Strand Core



6 x 3 x 19—Galvanized Spring-Lay



6 x 19—Type N—7 x 7 Independent Wire Rope Core

SELECTING WIRE ROPE

When selecting a wire rope to give the best service, there are four requirements which should be given consideration. A proper choice is made by correctly estimating the relative importance of these requirements and selecting a rope which has the qualities best suited to withstand the destructive effects of continued use.

The rope should possess:

Strength sufficient to take care of the maximum load that may be applied, with a proper factor of safety.

Ability to withstand repeated bending without failure of the wires from fatigue.

Ability to withstand abrasive wear.

Ability to withstand distortion and crushing—or, to put it more plainly, the ability to withstand abuse.

Strength

Wire rope in service is subjected to several kinds of stresses. The stresses most frequently encountered are direct tension, stress due to acceleration, stress due to sudden or shock loads, stress due to bending, and stress resulting from several forces acting at one time. For the most part, these stresses can be converted into terms of simple tension, and a rope of approximately the correct strength can be chosen. As the strength of a wire rope is determined by its size, grade, and construction, these three factors should be considered.

Safety Factors

The safety factor is the ratio of the strength of the rope to the working load. Thus, a wire rope with a strength of 10,000 pounds and a total working load of 2,000 pounds would be operating with a safety factor of five.

It is not possible to set proper safety factors for the various types of wire rope using equipment, as this factor can safely vary with conditions on individual units of equipment. The proper safety factor depends not only on the loads applied, but also on the speed of operation, the type of fittings used for securing the rope ends, the acceleration and deceleration, the length of rope, the number,

size and location of sheaves and drums, the factors causing abrasion and corrosion, the facilities for inspection, and the possible loss of life and property should a rope fail.

The following table of minimum safety factors is submitted only as a guide. These have been established by experience as the minimum safety factors required for safety and economy on the average installation. Larger safety factors are desirable, as they represent greater safety and, in most cases, increased economy.

TYPE OF SERVICE	MINIMUM SAFETY FACTOR
Track Cables	3.2
Guys	3.5
Mine Shafts	8.0 for depths to 500 ft. 7.0 for depths 500-1000 ft. 6.0 for depths 1000-2000 ft. 5.0 for depths 2000-3000 ft. 4.0 for depths 3000 ft. and more

Miscellaneous Hoisting Equipment	5.0
Haulage Ropes	6.0
Overhead and Gantry Cranes	6.0
Jib and Pillar Cranes	6.0
Derricks	6.0
Small Electric and Air Hoists	7.0
Hot Ladle Cranes	8.0
Slings	8.0

Elevators

Car Speed in Feet Per Minute	Passenger	Freight	Dumb- waiters
50	7.50	6.67	5.33
100	7.85	7.00	5.66
150	8.20	7.32	5.98
200	8.54	7.64	6.29
250	8.86	7.92	6.59
300	9.17	8.20	6.88
350	9.47	8.45	7.18
400	9.75	8.70	7.46
450	10.01	8.93	7.74
500	10.25	9.14	8.00
550	10.47	9.32	
600	10.68	9.50	
700	11.00	9.78	
800	11.25	10.02	
900	11.44	10.21	
1000	11.57	10.34	
1100	11.67	10.43	
1200	11.75	10.50	
1300	11.81	10.54	
1400	11.85	10.58	
1500	11.87	10.61	

Fatigue

Fatigue failure of the wires in a wire rope is the result of the propagation of small cracks under repeated applications of bending loads. It occurs when ropes operate over comparatively small sheaves or drums. The repeated bending of the individual wires, as the rope bends when passing over the sheaves or drums, and the straightening of the individual wires, as the rope leaves the sheaves or drums, causes fatigue. The effect of fatigue on wires is illustrated by bending a wire repeatedly back and forth until it breaks.

The best means of preventing early fatigue of wire ropes is to use sheaves and drums of adequate size. (See "Effects of Bending," page 32.) To increase the resistance to fatigue, a rope of more flexible construction should be used, as increased flexibility is secured through the use of smaller wires.

Abrasive Wear

The ability of a wire rope to withstand abrasion is determined by the size, the carbon and manganese content, and the heat treatment of the outer wires; and the construction of the rope. The larger outer wires of the less flexible constructions are better able to withstand abrasion than the finer outer wires of the more flexible ropes. The higher carbon and manganese content and the heat treatment used in producing wire for the stronger ropes, make the higher grade ropes better able to withstand abrasive wear than the lower grade ropes.

The construction of Lang Lay ropes makes them better adapted to resist abrasion than regular lay ropes of the same size, construction and grade.

Abuse

The ability of a wire rope to withstand distortion and crushing is governed principally by its construction. The use of large outer wires and wire cores increases the resistance to abuse. Types B and G Flattened Strand ropes, with their sector-shaped strands and large metallic areas, are well qualified for use where abuse of the rope is an important factor.

Grades

To meet the demand for strength, toughness, ability to withstand abrasive wear, and resistance to corrosion, in varying degrees for the many purposes for which wire rope is used, it is manufactured in the following grades:

Improved Plow Steel is the strongest, toughest and most wear resistant of the standard grades. These qualities make it the best suited for severe operating conditions. It is approximately 15% stronger than Plow Steel. The trade name of our Improved Plow Steel Wire Rope is "MONITOR".

Plow Steel is the intermediate of the three standard grades of steel wire ropes. Plow Steel Wire Ropes are approximately 15% stronger than similar wire ropes of Mild Plow Steel. This grade is extensively used on many types of equipment.

Mild Plow Steel is the lowest in strength and resistance to abrasion of the three standard grades of steel used for manufacturing wire rope. Its use is limited to installations requiring high fatigue resistance where strength and resistance to abrasion are not of primary importance.

Traction Steel is a special grade designed to meet the exacting requirements of hoisting ropes on high rise, high speed, electric traction type passenger elevators. Its use is limited to elevators.

Iron is of low tensile strength. It is soft and ductile, and its field is limited because of its low strength and resistance to abrasion.

Galvanized ropes have the individual wires protected by a uniform coating of pure zinc. These are used where ropes are exposed to the weather, to moisture, or to other corroding agencies, and their field is usually limited to stationary installations such as guys, standing rigging, towing hawsers, mooring lines, and the like. Heavily lubricated bright ropes are generally preferred to galvanized ropes on hoisting equipment, where corrosive conditions prevail.

Amgal Wire Ropes are our special galvanized ropes for installations requiring ropes possessing the toughness and strength of bright ropes and an additional protection against corrosion. These are used for hoisting, where bright ropes fail from corrosion.

Corrosion-Resisting Steel is the latest addition to the metals used for producing wire ropes. The 18 per cent chromium, 8 per cent nickel alloy commonly known as "18-8" has filled the need for a corrosion-resisting wire rope for both marine and industrial use.

Bronze is used to a limited extent for fabricating wire ropes. Bronze wire ropes are of both commercial bronze (90% copper—10% zinc) and phosphor-bronze. Phosphor-bronze ropes are the stronger and tougher of the two.

Wire Rope Lays

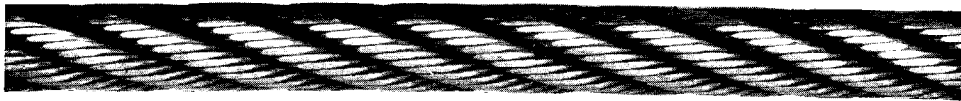
Regular Lay the accepted standard for wire ropes, denotes ropes in which the wires are twisted in one direction to form the strands, and the strands in the opposite direction to form the rope. In regular lay ropes the outer wires are approxi-

mately parallel to the longitudinal axis of the rope. Because of the difference in direction of the strand and rope lays, regular lay ropes are less likely to kink and untwist, and, therefore, are easier to handle than Lang lay ropes. Regular lay ropes are less subject to failure from crushing and distortion, due to the shorter length of exposed outer wires.

Lang Lay wire ropes have the wires in the strands, and the strands in the rope, twisted in the same direction. The outer wires run diagonally across the longitudinal axis of the rope, and are exposed for longer lengths than in regular lay ropes. Because of the longer length of exposed

outer wires, which presents greater wearing surface, Lang lay ropes have increased resistance to abrasion. They also possess greater flexibility and greater resistance to fatigue than regular lay ropes.

Greater care must be exercised when handling Lang lay ropes, as they are more likely to kink and to untwist than regular lay ropes. They are also less resistant to abuse from distortion and crushing. Lang lay ropes should have both ends permanently fastened to prevent untwisting. They are not recommended for installations where the untwisting tendency cannot be controlled, such as single part hoists, and should not be used with a swivel type end terminal.



A Worn Regular Lay Rope Showing Results of Abrasive Wear



A Worn Lang Lay Rope Showing Results of Abrasive Wear and Pounding Action

Right Lay or Left Lay depends on whether the strands of the rope rotate to the right or to the left while receding from the observer and when viewed from above. Right lay is the standard. There are very few types of installations requiring the use of left lay wire rope.

Reverse Lay applies to ropes in which the strands are alternately regular and Lang lay. The use of reverse lay ropes is usually limited to certain types of conveyors. The standard direction of lay is right, as it is for both regular lay and Lang lay ropes.

WIRE ROPE CORES

Fiber Cores are standard for most of the constructions of wire rope. The fiber core forms the heart of the rope, supports the strands, supplies internal lubrication, and contributes to the flexibility and resiliency of the rope.

Wire Cores consist of independent wire rope cores and wire strand cores.

Wire cores increase the resistance of wire rope to abuse, as steel cores will not yield to the compressive action of the outer strands as does the softer fiber. This tends to preserve the circular cross-section of the rope when it is crushed by overwinding, or when bent around small sheaves and drums while heavily loaded. It also prevents the strands of the rope from bridging, i. e., bearing forcibly against each other under these

conditions. Bridging of the strands prevents free movement of the strands and the wires in the strands as the rope bends and straightens, and results in early breakage of the wires from fatigue.

Wire cores add to the strength of ropes. They also increase the resistance of ropes to heat and should be used where ropes operate at temperatures sufficient to destroy fiber cores.

Ropes with wire cores are not less flexible, although they appear to be somewhat stiffer than fiber core ropes. They are less resilient than fiber core ropes. The smaller amount of stretch makes them suitable for bridges and the like, when the amount of elongation is important, and less suitable for operating duty where shock loads are frequent.

PREFORMED WIRE ROPES

Our preformed wire ropes are trade-marked "Excellay-Preformed". These preformed ropes differ from the standard, or non-preformed ropes, in that the individual wires in the strands and the strands in the rope are preformed, or preshaped, to their proper shape before they are assembled in the finished rope.

The preforming operation removes the natural tendency of the wires and strands to straighten, and causes them to retain their proper positions. This, in turn, results in preformed wire ropes having the following characteristics as compared to non-preformed ropes:

They can be cut without the seizings necessary to retain the rope structure of non-preformed ropes.

Broken rope ends do not untwist, as do the ends of the non-preformed ropes. This increases the salvage value of broken ropes.

They are substantially free from liveliness and

twisting tendencies. This makes installation and handling easier, and lessens likelihood of damage to the rope from kinking or fouling. Preforming permits the more general use of Lang lay and wire core constructions.

Removal of internal stresses increases resistance to fatigue from bending. This results in increased service where ability to withstand bending is the important requirement. It also permits the use of ropes with larger outer wires, when increased wear resistance is desired.

Outer wires will wear thinner before breaking, and broken wire ends will not protrude from the rope to injure workmen's hands, to nick and distort adjacent wires, or to wear sheaves and drums. Because of the fact that broken wire ends do not porcupine, they are not as noticeable as they are in non-preformed ropes. This necessitates the use of greater care when inspecting worn preformed ropes, to determine their true condition.

PRESTRESSED STRANDS AND ROPES

The purpose of prestressing is to remove the structural stretch. (See "Stretch of Wire Rope," page 29.) This is accomplished by subjecting the strand or rope to a predetermined load for a sufficient length of time to permit adjustment of the component parts to that load. The applied load should not exceed the elastic limit of the cable.

Prestressing is not recommended for fiber core ropes, nor is it desirable for operating ropes. Prestressing is of value for stationary strands and stationary wire core ropes on installations where a limited amount of elongation under load is re-

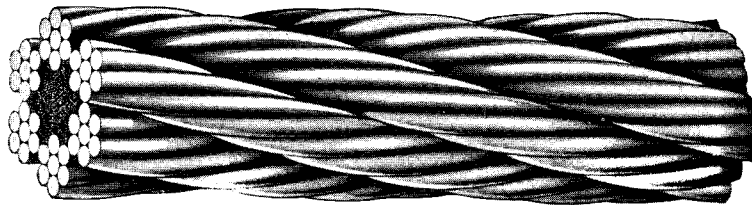
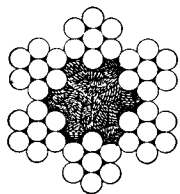
quired. Galvanized strands and galvanized wire core ropes used as main cables and as suspension cables on suspension bridges, and galvanized guy strands on vertical radio towers are examples of installations where prestressing is of definite value.

Prestressing permits accurate measuring under working loads. In order to assure its installation without twists being added or removed, a stripe is painted along the cable while it is in the prestressing equipment. After installation, the painted stripe should be parallel to the axis of the cable

CONSTRUCTIONS

Wire rope constructions are designated by the number of strands in the rope and the number of wires in each strand. Thus, a rope composed of

six strands of seven wires to the strand is a 6x7 rope. Unless noted, or specified by the number of strands, the rope has a fiber core.

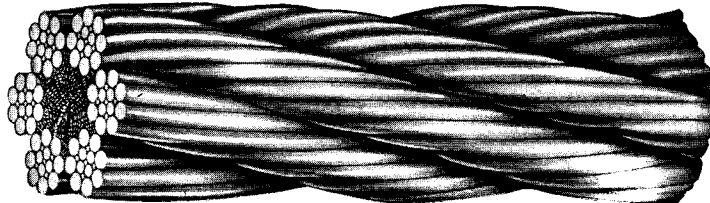
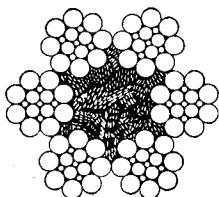


6 x 7—Regular Lay

Ropes of 6 strands, 7 wires to the strand, 1 fiber core, are known as "Haulage" ropes, as the large outer wires are well suited to withstand the abrasive wear to which haulage ropes are subjected. This construction is the least flexible of the standard rope constructions. Wire cores are usually

strands, making the rope construction 7x7.

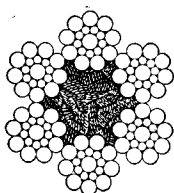
When fabricated of galvanized wires, this is termed a 6x7 Galvanized Guy Rope, and with a galvanized strand core, it becomes a 7x7 Galvanized Guy Rope.



6 x 17—Type L

This is a slightly more flexible haulage rope than the 6x7, as each strand has eight outer wires

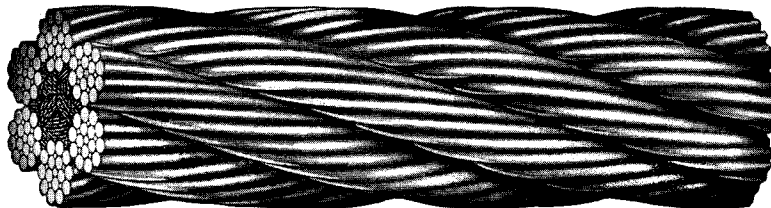
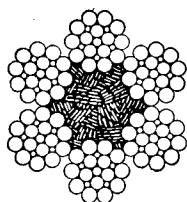
to six for the 6x7. The strengths of the 6x17—Type L are greater than those of the 6x7.



6 x 19—Seale Patent

This construction has nine outer wires per strand, and is therefore a little more flexible than the 6x17, Type L. The term "Seale Patent" refers to strands consisting of two concentric layers of

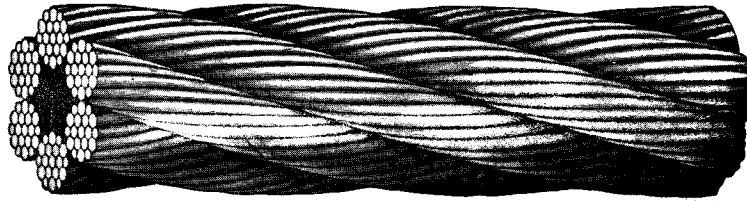
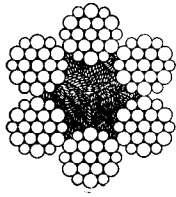
the same number of wires, all wires in each layer of one size, and each outer wire cradled on two inner wires. This arrangement of wires produces a rugged rope for severe service conditions.



6 x 21—Type M

The ten outer wires in each strand increase the flexibility of this rope over the three constructions shown above. It serves a field where resist-

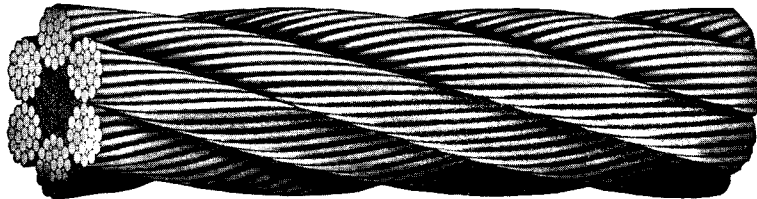
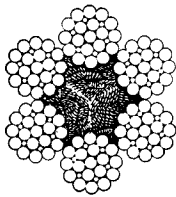
ance to abrasion and sufficient flexibility to permit winding on medium size drums are requisites. This rope is used for both hauling and hoisting.



6 x 19—Warrington

In this construction the outer layer of wires in each strand consists of six large and six smaller wires. The large outer wires prevent this rope from

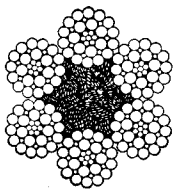
being as flexible as the 6x19 Type N Standard Hoisting Rope. 6x19 Galvanized Guy Ropes are usually of Warrington construction.



6 x 19—Type N

This Standard Hoisting Rope is more universally used than any other construction. In addition

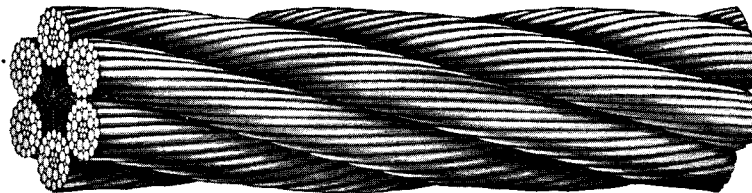
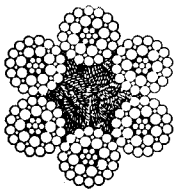
to the nineteen main wires in each strand there are six smaller intermediate wires.



6 x 29—Type P

This construction of wire rope, with fourteen outer wires to the strand, is next in flexibility to

the Standard Hoisting Rope with its twelve outer wires per strand.

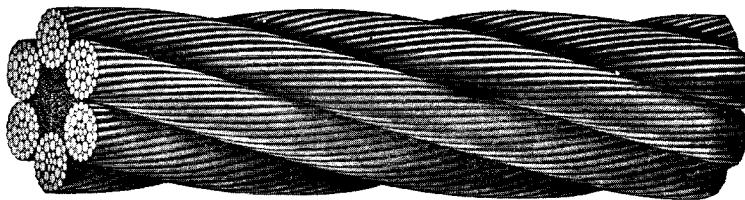
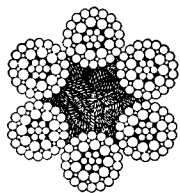


6 x 33—Type R

In this type of rope there are sixteen outer wires in each strand, making it more flexible than the constructions referred to, which have fewer outer wires. As the sixteen outer wires are larger than the outer wires of the 6x37 Type S Special

Flexible Hoisting Rope, this rope gives excellent service on cranes and similar equipment, where abrasive wear is too severe for the smaller wires of the 6x37 Crane Rope.

AMERICAN TIGER BRAND

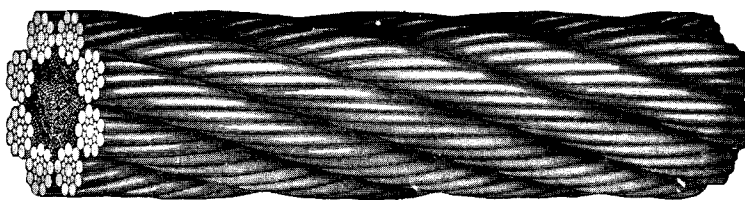
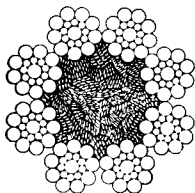


6 x 37—Type S

This rope is listed as "Special Flexible Hoisting Rope" and is also known as "Crane Rope" due to its widespread use on overhead traveling cranes. It is the most flexible of the standard constructions in which six strand hoisting ropes are commonly fabricated. Its flexibility permits its use with small sheaves and drums. Each strand has

eighteen outer wires which are comparatively small and therefore not suited to withstand excessive abrasion.

This construction has the highest reserve strength of the standard ropes. (See "Reserve Strengths," page 31).



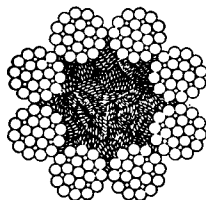
8 x 19—Seale Patent

The strands are of the same construction as those of the 6x19, Seale Patent, shown on page 10, but the strands are smaller and the fiber core larger. This increases its flexibility over the 6x19 Seale Patent construction, and because of the smaller metallic area, lowers the strength.

Ropes of eight strand nineteen wire construction are known as "Extra Flexible Hoisting Ropes." Because of the large fiber core, they

should not be used where overwinding conditions or other conditions tending toward abuse are severe, as eight strand ropes will not withstand distortion as well as six strand ropes.

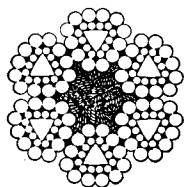
8x19 Seale Patent Traction Steel Special Flexible Hoisting Ropes have played an important role in increased speeds and smoother operation of high speed, high rise, traction type electric passenger elevators.



8 x 19—Type N

The standard construction of 8x19 Extra Flexible Hoisting Rope has eight smaller strands similar in design to the strands of 6x19, Type N rope shown on page 11. The eight strand rope is more flexible than the six strand construction,

because of the greater number of smaller strands composed of smaller wires. It is less resistant to abrasion, because of the smaller wires, and to distortion, because of the larger fiber core.

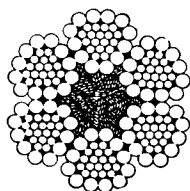


6 x 25—Type B, Flattened Strand

The sector-shaped strands of this rope have larger metallic areas than the strands of round strand ropes, resulting in additional strength; greatly increased wearing surfaces, which present greater resistance to abrasion; and larger bearing

surfaces between the adjacent strands, giving increased resistance to crushing.

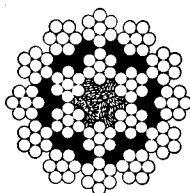
This type, in common with all types of flattened strand rope, is made Lang lay.



6 x 30—Type G, Flattened Strand

Type G differs from Type B in the construction of the strand cores. The single triangular-shaped core wire of Type B is replaced by a triangular-shaped core strand of six wires. The advantage of

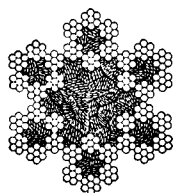
the six-wire strand core over the single-wire core is its greater resistance to fatigue. The six smaller round wires are more flexible and will withstand more bending than the larger shaped wire.



18 x 7—Non-Spinning

This is the only standard construction consisting of two concentric layers of strands. The six inner strands of seven wires are Lang lay, left lay. The twelve seven-wire outer strands are regular lay, right lay. The purpose of the two layers of

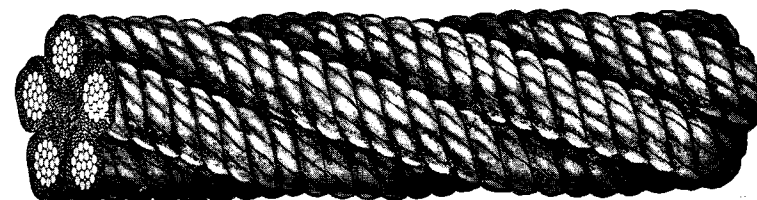
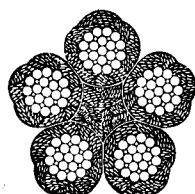
strands and the difference in lays is to counteract the untwisting tendency of each, and results in a wire rope with a minimum tendency to rotate while under load.



6 x 42—7 Fiber Cores—Tiller Rope

Tiller Rope may be described as 6x6x7, or as 6x42—7 Fiber Cores. Each of the six main strands of the rope is a smaller 6x7 rope. It is exceedingly

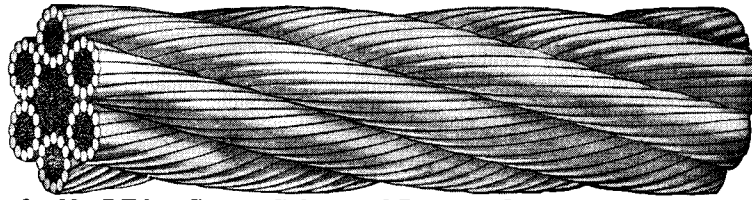
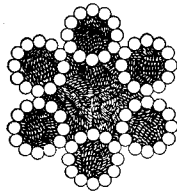
flexible, and the small size of the outer wires precludes sustained resistance to abrasive wear.



5 x 19—Marlin Clad

Each strand is served with a closely wrapped layer of tarred marlin before it is closed into the completed rope. The marlin forms a cushion for

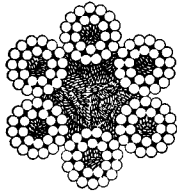
the strands, shields the strands against external wear as well as internal friction, and protects the hands of workmen handling the rope.



6 x 12—7 Fiber Cores—Galvanized Running Rope

This rope, of galvanized wires, is known as "Galvanized Running Rope" and sometimes as "Galvanized Hawser" or "Galvanized Mooring Line." Each strand consists of twelve wires around

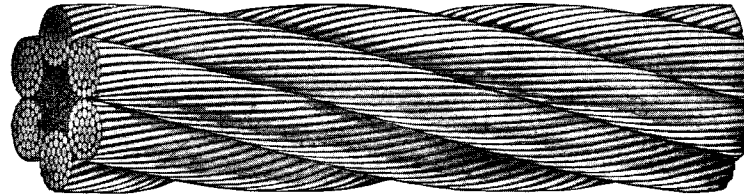
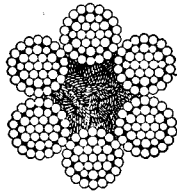
a fiber center. These six fiber centers add to the flexibility, but keep the strength below that of a rope of all metal strand construction.



6 x 24—7 Fiber Cores—Galvanized Mooring Line

This galvanized rope has six strands, each composed of two layers of wires, fifteen over nine, around a small fiber center. Because of its larger

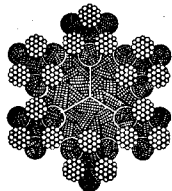
metallic area, it is stronger than the 6x12, 7 Fiber Core Running Rope. It is termed "Galvanized Hawser" and also "Galvanized Mooring Line."



6 x 37—Galvanized Hawser

Each strand of these Hawsers, or Towing Lines, consists of thirty-seven galvanized wires. The all-steel strand construction makes this rope stronger

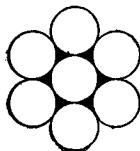
than the two marine ropes which have fiber centers in the strands.



6 x 3 x 19 Galvanized Spring-Lay

Each of the six strands of this Excellay Spring-Lay Wire rope consists of three steel strands and three fiber strands around a fiber core. This con-

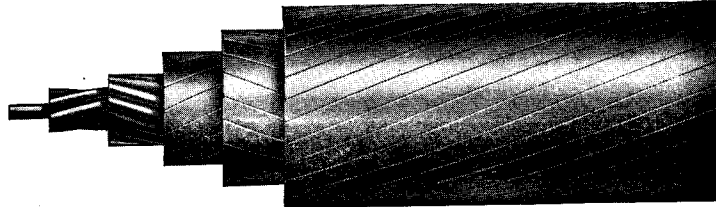
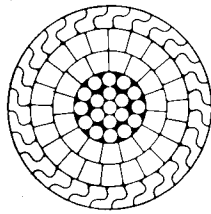
struction is designed for use in the marine field for warping and mooring where a flexible rope with high elasticity is required.



Galvanized Strand

Galvanized guy strands of seven and nineteen wires are made in four grades: Iron or Common Strand, Siemens-Martin, High Strength, and Extra High Strength. For sizes to and including $\frac{1}{2}$ inch diameter, guy strands are usually of seven wires; sizes $\frac{3}{16}$ inch to 1 inch are nineteen-wire con-

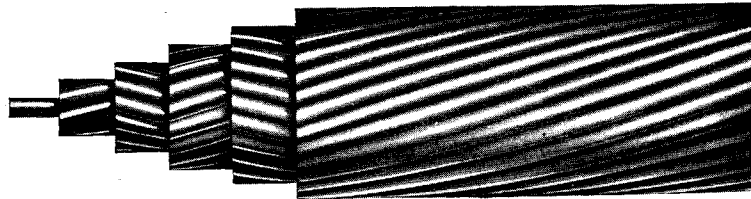
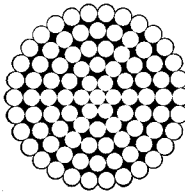
struction. Galvanized Bridge Strands are of 19, 37, 61 or 91 wire construction, depending on the size. Bridge strands are made of Galvanized Plow Steel wires with tensile strengths in excess of those used for the guy strands.



Locked Coil Track Strand

The smooth exterior surface of Locked Coil Track Cable eliminates the impact loads on the outer wires which occur when carriage wheels roll over any other type of track cable. The inter-

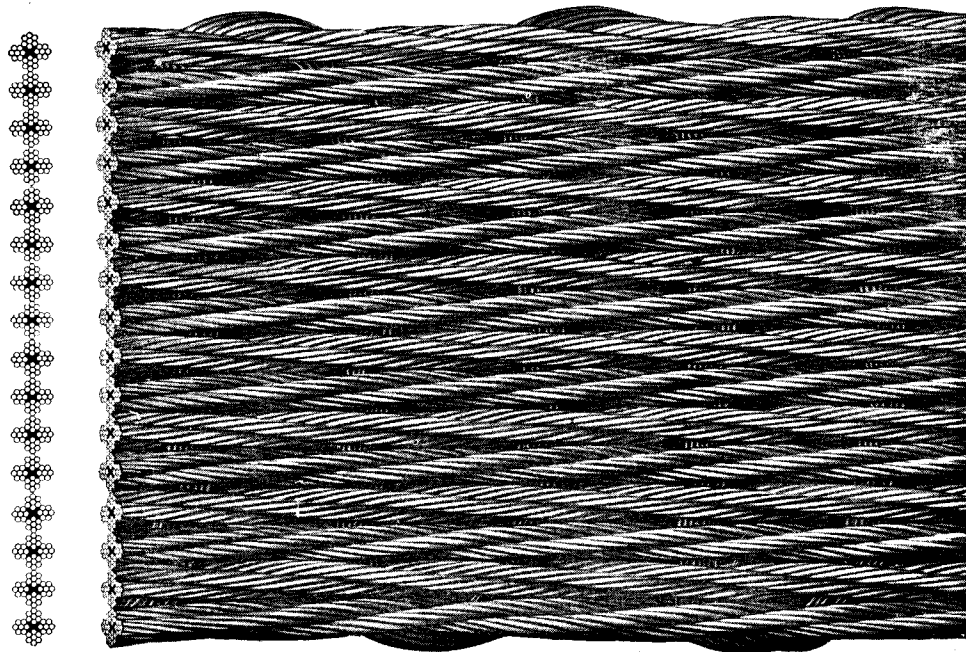
locking construction prevents broken outer wires from protruding. It is used for track cables on aerial tramways and cableways.



Smooth Coil Track Strand

This track strand consists of concentric layers of round wires, each layer reversed in direction of

lay to adjacent layers. This construction is also known as Round Wire Track Strand.



Flat Wire Rope

Flat Rope consists of a number of four-strand wire ropes, usually 4x7, of alternate right and left lay, held in position by soft iron sewing wires. The thickness of the rope is determined by the size of the four-strand ropes. The width is governed by the number of these ropes employed.

The advantage of this type of rope is that it winds upon itself, ribbon fashion, on a drum slightly wider than the rope. This permits locating the hoisting drum nearer the load than if round rope were used, as there is no fleet angle involved.

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

The Breaking Strengths and Weights given on pages 16, 17, 18, 19, 22, 23, 26 and in the first two tables on page 20 as well as in the second and third tables on page 24 are in accordance with Wire Rope Simplified Practice Recommendation R198-43. This simplification practice was issued by the United States Department of Commerce through the National Bureau of Standards and was made effective February 15, 1943.

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.			Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
	Monitor Steel	Plow Steel	Mild Plow Steel		
6 x 7 Haulage Rope					
1/4	2.64	2.30	2.00	0.094	3/4
5/16	4.10	3.56	3.10	.15	1
3/8	5.86	5.10	4.43	.21	1 1/8
7/16	7.93	6.90	6.00	.29	1 3/8
1/2	10.3	8.96	7.79	.38	1 5/8
9/16	13.0	11.3	9.82	.48	1 3/4
5/8	15.9	13.9	12.0	.59	2
3/4	22.7	19.8	17.2	.84	2 3/8
7/8	30.7	26.7	23.2	1.15	2 3/4
1	39.7	34.5	30.0	1.50	3 1/8
1 1/8	49.8	43.3	37.7	1.90	3 1/2
1 1/4	61.0	53.0	46.1	2.34	3 7/8
1 3/8	73.1	63.6	55.3	2.84	4 3/8
1 1/2	86.2	75.0	65.2	3.38	4 3/4

Wire Strand Cores and Independent Wire Rope Cores add 7½% to the above listed strengths and 10% to the weights. When these ropes are Galvanized, deduct 10% from the above listed strengths.

6 x 19 Standard Hoisting Rope

$\frac{1}{4}$	2.74	2.39	2.07	0.10	$\frac{3}{4}$
$\frac{5}{16}$	4.26	3.71	3.22	.16	1
$\frac{3}{8}$	6.10	5.31	4.62	.23	$1\frac{1}{8}$
$\frac{7}{16}$	8.27	7.19	6.25	.31	$1\frac{3}{8}$
$\frac{1}{2}$	10.7	9.35	8.13	.40	$1\frac{5}{8}$
$\frac{9}{16}$	13.5	11.8	10.2	.51	$1\frac{3}{4}$
$\frac{5}{8}$	16.7	14.5	12.6	.63	2
$\frac{3}{4}$	23.8	20.7	18.0	.90	$2\frac{3}{8}$
$\frac{7}{8}$	32.2	28.0	24.3	1.23	$2\frac{3}{4}$
1	41.8	36.4	31.6	1.60	$3\frac{1}{8}$
$1\frac{1}{8}$	52.6	45.7	39.8	2.03	$3\frac{1}{2}$
$1\frac{1}{4}$	64.6	56.2	48.8	2.50	$3\frac{7}{8}$
$1\frac{3}{8}$	77.7	67.5	58.8	3.03	$4\frac{3}{8}$
$1\frac{1}{2}$	92.0	80.0	69.6	3.60	$4\frac{3}{4}$
$1\frac{5}{8}$	107.0	93.4	81.2	4.23	$5\frac{1}{8}$
$1\frac{3}{4}$	124.0	108.0	93.6	4.90	$5\frac{1}{2}$
$1\frac{7}{8}$	141.0	123.0	107.0	5.63	$5\frac{7}{8}$
2	160.0	139.0	121.0	6.40	$6\frac{1}{4}$
$2\frac{1}{8}$	179.0	156.0		7.23	$6\frac{5}{8}$
$2\frac{1}{4}$	200.0	174.0		8.10	$7\frac{1}{8}$
$2\frac{1}{2}$	244.0	212.0		10.00	$7\frac{7}{8}$
$2\frac{3}{4}$	292.0	254.0		12.10	$8\frac{5}{8}$

This table applies to all types of 6 x 19, 6 x 17 and 6 x 21 Wire Ropes. Wire Strand Cores and Independent Wire Rope Cores add 7½% to the above listed strengths and 10% to the weights. When these ropes are Galvanized, deduct 10% from the above listed strengths.

WIRE ROPE ENGINEERING HANDBOOK

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.	Approx Circ. Inches
	Monitor Steel	Plow Steel		
6 x 37 Special Flexible Hoisting Rope				
$\frac{1}{4}$	2.59	2.25	0.10	$\frac{3}{4}$
$\frac{5}{16}$	4.03	3.50	.16	1
$\frac{3}{8}$	5.77	5.02	.22	$1\frac{1}{8}$
$\frac{7}{16}$	7.82	6.80	.30	$1\frac{3}{8}$
$\frac{1}{2}$	10.2	8.85	.39	$1\frac{5}{8}$
$\frac{9}{16}$	12.9	11.2	.49	$1\frac{3}{4}$
$\frac{5}{8}$	15.8	13.7	.61	2
$\frac{3}{4}$	22.6	19.6	.87	$2\frac{3}{8}$
$\frac{7}{8}$	30.6	26.6	1.19	$2\frac{3}{4}$
1	39.8	34.6	1.55	$3\frac{1}{8}$
$1\frac{1}{8}$	50.1	43.5	1.96	$3\frac{1}{2}$
$1\frac{1}{4}$	61.5	53.5	2.42	$3\frac{7}{8}$
$1\frac{3}{8}$	74.1	64.5	2.93	$4\frac{3}{8}$
$1\frac{1}{2}$	87.9	76.4	3.49	$4\frac{3}{4}$
$1\frac{5}{8}$	103.0	89.3	4.09	$5\frac{1}{8}$
$1\frac{3}{4}$	119.0	103.0	4.75	$5\frac{1}{2}$
$1\frac{7}{8}$	136.0	118.0	5.45	$5\frac{7}{8}$
2	154.0	134.0	6.20	$6\frac{1}{4}$
$2\frac{1}{8}$	173.0	150.0	7.00	$6\frac{5}{8}$
$2\frac{1}{4}$	193.0	168.0	7.85	$7\frac{1}{8}$
$2\frac{1}{2}$	236.0	205.0	9.69	$7\frac{7}{8}$
$2\frac{3}{4}$	284.0	247.0	11.72	$8\frac{5}{8}$
3	335.0	291.0	13.95	$9\frac{3}{8}$
$3\frac{1}{4}$	390.0	339.0	16.37	$10\frac{1}{4}$
$3\frac{1}{2}$	449.0	390.0	19.00	11

This table applies to 6 x 29 Type P, 6 x 33 Type R and 6 x 41 Type T Special Hoisting Ropes. Wire Strand Cores and Independent Wire Rope Cores add $7\frac{1}{2}\%$ to the above listed strengths and 10% to the weights.

When these ropes are Galvanized, deduct 10% from the above listed strengths.

8 x 19 Extra Flexible Hoisting Rope

$\frac{1}{4}$	2.35	2.04	0.09	$\frac{3}{4}$
$\frac{5}{16}$	3.65	3.18	.14	1
$\frac{3}{8}$	5.24	4.55	.20	$1\frac{1}{8}$
$\frac{7}{16}$	7.09	6.17	.28	$1\frac{3}{8}$
$\frac{1}{2}$	9.23	8.02	.36	$1\frac{5}{8}$
$\frac{9}{16}$	11.6	10.1	.46	$1\frac{3}{4}$
$\frac{5}{8}$	14.3	12.4	.57	2
$\frac{3}{4}$	20.5	17.8	.82	$2\frac{3}{8}$
$\frac{7}{8}$	27.7	24.1	1.11	$2\frac{3}{4}$
1	36.0	31.3	1.45	$3\frac{1}{8}$
$1\frac{1}{8}$	45.3	39.4	1.84	$3\frac{1}{2}$
$1\frac{1}{4}$	55.7	48.4	2.27	$3\frac{7}{8}$
$1\frac{3}{8}$	67.1	58.3	2.74	$4\frac{3}{8}$
$1\frac{1}{2}$	79.4	69.1	3.26	$4\frac{3}{4}$

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	BREAKING STRENGTH IN LBS.		Approx. Weight per Foot in Lbs.
	Traction Steel	Iron	
8 x 19 Extra Flexible Elevator Rope			
$\frac{3}{16}$		1,000	0.05
$\frac{1}{4}$	3,600	1,800	.09
$\frac{5}{16}$	5,600	2,900	.14
$\frac{3}{8}$	8,200	4,200	.20
$\frac{7}{16}$	11,000	5,600	.28
$\frac{1}{2}$	14,500	7,200	.36
$\frac{9}{16}$	18,500	9,200	.46
$\frac{5}{8}$	23,000	11,200	.57
* $\frac{11}{16}$	27,000		.69
$\frac{3}{4}$	32,000	16,000	.82
* $\frac{13}{16}$	37,000		.96
$\frac{7}{8}$	42,000	21,400	1.11
* $\frac{5}{16}$	48,000		1.27
1	54,000	28,000	1.45
$1\frac{1}{16}$	61,000		1.64
6 x 19 Elevator Rope			
$\frac{3}{16}$		1,300	0.06
$\frac{1}{4}$	3,600	2,200	.10
$\frac{5}{16}$	5,600	3,200	.16
$\frac{3}{8}$	8,200	5,000	.23
$\frac{7}{16}$	11,000	6,400	.31
$\frac{1}{2}$	14,500	8,400	.40
$\frac{9}{16}$	18,500	10,600	.51
$\frac{5}{8}$	23,000	12,800	.63
* $\frac{11}{16}$	27,000		.76
$\frac{3}{4}$	32,000	18,200	.90
* $\frac{13}{16}$	37,000		1.06
$\frac{7}{8}$	42,000	24,800	1.23
* $\frac{15}{16}$	48,000		1.41
1	54,000	32,000	1.60
$1\frac{1}{16}$	61,000		1.81

*This size can also be furnished in Special High Strength Traction Steel for High Rise Installations. Strengths: $\frac{11}{16}$ "—30,000 lbs.; $\frac{13}{16}$ "—46,000 lbs.; $\frac{15}{16}$ "—60,000 lbs.

Rope Diameter Inches	BREAKING STRENGTH IN LBS.		Approx. Weight per Foot in Lbs.
	Plow Steel	Iron	
6 x 42 Tiller Rope			
1/4	2,620	1,168	0.07
5/16	4,100	1,816	.11
3/8	5,860	2,600	.16
7/16	7,960	3,540	.21
1/2	10,360	4,600	.28
9/16	13,060	5,800	.35
5/8	16,080	7,140	.43

For Galvanized Tiller Rope, deduct 10% from the above listed strengths.

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
	Monitor Steel	Mild Plow Steel		
6 x 25 Type B } Flattened Strand Hoisting Ropes 6 x 30 Type G }				
$\frac{3}{8}$	6.71		0.25	$1\frac{1}{8}$
$\frac{1}{2}$	11.8	8.94	.45	$1\frac{5}{8}$
$\frac{9}{16}$	14.9	11.2	.57	$1\frac{3}{4}$
$\frac{5}{8}$	18.3	13.9	.70	2
$\frac{3}{4}$	26.2	19.8	1.01	$2\frac{3}{8}$
$\frac{7}{8}$	35.4	26.8	1.39	$2\frac{3}{4}$
1	46.0	34.8	1.80	$3\frac{1}{8}$
$1\frac{1}{8}$	57.9	43.8	2.28	$3\frac{1}{2}$
$1\frac{1}{4}$	71.0	53.7	2.81	$3\frac{7}{8}$
$1\frac{3}{8}$	85.5		3.40	$4\frac{3}{8}$
$1\frac{1}{2}$	101.0		4.05	$4\frac{3}{4}$
$1\frac{5}{8}$	118.0		4.75	$5\frac{1}{8}$
$1\frac{3}{4}$	136.0		5.51	$5\frac{1}{2}$

Wire Strand Cores and Independent Wire Rope Cores add 7½% to the above listed strengths and 5% to the weights.

6 x 8 Type D Flattened Strand Haulage Rope				
$\frac{1}{2}$	11.1	8.37	0.45	$1\frac{5}{8}$
$\frac{5}{8}$	17.1	12.9	.70	2
$\frac{3}{4}$	24.4	18.5	1.01	$2\frac{3}{8}$
$\frac{7}{8}$	33.0	24.9	1.39	$2\frac{3}{4}$
1	42.7	32.3	1.80	$3\frac{1}{8}$
$1\frac{1}{8}$	53.5	40.5	2.28	$3\frac{1}{2}$
$1\frac{1}{4}$	65.5	49.5	2.81	$3\frac{7}{8}$
$1\frac{3}{8}$	78.6	59.4	3.40	$4\frac{3}{8}$
$1\frac{1}{2}$	92.7	70.1	4.05	$4\frac{3}{4}$

Wire Strand Cores and Independent Wire Rope Cores add 7½% to the above listed strengths and 5% to the weights.

Cord Diameter Inches	BREAKING STRENGTH IN LBS.			Approx. Weight per Foot in Lbs.
	Hard Drawn Iron		Annealed Iron	
	Bright	Galvanized	Bright or Galvanized	
6 x 7 Sash Cord				
$\frac{1}{16}$	140	126	77	0.006
$\frac{3}{32}$	315	283	172	.013
$\frac{1}{8}$	560	504	306	.023
$\frac{5}{32}$	840	756	478	.038
$\frac{3}{16}$	1,150	1,035	688	.053
$\frac{7}{32}$	1,570	1,413	940	.072
$\frac{1}{4}$	2,040	1,836	1,225	.094

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
	Monitor Steel	Plow Steel		
18 x 7 Non-Spinning Rope				
$\frac{3}{8}$	5.59	4.86	0.24	$1\frac{1}{8}$
$\frac{7}{16}$	7.58	6.59	.33	$1\frac{3}{8}$
$\frac{1}{2}$	9.85	8.57	.43	$1\frac{5}{8}$
$\frac{9}{16}$	12.4	10.8	.55	$1\frac{3}{4}$
$\frac{5}{8}$	15.3	13.3	.68	2
$\frac{3}{4}$	21.8	19.0	.97	$2\frac{3}{8}$
$\frac{7}{8}$	29.5	25.7	1.32	$2\frac{3}{4}$
1	38.3	33.3	1.73	$3\frac{1}{8}$
$1\frac{1}{8}$	48.2	41.9	2.19	$3\frac{1}{2}$
$1\frac{1}{4}$	59.2	51.5	2.70	$3\frac{7}{8}$
$1\frac{3}{8}$	71.3	62.0	3.27	$4\frac{3}{8}$
$1\frac{1}{2}$	84.4	73.4	3.89	$4\frac{3}{4}$
$1\frac{5}{8}$	98.4	85.6	4.57	$5\frac{1}{8}$
$1\frac{3}{4}$	114.0	98.8	5.30	$5\frac{1}{2}$

Rope Diameter Inches	Breaking Strength in Lbs.	Approx. Weight per Foot in Lbs.
9 x 4 Galvanized Mast Arm Rope		
$\frac{1}{4}$	1100	0.070
$\frac{5}{16}$	1530	.107
$\frac{3}{8}$	2200	.158

Strand Diameter Inches	Approx. Diameter of Each Wire	Approx. Weight per 1000 Ft. in Lbs.
3 Wire Stone Sawing Strand		
$\frac{1}{8}$	0.054	26
$\frac{9}{64}$	.061	33
$\frac{5}{32}$	.067	40
$11/64$	.074	48
$\frac{3}{16}$	.080	56
$\frac{7}{32}$	.092	75
$\frac{1}{4}$	.106	99
$\frac{9}{32}$	.120	127

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	Breaking Strength in Tons of 2,000 Lbs.	Approx. Weight per Foot in Lbs.	Approx. Metallic Area in Sq. Inches	Approx. Circ. Inches
6 x 7 and 6 x 19, Wire Core, Galvanized Plow Steel Bridge Ropes				
1	45.7	1.67	0.471	3 $\frac{1}{8}$
1 $\frac{1}{8}$	57.8	2.11	.596	3 $\frac{1}{2}$
1 $\frac{1}{4}$	72.2	2.64	.745	3 $\frac{7}{8}$
1 $\frac{3}{8}$	87.8	3.21	.906	4 $\frac{3}{8}$
1 $\frac{1}{2}$	104.0	3.82	1.076	4 $\frac{3}{4}$
1 $\frac{5}{8}$	123.0	4.51	1.27	5 $\frac{1}{8}$
1 $\frac{3}{4}$	143.0	5.24	1.47	5 $\frac{1}{2}$
1 $\frac{7}{8}$	164.0	6.03	1.69	5 $\frac{7}{8}$
2	186.0	6.85	1.92	6 $\frac{1}{4}$
2 $\frac{1}{8}$	210.0	7.73	2.17	6 $\frac{5}{8}$
2 $\frac{1}{4}$	235.0	8.66	2.42	7 $\frac{1}{8}$
2 $\frac{3}{8}$	261.0	9.61	2.69	7 $\frac{1}{2}$
2 $\frac{1}{2}$	288.0	10.60	2.97	7 $\frac{7}{8}$
2 $\frac{5}{8}$	317.0	11.62	3.27	8 $\frac{1}{4}$
2 $\frac{3}{4}$	347.0	12.74	3.58	8 $\frac{5}{8}$
2 $\frac{7}{8}$	379.0	13.90	3.91	9
3	412.0	15.11	4.25	9 $\frac{3}{8}$
Galvanized Bridge Strand				
$\frac{7}{8}$	46.0	1.56	0.450	2 $\frac{3}{4}$
1	61.0	2.07	.596	3 $\frac{1}{8}$
1 $\frac{1}{8}$	78.0	2.64	.760	3 $\frac{1}{2}$
1 $\frac{1}{4}$	96.0	3.26	.940	3 $\frac{7}{8}$
1 $\frac{3}{8}$	116.0	3.94	1.135	4 $\frac{3}{8}$
1 $\frac{1}{2}$	138.0	4.69	1.35	4 $\frac{3}{4}$
1 $\frac{5}{8}$	162.0	5.50	1.59	5 $\frac{1}{8}$
1 $\frac{3}{4}$	188.0	6.38	1.84	5 $\frac{1}{2}$
1 $\frac{7}{8}$	216.0	7.32	2.11	5 $\frac{7}{8}$
2	245.0	8.34	2.40	6 $\frac{1}{4}$
2 $\frac{1}{8}$	277.0	9.42	2.71	6 $\frac{5}{8}$
2 $\frac{1}{4}$	310.0	10.55	3.04	7 $\frac{1}{8}$

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	Breaking Strength in Tons of 2,000 Lbs.	Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
6 x 7 Galvanized Iron Guy Rope			
$\frac{1}{4}$	0.918	0.094	$\frac{3}{4}$
$\frac{5}{16}$	1.42	.15	1
$\frac{3}{8}$	2.04	.21	$1\frac{1}{8}$
$\frac{7}{16}$	2.76	.29	$1\frac{3}{8}$
$\frac{1}{2}$	3.58	.38	$1\frac{5}{8}$
$\frac{9}{16}$	4.51	.48	$1\frac{3}{4}$
$\frac{5}{8}$	5.54	.59	2
$\frac{3}{4}$	7.90	.84	$2\frac{3}{8}$
$\frac{13}{16}$	9.23	.99	$2\frac{1}{2}$
$\frac{7}{8}$	10.7	1.15	$2\frac{3}{4}$
1	13.8	1.50	$3\frac{1}{8}$
$1\frac{1}{16}$	15.5	1.70	$3\frac{3}{8}$
$1\frac{1}{8}$	17.3	1.90	$3\frac{1}{2}$
$1\frac{3}{16}$	19.2	2.12	$3\frac{3}{4}$
$1\frac{1}{4}$	21.2	2.34	$3\frac{7}{8}$

Wire Strand Cores and Independent Wire Rope Cores add 7½% to the above listed strengths and 10% to the weights.

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.			Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
	Monitor Steel	Plow Steel	Iron		
6 x 12, 7 Fiber Cores, Galvanized Running Rope					
$\frac{5}{16}$	2.34	2.04	0.905	0.10	1
$\frac{3}{8}$	3.36	2.92	1.30	.15	$1\frac{1}{8}$
$\frac{7}{16}$	4.55	3.95	1.76	.20	$1\frac{3}{8}$
$\frac{1}{2}$	5.91	5.14	2.28	.26	$1\frac{5}{8}$
$\frac{9}{16}$	7.45	6.48	2.88	.33	$1\frac{3}{4}$
$\frac{5}{8}$	9.16	7.97	3.54	.41	2
$\frac{3}{4}$	13.1	11.4	5.06	.59	$2\frac{3}{8}$
$\frac{13}{16}$	15.3	13.3	5.92	.69	$2\frac{1}{2}$
$\frac{7}{8}$	17.7	15.4	6.85	.80	$2\frac{3}{4}$
1	23.0	20.0	8.89	1.05	$3\frac{1}{8}$
$1\frac{1}{16}$	25.9	22.5	10.0	1.19	$3\frac{3}{8}$
$1\frac{1}{8}$	29.0	25.2		1.33	$3\frac{1}{2}$
$1\frac{3}{16}$	32.2	28.0		1.48	$3\frac{3}{4}$
$1\frac{1}{4}$	35.6	30.9		1.64	$3\frac{7}{8}$
$1\frac{3}{8}$	42.8	37.2		1.99	$4\frac{3}{8}$
$1\frac{7}{16}$	46.7	40.6		2.17	$4\frac{1}{2}$
$1\frac{1}{2}$	50.7	44.1		2.36	$4\frac{3}{4}$
$1\frac{5}{8}$	59.2	51.4		2.77	$5\frac{1}{8}$
$1\frac{11}{16}$	63.6	55.3		2.99	$5\frac{1}{4}$
$1\frac{3}{4}$	68.3	59.4		3.22	$5\frac{1}{2}$
$1\frac{13}{16}$	73.0	63.5		3.45	$5\frac{3}{4}$
$1\frac{15}{16}$	83.0	72.2		3.94	$6\frac{1}{8}$
2	88.2	76.7		4.20	$6\frac{1}{4}$
$2\frac{1}{16}$	93.6	81.4		4.47	$6\frac{1}{2}$

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Ft. in Lbs.	Approx. Circ. Inches
	Monitor Steel	Plow Steel		
6 x 24, 7 Fiber Cores, Galvanized Mooring Line				
$\frac{3}{8}$	4.77	4.14	0.194	$1\frac{1}{8}$
$\frac{1}{2}$	8.40	7.30	.35	$1\frac{5}{8}$
$\frac{5}{8}$	13.0	11.3	.54	2
$\frac{3}{4}$	18.6	16.2	.78	$2\frac{3}{8}$
$\frac{13}{16}$	21.8	19.0	.91	$2\frac{1}{2}$
$\frac{7}{8}$	25.2	21.9	1.06	$2\frac{3}{4}$
1	32.3	28.5	1.38	$3\frac{1}{8}$
$1\frac{1}{16}$	36.9	32.1	1.56	$3\frac{3}{8}$
$1\frac{1}{8}$	41.2	35.9	1.75	$3\frac{1}{2}$
$\frac{13}{16}$	45.9	39.9	1.95	$3\frac{3}{4}$
$1\frac{1}{4}$	50.7	44.1	2.16	$3\frac{7}{8}$
$1\frac{3}{8}$	61.0	53.1	2.61	$4\frac{3}{8}$
$1\frac{7}{16}$	66.5	57.9	2.85	$4\frac{1}{2}$
$1\frac{1}{2}$	72.3	62.9	3.11	$4\frac{3}{4}$
$1\frac{5}{8}$	84.5	73.4	3.64	$5\frac{1}{8}$
$1\frac{11}{16}$	90.9	79.0	3.93	$5\frac{1}{4}$
$1\frac{3}{4}$	97.5	84.8	4.23	$5\frac{1}{2}$
$1\frac{13}{16}$	104.0	90.8	4.53	$5\frac{3}{4}$
$1\frac{15}{16}$	119.0	103.0	5.18	$6\frac{1}{8}$
2	126.0	110.0	5.52	$6\frac{1}{4}$
$2\frac{1}{16}$	134.0	116.0	5.87	$6\frac{1}{2}$
6 x 37 Galvanized Hawser				
$\frac{3}{4}$	21.0	18.2	0.87	$2\frac{3}{8}$
$\frac{13}{16}$	24.5	21.3	1.02	$2\frac{1}{2}$
$\frac{7}{8}$	28.4	24.7	1.19	$2\frac{3}{4}$
1	36.9	32.1	1.55	$3\frac{1}{8}$
$1\frac{1}{16}$	41.6	36.1	1.75	$3\frac{3}{8}$
$1\frac{1}{8}$	46.5	40.4	1.96	$3\frac{1}{2}$
$\frac{13}{16}$	51.7	44.9	2.19	$3\frac{3}{4}$
$1\frac{1}{4}$	57.1	49.7	2.42	$3\frac{7}{8}$
$1\frac{3}{8}$	68.8	59.8	2.93	$4\frac{3}{8}$
$1\frac{7}{16}$	75.0	65.3	3.20	$4\frac{1}{2}$
$1\frac{1}{2}$	81.5	70.9	3.49	$4\frac{3}{4}$
$1\frac{5}{8}$	95.3	82.9	4.09	$5\frac{1}{8}$
$1\frac{11}{16}$	103.0	89.2	4.41	$5\frac{1}{4}$
$1\frac{3}{4}$	110.0	95.7	4.75	$5\frac{1}{2}$
$1\frac{13}{16}$	118.0	102.0	5.09	$5\frac{3}{4}$
$1\frac{15}{16}$	134.0	117.0	5.82	$6\frac{1}{8}$
2	143.0	124.0	6.20	$6\frac{1}{4}$
$2\frac{1}{16}$	151.0	132.0	6.59	$6\frac{1}{2}$
$2\frac{1}{8}$	160.0	139.0	7.00	$6\frac{5}{8}$
$2\frac{1}{4}$	179.0	156.0	7.85	$7\frac{1}{8}$
$2\frac{5}{16}$	189.0	164.0	8.29	$7\frac{1}{4}$
$2\frac{3}{8}$	199.0	173.0	8.74	$7\frac{1}{2}$

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Rope Diameter Inches	Breaking Strength in Tons of 2,000 Lbs.	Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
6 x 3 x 19 Galvanized, Excellay, Spring-Lay Rope			
$\frac{1}{2}$	4.0	0.22	$1\frac{5}{8}$
$\frac{9}{16}$	5.0	.28	$1\frac{3}{4}$
$\frac{5}{8}$	6.25	.34	2
$\frac{3}{4}$	9.0	.49	$2\frac{3}{8}$
$\frac{7}{8}$	12.25	.63	$2\frac{3}{4}$
1	15.0	.88	$3\frac{1}{8}$
$1\frac{1}{8}$	19.0	1.14	$3\frac{1}{2}$
$1\frac{1}{4}$	23.5	1.36	$3\frac{7}{8}$
$1\frac{3}{8}$	28.0	1.66	$4\frac{3}{8}$
$1\frac{1}{2}$	36.0	1.97	$4\frac{3}{4}$
$1\frac{5}{8}$	42.0	2.28	$5\frac{1}{8}$
$1\frac{3}{4}$	49.0	2.67	$5\frac{1}{2}$
$1\frac{7}{8}$	56.0	3.09	$5\frac{7}{8}$
2	60.0	3.53	$6\frac{1}{4}$

Rope Diameter Inches		BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
Before Serving	After Serving	Plow Steel	Mild Plow Steel		
5 x 19 Marlin Clad Hoisting Rope					
$\frac{1}{4}$ x $\frac{9}{16}$		2.17	1.89	0.21	$1\frac{3}{4}$
$\frac{5}{16}$ x $\frac{5}{8}$		3.37	2.93	.28	2
$\frac{3}{8}$ x $1\frac{1}{16}$		4.82	4.20	.36	$2\frac{1}{8}$
$\frac{7}{16}$ x $\frac{3}{4}$		6.53	5.68	.42	$2\frac{3}{8}$
$\frac{1}{2}$ x $1\frac{3}{16}$		8.50	7.39	.51	$2\frac{1}{2}$
$\frac{9}{16}$ x $\frac{7}{8}$		10.7	9.31	.62	$2\frac{3}{4}$
$\frac{5}{8}$ x 1		13.2	11.4	.81	$3\frac{1}{8}$
$\frac{3}{4}$ x $1\frac{1}{8}$		18.8	16.4	1.10	$3\frac{1}{2}$
$\frac{7}{8}$ x $1\frac{1}{4}$		25.5	22.1	1.32	$3\frac{7}{8}$
1 x $1\frac{3}{8}$		33.1	28.7	1.70	$4\frac{3}{8}$
$1\frac{1}{8}$ x $1\frac{1}{2}$		41.6	36.2	2.12	$4\frac{3}{4}$
$1\frac{1}{4}$ x $1\frac{5}{8}$		51.1	44.4	2.58	$5\frac{1}{8}$
$1\frac{3}{8}$ x $1\frac{3}{4}$		61.4	53.4	3.14	$5\frac{1}{2}$

Rope Diameter Inches		Breaking Strength in Tons of 2,000 Lbs.	Approx. Weight per Foot in Lbs.	Approx. Circ. Inches
Before Serving	After Serving			
6 x 19, Mild Plow Steel, Marlin Clad Grain Shovel Rope				
$\frac{1}{4}$ x $\frac{5}{8}$		2.50	0.25	2
$\frac{3}{8}$ x $\frac{3}{4}$		5.50	.43	$2\frac{3}{8}$

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Strand Diameter Inches	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.
	Special	Standard	
Locked Coil Track Strand			
$\frac{3}{4}$	31.5	25.0	1.41
$\frac{7}{8}$	41.5	32.0	1.92
1	52.5	42.0	2.50
$1\frac{1}{8}$	66.0	54.0	3.16
$1\frac{1}{4}$	81.0	65.0	3.91
$1\frac{3}{8}$	100.0	78.0	4.73
$1\frac{1}{2}$	120.5	93.0	5.63
$1\frac{5}{8}$	140.0	108.0	6.60
$1\frac{3}{4}$	165.0	125.0	7.66
$1\frac{7}{8}$	187.5	138.0	8.79
2	215.0	158.0	10.00
$2\frac{1}{4}$	280.0		12.50
$2\frac{1}{2}$	345.0		15.20
$2\frac{3}{4}$	420.0		18.30
3	500.0		22.20

Strand Diameter Inches	No. of Wires	BREAKING STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight per Foot in Lbs.
		Extra High Strength	High Strength	
Smooth Coil Track Strand				
1/2	19	15.3	12.6	0.55
5/8	19	22.3	19.2	.86
3/4	19	32.5	27.6	1.24
7/8	19	44.4	37.6	1.69
1	19	58.0	49.2	2.20
1 1/8	37	70.7	60.0	2.70
1 1/4	37	84.6	71.8	3.23
1 3/8	37	105.0	88.8	4.01
1 1/2	37	127.5	108.4	4.88
1 5/8	61	146.0	124.0	5.63
1 3/4	61	171.0	145.8	6.59
1 7/8	61	189.0	161.0	7.28
2	91	218.0	185.0	8.40
2 1/8	91	240.0	204.0	9.35
2 1/4	91	266.0	233.0	10.36
2 1/2	91	335.0	285.0	13.10

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Thickness and Width in Inches	BREAKING STRENGTH IN TONS OF 2000 LBS.		Approximate Weight Per Ft. in Lbs.
	Plow Steel	Mild Plow Steel	
Flat Wire Rope			
$\frac{1}{4}$ x $1\frac{1}{2}$	16.8	14.6	0.69
$\frac{1}{4}$ x 2	21.7	18.8	.88
$\frac{1}{4}$ x $2\frac{1}{2}$	26.5	23.0	1.15
$\frac{1}{4}$ x 3	31.3	27.2	1.34
$\frac{5}{16}$ x $1\frac{1}{2}$	18.5	16.0	.77
$\frac{5}{16}$ x 2	25.8	22.4	1.05
$\frac{5}{16}$ x $2\frac{1}{2}$	33.2	28.8	1.33
$\frac{5}{16}$ x 3	40.5	35.3	1.61
$\frac{5}{16}$ x $3\frac{1}{2}$	47.9	41.7	1.89
$\frac{5}{16}$ x 4	55.3	48.1	2.17
$\frac{3}{8}$ x 2	31.4	27.3	1.25
$\frac{3}{8}$ x $2\frac{1}{2}$	41.8	36.4	1.64
$\frac{3}{8}$ x 3	47.1	40.9	1.84
$\frac{3}{8}$ x $3\frac{1}{2}$	57.5	50.0	2.23
$\frac{3}{8}$ x 4	62.7	54.6	2.44
$\frac{3}{8}$ x $4\frac{1}{2}$	73.2	63.7	2.83
$\frac{3}{8}$ x 5	78.4	68.2	3.03
$\frac{3}{8}$ x $5\frac{1}{2}$	88.9	77.3	3.42
$\frac{3}{8}$ x 6	94.1	81.9	3.63
$\frac{1}{2}$ x $2\frac{1}{2}$	54.5	47.4	2.13
$\frac{1}{2}$ x 3	63.6	55.4	2.47
$\frac{1}{2}$ x $3\frac{1}{2}$	72.7	63.3	2.82
$\frac{1}{2}$ x 4	81.8	71.2	3.16
$\frac{1}{2}$ x $4\frac{1}{2}$	90.9	79.1	3.82
$\frac{1}{2}$ x 5	109.0	94.9	4.16
$\frac{1}{2}$ x $5\frac{1}{2}$	118.0	103.0	4.50
$\frac{1}{2}$ x 6	127.0	111.0	4.85
$\frac{1}{2}$ x 7	145.0	126.0	5.85
$\frac{5}{8}$ x $3\frac{1}{2}$	85.8	74.6	3.40
$\frac{5}{8}$ x 4	100.0	87.1	3.95
$\frac{5}{8}$ x $4\frac{1}{2}$	114.0	99.5	4.50
$\frac{5}{8}$ x 5	129.0	112.0	5.04
$\frac{5}{8}$ x $5\frac{1}{2}$	143.0	124.0	5.59
$\frac{5}{8}$ x 6	157.0	137.0	6.14
$\frac{5}{8}$ x 7	186.0	162.0	7.23
$\frac{5}{8}$ x 8	214.0	186.0	8.32
$\frac{3}{4}$ x 5	165.0	143.0	6.50
$\frac{3}{4}$ x 6	185.0	161.0	7.31
$\frac{3}{4}$ x 7	206.0	179.0	8.13
$\frac{3}{4}$ x 8	227.0	197.0	9.70
$\frac{7}{8}$ x 5	190.0	165.0	7.50
$\frac{7}{8}$ x 6	217.0	188.0	8.56
$\frac{7}{8}$ x 7	244.0	212.0	9.63
$\frac{7}{8}$ x 8	271.0	236.0	10.69

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Diameter in Inches	BREAKING STRENGTH IN LBS.			APPROXIMATE WEIGHT PER 1000 FEET IN LBS.		
	19 Wire	7x7	7x19	19 Wire	7x7	7x19

Corrosion-Resisting Steel Aircraft Cables

$\frac{1}{32}$	*150			*2.5		
$\frac{3}{64}$	*375			*5.5		
$\frac{1}{16}$	500	480		8.5	7.5	
$\frac{5}{64}$	800	650		14	11	
$\frac{3}{32}$	1,200	920		20	16	
$\frac{7}{64}$	1,600	1,260		27	22	
$\frac{1}{8}$	2,100	1,700	1,900	35	28	29
$\frac{5}{32}$	3,300	2,600	2,600	55	43	45
$\frac{3}{16}$	4,700	3,700	3,900	77	62	65
$\frac{7}{32}$	6,300	4,800	5,200	102	83	86
$\frac{1}{4}$	8,200	6,100	6,600	135	106	110
$\frac{9}{32}$	10,300	7,600	8,000	170	134	139
$\frac{5}{16}$	12,500	9,100	**8,200	210	167	173
$\frac{1}{2}$		10,800	11,700		201	207
$\frac{3}{8}$		12,600	**12,000		236	243

*7 Wire.

**Made of Molybdenum Bearing Corrosion-Resisting Steel.

Diameter in Inches	BREAKING STRENGTH IN LBS.		APPROX. WEIGHT PER 1000 FT. IN LBS.	
	6x7 With Wire Core	6x19 With Wire Core	6x7 With Wire Core	6x19 With Wire Core

Corrosion-Resisting Steel Ropes

$\frac{7}{16}$	16,500	**16,000	330	355
$\frac{1}{2}$	21,300	22,800	430	460
$\frac{9}{16}$	26,600	28,500	540	585
$\frac{5}{8}$	32,500	35,000	665	725
$\frac{3}{4}$	45,500	49,600	935	1,030
$\frac{7}{8}$	60,200	66,500	1,300	1,410
1	76,700	85,400	1,690	1,840
$1\frac{1}{8}$	94,600	106,400	2,140	2,330
$1\frac{1}{4}$	113,300	129,400	2,630	2,870

**Made of Molybdenum Bearing Corrosion-Resisting Steel.

AMERICAN TIGER BRAND

STRENGTHS AND WEIGHTS OF WIRE ROPES AND STRANDS

Diameter in Inches	BREAKING STRENGTH IN POUNDS				APPROX. WEIGHT PER THOUSAND FT. IN LBS.			
	19-Wire Strand	6 x 7	7 x 7	7 x 19	19-Wire Strand	6 x 7	7 x 7	7 x 19
Carbon Steel Aircraft Cables								
$\frac{1}{32}$	*185				*2.5			
$\frac{3}{64}$	*375				*5.5			
$\frac{1}{16}$	500	400	480		8.5	6.8	7.5	
$\frac{5}{64}$	800	550	650		14	10	11	
$\frac{3}{32}$	1200	800	920		20	14.5	16	
$\frac{7}{64}$	1600	1050	1260		27	20	22	
$\frac{1}{8}$	2100	1440	1700	2000	35	25.5	28	29
$\frac{5}{32}$	3300	2200	2600	2800	55	39	43	45
$\frac{3}{16}$	4700	3150	3700	4200	77	56	62	65
$\frac{7}{32}$	6300	4100	4800	5600	102	75	83	86
$\frac{1}{4}$	8200	5200	6100	7000	135	97	106	110
$\frac{9}{32}$	10300	6600	7600	8000	170	122	134	139
$\frac{5}{16}$	12500	8000	9200	9800	210	152	167	173
$\frac{11}{32}$		9500	11100	12500		182	201	207
$\frac{3}{8}$		11500	13100	14400		215	236	243

*7-Wire.

Strand Diameter Inches	BREAKING STRENGTH IN POUNDS				Approx. Weight per Foot in Lbs.
	Extra High Strength	High Strength	Siemens-Martin	Common	
7-Wire Galvanized Strand					
$\frac{1}{8}$	1830	1330	910	540	0.032
$\frac{5}{32}$	2940	2140	1470	870	.051
$\frac{3}{16}$	3990	2850	1900	1150	.073
$\frac{7}{32}$	5400	3850	2560	1540	.098
$\frac{1}{4}$	6650	4750	3150	1900	.121
$\frac{9}{32}$	8950	6400	4250	2570	.164
$\frac{5}{16}$	11200	8000	5350	3200	.205
$\frac{3}{8}$	15400	10800	6950	4250	.273
$\frac{7}{16}$	20800	14500	9350	5700	.399
$\frac{1}{2}$	26900	18800	12100	7400	.517
$\frac{9}{16}$	35000	24500	15700	9600	.671
$\frac{5}{8}$	42400	29600	19100	11600	.813
19-Wire Galvanized Strand					
$\frac{1}{2}$	26700	19100	12700	7620	0.504
$\frac{9}{16}$	33700	24100	16100	9640	.637
$\frac{5}{8}$	40200	28100	18100	11000	.796
$\frac{3}{4}$	58300	40800	26200	16000	1.155
$\frac{7}{8}$	79700	55800	35900	21900	1.581
1	104500	73200	47000	28700	2.073

STRETCH OF WIRE ROPE

The stretch of a wire rope under load is the result of two components: the structural stretch, caused by lengthening of the rope lay, compression of the core, and adjustment of the wires and strands to the load; and the elastic stretch, caused by elongation of the wires.

The structural stretch varies with the size of core, the lengths of lays, and the construction of the rope. This stretch also varies with the loads imposed and the amount of bending to which the rope is subjected. For estimating this stretch the value of $\frac{1}{2}\%$, or .005 times the length of rope under load, gives an approximate figure. If loads

are light, $\frac{1}{4}\%$ or .0025 times the rope length may be used. With heavy loads, this stretch may approach 1%, or .01 times the rope length.

The elastic stretch of a wire rope is directly proportional to the load and the length of rope under load, and inversely proportional to the metallic area and modulus of elasticity. This applies only to loads which do not exceed the elastic limit of a wire rope. The elastic limit of a bright wire rope is approximately 55% of its breaking strength; and for galvanized ropes it is approximately 50%. This may be expressed as:

$$\text{Elastic Stretch} = \frac{(\text{Load in Lbs.}) (\text{Length of rope under load in feet})}{(\text{Metallic area of rope in square inches}) (\text{Modulus of Elasticity})}$$

The approximate metallic areas of the principal constructions of American Tiger Brand Wire

Ropes and Strands are shown below.

APPROXIMATE METALLIC AREAS IN SQUARE INCHES

Dia.	6x7 6x17 Type L	6x19 Seale Patent 6x21 Type M	6x19 Type N 6x29 Type P 6x33 Type R 6x37 Type S	6x25 Type B	8x19	Galv. Wire Core Bridge Ropes	Galv. Bridge Strands	TRACK STRANDS		GALVANIZED GUY STRANDS	
								Locked Coil	Smooth Coil	7-Wire	19-Wire
$\frac{3}{8}$	0.054	0.055	0.057	0.062	0.050					0.079	
									19-Wire		
$\frac{1}{2}$	.095	.098	.101	.110	.088				.143	.149	0.145
$\frac{9}{16}$	.121	.124	.128	.140	.112						.184
$\frac{5}{8}$	.149	.154	.158	.172	.139				.223		.228
$\frac{3}{4}$	.215	.221	.227	.248	.199			.385	.301		.328
$\frac{7}{8}$	.294	.302	.309	.338	.270		0.450	.531	.438		.442
1	.380	.394	.404	.440	.352	.471	.596	.677	.576		.580
									37-Wire		
$1\frac{1}{8}$	.484	.498	.512	.560	.448	.596	.760	.850	.723		
$1\frac{1}{4}$	.596	.616	.632	.688	.556	.745	.940	1.041	.897		
$1\frac{3}{8}$	.725	.747	.762	.836	.670	.906	1.135	1.244	1.081		
$1\frac{1}{2}$	.860	.888	.908	.992	.796	1.076	1.350	1.490	1.292		
									61-Wire		
$1\frac{5}{8}$		1.042	1.065	1.165		1.27	1.590	1.782	1.511		
$1\frac{3}{4}$		1.207	1.236	1.352		1.47	1.840	2.061	1.758		
$1\frac{7}{8}$		1.386	1.419			1.69	2.110	2.334	2.007		
									91-Wire		
2		1.576	1.616			1.92	2.400	2.659	2.291		
$2\frac{1}{4}$		1.994	2.048			2.42	3.040	3.432	2.885		
$2\frac{1}{2}$			2.528			2.97		4.168	3.557		
$2\frac{3}{4}$			3.048			3.58		4.987			
3			3.632			4.25		6.044			

Independent Wire Rope Cores add approximately 15% to the metallic area of ropes of six round strands; and approximately 10% to 6x25, Type B. Wire Strand Cores add approximately 20% to the metallic area of six-strand, round strand, wire ropes.

APPROXIMATE MODULI OF ELASTICITY

The commonly used approximate values for moduli of elasticity of the various constructions are listed below.

New or unused wire ropes will have a greater elongation than used ropes, because the greater portion of the structural stretch of a rope occurs during the initial period of its useful life. The modulus of elasticity is also the smallest during this period.

Construction	Modulus of Elasticity
6x7.....	14,000,000
6x17, Type L.....	13,000,000
6x19, Seale Patent.....	
6x21, Type M.....	
6x19.....	12,000,000
6x29, Type P.....	
6x25, Type B.....	
6x30, Type G.....	
6x19 with Independent Wire Rope Core.....	14,000,000
6x33, Type R.....	11,000,000
6x37, Type S.....	
8x19.....	10,000,000
Galvanized Wire Core Bridge Ropes { 6 x 7.....	16,000,000
{ 6 x 19.....	15,000,000
{ 6 x 37.....	14,000,000
Prestressed Galvanized Wire Core Bridge Ropes.....	20,000,000
Galvanized Bridge Strands { 7 Wire.....	21,000,000
{ 19 Wire.....	19,000,000
Galvanized Guy Strands { 37 Wire.....	18,000,000
{ 61 Wire.....	17,000,000
{ 91 Wire.....	16,000,000
Prestressed Galvanized Bridge Strands.....	25,000,000
Locked Coil Track Strand.....	19,000,000
Smooth Coil Track Strand.....	19,000,000

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RESERVE STRENGTHS

The reserve strength of a wire rope is the strength of the rope exclusive of the outer wires, which are the first to be destroyed by wear and abrasion. As the number of layers of wires in each strand

increases, the reserve strength increases. Well lubricated ropes have the following approximate reserve strengths in terms of total strengths of new ropes.

Construction	PERCENTAGE OF TOTAL	
	Outer Wires	Inner Wires (Reserve Strength)
6x7.....	83	17
6x6x7 Tiller Rope.....		
6x17 Type L.....	73	27
*6x25 Type B.....		
6x19 Seale Patent.....	69	31
8x19 Seale Patent.....		
6x30 Type G.....	66	34
6x21 Type M.....	64	36
6x19 Warrington.....	59	41
6x19 Type N.....	57	43
8x19.....		
18x7 Non-Spinning.....	54	46
6x29 Type P.....	50	50
6x33 Type R.....	48	52
6x37 Type S.....	43	57

*Triangular core wires not included.

SIZE OF OUTER WIRES

Rope Construction	Factor for Determining Approx. Size of Outer Wires (Multiply Dia. of Rope by Factor)
6x7.....	1/9
6x12—1 Fiber Core.....	1/12
6x17 Type L.....	1/12
6x19 Seale Patent.....	1/13
6x19 Warrington (Large Outer Wires).....	1/14
6x21 Type M.....	1/14
6x25 Type B Flattened Strand.....	1/14
6x30 Type G Flattened Strand.....	1/14
6x12—7 Fiber Cores.....	1/15
8x19 Seale Patent.....	1/15
6x19 Type N.....	1/16
18x7 Non-Spinning.....	1/16
6x29 Type P.....	1/18
8x19.....	1/19
6x33 Type R.....	1/20
6x37.....	1/22
6x61.....	1/29
6x6x7 Tiller Rope.....	1/29

EFFECTS OF BENDING

All wire ropes, except stationary ropes used as guys or supports, are subjected to bending around sheaves or drums. The service obtained from wire ropes is, to a large extent, dependent upon the proper choice and location of the sheaves and drums about which it operates.

A wire rope may be considered as a machine in which the individual elements (wires and strands) slide upon each other when the rope is bent. Therefore, as a prerequisite to the satisfactory operation of wire rope over sheaves and drums, the rope must be properly lubricated. (See Lubrication—page 113.) With this in mind, the effects of bending may be classified as:

Loss of strength due to bending.

Fatigue effect of bending.

Loss of strength due to bending is caused by the inability of the individual strands and wires to adjust themselves to their changed position when the rope is bent. Tests made by the Bureau of Standards and reported in Technologic Paper No. 229 show that the rope strength decreases in a marked degree as the sheave diameter grows smaller with respect to the diameter of the rope. The loss of strength due to bending wire ropes over the sheaves found in common use will not exceed 6% and will usually be about 4%.

The bending of a wire rope is accompanied by readjustments in the positions of the strands and wires and results in actual bending of the wires. Repetitive flexing of the wires develops bending loads which, even though well within the elastic limit of the wires, set up points of stress concentration.

The fatigue effect of bending appears in the form of small cracks in the wires at these overstressed foci. These cracks propagate, under repeated stress cycles, until the remaining sound metal is inadequate to withstand the bending load. This results in broken wires showing no apparent contraction of cross section.

Experience has established the fact that from the service viewpoint, a very definite relationship exists between the size of the individual outer wires of a wire rope and the size of the sheave or drum about which it operates. Sheaves and drums smaller than 200 times the diameter of the outer wires will cause permanent set in a heavily loaded rope. Good practice requires the use of sheaves and drums with diameters 800 times the diameter of the outer wires in the rope for heavily loaded fast-moving ropes. For mine hoists, the factors are usually about 1,000; for elevators, approximately 900.

It is impossible to give a definite minimum size of sheave or drum about which a wire rope will operate with satisfactory results, because of the other factors affecting the useful life of the rope. If the loads are light or the speed slow, smaller

sheaves and drums can be used without causing early fatigue of the wires than if the loads are heavy or the speed fast. Reverse bends, where a rope is bent in one direction and then in the opposite direction, cause excessive fatigue and should be avoided whenever possible. When a reverse bend is necessary, larger sheaves are required than would be the case if the rope were bent in one direction only.

Tables I and II show the minimum tread diameters of sheaves and drums for use with the various sizes, grades, and constructions of wire rope. These diameters are based on factors of 600 times the diameters of the outer wires for the table covering iron ropes, and 400 for the table covering the higher grades of wire rope, with the exception of the 18x7 Non-Spinning, for which a factor of 500 is used.

It should be clearly understood that these are not the recommended diameters of sheaves and drums for use with American Tiger Brand Wire Rope. These are the minimum sizes which, under favorable operating conditions, can be expected to give reasonable wire rope service. If the other features of operation, such as speeds and loads, are severe, larger sheaves and drums should be used; the amount by which they exceed these minimum figures depending upon the severity of the conditions of service. The use of sheaves and drums larger than shown in these tables will result in increased wire rope service, which usually will more than warrant the additional cost of the larger sheaves and drums.

TABLE I
MINIMUM TREAD DIAMETER OF
SHEAVES AND DRUMS IN INCHES
Bright Iron Wire Ropes

Rope Dia.	6x7	6x19	8x19	6x6x7 Tiller
$\frac{1}{4}$	$15\frac{3}{4}$	10	8	5
$\frac{5}{16}$	$19\frac{3}{4}$	$12\frac{1}{2}$	10	$6\frac{1}{4}$
$\frac{3}{8}$	$23\frac{1}{2}$	15	12	$7\frac{1}{2}$
$\frac{7}{16}$	$27\frac{1}{2}$	$17\frac{1}{2}$	14	$8\frac{3}{4}$
$\frac{1}{2}$	$31\frac{1}{2}$	20	16	10
$\frac{9}{16}$	$35\frac{1}{2}$	$22\frac{1}{2}$	18	$11\frac{1}{4}$
$\frac{5}{8}$	$39\frac{1}{2}$	25	20	$12\frac{1}{2}$
$\frac{3}{4}$	$47\frac{1}{4}$	30	24	15
$\frac{7}{8}$	$55\frac{1}{4}$	35	28	$17\frac{1}{2}$
1		40	32	20
$1\frac{1}{8}$		45		
$1\frac{1}{4}$		50		

These Minimum Tread Diameters are based on factors of 600 times the diameters of the outer wires.

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TABLE II
MINIMUM TREAD DIAMETERS OF SHEAVES
AND DRUMS IN INCHES

Mild Plow Steel, Plow Steel, and Monitor Steel Wire Ropes

Rope Dia.	6x7	6x19 Warrington			6x29 Type P	6x33 Type R 8x19	6x37 Type S 5x19 Marlin Clad	6x6x7 Tiller
		6x19 Seale Pat.	6x25 Type B					
		6x17 Type L	6x30 Type G	6x19 Type N				
		18x7 Non-Spin.	6x21 Type M	8x19 Seale Pat.				
1/4	10 1/2	8 1/2	7 1/2	6 1/2				3 1/2
5/16	13 1/4	10 3/4	9 1/2	8 1/4				4 1/2
3/8	15 3/4	12 3/4	11 1/4	9 3/4	8 3/4	8	6 3/4	5 1/4
7/16	18 1/2	15	13 1/4	11 1/2	10	9 1/4	8	6 1/4
1/2	21	17	15	13	11 1/2	10 1/2	9	7
9/16	23 1/2	19 1/4	17	14 3/4	13	11 3/4	10 1/4	8
5/8	26 1/4	21 1/4	18 3/4	16 1/4	14 1/2	13 1/4	11 1/4	8 3/4
3/4	31 1/2	25 1/2	22 1/2	19 1/2	17 1/4	15 3/4	13 1/2	10 1/2
7/8	36 3/4	29 3/4	26 1/4	22 3/4	20 1/4	18 1/2	15 3/4	12 1/4
1	42	34	30	26	23	21	18	14
1 1/8	47 1/4	38 1/4	33 3/4	29 1/4	26	23 3/4	20 1/4	
1 1/4	52 1/2	42 1/2	37 1/2	32 1/2	28 3/4	26 1/4	22 1/2	
1 3/8	57 3/4	46 3/4	41 1/4	35 3/4	31 3/4	29	24 3/4	
1 1/2	63	51	45	39	34 1/2	31 1/2	27	
1 5/8		55 1/4	48 3/4	42 1/4	37 1/2	34 1/4	29 1/4	
1 3/4		59 1/2	52 1/2	45 1/2	40 1/4	36 3/4	31 1/2	
1 7/8		63 3/4	56 1/4	48 3/4	43 1/4	39 1/2	33 3/4	
2		68	60	52	46	42	36	
2 1/4		76 1/2	67 1/2	58 1/2	51 3/4	47 1/4	40 1/2	
2 1/2			75	65	57 1/2	52 1/2	45	
2 3/4					63 1/4	57 3/4	49 1/2	
3					69	63	54	
3 1/4						68 1/4	58 1/2	
3 1/2							63	

These Minimum Tread Diameters are based on factors of 400 times the diameters of outer wires for all except the 18x7 Non-Spinning Rope, for which a factor of 500 is used.

STRESSES DUE TO ACCELERATION

In order to cause a body to move from one point to another, a force must be applied to the body. If the rate at which the body moves from point to point does not change during successive intervals of time, we say that the body has a constant speed or a uniform velocity. Velocity is always expressed as a ratio of distance to time, and for our purposes, we shall use the ratio, feet per second (ft./sec.) as most convenient. The force necessary to move a body with uniform velocity is constant, and is that force required to overcome frictional and gravitational resistance.

On the other hand, if the velocity of a body changes, additional force is necessary to cause this change. Suppose a body initially at rest starts to move so that it has velocities at the end of certain time intervals as follows:

At end of 1st second velocity is 5 ft./sec.

At end of 2nd second velocity is 10 ft./sec.

At end of 3rd second velocity is 15 ft./sec.

At end of 4th second velocity is 20 ft./sec.

We see that at the end of each second the velocity has been increased 5 ft./sec., or, in other words, the body has been accelerated at the rate of 5 feet per second per second. (Written 5 ft./sec./sec. or 5 ft./sec.²) If the acceleration is not at a uniform rate, we can only express the acceleration as it is at the end of a certain time interval. It will be found that the great majority of wire rope installations operate with a uniform acceleration, or that the acceleration decreases as the equipment gets up to maximum speed.

There are certain fundamental relations between weight, force, acceleration, velocity, distance and time which fortunately are very simple.

Let W = weight of a body and rope.

F = force necessary to cause a change of velocity. This is sometimes called the inertia force and does not include the forces needed to overcome friction or gravity.

g = acceleration due to gravity = 32.2 ft. per sec. per sec.

a = linear acceleration in ft. per sec. per sec.

v = linear velocity in ft. per sec.

s = distance in feet.

t = time in seconds of the acceleration period.

The formulas connecting these various quantities that we shall need to use are as follows:

$$(1) a = \frac{v}{t}$$

$$(2) a = \frac{2s}{t^2} \text{ when starting from rest}$$

$$(3) F = \frac{W}{g} a$$

From which

$$(4) F = \frac{W}{g} \times \frac{v}{t} = \frac{Wv}{32.2t}$$

$$(5) F = \frac{W}{g} \times \frac{2s}{t^2} = \frac{Ws}{16.1t^2}$$

Let us take, for example, a vertical mine hoist. W then represents the weight of the rope hanging in the shaft, the weight of the cage, the weight of the car, and the weight of whatever material is being brought to the surface. From the mine data, we can get v or the maximum speed of the hoist in feet per second. That still leaves the time t to be determined. By standing near the hoisting drum and listening to the engine during a few trips, the observer can very easily determine the number of seconds t it takes the engine to get up to speed. A stop watch will help considerably in determining this acceleration period. From formula (4), we can now get F , the extra force needed for acceleration.

In case the speed of the hoist is not available, then recourse is had to formula (5). t is determined as described above, and the acceleration distance s must also be found by observation. The hoist indicator can be used in estimating the distances traveled during the acceleration period, the uniform speed period, and the slowing

down period by noting the position of the indicator when the time is taken. A very close estimate of distance can be made in this manner. Local conditions may give the observer other means of determining the maximum speeds, time and distances.

The fact that all wire rope possesses a certain amount of elasticity makes conditions easier for long ropes on deep shafts or slopes. A certain amount of motion of the winding drum is absorbed in stretching the rope, and therefore the load is not accelerated as rapidly. The force necessary to overcome the inertia of sheaves, etc., in shallow shafts is also greater in proportion to the whole inertia force, and must be taken into consideration. Good practice therefore calls for the use of a higher factor of safety in shafts where the total hoisting distance is not great. See Safety Factors, page 6.

The rope user will find that acceleration stresses are generally much less in electrically operated hoists, for the automatic control prevents rapid starting as a protective measure for the electrical equipment. Steam hoists, on the other hand, are often started by opening the throttle wide, with consequent sudden jerks on the rope and extremely rapid acceleration for the first second or

two. In such cases, it is a safe precaution to add arbitrarily about 25% to calculated stresses obtained as above.

The preceding discussion has only considered speeding up or acceleration. The same forces come into play when apparatus is slowing down during retardation. In most hoists, the acceleration forces are greater because loads are being lifted, but occasionally conditions of extremely rapid retardation are met which necessitate careful checking to avoid undue stresses being placed on the rope. The same formulas apply in either case.

The following table gives the percentage of increase over the static load in hoisting cables due to accelerations of from .25 to 32 feet per second per second. These are based on $g = 32$ ft. per second per second.

Acceleration Feet/sec./sec.	% Increase of Load	Acceleration Feet/sec./sec.	% Increase of Load
.25	0.78	14	43.75
.50	1.56	16	50.00
.75	2.34	18	56.25
1	3.13	20	62.50
2	6.25	22	68.75
4	12.50	24	75.00
6	18.75	26	81.25
8	25.00	28	87.50
10	31.25	30	93.75
12	37.50	32	100.00

STRESSES IN GUYS

Guys are wire ropes or strands used to hold a vertical structure in position against an overturning force. The most common types of guyed structures are stacks, derricks, and masts for draglines, reversible tramways and radio transmission.

As a general rule stresses in guys from temperature changes are neglected, but in structures such as radio masts this is an important feature, and must be subject to special analysis.

The number of guys used for any particular installation is contingent on several variable factors, such as type of structure, space available for guys, contour of the ground, etc., and is not a part of this discussion.

It is desirable to space guys uniformly wherever possible in order to equalize the pull, P , on each guy insofar as possible, particularly against forces which change in direction, as when a derrick boom swings in its circle.

It is also desirable to equalize the erection tensions on the guys. When no external force is acting on the structure, the tension in each guy should be the same. A "Tension Indicator" is

sometimes used to determine the tension in guys. If this instrument is not available, the tension can be approximated very closely by measuring the deflection at the center of the span from the chord drawn from the guy anchorage to the point of support on the structure. The formulas for uniformly loaded cables will be found under "Stresses in Suspended Cables", and the initial tension may be found when the deflection, span and weight per foot are known. A good average figure to use for erection tension of guys is 20% of the maximum working tension of the guy.

Our purpose is to outline the method of determining the stresses in guys. One of the first considerations is the location of the guy anchorages. The anchorages should be so located that the angle α , (Alpha) between the horizontal plane and the guy line, is the same for all guys (to equalize erection tensions). Angle α , in good practice, seldom exceeds 45 degrees; 30 degrees being quite commonly used. The tension in the guys decreases as angle α becomes less. The direct load on the structure is also less with smaller values of α .

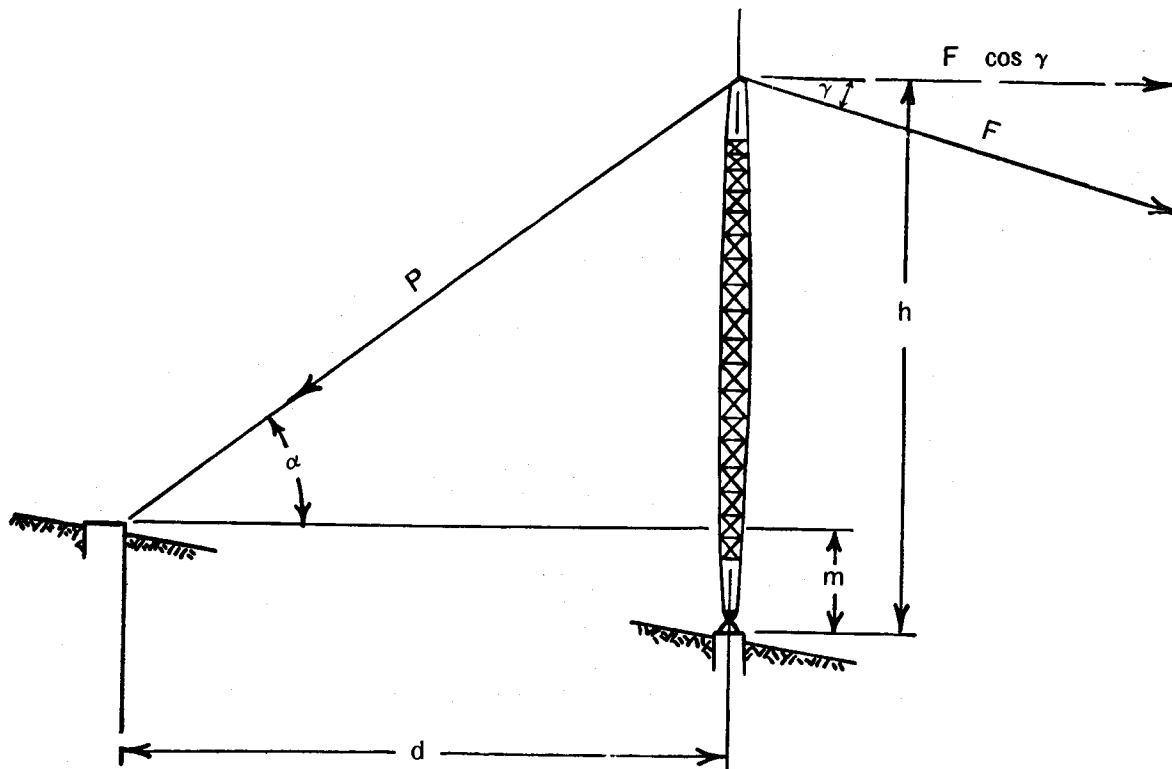


Figure 1

To find the maximum extra tension, T , that will be applied to any single guy by the force, F ; first, determine the pull, P , which is the amount required along the guys, in the same vertical plane as the force, F , to resist the horizontal component of the force, F . This pull, P , is entirely independent of the number of guys. Assume that the following are known:

F = The total resultant external force acting on the structure.

γ = Gamma = The angle between the horizontal plane and the force, F .

h = The height of structure.

d = The horizontal distance from structure to guy anchorage.

m = The vertical height of anchorage above or below base of structure.

The horizontal component of the force, F , = $F \cos \gamma$.

α = Alpha = the angle whose tangent is $\frac{h \pm m}{d}$.

m is plus if the anchorage is below the base of the structure and minus if it is above.

$$P = \frac{F \cos \gamma}{\cos \alpha}$$

As $\cos \alpha$ is always less than one, P is always greater than $F \cos \gamma$, the horizontal component of force, F .

It must be remembered that P represents the total pull acting along the guys at an angle, α , with the horizontal and in the same vertical plane as the force, F .

If only one guy were used, P would represent the extra tension, T . In practice, however, a number of guys are always used and, therefore, the pull on any one guy will not be equal to P . The following table gives factors for any number of guys from 3 to 15, equally spaced about a central structure. To find the maximum extra tension, T , that will be applied to any single guy by the force, F , capable of rotating 360 degrees around a vertical axis, it is only necessary to multiply the value of P , as determined above, by the factor for the number of guys used. It must be clearly understood in using this table that the guys are uniformly spaced and under equal tension when no load is acting on the structure.

TABLE III

No. of Guys	Factors*	No. of Guys	Factors*
3	1.15	10	.45
4	1.00	11	.40
5	.90	12	.37
6	.75	13	.35
7	.65	14	.32
8	.55	15	.30
9	.50		

*These factors are for average conditions. If the guys are erected under accurately measured tensions of not less than 20% of the working load the factors for 5 or more guys may be reduced 10%. If the erecting tensions are low or not accurately equalized, the factors for 5 or more guys should be increased 10%.

Example—A derrick mast 90 ft. high is supported by nine equally spaced guys anchored at a horizontal distance of 170 ft. from the mast and the elevations of the guy anchorages are 10 ft. below the base of the mast. The load on the structure is equivalent to a force of 10,000 lbs. acting on an angle of 10 degrees below the horizontal. What is the maximum pull on any single cable and what size guy rope should be used?

From Fig. I

$$\begin{aligned} h &= 90 \text{ Ft.} \\ d &= 170 \text{ Ft.} \\ m &= 10 \text{ Ft.} \\ \gamma &= 10^\circ - 00' \\ F &= 10,000 \text{ Lbs.} \end{aligned}$$

$$\tan \alpha = \frac{90 + 10}{170} = \frac{100}{170} = .588$$

$$\alpha = 30^\circ - 28'$$

$$P = \frac{F \cos \gamma}{\cos \alpha} = \frac{10,000 \times .985}{.862} = 11,427 \text{ Lbs.}$$

$$\text{From Table III, } T = 11,427 \times .50 = 5,714 \text{ Lbs.}$$

If erection tension is 10 per cent of total working tension, 5,714 is 90 per cent of total working tension. Therefore, working tension = $\frac{5714 \times 100}{90} = 6,349 \text{ Lbs.}$

With a factor of safety of 5, the guy ropes should have a breaking strength of 31,745 pounds. By referring to pages 22 and 28, it will be found that a $1\frac{1}{8}$ " diameter 6x7 Galvanized Iron Guy Rope, or a $\frac{7}{8}$ " diameter Extra Galvanized Siemens-Martin 19-wire strand, could be specified for this installation.

STRESSES IN SUSPENDED CABLES

Cable spans may be divided into two general classes, Anchored Spans, and Counterweighted Spans. In each of these divisions, we find it necessary to solve for stresses and deflections of uniformly loaded spans and also of spans supporting one or more individual concentrated loads. It is, therefore, necessary to analyze the conditions of each problem carefully and the following points must be considered:

1. Horizontal distance between supports.
2. Difference in elevation between supports.
3. Maximum allowable deflection, measured vertically from chord to cable.
4. Length of cable between supports.
5. Weight per foot of cable, to which must be added in certain cases the additional weight imposed by snow and ice.
6. Maximum load to be supported by the cable.
 - a. Load uniformly distributed over the length of the span.
 - b. A single load supported at any point in the span.
 - c. Multiple individual loads.
7. Is the cable anchored at both ends or is it anchored at one end and counterweighted at the other end?
8. Modulus of elasticity in tension.
9. Wind loads on the cable and on the suspended load.
10. Changes in length of cable due to changes in temperature.

Since our purpose is to present means for obtaining results quickly, we will not give derivations of the following formulas. Computations are simplified by the assumption that uniform loading is distributed horizontally, and that the cable assumes a parabolic arc. For the great majority of cases encountered in practice, the results thus obtained are sufficiently accurate. If special cases occur where the ratio of deflection to span is very large, then the catenary equations should be applied. These are available in several textbooks.

The following nomenclature will be used:

- A = Net cross sectional area of cable.
 a = Horizontal spacing of loads.
 $b = \frac{n(n-1)}{2}$ $c = \frac{u(u-1)}{2}$
 e = Base of Napierian system of logarithms = 2.7182818.
 E = Modulus of elasticity in tension.
 G = Weight of an individual concentrated load.
 h = Vertical difference in elevation of supports.

- k = Ratio of deflection to span = $\frac{y}{s}$ for level spans and $\frac{ws \cos^2 \alpha}{8t}$ for inclined spans.
 L_1 = Length along cable when the cable only is supported in a span.
 L_2 = Hypothetical length along cable at zero tension.
 L = Length along cable when either a uniformly distributed load or one or more concentrated loads are suspended.
 m = Horizontal distance from left support to the first load.
 n = Number of concentrated loads.
 P = Change in total length of cable per pound of tension = $\frac{L}{AF}$
 s = Horizontal distance between supports.
 s_1 = Chord length of sub-span between load and support or between two loads.
 t = Horizontal component of cable tension.
 t' = Maximum cable tension at left support.
 t'' = Maximum cable tension at right support.
 t_e = Erection tension of empty cable in an anchored span.
 u = Number of loads to left of xy in a multiple loaded span.
 w = Weight per foot of horizontal length of span for a uniformly distributed load, $w = w' \sec \alpha$.
 w' = Weight per foot of uniformly distributed load along the cable, which is assumed for purposes of parabolic curve calculations, as equivalent to uniformly distributed load along the chord.
 w'' = Weight per foot of uniformly distributed load along the cable for purposes of catenary curve calculations.
 x = Horizontal distance from support to xy.
 y = Vertical deflection from support to xy.
 y_e = Vertical deflection from support at center of span.
 z = A term in the general formula for multiple loaded counterweighted spans.
 α = Alpha = Angle between the horizontal and a chord between supports.
 β_1 = Beta₁ = Angle between the horizontal and a tangent to a cable curve at the left support.
 β_2 = Beta₂ = Angle between the horizontal and a tangent to a cable curve at the right support.
 β_3 = Beta₃ = Angle between the horizontal and a tangent to a cable curve at any point in a span.

- β_4 = Beta₄ = Angle between the horizontal and a tangent to a cable curve at a load.
 λ = Lambda = Change in length of cable per foot of length, per pound of tension.
 Δ = Delta = Total change in length of cable = $\lambda t L_2$.
 θ = Theta = Angle between the horizontal and the chord of a half span.
 sec = Secant of an angle = $\frac{1}{\cosine}$

ANCHORED SPANS are principally employed for supporting electrical cables, for guy lines, for suspension bridges, and usually for track cables of cableways and reversible aerial tramways where a single moving load is supported in a clear span.

When a cable span is erected, anchored at both ends, and a load of any kind supported from the cable, the deflection increases because of the elastic properties of the cable. The tension also increases when the load is applied.

It is necessary to select the size, construction, and grade of the cable, with a proper factor of safety, after having determined the maximum tension in the cable due to dead and live loads. It is then necessary to erect the cable at such a deflection that the maximum safe working ten-

sion will not be exceeded when the load is applied.

In the case of cableways with high self-supporting towers, the cable tension and deflection may be affected by yielding of the supports. A complete study of such a span includes the application of the theory of deflection in framed structures, but such a special condition does not come within the scope of this handbook. In all cases we will assume that cables are anchored to rigid supports or immovable ground anchorages.

The determination of the proper erection deflection and tension involves the use of the modulus of elasticity in tension for the particular construction of cable which is being used.

It is well known that the modulus of elasticity ranges between 28,000,000 and 30,000,000 for structural steel, but the modulus of elasticity of a wire cable, considering the cable as a whole, has various values depending on its construction, and also on the work that has been put into it.

The modulus can be appreciably increased by a prestressing operation. This is frequently done to bridge cables. In the case of track cables carrying rolling loads somewhat the same effect is secured after a period of operation, as most of the structural stretch is removed. See Moduli of Elasticity page 30, and Prestressed Strands and Ropes page 9.

LEVEL SPAN—UNIFORMLY LOADED—ANCHORED

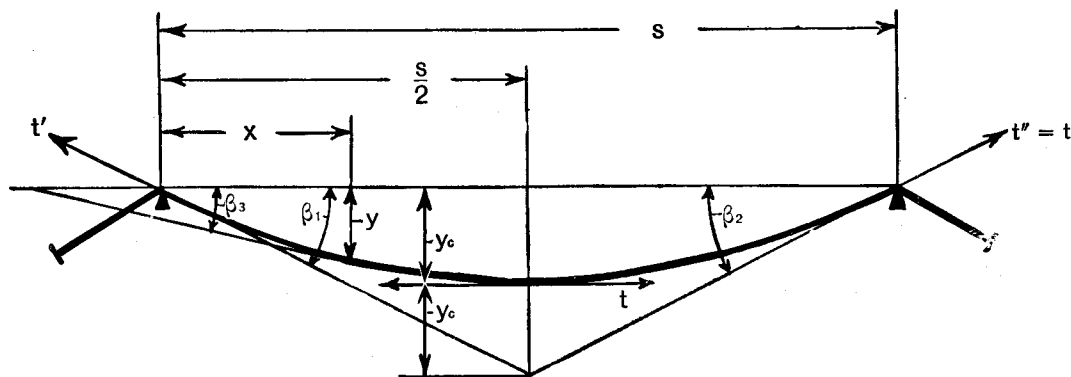


Figure 2

When the tension is known, the center deflection is found from:

$$y_c = \frac{ws^2}{8t} \quad (1)$$

and the deflection at any point in the span is:

$$y = \frac{wx(s-x)}{2t} \quad (2)$$

When the center deflection is known, the horizontal component of tension is found from:

$$t = \frac{ws^2}{8y_c} \quad (3)$$

When the deflection at some point other than the center of span is known:

$$t = \frac{wx(s-x)}{2y} \quad (4)$$

$$t' = t \sec \beta_1 \quad (5)$$

The cable slope at any point in the span is:

$$\tan \beta_3 = \frac{w}{t} \left(\frac{s}{2} - x \right) \quad (6)$$

At either support the cable slope is:

$$\tan \beta_1 \text{ or } \beta_2 = \frac{4y_c}{s} \quad (7)$$

$$\text{also } \tan \beta_1 \text{ or } \beta_2 = \frac{ws}{2t} \quad (8)$$

When the tension is known, the length of cable is:

$$L_1 \text{ or } L = s + \frac{w^2 s^3}{24t^2} (\text{approx.}) \quad (9)$$

When the deflection is known:

$$L_1 \text{ or } L = s \left[\frac{1}{2} \sqrt{1 + 16k^2} + \frac{1}{8k} \log_e \left(4k + \sqrt{1 + 16k^2} \right) \right] \quad (10)$$

An easier formula, giving closely approximate results is:

$$L_1 \text{ or } L = s \left(1 + \frac{8}{3} k^2 - \frac{32}{5} k^4 + \frac{256}{7} k^6 \right) \quad (11)$$

Sufficient accuracy can be secured, for many of the cases encountered in practice, by contracting formula (11) to:

$$L_1 \text{ or } L = s \left(1 + \frac{8}{3} k^2 \right) \quad (12)$$

In determining the erection tension for a uniformly loaded span, the values of L_1 and t_e must satisfy the equation:

$$L - L_1 = \frac{(t - t_e) L}{AE} = P (t - t_e) \quad (13)$$

By substitution of (9) for L_1 in (13) and using corresponding values of w and t_e ,

$$Pt_e + L - (Pt + s) = \frac{w^2 s^3}{24 t_e^2} \quad (14)$$

This equation can be solved for t_e , using the trial and error method.

EXAMPLE:

A 750,000 C.M. bare, hard drawn, stranded copper cable is to be supported across a river. Supports will be at the same elevation and 1350 feet centers. The copper cable is .9981" diameter, and will weigh 2.325 pounds per foot. N.E.L.A. class "B" loading is to be used, that is, a coating of ice 1/2 inch thick plus a horizontal wind load of eight pounds per square foot on the projected area of the ice coated cable. The center deflection can not exceed 75 feet.

(a) What are the specifications of the necessary messenger cable, assuming the same ice and wind loads?

(b) What is the cable slope at supports and at the quarter points of the span?

(c) What is the erection tension and deflection for the messenger strand only, assuming there are no ice or wind conditions at time of erection?

It is necessary to assume the diameter of messenger strand to figure the loading on the span. It may then be necessary to revise the figures if the first selection does not prove suitable. We will assume a 7/8" diameter strand weighing 1.581 pounds per foot.

Copper cable + ice = 3.240 pounds per foot

Messenger strand + ice = 2.421 pounds per foot

Total vertical load = 5.661 pounds per foot

Horizontal wind load on both cables = 2.582 pounds per foot

Total resultant load = 6.222 pounds per foot

$$\text{Then from (3)} \quad t = \frac{6.222 \times 1350^2}{8 \times 75} = 18,900 \text{ pounds}$$

$$\text{Then from (8)} \quad \tan \beta_1 = \frac{6.222 \times 1350}{2 \times 18900} = .2222, \quad \beta_1 = 12^\circ-32'$$

$$\text{Then from (5)} \quad t' = 18900 \times \sec 12^\circ-32' = 19365 \text{ pounds}$$

$$\text{Then from (6)} \quad \text{when } x = 337.5 \text{ feet}$$

$$\tan \beta_3 = \frac{6.222}{18900} (675 - 337.5) = .1111, \quad \beta_3 = 6^\circ-20'$$

With a factor of safety of 4, the required breaking strength will be $4 \times 19365 = 77460$ pounds. Page 28 shows 7/8" diameter, 19 wire, Extra Galvanized Extra High Strength Strand has a break-

ing strength of 79,700 pounds, and will be satisfactory for the purpose intended. $A = .4455$ square inches. $w = 1.581$ pounds.

$$(9) \quad L = 1350 + \frac{6.222^2 \times 1350^3}{24 \times 18900^2} = 1361.110 \text{ ft.}$$

In order to set up (14) in convenient form, first calculate the following:

$$P = \frac{1361.110}{.4455 \times 19,000,000} = .0001608$$

$$Pt + s = (.0001608 \times 18900) + 1350 = 1353.039$$

$$\frac{w^2 s^3}{24} = \frac{1.581^2 \times 1350^3}{24} = 256,240,000$$

Substituting these values in (14),

$$.0001608 t_e + 8.071 = \frac{256,240,000}{t_e^2}$$

WIRE ROPE ENGINEERING HANDBOOK

The following shows the results of a series of slide rule computations for assumed values of t_e until the above equation is satisfied (the values in the last two columns are equal).

t_e	$.0001608 t_e$	$+ 8.071$	$\frac{256,240,000}{t_e^2}$
5200	.836	8.907	9.476
5300	.852	8.923	9.122
5350	.860	8.931	8.952
5356	.861	8.932	8.932

$t_e = 5356$ pounds.

From (1) $y_e = \frac{1.581 \times 1350^2}{8 \times 5356} = 67.25$ ft.

From (9) $L_1 = 1350 + \frac{1.581^2 \times 1350^3}{24 \times 5356^2} = 1358.932$ ft.

Therefore:

- (a) One piece $\frac{7}{8}$ " diam. 19 wire Extra Galvanized Extra High Strength Strand with sockets attached so as to give a length of 1358.93 feet center to center of supports.

- (b) Maximum cable slope at supports = $12^\circ-32'$.

Maximum cable slope at quarter points of span = $6^\circ-20'$.

- (c) Erection tension = 5356 pounds
Erection deflection = 67.25 ft.

The following table No. IV gives factors for obtaining maximum tension t' at the supports of a uniformly loaded level span when w , the weight per horizontal foot and s , the horizontal length of span, are known. See column 2. The close relation between the parabola and the catenary is shown by a comparison of the values in columns 2 and 3. Column 3 gives the factor for obtaining t' when w'' , the weight per foot along the cable, and s is known. The length of a uniformly loaded level span, based on a parabolic curve, can be obtained from the factors in column 4. If the span is inclined see (24) and (25).

The factors in column 4 can also be used for the catenary for k ratios up to 0.12 with an error less than 0.02%, and for k ratios as high as 0.20 with an error of only 0.1%.

TABLE IV

Ratio of center deflection to chord length of span k	FACTORS FOR MAXIMUM TENSION		To get length of cable, multiply total span by factor below $L_1 = s \times \text{factor}$
	When weight of load per foot of span, w , is known $t' = ws \times \text{factor}$	When weight of load per foot of cable, w'' , is known $t' = w''s \times \text{factor}$	
	COLUMN 2	COLUMN 3	COLUMN 4
COLUMN 1			
.01	12.510	12.51	1.00027
.012	10.429	10.43	1.00038
.014	8.942	8.94	1.00052
.016	7.828	7.83	1.00068
.018	6.962	6.96	1.00086
.02	6.270	6.27	1.00107
.022	5.704	5.70	1.00129
.024	5.232	5.23	1.00153
.026	4.834	4.83	1.00180
.028	4.492	4.49	1.00209
.03	4.196	4.20	1.00240
.032	3.938	3.94	1.00272
.034	3.710	3.71	1.00307
.036	3.508	3.51	1.00344
.038	3.327	3.33	1.00384

AMERICAN TIGER BRAND

TABLE IV (Cont.)

Ratio of center deflection to chord length of span $\frac{\delta}{L}$	FACTORS FOR MAXIMUM TENSION		To get length of cable, multiply total span by factor below $L_1 = s \times \text{factor}$
	When weight of load per foot of span, w , is known $t' = ws \times \text{factor}$	When weight of load per foot of cable, w'' , is known $t' = w''s \times \text{factor}$	
COLUMN 1	COLUMN 2	COLUMN 3	COLUMN 4
.04	3.165	3.17	1.00425
.042	3.018	3.02	1.00468
.044	2.884	2.88	1.00514
.046	2.763	2.76	1.00561
.048	2.652	2.65	1.00611
.05	2.549	2.55	1.00663
.055	2.327	2.33	1.00801
.06	2.142	2.14	1.00952
.065	1.987	1.99	1.01115
.07	1.854	1.86	1.01291
.075	1.740	1.74	1.01480
.08	1.640	1.64	1.01681
.085	1.553	1.56	1.01894
.09	1.476	1.49	1.02119
.095	1.408	1.42	1.02356
.10	1.346	1.36	1.02604
.105	1.291	1.31	1.02865
.11	1.242	1.26	1.03136
.115	1.197	1.22	1.03419
.12	1.155	1.18	1.03713
.125	1.118	1.14	1.04021
.13	1.084	1.11	1.04333
.135	1.052	1.08	1.04659
.14	1.023	1.05	1.04995
.145	0.997	1.03	1.05341
.15	0.972	1.005	1.05711
.16	0.928	0.964	1.06455
.17	0.890	0.930	1.07236
.18	0.856	0.900	1.08063
.19	0.826	0.874	1.08919
.20	0.800	0.853	1.09822
.21	0.777	0.834	1.10749
.22	0.757	0.820	1.11706
.23	0.738	0.807	1.12701
.24	0.722	0.796	1.13724
.25	0.707	0.788	1.14778

INCLINED SPAN—UNIFORMLY LOADED—ANCHORED

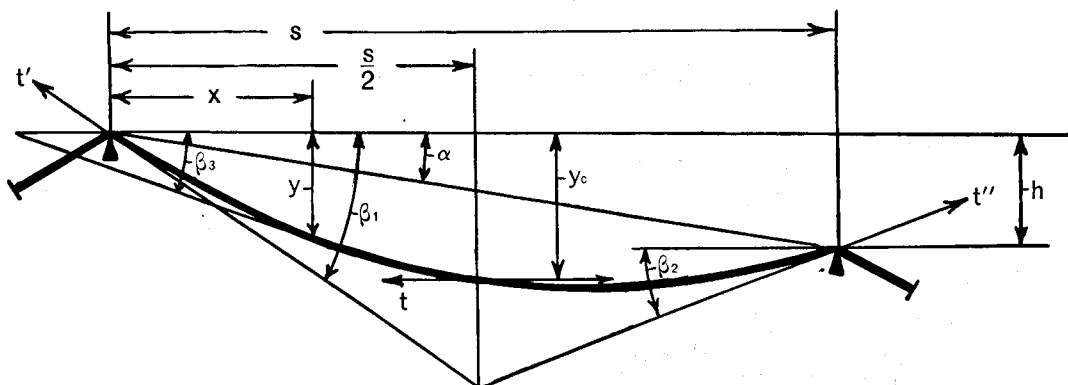


Figure 3

The following formulas give the increments of deflection and slope due to inclination of the chord.

"Down" slopes are usually considered as plus values and "up" slopes as minus values.

$$y_c = \frac{ws^2}{8t} + \frac{h}{2} \quad (15)$$

$$\tan \alpha = \frac{h}{s} \quad (16)$$

At any point—

$$y = \frac{wx(s-x)}{2t} \pm x \tan \alpha \quad (17)$$

$$\tan \beta_1 = -\frac{ws}{2t} + \tan \alpha \quad (18)$$

$$\tan \beta_2 = -\frac{ws}{2t} - \tan \alpha \quad (19)$$

$$\tan \beta_3 \text{ (at any point)} = \frac{w}{t} \left(\frac{s}{2} - x \right) \pm \tan \alpha \quad (20)$$

When center deflection is known:

$$t = \frac{ws^2}{8y_c - 4h} \quad (21)$$

Low point of an inclined span occurs when $\tan \beta_3 = 0$

$$\therefore x = \frac{s}{2} + \frac{t}{w} \tan \alpha$$

When deflection at any other point is known:

$$t = \frac{wx(s-x)}{2(y-x \tan \alpha)} \quad (22)$$

$$t' = t \sec \beta_1 \quad (23)$$

$$t'' = t \sec \beta_2$$

To find the lengths of cable in an inclined span formulas (9) and (11) are modified:

$$L_1 \text{ or } L = \sqrt{s^2 + h^2} + \frac{w^2 s^3 \cos^3 \alpha}{24t^2} \text{ (approx.)} \quad (24)$$

$$L_1 \text{ or } L = \sqrt{s^2 + h^2} \left(1 + \frac{8}{3}k^2 - \frac{32}{5}k^4 + \frac{256}{7}k^6 \right) \quad (25)$$

It will be seen that the solutions for inclined spans are quite similar to those for level spans.

LEVEL SPAN—SINGLE LOAD AT CENTER—ANCHORED

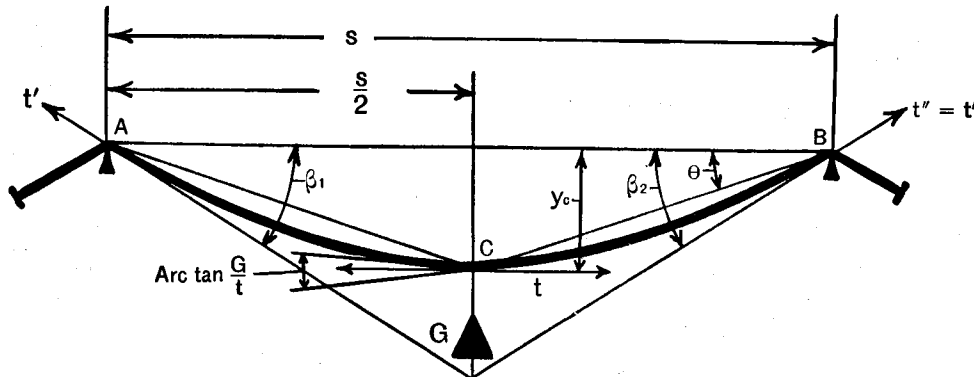


Figure 4

The deflection produced by a concentrated load suspended midway between two fixed points A and B forms two equal sub-chords AC and CB. The cable assumes two catenary arcs which intersect at C. The following formulas are, however, based on the parabola, as the difference in results is negligible.

The center deflection is found from:

$$y_c = \frac{Gs}{4t} + \frac{ws^2}{8t} = \frac{s(2G + ws)}{8t} \quad (26)$$

$$\text{and } t = \frac{s(2G + ws)}{8y_c} \quad (27)$$

$$t' = t \sec \beta_1 = t \sec \beta_2 = t'' \quad (28)$$

$$\tan \beta_1 = \frac{G + ws}{2t} = \tan \beta_2 \quad (29)$$

Example: A rolling load weighing 2000 pounds is to be supported in a level span 2000 ft. long by

a cable anchored at both ends. The deflection must not exceed 83 feet. No wind or ice conditions.

- What are the specifications of the cable?
- What is the maximum tension in the cable?
- What is the slope at the supports with the load at center of span?
- What is the cable length between supports, with no rolling load on the cable?
- What is the erection tension and erection deflection of the cable?

It is necessary to assume a size and grade of cable for the calculations. If the first selection does not prove suitable, the calculations must be revised. We shall assume that a $1\frac{1}{8}$ " diameter Standard Locked Coil Cable will be suitable.

Since this is a level span, $\alpha = 0$ and $w = w'$

$w = 3.16$ pounds per foot

$A = .8503$ square inches

$$\text{From (27)} \quad t = \frac{2000(2 \times 2000 + 3.16 \times 2000)}{8 \times 83} = 31,084 \text{ pounds}$$

$$\text{From (29)} \quad \tan \beta_1 = \frac{2000 + (3.16 \times 2000)}{2 \times 31084} = .1338 \quad \beta_1 = 7^\circ-37'$$

$$\text{From (28)} \quad t' = 31084 \times 1.0089 = 31360 \text{ pounds}$$

The maximum cable length occurs when load is at center of span.

$$s_1 = \sqrt{\left(\frac{s}{2}\right)^2 + y^2} = \sqrt{1000^2 + 83^2} = 1003.439 \text{ ft.}$$

$$\tan \theta = \frac{83}{1000} = .083 \quad \theta = 4^\circ-45'$$

$$L = 2 \left(s_1 + \frac{w^2 \left(\frac{s}{2}\right)^3 \cos^3 \theta}{24 t^2} \right) = 2 \left(1003.439 + \frac{3.16^2 \times 1000^3 \times \cos^3 4^\circ-45'}{24 \times 31084^2} \right) = 2007.730$$

In order to set up (14) in convenient form, first calculate the following:

$$P = \frac{2007.730}{.8503 \times 19,000,000} = .0001243$$

$$Pt + s = .0001243 \times 31,084 + 2000 = 2003.864$$

$$\frac{w^2 s^3}{24} = \frac{3.16^2 \times 2000^3}{24} = 3,328,533,333$$

Substituting these values in (14)

$$.0001243 t_e + 2007.730 - 2003.864 = \frac{3,328,533,333}{t_e^2}$$

The following shows the results of a series of slide rule computations for assumed values of t_e until the above equation is satisfied (the values in the last two columns are equal).

t_e	$.0001243 t_e$	$+ 3.866$	$\frac{3,328,533,333}{t_e^2}$
22,000	2.735	6.601	6.877
22,300	2.772	6.638	6.693
22,380	2.782	6.648	6.646

$t_e = 22,380$ pounds

$$\text{from (1) } y_c = \frac{3.16 \times 2000^2}{8 \times 22380} = 70.60 \text{ ft.}$$

$$\text{from (9) } L_1 = 2000 + \frac{3.16^2 \times 2000^3}{24 \times 22380^2} = 2006.65 \text{ ft.}$$

(a) With a factor of safety of 3.2, the required breaking strength will be $3.2 \times 31360 = 100352$

pounds = 50.18 tons. The breaking strength of a $1\frac{1}{8}$ " diameter Standard Locked Coil Cable is 54 tons. Therefore, this size cable is satisfactory, and our Locked Coil Cable is the most suitable construction where rolling loads are to be handled. If the proposed installation is temporary, or if first cost of the cable is a prime consideration, we may consider the use of $1\frac{1}{8}$ " diameter High Strength, or 1" diameter Extra High Strength Smooth Coil Track Strands, as well as $1\frac{1}{4}$ " diameter 6x19 Plow Steel Rope. However, it would be necessary to revise the calculations for either of the latter selections, because the weight, area, and modulus of elasticity change.

(b) $t' = 31360$ pounds

(c) $\beta_1 = 7^\circ-37'$

(d) $L_1 = 2006.65$ ft.

(e) $t_e = 22380$ pounds

$y_c = 70.60$ ft.

INCLINED SPAN—SINGLE LOAD AT CENTER—ANCHORED

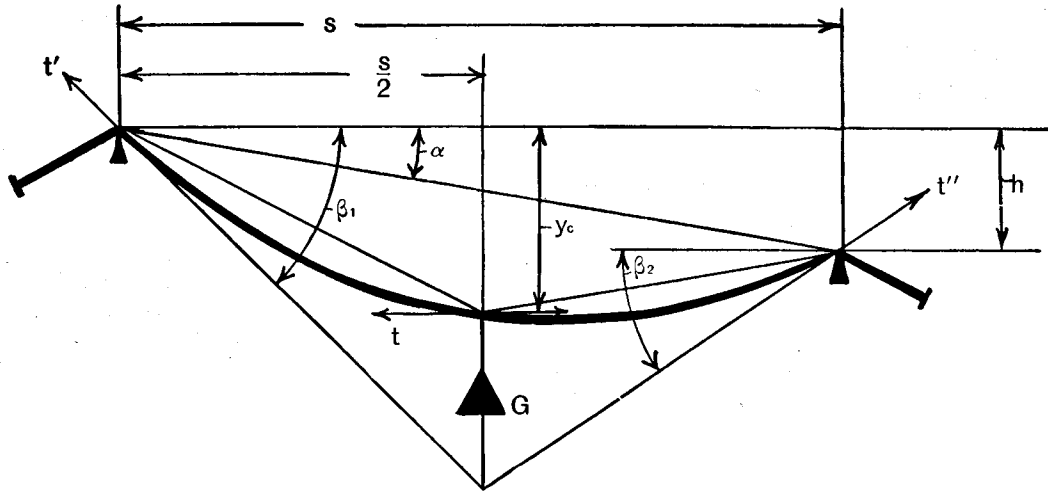


Figure 5

If the chord is inclined, similar to Fig. 5, then the center deflection is found by adding $\frac{h}{2}$ to (26). Then:

$$y_c = \frac{Gs}{4t} + \frac{ws^2}{8t} + \frac{h}{2} = \frac{s(2G + ws)}{8t} + \frac{h}{2} \quad (30)$$

$$t = \frac{s(2G + ws)}{8y_c - 4h} \quad (31)$$

$$t' = t \sec \beta_1$$

See (28)

$$t'' = t \sec \beta_2$$

See (28)

$$\tan \beta_1 = \frac{G + ws}{2t} + \frac{h}{s}$$

See (29)

$$\tan \beta_2 = \frac{G + ws}{2t} - \frac{h}{s}$$

See (29)

LEVEL SPAN WITH SINGLE LOAD AT ANY POINT

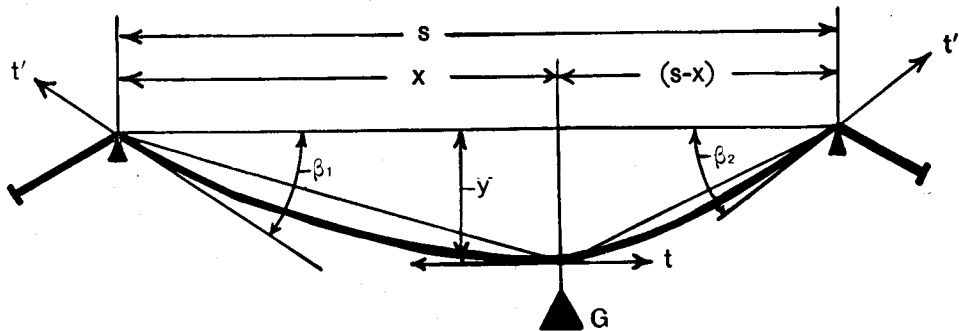


Figure 6

When the cable hangs between fixed points of suspension, with supports at the same elevation, the tension t varies with different positions of the load G and is a maximum only when G is at the center of the span.

Knowing the tension t at the center of the span from (27), the deflection at other points may be determined from:

$$y = \frac{x (ws + 2G)^2 (s-x)}{2t (ws^2 + 4G \sqrt{x(s-x)})} \quad (32)$$

However, it must be understood this formula will only give approximate results, as it is based on constant cable length, neglecting the elastic properties of the cable. As the load moves away from center of span, the tension decreases and

cable decreases in length. Therefore, the results obtained from (32) are somewhat greater than actual deflections. Formula (32) is, however, sufficiently accurate for many problems encountered in practice.

After determining the deflection by (32) for any position of the load, the corresponding approximate tension at xy can be found from:

$$t = \frac{x(s-x) (ws + 2G)}{2sy} \quad (33)$$

To determine the deflection of the cable at any point, when the load is at xy , consider x or $(s-x)$, Fig. 6, as separate inclined spans, with y as the difference in elevation. Then formula (17) can be applied.

INCLINED SPAN—SINGLE LOAD AT ANY POINT—ANCHORED

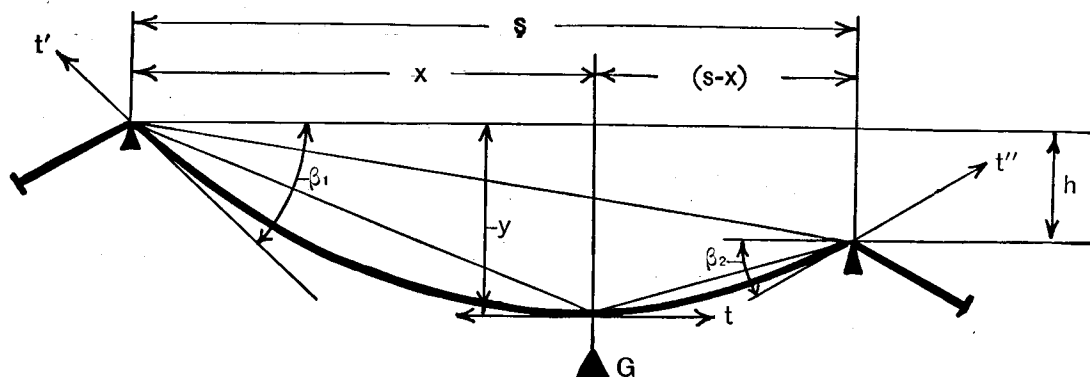


Figure 7

Formula (32) can be applied to inclined spans by adding $\frac{hx}{s}$, which becomes $\frac{h}{2}$ when $x = \frac{s}{2}$. Then, for inclined spans:

$$y = \frac{x (ws + 2G)^2 (s-x)}{2t(ws^2 + 4G \sqrt{x(s-x)})} + \frac{hx}{s} \quad (34)$$

Multiple Loads in Anchored Spans

Multiple loads in anchored spans are seldom encountered in practice. However, the subject is important enough to merit some attention. When speaking of multiple loads, it will be assumed loads are equal in amount and spaced uniformly.

The loads should be placed symmetrically about the center line of the span to compute the maximum tension or deflection in the span. Use formula (52), page 54, to determine the deflection and

formula (54) to determine the maximum tension. To determine the length along the cable at maximum tension, consider the loads as stationary in the position stated above and treat the lengths of cable between supports and the first load, and the lengths between loads, as separate spans. After this length, L , has been determined, the erection tension, deflection of empty cable, etc., are calculated by the trial method in a similar manner to that for a single load in an anchored span.

LEVEL SPAN—UNIFORMLY LOADED—COUNTERWEIGHTED

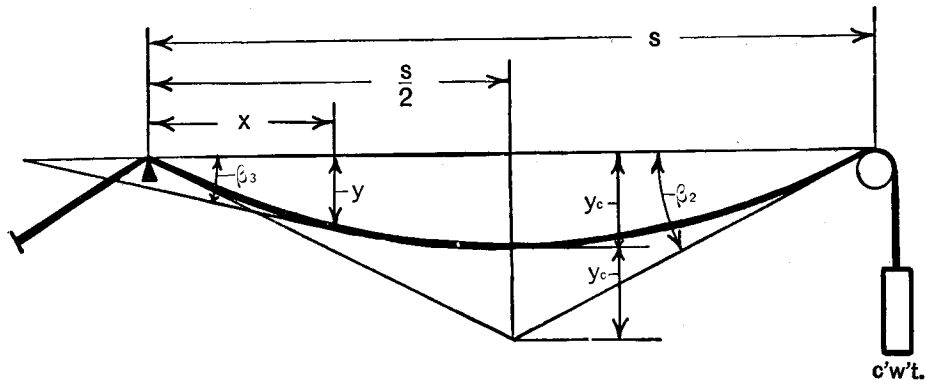


Figure 8

The tension and deflection of either an anchored or a counterweighted span are the same, under the same conditions of loading, when the cable supports a uniformly distributed load. However, an important difference occurs when the live load is removed. In the case of an anchored span, the deflection and length of the cable remain constant, except as they are affected by the elastic properties of the cable, backstays, and supports. The tension, however, decreases when the live load is removed. Comparing this performance with a counterweighted span, we find that the tension remains constant when the live load is removed, while the deflection and length of the cable decrease in proportion to the change in loading. These are the effects due to equalizing the moment-

sum of all forces for any origin of moments.

The same comparison holds true of spans supporting one or more individual concentrated loads, when the loads are so placed as to produce the maximum deflection.

The use of a counterweighted track cable for rolling loads results in a constant angle under the load, the angle whose tangent is $\frac{G}{t}$, at all points of a span. Also, it produces a smaller angle at each support than would be the case with an anchored span. These two factors are of definite advantage in the design of aerial tramways having intermediate supports.

Apply formulas (1) to (12) inclusive, pages 39 and 40 under "Anchored Spans."

INCLINED SPAN—UNIFORMLY LOADED—COUNTERWEIGHTED

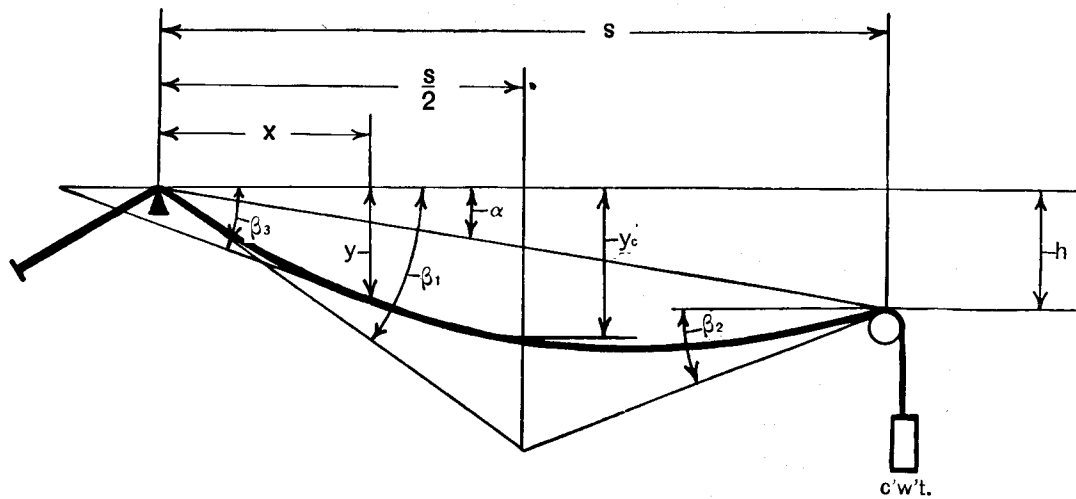


Figure 9

Apply formulas (15) to (25) inclusive, page 43 under "Anchored Spans."

LEVEL SPAN—SINGLE LOAD AT CENTER—COUNTERWEIGHTED

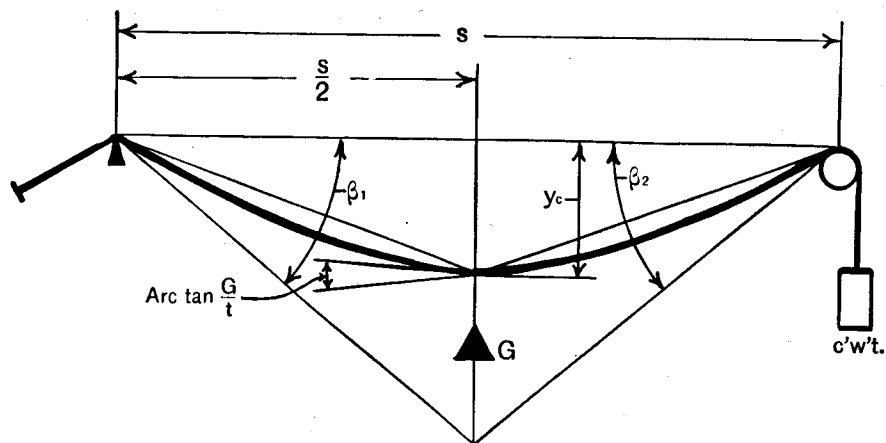


Figure 10

Apply formulas (26) to (29) inclusive, page 44, under "Anchored Spans."

INCLINED SPANS—SINGLE LOAD AT CENTER—COUNTERWEIGHTED

Apply formulas (30) and (31) inclusive, page 46, under "Anchored Spans."

LEVEL SPAN—SINGLE LOAD AT ANY POINT—COUNTERWEIGHTED

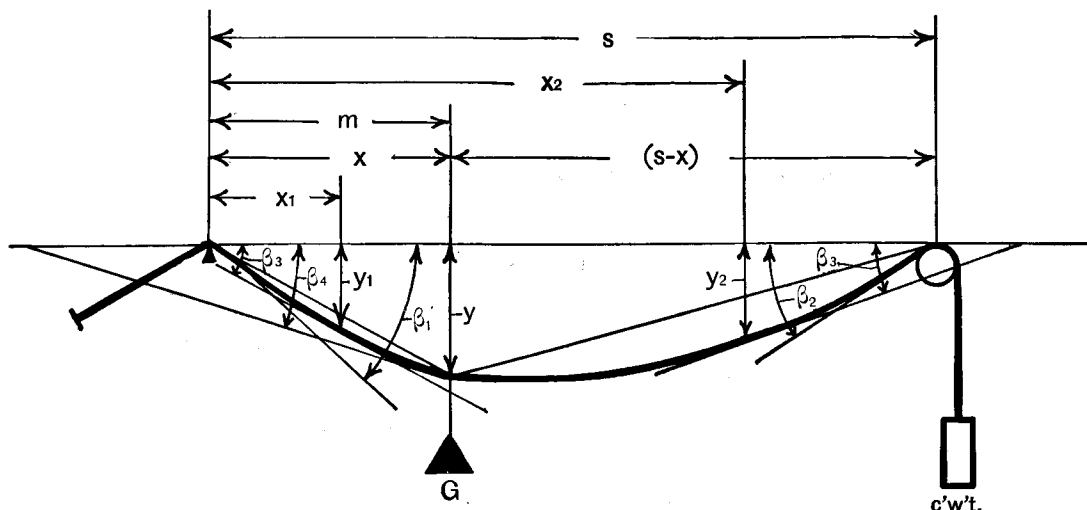


Figure 11

In a constant tension span the deflection *at the load* may be determined from:

$$y = \frac{Gx(s-x)}{st} + \frac{wx(s-x)}{2t} \quad (35)$$

Also the deflection of the cable may be determined for any point in the span, with the load at any point, $x_1 y_1$ being coordinates to points to the left of G and $x_2 y_2$ being coordinates of points to the right of G.

$$y_1 \text{ (points left of G)} = \frac{Gx_1}{st} (s-m) + \frac{wx_1}{2t} (s-x_1) \quad (36)$$

$$y_2 \text{ (points right of G)} = \frac{Gm}{st} (s-x_2) + \frac{wx_2}{2t} (s-x_2) \quad (37)$$

The cable slope at left support, when $x_1 = 0$, is:

$$\tan \beta_1 = \frac{G}{st} (s-m) + \frac{ws}{2t} \quad (38)$$

The cable slope at right support, when $x_2 = s$, is:

$$\tan \beta_2 = \frac{Gm}{st} + \frac{ws}{2t} \quad (39)$$

The cable slope at any point between the load and either support is:

$$\tan \beta_3 \text{ (points to left of G)} = \frac{G}{st} (s-m) + \frac{w}{t} \left(\frac{s}{2} - x_1 \right) \quad (40)$$

$$\tan \beta_3 \text{ (points to right of G)} = \frac{Gm}{st} + \frac{w}{t} \left(x_2 - \frac{s}{2} \right) \quad (41)$$

When $x = m$, the slope *at* and to the left of the load is:

$$\tan \beta_4 \text{ (sloping to left of G)} = \frac{G}{t} - \frac{Gx}{st} + \frac{w}{t} \left(\frac{s}{2} - x \right) \quad (42)$$

The slope *at* and to the right of the load is:

$$\tan \beta_4 \text{ (Sloping to the right of G)} = \frac{Gx}{st} + \frac{w}{t} \left(x - \frac{s}{2} \right) \quad (43)$$

The tangent of the angle under the load is equal to (42) + (43) = $\frac{G}{t}$

If we take half the difference between the angles *ned* from (42) and (43), the tangent of the *resulting* angle will be the slope which a moving

load must climb. The maximum slope thus obtained will determine the maximum pull on a haulage rope.

INCLINED SPAN—SINGLE LOAD AT ANY POINT—COUNTERWEIGHTED

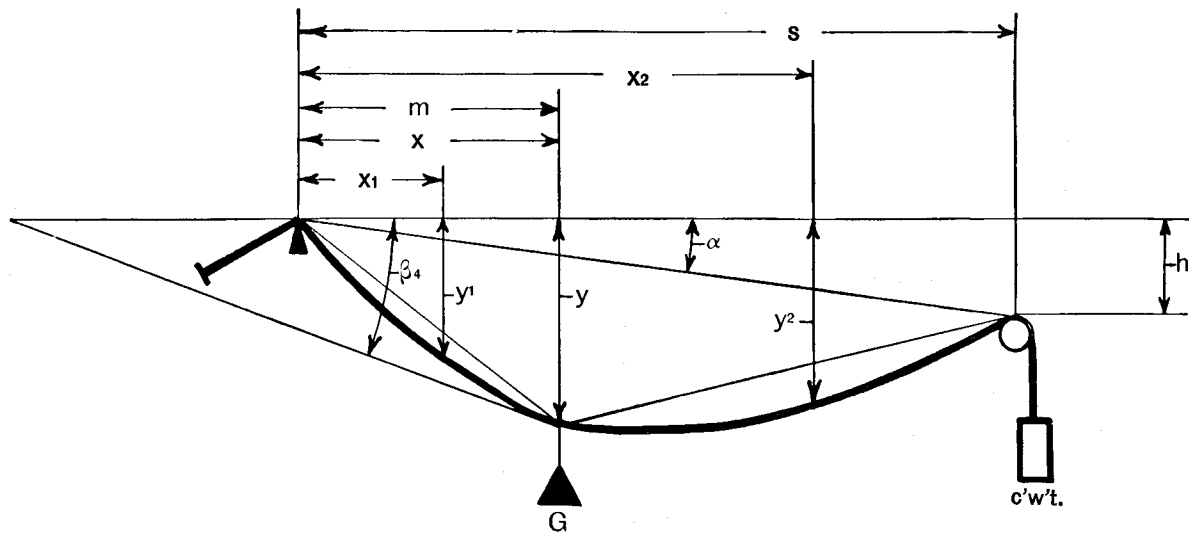


Figure 12

In these formulas, as in all others, we have placed the higher support at the left-hand end of span, and have made this point the origin of moments.

$$\text{For } y \text{ — at the load — add to formula (35)} \\ x \tan \alpha \quad (44)$$

$$\text{For } y_1 \text{ — points left of } G \text{ — add to formula (36)} \\ x_1 \tan \alpha \quad (45)$$

$$\text{For } y_2 \text{ — points right of } G \text{ — add to formula (37)} \\ x_2 \tan \alpha \quad (46)$$

The cable slopes are determined by taking the chord into account as an additional term in the above equations.

$$\tan \beta_1 \text{ — at left support — formula (38)} \\ + \tan \alpha \quad (47)$$

$$\tan \beta_2 \text{ — at right support — formula (39)} \\ - \tan \alpha \quad (48)$$

$$\tan \beta_3 \text{ — points to left of } G \text{ — formula (40)} \\ \pm \tan \alpha \quad (49)$$

$$\tan \beta_3 \text{ — points to right of } G \text{ — formula (41)} \\ \pm \tan \alpha \quad (50)$$

$$\tan \beta^4 \text{ — at the load — formula (42) and (43)} \\ \pm \tan \alpha \quad (51)$$

EXAMPLE: A 2,000 pound rolling load is to be supported on an inclined span 800 ft. long with difference in elevation of 67 ft. The cable is $1\frac{3}{8}$ " diameter Standard Locked Coil; $w' = 4.73$ pounds per foot, $A = 1.2437$ sq. in. The center deflection must not exceed 18 ft. from the chord.

(a) What is the horizontal component of cable

tension with load at center of span?

(b) What is the slope of the cable at the higher support (1) with the load at center of span, (2) with the load 100 ft. horizontally away from the upper support and (3) with the cable empty?

(c) What is the center deflection of the empty cable?

$$\tan \alpha = \frac{67}{800} = .08375 \quad \alpha = 4^\circ - 47', \sec \alpha = 1.0035$$

$$w = 4.73 \times 1.0035 = 4.75$$

$$\text{From (31), } t = \frac{800 (2 \times 2000 + 4.75 \times 800)}{8 \times 51.5 - 4 \times 67} = 43,333 \text{ pounds}$$

$$\begin{aligned} \text{From (47), } \tan \beta_1 &= \frac{2000 \times 400}{800 \times 43333} + \frac{4.75 \times 800}{2 \times 43333} + \frac{67}{800} \\ &= .1507, \quad \beta_1 = 8^\circ - 34' \end{aligned}$$

$$\begin{aligned} \text{From (47), } \tan \beta_1 &= \frac{2000 \times 700}{800 \times 43333} + \frac{4.75 \times 800}{2 \times 43333} + \frac{67}{800} \\ &= .1680, \quad \beta_1 = 9^\circ - 32' \end{aligned}$$

$$\begin{aligned} \text{From (18), } \tan \beta_1 &= \frac{4.75 \times 800}{2 \times 43333} + \frac{67}{800} \\ &= .1276, \quad \beta_1 = 7^\circ - 16' \end{aligned}$$

$$\text{From (15), } y_c = \frac{4.75 \times 800^2}{8 \times 43333} + \frac{67}{2} = 42.27 \text{ ft.}$$

(a) 43,333 pounds.

(b1) Slope $8^\circ - 34'$ with load at center of span

(b2) Slope $9^\circ - 32'$ with $m = 100$ feet

(b3) Slope $7^\circ - 16'$ with cable empty

(c) Center deflection 42.27 feet with cable empty

LEVEL SPAN—MULTIPLE LOADS—COUNTERWEIGHTED

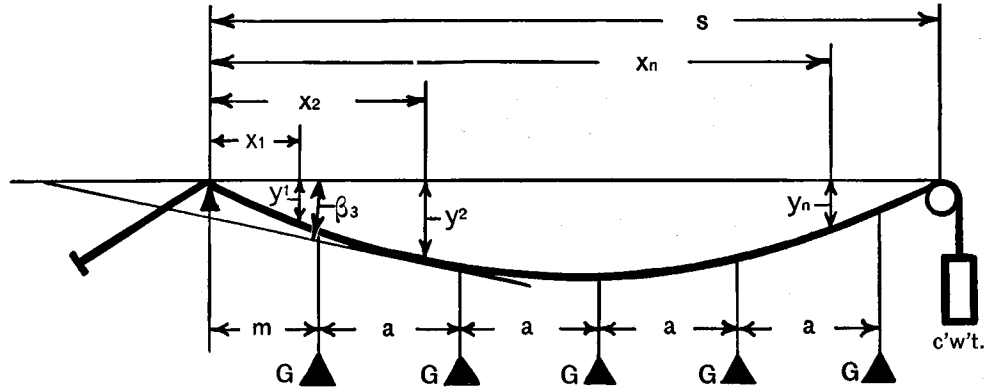


Figure 13

A cable supporting multiple loads forms a series of parabolic arcs between the loads. For many cases encountered in practice, it will be sufficiently accurate to calculate spans carrying more than five loads as uniformly loaded spans. If this is done, the load per foot equals weight of cable

plus $\frac{G}{a}$.

However, the general formula for deflection y , at any point xy , of a span supporting n loads of uniform spacing and equal weight, the cable tension being constant, is:

$$y = \frac{G}{t} \left[x(n-u) - m \left(\frac{xn}{s} - u \right) - a \left(\frac{bx}{s} - c \right) \right] + \frac{wx(s-x)}{2t} \quad (52)$$

$$\text{If } z = \left[x(n-u) - m \left(\frac{xn}{s} - u \right) - a \left(\frac{bx}{s} - c \right) \right]$$

$$\text{Then } y = \frac{2Gz + wx(s-x)}{2t} \quad (53)$$

$$\text{and } t = \frac{2Gz + wx(s-x)}{2y} \quad (54)$$

The cable slope at any point may be found from the general formula:

$$\tan \beta_3 = \frac{G}{t} \left[(n-u) - \frac{nm + ab}{s} \right] + \frac{w}{t} \left(\frac{s}{2} - x \right) \quad (55)$$

Example: A 1½" diameter Standard Locked Coil Cable is to be used to support 5 loads, each weighing 2000 pounds, and spaced 400 feet.

Length of span 2000 feet. Horizontal component of working tension $t = 45,964$ pounds.
 $w = w' = 5.63$ pounds.

- (a) What is maximum deflection?
- (b) What is the slope of the cable at a point 500 feet from the support?

Maximum deflection occurs with one load at center of span.

$$x = 1000 \text{ ft.}$$

$$m = 200 \text{ ft.}$$

$$u = 2$$

$$b = \frac{5 \times 4}{2} = 10$$

$$c = \frac{2 \times 1}{2} = 1$$

$$\begin{aligned} \text{From (52) } y_c &= \frac{2000}{45964} \left[1000 (5-2) - 200 \left(\frac{1000 \times 5}{2000} - 2 \right) - 400 \left(\frac{10 \times 1000}{2000} - 1 \right) \right] \\ &\quad + \frac{5.63 \times 1000 \times 1000}{2 \times 45964} \\ &= 56.566 + 61.244 = 117.81 \text{ feet} \end{aligned}$$

From (55) with $x = 500$ feet, $u = 1$

$$\begin{aligned} \tan \beta_3 &= \frac{2000}{45964} \left[(5-1) - \frac{5 \times 200 + 400 \times 10}{2000} \right] + \frac{5.63}{45964} (1000-500) \\ &= .0653 + .0612 = .1265, \quad \beta_3 = 7^\circ-13' \end{aligned}$$

$$(a) = 117.81 \text{ feet}$$

$$(b) = 7^\circ-13'$$

INCLINED SPAN—MULTIPLE LOADS—COUNTERWEIGHTED

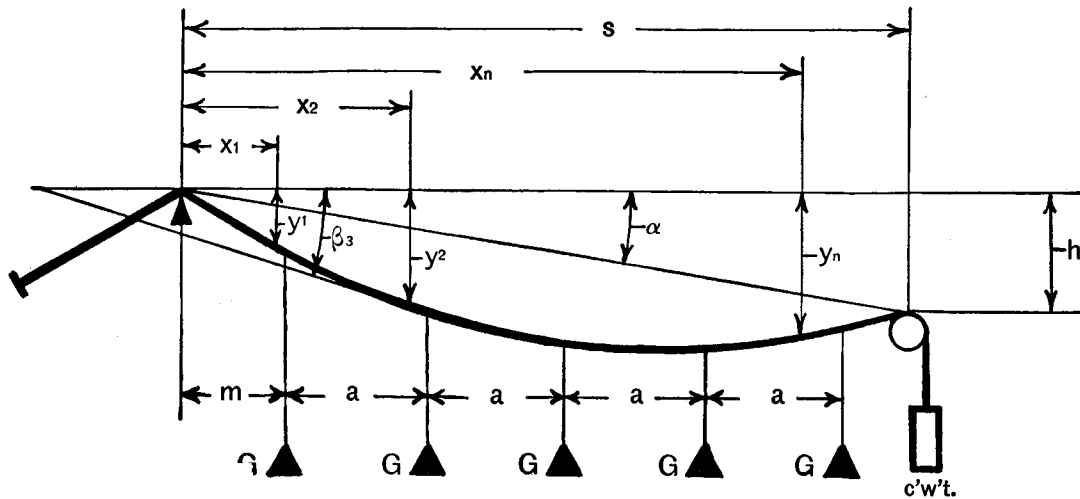


Figure 14

For y add to formula (52) and (53) $x \tan \alpha$ (56)

Formula (54) becomes $t = \frac{2Gz + wx(s-x)}{2(y-x \tan \alpha)}$ (57)

$\tan \beta_3$ is found by completing formula (55) with $\pm \tan \alpha$ (58)

WIND AND ICE LOADS

The change in length of cables due to change in temperature has not been taken into account in the examples given above. In counterweighted spans such a change in length results in a small movement of the counterweight, the tension and deflection remaining constant. However, in anchored spans the change in length due to temperature changes results in changes in cable tension, and frequently the effect of such changes must be carefully considered.

To find the change in length, multiply the length of the cable by the number of degrees (F.) variation in temperature and the product by the coefficient .0000689 for steel rope wire.

Wind loads on cylindrical surfaces, such as wire cables, are determined from maximum wind velocities. If P equals wind pressure in pounds per square foot of projected area and V = actual

wind velocity in miles per hour, then $P = 0.0025 V^2$. This gives 4.0 pounds per square foot for 40 miles per hour, 12.2 pounds per square foot for 70 miles per hour, and 20.2 pounds per square foot for 90 miles per hour.

Where exceptionally severe sleet conditions occur, the cables are assumed to be covered with a coating of ice $\frac{3}{4}$ inch thick, or a total of $1\frac{1}{2}$ inches of ice plus the diameter of cable. Where the sleet conditions are less severe, the ice coating is assumed to be $\frac{1}{2}$ inch thick, or a total of 1 inch plus the diameter of cable. Then wind load is based on the total diameter of ice plus cable, and the resultant cable load is determined from the horizontal wind load and vertical load of cable and ice. The weight of ice is approximately 56 pounds per cubic foot, or .0324 pounds per cubic inch.

MULTIPLE SHEAVE BLOCKS

When a load is supported by multiple part wire rope tackle, as shown on page 59, the load upon the rope is equal to the load supported, divided by the number of parts of rope between the blocks.

When a load is raised by this method, the load upon the rope increases progressively from the dead end due to the friction of the individual sheaves and the force required to bend the rope about the sheaves. The amount of this increased load depends upon the size and construction of rope, size of sheaves, size of sheave pins, and coefficient of friction at the sheave pins.

The formula for determining the load upon the rope for various loads raised by multiple part tackle is:

$$P = W \frac{K^n (K - 1)}{K^n - 1} \quad (1)$$

$$W = \frac{P (K^n - 1)}{K^n (K - 1)} \quad (2)$$

Where P = Load on Rope

W = Load Lifted

K = Ratio of stress in rope unwinding from sheave to stress in rope winding on sheave. (These stresses differ by the stress due to resistance at sheave.)

n = Number of parts of rope between blocks.

If the rope bends 180 degree at the sheaves, as is the usual case:

$$K = 1 + C \frac{d^2}{16} + 2 f \frac{r}{R} \quad (3)$$

Where C = Constant depending on the wire rope

d = Rope diameter

R = Radius of center line of rope at sheave

f = Coefficient of friction at sheave pin

r = Radius of sheave pin

For 6 x 19 Plow Steel Hoisting Rope, C = 0.3.

For metaline bushed sheaves on steel pins, f = 0.08. Inserting these values in (3):

$$K = 1 + 0.3 \frac{d^2}{R} + 0.16 \frac{r}{R} \quad (4)$$

The following tables will facilitate the finding of K:

TABLE V
Values of $0.3 \frac{d^2}{R}$

Diameter of Rope	DIAMETER OF SHEAVES AT BOTTOM OF GROOVE								
	6½"	9"	10"	13"	14"	16"	18"	20"	24"
5/8"	.033	.024	.022	.017	.016	.014	.013	.011	.010
¾"	.047	.035	.031	.025	.023	.020	.018	.016	.014
7/8"	.062	.047	.042	.033	.031	.027	.024	.022	.018
1"	.080	.060	.055	.043	.040	.035	.032	.029	.024

AMERICAN TIGER BRAND

TABLE VI
Values of $0.16 \frac{r}{R}$

Diam. of Sheave Pin	DIAMETER OF SHEAVES AT BOTTOM OF GROOVE								
	6½"	9"	10"	13"	14"	16"	18"	20"	24"
1"	.022	.016	.015	.012	.011	.010	.009	.008	.006
1⅛"	.025	.018	.017	.013	.012	.011	.010	.009	.007
1¼"	.028	.020	.019	.015	.014	.012	.011	.010	.008
1⅜"	.030	.023	.020	.016	.015	.013	.012	.011	.009
1½"	.033	.025	.022	.017	.016	.014	.013	.012	.010
1¾"	.039	.029	.026	.020	.019	.017	.015	.013	.011
2"	.044	.033	.030	.023	.022	.019	.017	.015	.013
2¼"	.050	.037	.034	.026	.024	.022	.019	.017	.015
2½"	.055	.041	.037	.029	.027	.024	.021	.019	.016
2¾"	.061	.045	.041	.032	.030	.026	.023	.021	.018
3"	.066	.049	.045	.035	.033	.029	.026	.023	.019
3½"	.077	.057	.052	.041	.038	.033	.030	.027	.023
4"	.088	.066	.060	.047	.043	.038	.034	.031	.026

Example—What safe load can be lifted by a 9-part tackle reeved with ¾" diameter 6x19 Plow Steel Rope, if the block sheaves are 14" groove diameter, metaline bushed, with 2" dia. pins.

Breaking strength of ¾" dia. 6x19 plow steel rope is 20.7 tons = 41,400 pounds.

With factor of safety of 5 the maximum working load is 8,280 pounds.

From Tables V and VI, $K = 1 + .023 + .022 = 1.045$. Substituting K in (2)

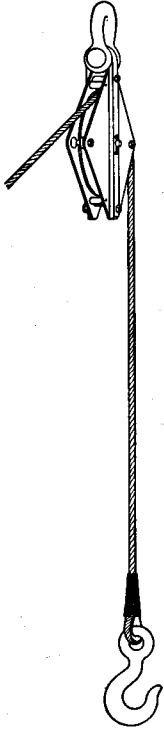
$$W = \frac{8280 (\overline{1.045^9} - 1)}{\overline{1.045^9} (1.045 - 1)}$$

$$= 60,200 \text{ pounds or practically 30 tons.}$$

When the lead rope passes around snatch blocks the load on the rope is increased approximately 8% at each snatch block, where the rope bends 180 degrees, and approximately 4% at each snatch block, where it bends 90 degrees.

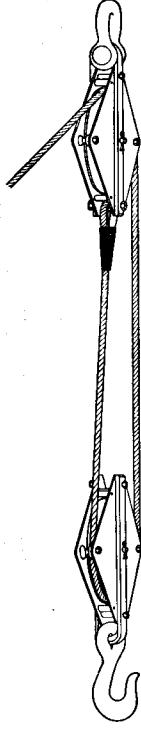
WIRE ROPE ENGINEERING HANDBOOK

One-part Line



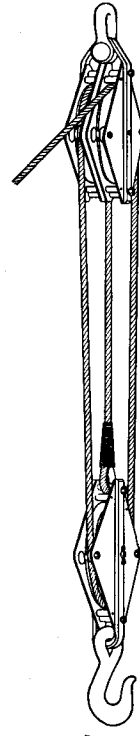
Load on Rope
same as Sup-
ported Load

Two-part Line



Load on Rope
is one-half
Supported
Load

Three-part Line



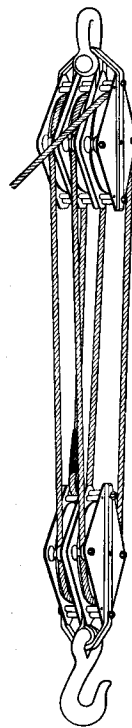
Load on Rope
is one-third
Supported
Load

Four-part Line



Load on Rope
is one-fourth
Supported
Load

Five-part Line



Load on Rope
is one-fifth
Supported
Load

STRESSES IN WIRE ROPES ON INCLINED PLANES AND SLOPES

There are two methods of designating the grade or pitch of an inclined plane or slope. The first is by using the angle which the line of the incline or slope makes with the horizontal. The second is by using a percentage figure which is the ratio of the vertical rise to the horizontal distance, and which is equivalent to 100 times the tangent of the angle of incline.

Neglecting friction, the pull on the rope due to loaded cars on an incline or slope is equivalent to the total weight of the loaded cars multiplied by the sine of the angle of incline. In cases where this angle is not the same throughout, the value to be used must be the angle of that part on which the cars are resting. It must be remembered that the weight of the rope is to be added to the weight of the loaded cars, as this has a bearing on the final result, especially in the case of long inclines or slopes.

In most cases, friction must be considered in determining the pull on the rope. The frictional load is dependent on the car friction, which remains constant for a given angle of incline, and the rope friction, which varies depending on how much rope is resting on the incline or slope. Particular attention must be paid to the fact that when a rope is used to haul loaded cars up a grade, the frictional load is added to the pull caused by the weight of the material moved, while if the rope is used to lower loaded cars downhill, the friction load acts opposite to the main load, and is subtracted from it.

Car friction is greatest on a level stretch of track. On an incline, it depends on the cosine of the angle of incline and will usually be found to be equivalent to the weight of the loaded car multiplied by .03 times the cosine of the angle of incline.

Rope friction is an exceedingly variable factor. It depends on the spacing and condition of track rollers or rubbing boards, the contour of the incline or slope, and whether or not the tension on the rope is sufficient to raise it off its supports. In order to be sure of a large enough allowance, a

coefficient should be used having at least twice the value of that used for car friction; therefore, the rope friction would be equivalent to the weight of the rope between car and drum multiplied by .06 times the cosine of the angle of incline. In this case, for the sake of safety, use the minimum angle where there is more than one grade.

To find the total pull on a rope, we must therefore determine three values. First, the gravity load; second, the car friction load; and third, the rope friction load.

The gravity load is found by multiplying the total weight of loaded cars plus the weight of rope between car and drum by the load factor for the proper grade as found in column 3 of table on page 62.

The car friction load is found by multiplying the weight of the loaded car by the proper car friction factor given in column 4.

The rope friction load is found by multiplying the weight of rope between car and drum by the rope friction factor (see column 5) for the minimum grade between car and drum.

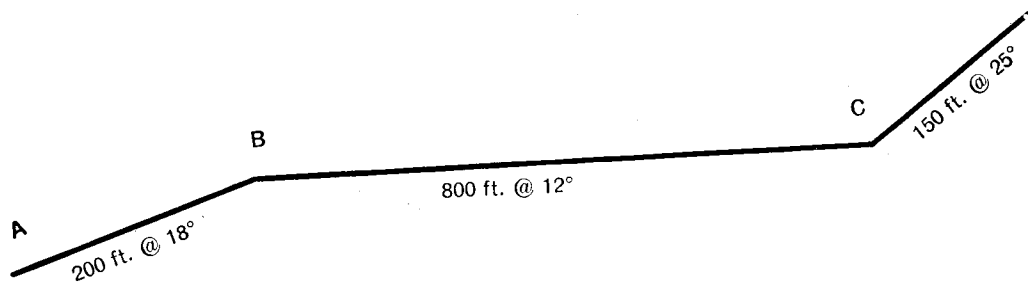
For ascending trips, the friction loads must be added to the gravity load, and for descending trips they must be subtracted. These values should be determined for the lowest point of each section when there is more than one grade.

To the sum of these three loads should be added a factor to cover the stress due to acceleration. For speeds below 500 ft. per minute, a 5% addition to the total stress in the wire rope is usually adequate. For speeds of 500 ft. to 1000 ft. per minute, the factor should be 10%. For higher speeds, the stress due to acceleration should be calculated. See Stresses due to Acceleration, page 34.

In selecting a proper rope, the total pull should be multiplied by a factor of safety of at least 6, and in cases where the abrasive conditions are particularly severe this factor should be increased in order to make allowance for the loss of strength that will result from wearing of the rope. See Safety Factors, page 6.

EXAMPLE: A trip of cars weighing 27,500 lbs. when loaded is being hauled up a slope with contour as shown in the sketch. The speed is 850

feet per minute. What is the maximum rope pull? What rope should be used?



It is necessary to assume a certain size rope at the outset in order to get the gravity and rope friction loads. If the results are close to the assumed size, the figures may stand; but if there is much of a variation, the process should be repeated, using the size rope found to be approxi-

mately correct in the first solution. $1\frac{1}{8}$ " 6x7 Rope weighing 1.90 lbs. per ft. will be assumed, together with the fact that sheaves and drums are sufficiently large for this construction. The values must be found for car load at points A, B, and C.

Point A

Gravity load	=	$(27,500 + (1150 \times 1.90)) \times .3090$	=	9173
Car friction load	=	$27,500 \times .0285$	=	784
Rope friction load	=	$1150 \times 1.90 \times .0587$	=	128
10% Acceleration Stress			=	10085
			=	1009
Total				11094

Point B

Gravity load	=	$(27,500 + (950 \times 1.90)) \times .2079$	=	6093
Car friction load	=	$27,500 \times .0293$	=	806
Rope friction load	=	$950 \times 1.90 \times .0587$	=	106
10% Acceleration Stress			=	7005
			=	701
Total				7706

Point C

Gravity load	=	$(27,500 + (150 \times 1.90)) \times .4226$	=	11742
Car friction load	=	$27,500 \times .0272$	=	748
Rope friction load	=	$150 \times 1.90 \times .0544$	=	16
10% Acceleration Stress			=	12506
			=	1251
Total				13757

It will be seen from the above that the load is greatest when the cars are at point C, and are traveling on the steepest grade. This will be found true in the great majority of cases, but to be absolutely certain of the correct figures all problems should be solved as above. Again attention is called to the fact that for descending loads the friction values must be subtracted from the load values,

and in this case the load at point C would be 12,076 lbs.

Assuming a factor of safety of 6, the rope selected should have a breaking strength of $6 \times 13,757 = 82,542$ lbs. A $1\frac{1}{8}$ " diameter 6x7 Plow Steel rope would be sufficient for this installation. If abrasive conditions were severe, a Monitor Steel rope of the same size and construction should be used.

AMERICAN TIGER BRAND

LOAD FACTORS

Angle of Incline Degrees	% Grade	Load Factor	Car Friction Factor	Rope Friction Factor	Angle of Incline Degrees	% Grade	Load Factor	Car Friction Factor	Rope Friction Factor
0	0.0	.0000	.0300	.0600	46	103.5	.7193	.0208	.0416
1	1.7	.0174	.0300	.0600	47	107.2	.7313	.0205	.0409
2	3.5	.0349	.0300	.0600	48	111.1	.7431	.0201	.0401
3	5.2	.0523	.0299	.0599	49	115.0	.7547	.0197	.0394
4	7.0	.0698	.0299	.0599	50	119.2	.7660	.0193	.0385
5	8.7	.0872	.0299	.0598	51	123.5	.7771	.0189	.0377
6	10.5	.1045	.0298	.0597	52	128.0	.7880	.0185	.0370
7	12.3	.1219	.0298	.0595	53	132.7	.7986	.0180	.0361
8	14.0	.1392	.0297	.0594	54	137.6	.8090	.0176	.0352
9	15.8	.1564	.0296	.0593	55	142.8	.8191	.0172	.0344
10	17.6	.1737	.0295	.0591	56	148.2	.8290	.0168	.0335
11	19.4	.1908	.0294	.0589	57	154.0	.8387	.0163	.0327
12	21.3	.2079	.0293	.0587	58	160.0	.8480	.0159	.0318
13	23.1	.2249	.0292	.0585	59	166.4	.8572	.0154	.0309
14	24.9	.2419	.0291	.0583	60	173.2	.8660	.0150	.0300
15	26.8	.2588	.0290	.0580	61	180.4	.8746	.0145	.0290
16	28.7	.2756	.0288	.0577	62	188.1	.8829	.0141	.0282
17	30.6	.2924	.0287	.0574	63	196.3	.8910	.0136	.0272
18	32.5	.3090	.0285	.0571	64	205.0	.8988	.0131	.0263
19	34.4	.3256	.0284	.0567	65	214.4	.9063	.0127	.0253
20	36.4	.3420	.0282	.0564	66	224.6	.9135	.0122	.0244
21	38.4	.3584	.0280	.0560	67	235.6	.9205	.0117	.0234
22	40.4	.3746	.0278	.0556	68	247.5	.9272	.0112	.0224
23	42.4	.3907	.0276	.0552	69	260.5	.9336	.0107	.0215
24	44.5	.4067	.0274	.0548	70	274.7	.9397	.0102	.0205
25	46.6	.4226	.0272	.0544	71	290.4	.9455	.0098	.0195
26	48.8	.4384	.0270	.0540	72	307.8	.9511	.0093	.0185
27	50.9	.4540	.0267	.0535	73	327.1	.9563	.0088	.0175
28	53.2	.4695	.0265	.0530	74	348.7	.9613	.0083	.0165
29	55.4	.4848	.0262	.0525	75	373.2	.9659	.0078	.0155
30	57.7	.5000	.0260	.0520	76	401.1	.9703	.0073	.0145
31	60.1	.5150	.0257	.0515	77	433.1	.9744	.0067	.0135
32	62.5	.5299	.0254	.0509	78	470.5	.9781	.0062	.0124
33	64.9	.5446	.0252	.0503	79	514.5	.9816	.0057	.0114
34	67.4	.5592	.0249	.0497	80	567.1	.9848	.0052	.0104
35	70.0	.5736	.0246	.0491	81	631.4	.9877	.0047	.0094
36	72.6	.5878	.0243	.0485	82	711.5	.9903	.0042	.0083
37	75.3	.6018	.0240	.0479	83	814.4	.9925	.0036	.0073
38	78.1	.6157	.0237	.0473	84	951.4	.9945	.0031	.0063
39	81.0	.6293	.0233	.0467	85	1143.0	.9962	.0026	.0052
40	83.9	.6428	.0230	.0460	86	1430.0	.9976	.0021	.0042
41	86.9	.6561	.0226	.0453	87	1908.1	.9986	.0015	.0031
42	90.0	.6691	.0223	.0446	88	2863.6	.9994	.0010	.0021
43	93.2	.6820	.0219	.0439	89	5728.9	.9998	.0005	.0010
44	96.6	.6947	.0216	.0431	90	∞	1.0000	.0000	.0000
45	100.0	.7071	.0212	.0424					

MAXIMUM LOAD ON A WIRE ROPE

It is sometimes impossible to estimate the load imposed on a rope by totaling the stresses due to weight of live and dead loads, weight of rope, friction, acceleration, and stresses due to bending. In these cases the maximum stress on the rope may be approximated by a more indirect method. This method is not intended for very close calculations, and should not be used where the rope is subjected to sudden jerks. It will serve as a guide, however, to show whether or not a rope is overloaded.

Practically every steam engine or electric motor has a known horse power rating. For short periods of time, these may usually be operated with a very high overload, such as 25% for steam and 50% for electricity. Knowing the horse power of the prime mover provides a basis for this calculation. When hard pulling is required, the overload values mentioned above should be added to the horse power rating.

One horse power is the equivalent of 550 ft. lbs. of work per second; therefore, the total horse power exerted by a steam engine or electric motor in pulling a rope may be expressed:

$$\text{Horse power} = \frac{\text{feet} \times \text{pounds}}{\text{seconds} \times 550}$$

where feet represents the number of feet of rope wound on the drum in the measured time.

By rearrangement:

$$\text{Pull on rope in pounds} = \frac{\text{Horse power} \times \text{seconds} \times 550}{\text{feet}}$$

There are now on the right hand side of the equation two unknown factors, time and distance. By placing a marker on the rope or watching the end connection, it is very easy, with the aid of a watch and a rule, to determine very closely what these factors actually are.

The time at which these observations are taken should be when the engine or motor is pulling the hardest, as indicated by sound and the slowing down of speed. If the equipment is stalled by excessive loads, these observations should be made just before motion ceases.

EXAMPLE: A slack-line excavator driven by a 25 h.p. electric motor is digging in very hard ground. When the bucket strikes the hard places, it digs 15 feet in 10 seconds. What is the pull on the line?

By using the formula we find:

$$\text{Pull on rope in pounds} = \frac{25 \times 1.5 \times 10 \times 550}{15} = 13,750 \text{ lbs.}$$

where the factor 1.5 in the above represents the 50% overload on the motor.

Providing a safety factor of five for the rope on this installation requires a rope with a strength of $5 \times 13,750 = 68,750$ pounds.

GROOVES

Grooves in sheaves and drums should be slightly larger than the rope, in order to avoid pinching and binding of the strands, and to permit the rope to adjust itself to the radius of curvature. The greater the angle of approach to the groove, the larger the tolerance required to prevent excessive flange wear.

The diameter of an unused rope may exceed the nominal diameter by the amounts specified

in the United States Master Specification for Wire Rope, as shown in the following table.

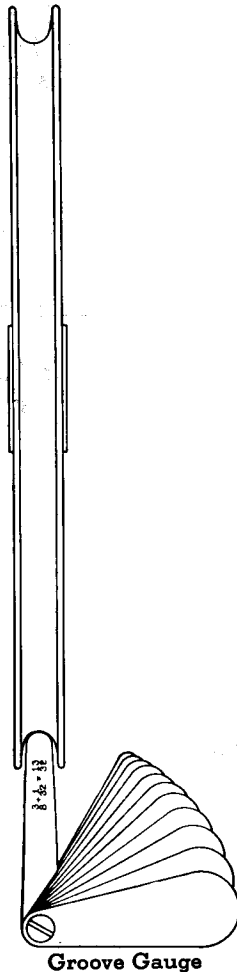
Diameter Tolerances for Wire Rope

Nominal Diameter of Rope in Inches	Undersize Inch	Oversize Inch
0 to $\frac{3}{4}$	0	$\frac{1}{32}$
$\frac{13}{16}$ to $1\frac{1}{8}$	0	$\frac{3}{64}$
$\frac{13}{16}$ to $1\frac{1}{2}$	0	$\frac{1}{16}$
$\frac{19}{16}$ to $2\frac{1}{4}$	0	$\frac{3}{32}$
$2\frac{5}{16}$ and larger	0	$\frac{1}{8}$

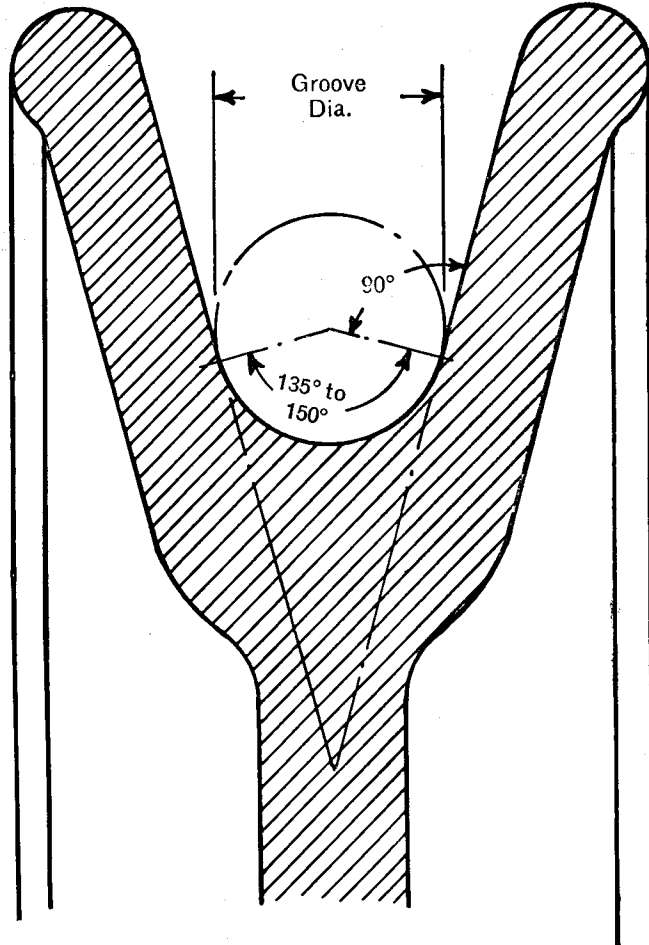
Tolerance Groove Diameter Should Exceed Nominal Rope Diameter

Nominal Diameter of Rope in Inches	Minimum	New or Remachined Grooves
$\frac{1}{4}$ — $\frac{5}{16}$	$\frac{1}{64}$	$\frac{1}{32}$
$\frac{3}{8}$ — $\frac{3}{4}$	$\frac{1}{32}$	$\frac{1}{16}$
$\frac{13}{16}$ — $1\frac{1}{8}$	$\frac{3}{64}$	$\frac{3}{32}$
$\frac{13}{16}$ — $1\frac{1}{2}$	$\frac{1}{16}$	$\frac{1}{8}$
$\frac{19}{16}$ — $2\frac{1}{4}$	$\frac{3}{32}$	$\frac{5}{32}$
$2\frac{5}{16}$ and Larger	$\frac{1}{8}$	$\frac{3}{16}$

In the recommended minimum tolerances of groove diameters shown on the right, allowances have been made for the new rope being slightly oversize. Grooves of too large diameter do not properly support the rope, and permit it to become elliptical.



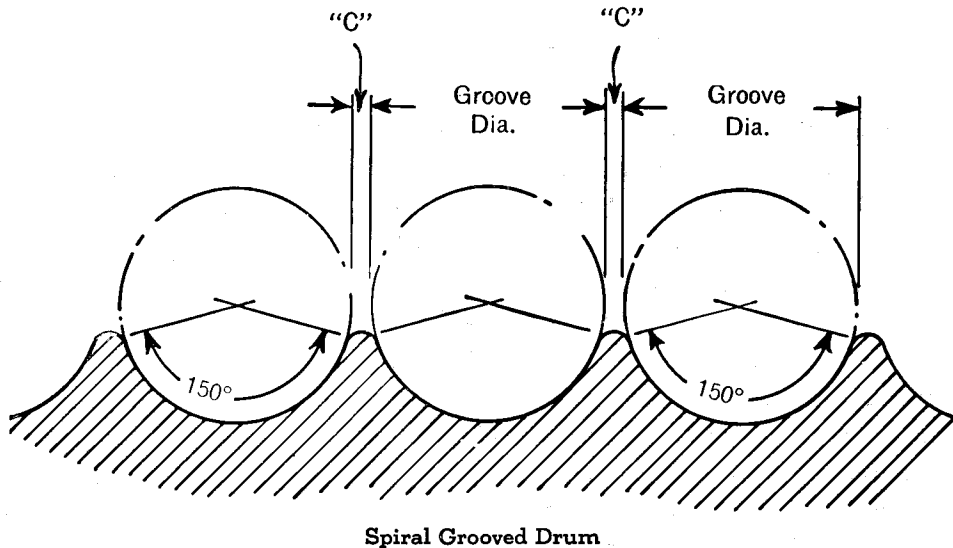
Groove Gauge



Sheave

These tolerances are too great for traction type elevators. New and re-machined grooves on "U" type machines should be $\frac{1}{32}$ inch greater than nominal rope diameter. Grooves should be re-machined when worn to less than $\frac{1}{64}$ inch larger than nominal rope diameter.

Rope Dia. Inches	Clearance "C" in Inches
$\frac{1}{2} - 1\frac{1}{4}$	$\frac{1}{16}$
$1\frac{3}{8} - 1\frac{3}{4}$	$\frac{3}{32}$
$1\frac{7}{8} - 2\frac{1}{2}$	$\frac{1}{8}$



Grooved drums are recommended in preference to smooth drums as the grooves furnish better support for the rope than the flat surfaces of smooth drums, and the more uniform winding results in less abrasive wear on the rope.

Annular, or concentric grooves in drums should not be greater in depth than 10% of the rope diameter. Deeper grooves will cause undue distortion of the rope at the points of cross-over from one groove to the next. Clearances recommended for spiral type grooves are suitable for annular grooves.

Grooves should be smooth. Those which have taken the imprint of the outer wires of previous ropes exert a grinding action on new ropes. A harder metal is recommended for installations where the radial pressure of the rope on the groove scores the groove. This radial pressure is directly proportional to the load on the rope, and inversely proportional to the diameter of the rope and the

tread diameter of the sheave or drum. This may be expressed as:

$$P = \frac{L}{RD}$$

Where: P = pressure in pounds per square inch.

L = load on the rope in pounds.

R = tread radius (one-half tread diameter) of the sheave or drum in inches.

D = diameter of the rope in inches.

Maximum Radial Pressure

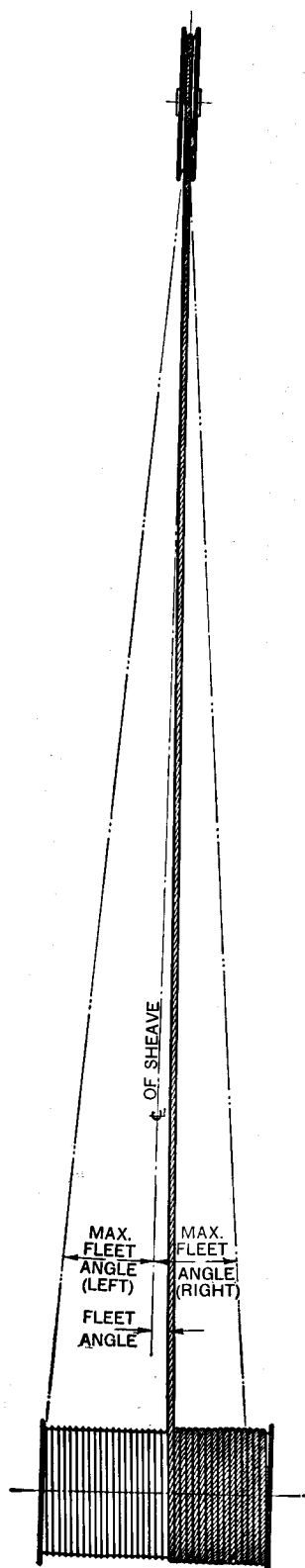
Rope Construction	Cast Iron	Cast Steel
6x19 Regular Lay	500	900
6x19 Lang Lay	575	1025
8x19 Regular Lay	600	1075
6x37 Regular Lay	600	1075
6x37 Lang Lay	700	1225
Flattened Strand	800	1450

For greater pressures, manganese or special alloy steels, heat treated, should be used.

FLEET ANGLE

On installations where the wire rope passes over a lead sheave then on to a drum, it is important that the lead sheave be located at a sufficient distance from the drum to maintain a small fleet angle at all times. The fleet angle is the side angle at which the rope approaches the sheave from the drum. It is the angle between the center line of the sheave and the wire rope.

Experience has proven that the best wire rope service is obtained when the maximum fleet angle is not more than $1\frac{1}{2}$ degrees for smooth drums and 2 degrees for grooved drums. The maximum fleet angle is the angle between the center line of the sheave and the rope when it is at the end of its traverse travel on drum. Fleet angles of $1\frac{1}{2}$ and 2 degrees are the equivalents of approximately 38 and 29 feet, respectively, of lead for each foot of rope traverse travel either side of the center line of the sheave. Thus a smooth drum with 3 ft. traverse travel with the center of travel in line with the lead sheave should be located not less than 57 ft. from the lead sheave. If the drum were grooved, the minimum distance should be approximately 43.5 ft.



UNREELING AND UNCOILING

When removing wire rope from the reel on which it is received, or from the coil if it is a coil shipment, it is imperative that the reel or coil

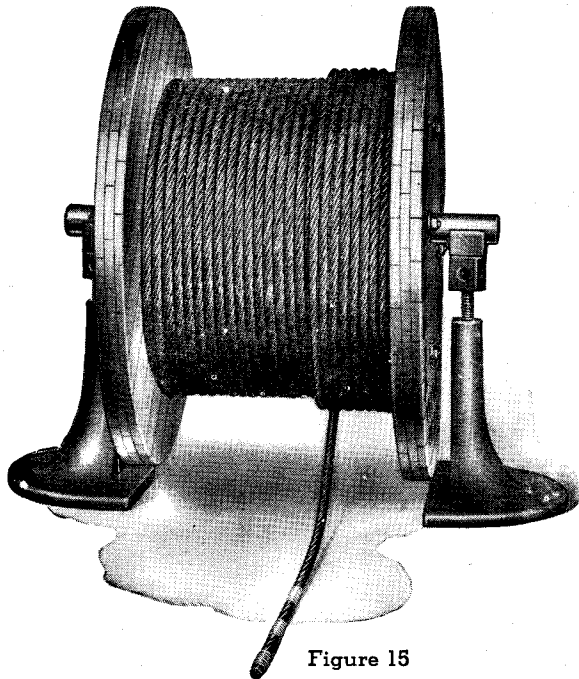


Figure 15

rotates as the rope unwinds. Attempts to unwind rope from stationary coils or reels will result in

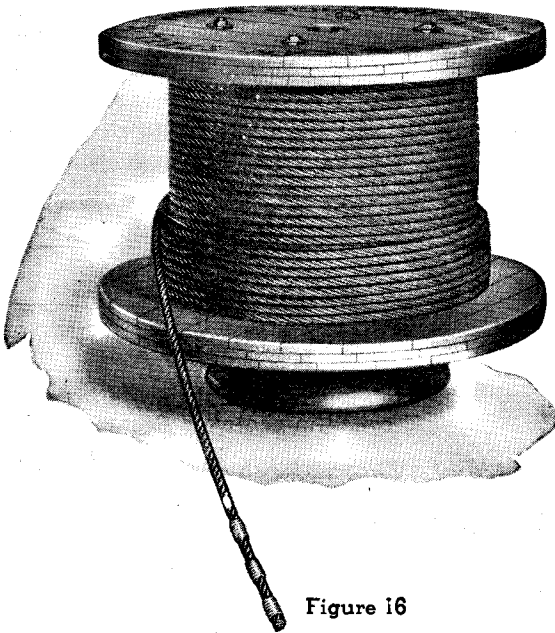


Figure 16

kinking the rope, and once a kink is formed the rope at that point is ruined beyond repair.

Unreeling: If the rope is to be unwound from a reel, there are three correct methods of unreeling.

1. The reel may be mounted on a shaft supported by two jacks as shown in Figure 15. The rope is then pulled from the reel by a workman holding the end of the rope and walking away from the reel which rotates as the rope unwinds. This is the common approved method of unreeling wire rope.

2. The reel may be mounted on an unreeling stand as shown in Figure 16. It is then unwound in the same manner as described above. Care must be exercised to keep the rope under sufficient tension to prevent slack accumulating and the rope dropping below the lower reel head.

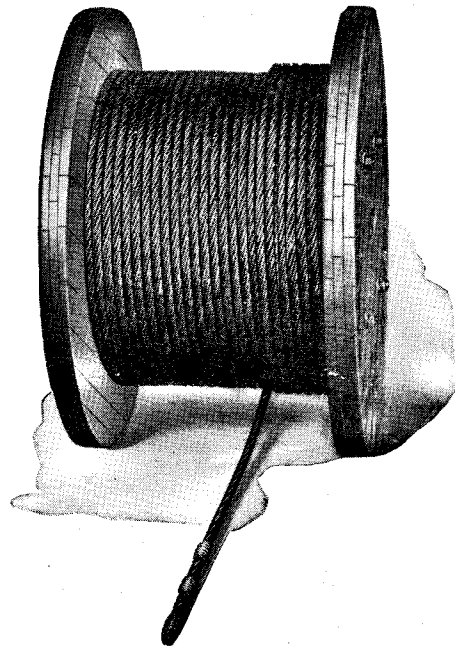


Figure 17

3. The end of the rope may be held and the reel rolled along the ground as shown in Figure 17.

When re-reeling rope from one horizontal reel to a second horizontal reel, the rope should travel from the top of the full reel to the top of the empty reel; or, from the bottom of the full reel to the bottom of the empty reel.

When re-reeling rope from one vertical reel to a second vertical reel, the rope should travel in a line parallel to a line drawn between the axes of the two reels, and the reels should rotate in the same direction.

The object of these instructions is to avoid putting a reverse bend into the rope as it is being re-reeled. Reeling the ropes so that a reverse bend is put into the rope causes it to become livelier and harder to handle.

Uncoiling: If the rope is to be unwound from a coil, there is only one correct method of uncoiling. The end of the rope should be held and the coil rolled on the ground like a hoop as shown in Figure 18.

Failure to use one of these methods has ruined many lengths of wire rope. Hemp rope can be unwound by pulling through the eye of the coil or from the stationary reel standing on end without seriously injuring it. These methods should never be attempted when handling wire rope.

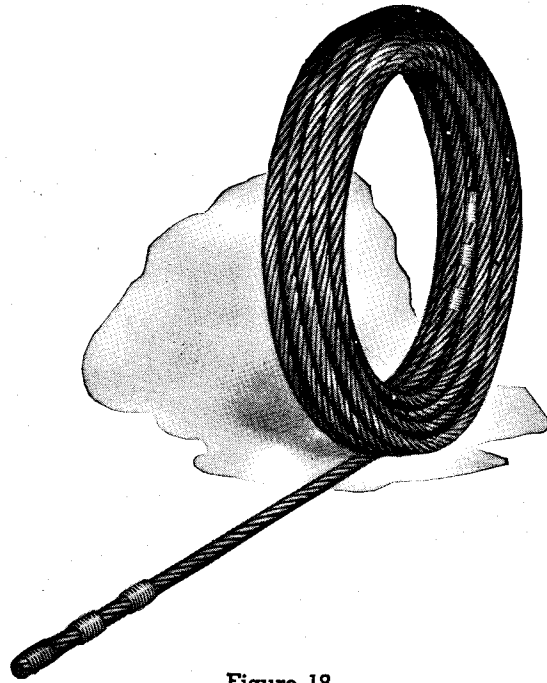


Figure 18

SPOOLING AND COILING

Correct Method of Winding First Layer on Drum

When winding the first layer of wire rope on a smooth drum, it should be started from the side which causes the coils on the drum to hug together. This tends to produce a uniform and close wound first layer, which, in turn, tends toward uniformity of successive layers. It also results in an even wind of the coils of the first layer on the drum when the rope is re-wound after the load has been slacked off and then picked up.

When the first layer of wire rope is wound on a smooth drum in the wrong direction, the coils tend to spread apart. Coils of the second layer wedge themselves between the open coils, causing non-uniform winding, which may result in damaging the rope from crushing and abrasion. There is also a tendency for the remaining coils on the drum, when the rope is out and the load slacked off and then applied again, to cross other coils with resultant crushing of the rope at the points of cross-over.

The proper direction of winding the first layer on a smooth drum is determined by standing behind the drum and looking along the path the rope travels.

Right Lay Wire Ropes	Overwind: From Left to Right
	Underwind: From Right to Left
Left Lay Wire Ropes	Overwind: From Right to Left
	Underwind: From Left to Right

When overwinding the top of the drum rotates toward the observer while the rope is winding on. When underwinding the top of the drum rotates away from the observer while the rope is winding on.

Correct Method of Coiling Wire Rope

When hand coiling wire rope into a coil on the floor or bench, coil it in the direction that will take twist out of the rope. When coiled in the proper direction little difficulty will be encountered, but if coiled in the wrong direction twists are added and the rope becomes too lively to readily form into a coil.

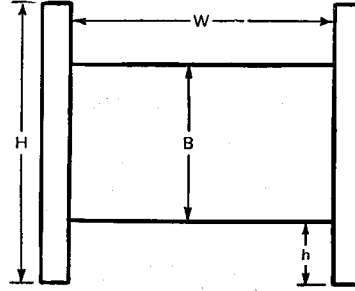
Looking down on the coil, the proper directions are:

Right Lay Wire Ropes—Coil in a Clockwise Direction.

Left Lay Wire Ropes—Coil in a Counter-Clockwise Direction.

DRUM AND REEL CAPACITIES

Let H = Diameter of Head in Inches.
 B = Diameter of Barrel in Inches.
 h = Depth of Cable in Inches.
 W = Width between Flanges in Inches.
 d = Diameter of Cable in Inches.
 L = Length of Cable in Feet.



$$L = \frac{\pi W (H + B) (H - B)}{48 d^2} = \frac{W (H + B) (H - B)}{15.28 d^2} = \frac{.06545 W (H + B) (H - B)}{d^2} = \frac{.2618 W h (B + h)}{d^2}$$

Table of Factors for $\frac{.2618}{d^2}$

Cable Dia.	$\frac{1}{4}$	$\frac{3}{8}$	$\frac{7}{16}$	$\frac{1}{2}$	$\frac{9}{16}$	$\frac{5}{8}$	$\frac{3}{4}$	$\frac{7}{8}$	1	$1\frac{1}{8}$	$1\frac{1}{4}$	$1\frac{3}{8}$	$1\frac{1}{2}$
Factor	4.19	1.86	1.37	1.05	.828	.670	.465	.342	.262	.207	.168	.138	.116
Cable Dia.	$1\frac{5}{8}$	$1\frac{3}{4}$	$1\frac{7}{8}$	2	$2\frac{1}{8}$	$2\frac{1}{4}$	$2\frac{3}{8}$	$2\frac{1}{2}$	$2\frac{5}{8}$	$2\frac{3}{4}$	$2\frac{7}{8}$	3	$3\frac{1}{4}$
Factor	.099	.085	.074	.066	.058	.052	.046	.042	.038	.035	.032	.029	.025

To Compute Length of Cable in Feet for any Reel or Drum: $L = \text{Factor} \times W \times h \times (B + h)$

The Formula can be readily derived:

$$(1) \text{ Length of Coil of Middle Layer} = \frac{\pi}{12} \left(B + \frac{H - B}{2} \right)$$

$$\text{Number of Coils} = \frac{W}{d} \quad \text{Number of Layers} = \frac{H - B}{2d}$$

$$L = \frac{\pi}{12} \left(B + \frac{H - B}{2} \right) \times \frac{W}{d} \times \frac{H - B}{2d} = \frac{\pi W (H + B) (H - B)}{48d^2}$$

$$(2) \text{ Volume of Drum in Cubic Inches} = W \left(\frac{\pi H^2}{4} - \frac{\pi B^2}{4} \right)$$

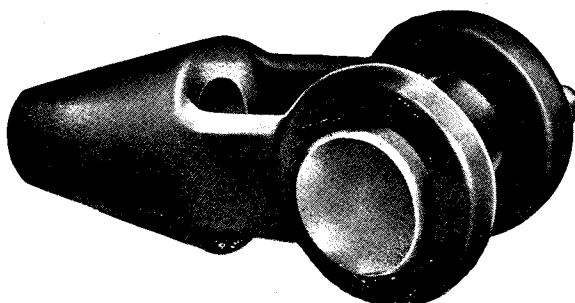
$$L = \frac{W}{12d^2} \left(\frac{\pi H^2}{4} - \frac{\pi B^2}{4} \right) = \frac{\pi W}{48d^2} (H^2 - B^2) = \frac{\pi W (H + B) (H - B)}{48d^2}$$

When shipping rope on reels, the reels should not be completely filled. A margin (m) should be left to protect the rope. H then becomes $H - 2m$ and h becomes $h - m$.

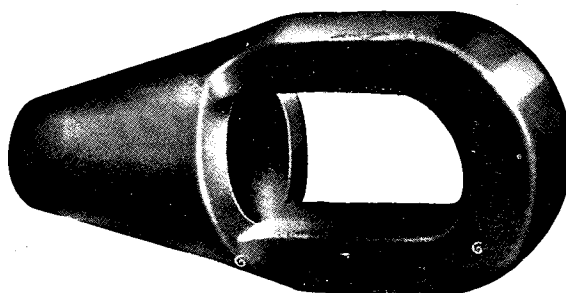
This Formula is based on the assumption that: the cable is exact size and does not flatten

when coiled; and that it is in perfectly uniform layers with no meshing of the coils. These factors vary with size and construction of the cable and with the dimensions of the reel or drum. As these variables tend to offset each other, this method of computing reel and drum capacities has proved to be reliable.

WIRE ROPE FITTINGS



Tiger Open Socket



Tiger Closed Socket

WIRE ROPE SOCKETS attached with pure zinc in accordance with directions on page 100 are the most reliable of all wire rope terminal fittings. When properly attached, standard drop forged sockets develop the full strength of the rope.

Wire rope sockets are recommended for all permanent installations, and for all main hoisting ropes. Sockets should be used where safety is required and where service conditions are severe.

The method of socketing in which the strand ends are bent back into the socket basket will not develop the full strength of a high strength rope,

as there is not sufficient space in the basket for enough zinc to properly secure the strand ends. This method is not recommended as it is not efficient, and it is very apt to distort the rope structure causing unequal loading of the individual strands of the rope. This, in turn, tends to develop early rope failure.

The use of babbitt, or other low-melting alloys, is not recommended for heavy duty ropes, as they will not develop the holding power secured with pure zinc.

LIST OF WIRE ROPE FITTINGS

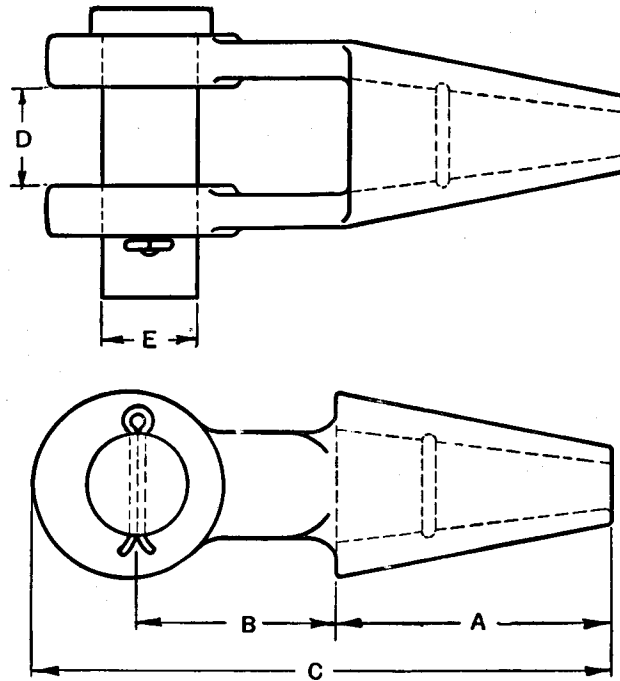
Tiger Open Wire Rope Sockets
Tiger Closed Wire Rope Sockets
Tiger Open Wire Rope Bridge Sockets
Tiger Closed Wire Rope Bridge Sockets
Open Wire Rope Wedge Type Sockets
Tiger Wire Rope Clips
Galvanized Light Wire Rope Thimbles
Galvanized Heavy Wire Rope Thimbles

Galvanized Standard Hawser Thimbles
Tiger Wire Rope Hooks
Tiger Wire Rope Links
Anchor Shackles
Shackle End Turnbuckles
Eye End Turnbuckles
Special Turnbuckles

Special fittings for Wire Rope Slings are shown in our catalog on Tiger Wire Rope Slings.

TIGER OPEN WIRE ROPE SOCKETS

Tiger Wire Rope Sockets are drop-forged from high grade SAE forging steel and normalized to give a fine grain structure.



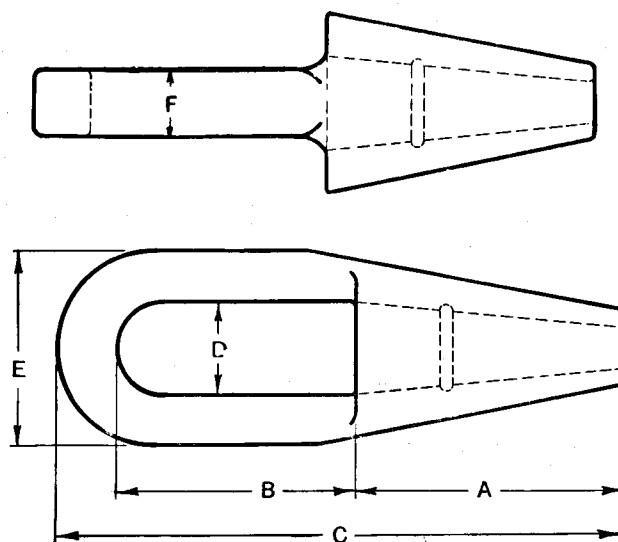
Dimensions of Sockets

Rope Diam.	Length of Basket A	Distance from Basket to Center Line of Pin B	Overall Length C	Opening Between Jaws D	Diam. of Pin E	Approx. Weight Pounds
$\frac{1}{4}$	2	$\frac{19}{16}$	$\frac{45}{16}$	$\frac{11}{16}$	$\frac{11}{16}$	0.9
$\frac{5}{16}$ - $\frac{3}{8}$	2	$\frac{13}{4}$	$\frac{45}{8}$	$\frac{13}{16}$	$\frac{13}{16}$	1.1
$\frac{7}{16}$ - $\frac{1}{2}$	$2\frac{1}{2}$	2	$\frac{59}{16}$	1	1	2.3
$\frac{9}{16}$ - $\frac{5}{8}$	3	$2\frac{1}{2}$	$6\frac{3}{4}$	$1\frac{1}{4}$	$1\frac{3}{16}$	3.8
$\frac{3}{4}$	$3\frac{1}{2}$	3	$7\frac{15}{16}$	$1\frac{1}{2}$	$1\frac{3}{8}$	6.0
$\frac{7}{8}$	4	$3\frac{1}{2}$	$9\frac{1}{4}$	$1\frac{3}{4}$	$1\frac{5}{8}$	10.0
1	$4\frac{1}{2}$	4	$10\frac{9}{16}$	2	2	15.5
$1\frac{1}{8}$	5	$4\frac{1}{2}$	$11\frac{13}{16}$	$2\frac{1}{4}$	$2\frac{1}{4}$	22.0
$1\frac{1}{4}$	$5\frac{1}{2}$	5	$13\frac{3}{16}$	$2\frac{1}{2}$	$2\frac{1}{2}$	32.0
$1\frac{3}{8}$	$5\frac{1}{2}$	5	$13\frac{3}{16}$	$2\frac{1}{2}$	$2\frac{1}{2}$	32.0
$1\frac{1}{2}$	6	6	$15\frac{1}{8}$	3	$2\frac{3}{4}$	46.0
$1\frac{5}{8}$	$6\frac{1}{2}$	$6\frac{1}{2}$	$16\frac{1}{4}$	3	3	55.0
$1\frac{3}{4}$	$7\frac{1}{2}$	7	$18\frac{1}{4}$	$3\frac{1}{2}$	$3\frac{1}{2}$	85.0
$1\frac{7}{8}$	$7\frac{1}{2}$	7	$18\frac{1}{4}$	$3\frac{1}{2}$	$3\frac{1}{2}$	85.0
2 - $2\frac{1}{8}$	$8\frac{1}{2}$	9	$21\frac{1}{2}$	4	$3\frac{3}{4}$	125.0
$2\frac{1}{4}$ - $2\frac{3}{8}$	9	10	$23\frac{1}{2}$	$4\frac{1}{2}$	$4\frac{1}{4}$	165.0

Pins are turned. Holes are $\frac{1}{32}$ -inch larger than pin diameters.

TIGER CLOSED WIRE ROPE SOCKETS

Tiger Wire Rope Sockets are of generous dimensions and develop 75,000 lbs. per square inch tensile strength.



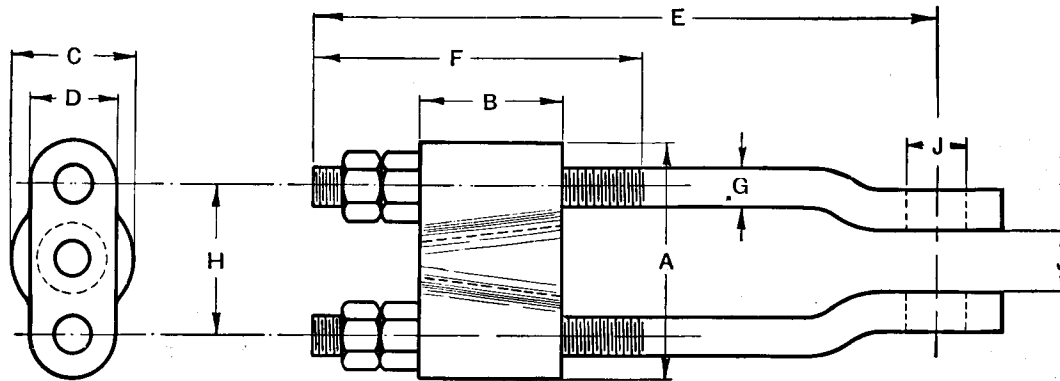
Dimensions of Sockets

Rope Diam.	Length of Basket A	Length of Opening B	Overall Length C	Width of Opening D	Width of Loop E	Depth of Loop F	Approx. Weight Pounds
$\frac{1}{4}$	2	$1\frac{13}{16}$	$4\frac{1}{4}$	$\frac{13}{16}$	$\frac{17}{16}$	$\frac{1}{2}$	0.5
$\frac{5}{16}$ - $\frac{3}{8}$	2	$2\frac{1}{16}$	$4\frac{5}{8}$	$\frac{15}{16}$	$1\frac{11}{16}$	$\frac{5}{8}$	0.8
$\frac{7}{16}$ - $\frac{1}{2}$	$2\frac{1}{2}$	$2\frac{5}{16}$	$5\frac{1}{2}$	$1\frac{1}{8}$	2	$\frac{7}{8}$	1.6
$\frac{9}{16}$ - $\frac{5}{8}$	3	$2\frac{9}{16}$	$6\frac{3}{8}$	$1\frac{3}{8}$	$2\frac{5}{8}$	1	3.0
$\frac{3}{4}$	$3\frac{1}{2}$	$3\frac{1}{16}$	$7\frac{5}{8}$	$1\frac{5}{8}$	3	$1\frac{1}{4}$	4.5
$\frac{7}{8}$	4	$3\frac{5}{8}$	$8\frac{7}{8}$	$1\frac{7}{8}$	$3\frac{5}{8}$	$1\frac{1}{2}$	7.0
1	$4\frac{1}{2}$	$4\frac{1}{8}$	10	$2\frac{1}{4}$	$4\frac{1}{8}$	$1\frac{3}{4}$	11.0
$1\frac{1}{8}$	5	$4\frac{5}{8}$	$11\frac{1}{8}$	$2\frac{1}{2}$	$4\frac{1}{2}$	2	16.0
$1\frac{1}{4}$	$5\frac{1}{2}$	$5\frac{3}{16}$	$12\frac{5}{16}$	$2\frac{3}{4}$	5	$2\frac{1}{4}$	22.0
$1\frac{3}{8}$	$5\frac{1}{2}$	$5\frac{3}{16}$	$12\frac{5}{16}$	$2\frac{3}{4}$	5	$2\frac{1}{4}$	22.0
$1\frac{1}{2}$	6	$6\frac{3}{16}$	$14\frac{1}{8}$	$3\frac{1}{8}$	$5\frac{3}{8}$	$2\frac{1}{2}$	28.0
$1\frac{5}{8}$	$6\frac{1}{2}$	$6\frac{3}{4}$	$15\frac{3}{8}$	$3\frac{1}{4}$	$5\frac{3}{4}$	$2\frac{3}{4}$	36.0
$1\frac{3}{4}$	$7\frac{1}{2}$	$7\frac{13}{16}$	$17\frac{1}{2}$	$3\frac{17}{32}$	$6\frac{3}{4}$	3	58.0
$1\frac{7}{8}$	$7\frac{1}{2}$	$7\frac{13}{16}$	$17\frac{1}{2}$	$3\frac{17}{32}$	$6\frac{3}{4}$	3	58.0
2 - $2\frac{1}{8}$	$8\frac{1}{2}$	$8\frac{13}{16}$	$19\frac{3}{4}$	$3\frac{25}{32}$	$7\frac{5}{8}$	$3\frac{1}{4}$	80.0
$2\frac{1}{4}$ - $2\frac{3}{8}$	9	$9\frac{3}{4}$	$21\frac{5}{8}$	$4\frac{9}{32}$	$8\frac{1}{2}$	$3\frac{5}{8}$	105.0

TIGER OPEN WIRE ROPE BRIDGE SOCKETS

The distance between eyes, dimension "J," and length of eye-bolt, dimension "E," can be varied to meet requirements. These sockets are made of steel throughout and develop the full strength of

the rope to which they are attached. These sockets are suitable for attaching to Galvanized Bridge Cables, Locked Coil Track Cable and any type of Wire Rope.



Dimensions of Sockets

Rope Diam.	SOCKET CASTINGS				Length of Eye Bolts	Take- Up	Diam. of Eye Bolts	Center to Center Distance of Eye Bolts	Diam. of Pin Dist. Between Eyes	APPROX. WEIGHT POUNDS			
	A	B	C	D						Cast- ing	Two Eye Bolts and Nuts	Each Addi- tional Foot of Eye Bolts	Pin*
1/2	5	3 1/2	2 3/4	2	16	8	3/4	3	1	5.5	5.5	3	1
5/8	5 7/8	3 3/4	3	2 3/8	18	8	7/8	3 1/2	1 1/4	10	9	4.1	1.3
3/4	6 1/2	4	3 1/4	2 1/2	22	10	1	4	1 1/2	14	16	5.3	2.3
7/8	7 1/4	4 1/2	3 3/4	2 3/4	24	10	1 1/8	4 1/2	1 3/4	18	20	6.7	3.5
1	10	6	4 1/4	3	27	12	1 1/4	7	2	35	33	7	6
1 1/8	10	6	4 5/8	3 1/4	27	12	1 3/8	7	2 1/4	38	37	10	8
1 1/4	10 3/4	6	5	3 1/2	27	12	1 1/2	7 1/4	2 1/2	48	41	12	12
1 3/8	11 1/2	6 1/2	5 5/8	4	28	12	1 3/4	7 1/2	2 3/4	62	69	16	16
1 1/2	12	7	6	4 1/4	31	15	1 7/8	7 3/4	3	74	86	19	20
1 5/8	12 1/2	7 1/2	6 3/8	4 1/2	32	15	2	8	3 1/4	90	98	21	25
1 3/4	13 1/2	8	7	5	36	18	2 1/4	8 1/2	3 1/2	108	136	27	32
1 7/8	15	9	7 3/4	5 1/2	38	18	2 1/2	9 1/2	3 3/4	150	182	33	40
2	15	9	7 3/4	5 1/2	38	18	2 1/2	9 1/2	3 3/4	150	182	33	40
2 1/8	16 1/4	10	8 1/2	6	40	18	2 3/4	10 1/4	4	195	227	40	48
2 1/4	16 1/4	10	8 1/2	6	40	18	2 3/4	10 1/4	4	195	227	40	48
2 3/8	17 3/4	11	9 1/4	6 1/2	42	18	3	11 1/4	4 1/2	254	284	48	65
2 1/2	17 3/4	11	9 1/4	6 1/2	42	18	3	11 1/4	4 1/2	254	284	48	65
2 5/8	19	12	10	7	42	18	3 1/4	12	5	318	341	56	89
2 3/4	19	12	10	7	42	18	3 1/4	12	5	318	341	56	89
3	21	13	10 3/4	7 1/2	44	18	3 1/2	13 1/2	5 1/2	410	415	65	114

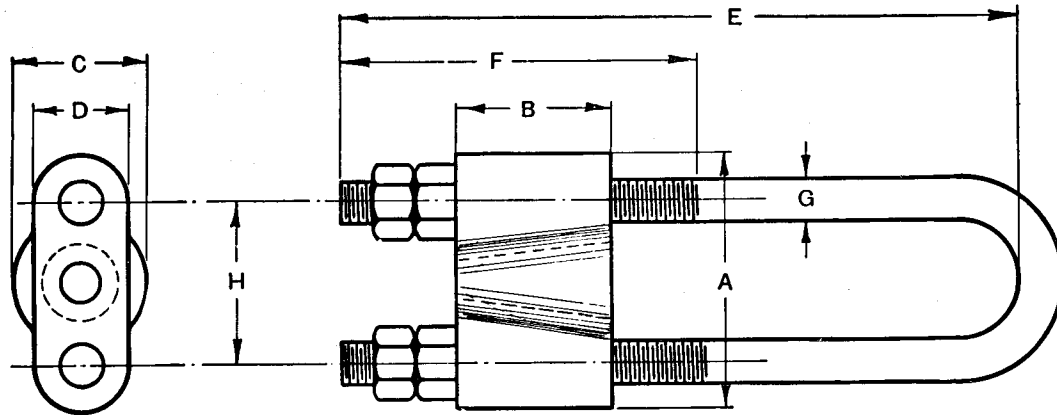
*Pin not included unless specified.

TIGER CLOSED WIRE ROPE BRIDGE SOCKETS

Length of U-bolt, dimension "E," may be varied to suit requirements.

These sockets are constructed throughout of

steel and are suitable for attaching to Galvanized Bridge Cables, Locked Coil Track Cable, and any type of Wire Rope.

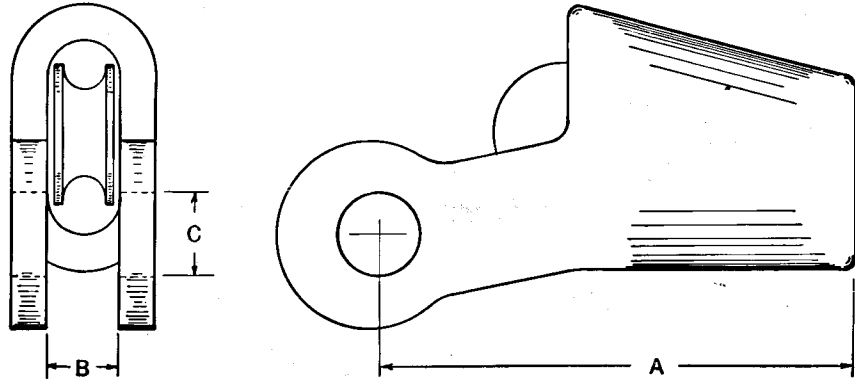


Dimensions of Sockets

Rope Diam.	Socket Castings				Length of U-Bolt	Takeup	Diam. of U-Bolt	Center to Center Distance of U-Bolt	APPROX. WEIGHT POUNDS		
	A	B	C	D					Casting	U-Bolt and Nuts	Each Additional Foot of U-Bolt
$\frac{1}{2}$	5	$3\frac{1}{2}$	$2\frac{3}{4}$	2	16	8	$\frac{3}{4}$	3	5.5	4.5	3
$\frac{5}{8}$	$5\frac{7}{8}$	$3\frac{3}{4}$	3	$2\frac{3}{8}$	18	8	$\frac{7}{8}$	$3\frac{1}{2}$	10	8	4.1
$\frac{3}{4}$	$6\frac{1}{2}$	4	$3\frac{1}{4}$	$2\frac{1}{2}$	22	10	1	4	14	13	5.3
$\frac{7}{8}$	$7\frac{1}{4}$	$4\frac{1}{2}$	$3\frac{3}{4}$	$2\frac{3}{4}$	24	10	$1\frac{1}{8}$	$4\frac{1}{2}$	18	19	6.7
1	10	6	$4\frac{1}{4}$	3	27	12	$1\frac{1}{4}$	7	35	27	7
$1\frac{1}{8}$	10	6	$4\frac{5}{8}$	$3\frac{1}{4}$	27	12	$1\frac{3}{8}$	7	38	32	10
$1\frac{1}{4}$	$10\frac{3}{4}$	6	5	$3\frac{1}{2}$	27	12	$1\frac{1}{2}$	$7\frac{1}{4}$	48	39	12
$1\frac{3}{8}$	$11\frac{1}{2}$	$6\frac{1}{2}$	$5\frac{5}{8}$	4	28	12	$1\frac{3}{4}$	$7\frac{1}{2}$	62	57	16
$1\frac{1}{2}$	12	7	6	$4\frac{1}{4}$	31	15	$1\frac{7}{8}$	$7\frac{3}{4}$	74	68	19
$1\frac{5}{8}$	$12\frac{1}{2}$	$7\frac{1}{2}$	$6\frac{3}{8}$	$4\frac{1}{2}$	32	15	2	8	90	80	21
$1\frac{3}{4}$	$13\frac{1}{2}$	8	7	5	36	18	$2\frac{1}{4}$	$8\frac{1}{2}$	108	110	27
$1\frac{7}{8}$	15	9	$7\frac{3}{4}$	$5\frac{1}{2}$	38	18	$2\frac{1}{2}$	$9\frac{1}{2}$	150	140	33
2	15	9	$7\frac{3}{4}$	$5\frac{1}{2}$	38	18	$2\frac{1}{2}$	$9\frac{1}{2}$	150	140	33
$2\frac{1}{8}$	$16\frac{1}{4}$	10	$8\frac{1}{2}$	6	40	18	$2\frac{3}{4}$	$10\frac{1}{4}$	195	183	40
$2\frac{1}{4}$	$16\frac{1}{4}$	10	$8\frac{1}{2}$	6	40	18	$2\frac{3}{4}$	$10\frac{1}{4}$	195	183	40
$2\frac{3}{8}$	$17\frac{3}{4}$	11	$9\frac{1}{4}$	$6\frac{1}{2}$	42	18	3	$11\frac{1}{4}$	254	231	48
$2\frac{1}{2}$	$17\frac{3}{4}$	11	$9\frac{1}{4}$	$6\frac{1}{2}$	42	18	3	$11\frac{1}{4}$	254	231	48
$2\frac{5}{8}$	19	12	10	7	42	18	$3\frac{1}{4}$	12	318	271	56
$2\frac{3}{4}$	19	12	10	7	42	18	$3\frac{1}{4}$	12	318	271	56
3	21	13	$10\frac{3}{4}$	$7\frac{1}{2}$	44	18	$3\frac{1}{2}$	$13\frac{1}{2}$	410	325	65

OPEN WIRE ROPE WEDGE TYPE SOCKETS

These Wedge Type Sockets are easily and quickly attached in the field by bending the rope end around the tapered wedge. These Sockets are furnished without pins.



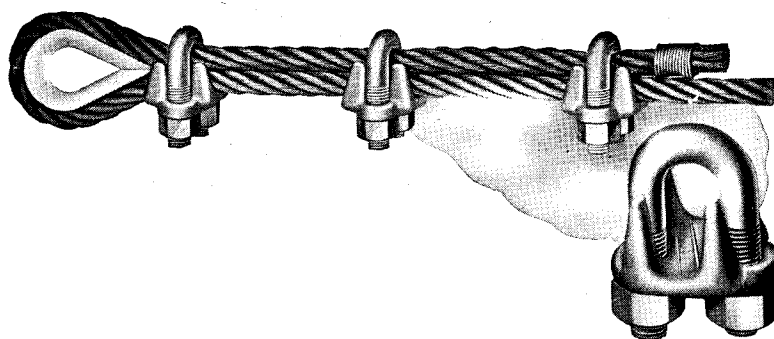
Dimensions of Sockets

Diameter of Rope	Center of Pin Hole to End of Socket A	Opening Between Ears B	Diameter of Pin Hole C	Approximate Weight Pounds
$\frac{3}{8}$	$5\frac{1}{2}$	$\frac{5}{8}$	$1\frac{1}{16}$	2.5
$\frac{1}{2}$	$5\frac{1}{2}$	$\frac{5}{8}$	$1\frac{1}{16}$	2.5
$\frac{5}{8}$	7	$1\frac{3}{8}$	$1\frac{1}{4}$	5
$\frac{3}{4}$	$7\frac{1}{2}$	$1\frac{1}{4}$	$1\frac{1}{4}$	9
$\frac{7}{8}$	9	$1\frac{3}{4}$	$1\frac{5}{8}$	15
1	$9\frac{3}{4}$	$1\frac{5}{8}$	$1\frac{5}{8}$	20
$1\frac{1}{8}$	$10\frac{5}{8}$	$1\frac{1}{2}$	$1\frac{5}{8}$	23
$1\frac{1}{4}$	$11\frac{3}{4}$	$1\frac{3}{4}$	$2\frac{1}{8}$	32
$1\frac{3}{8}$	$11\frac{3}{4}$	$1\frac{3}{4}$	$2\frac{1}{8}$	32
$1\frac{1}{2}$	$13\frac{1}{4}$	$2\frac{1}{2}$	$3\frac{1}{8}$	52
$1\frac{5}{8}$	$13\frac{1}{4}$	$2\frac{1}{2}$	$3\frac{1}{8}$	52

Furnished without pin.

AMERICAN TIGER BRAND

TIGER WIRE ROPE CLIPS



DROP FORGED STEEL

HOT GALVANIZED

Rope Diam.	Diam. of U-Bolt	Approx. Wt. in Lbs.	No. Clips for Each Rope End*
$\frac{3}{16}$	$\frac{5}{16}$	.14	2
$\frac{1}{4}$	$\frac{3}{8}$	.29	2
$\frac{5}{16}$	$\frac{3}{8}$	.30	2
$\frac{3}{8}$	$\frac{7}{16}$	.46	2
$\frac{7}{16}$	$\frac{1}{2}$	.73	2
$\frac{1}{2}$	$\frac{1}{2}$	.73	3
$\frac{5}{8}$	$\frac{9}{16}$	1.00	3
$\frac{3}{4}$	$\frac{5}{8}$	1.46	3
$\frac{7}{8}$	$\frac{3}{4}$	2.41	4
1	$\frac{3}{4}$	2.66	4
$1\frac{1}{8}$	$\frac{3}{4}$	3.35	5
$1\frac{1}{4}$	$\frac{7}{8}$	4.63	5
$1\frac{3}{8}$	$\frac{7}{8}$	5.00	6
$1\frac{1}{2}$	$\frac{7}{8}$	5.48	6
$1\frac{5}{8}$	1	6.91	6
$1\frac{3}{4}$	1	7.67	6
2	1	10.4	6
$2\frac{1}{4}$	$1\frac{1}{8}$	13.6	6
$2\frac{1}{2}$	$1\frac{1}{8}$	15.9	6

*For Ropes with Wire Cores and for High Strength and Extra High Strength Strands, one additional clip should be installed on each end.

Tiger Wire Rope Clips are full size clips with genuine drop forged steel bases. They should not be confused with the lighter type malleable iron base clips.

Tiger Wire Rope Clips are easy to apply, reliable, and durable. Protected from corrosion by a heavy coating of pure zinc applied by the Hot Galvanizing Process, they can be used repeatedly.

In addition to the Drop Forged Steel Clips, Tiger Wire Rope Clips are forged from High Strength Bronze. These Bronze Clips are designed for use where electrolysis or corrosion makes the use of steel clips impractical. Dropped from the same forging dies as are the steel clips, these high strength bronze clips are full size. They weigh approximately 10% more than the steel clips, and are approximately 25% less strong. When High Strength Bronze Clips are used on wire ropes, one

clip more than recommended above should be used for each fastening.

It is possible to hold wire ropes with smaller number of clips than specified above, but the increased pressure required reduces the efficiency of the connection.

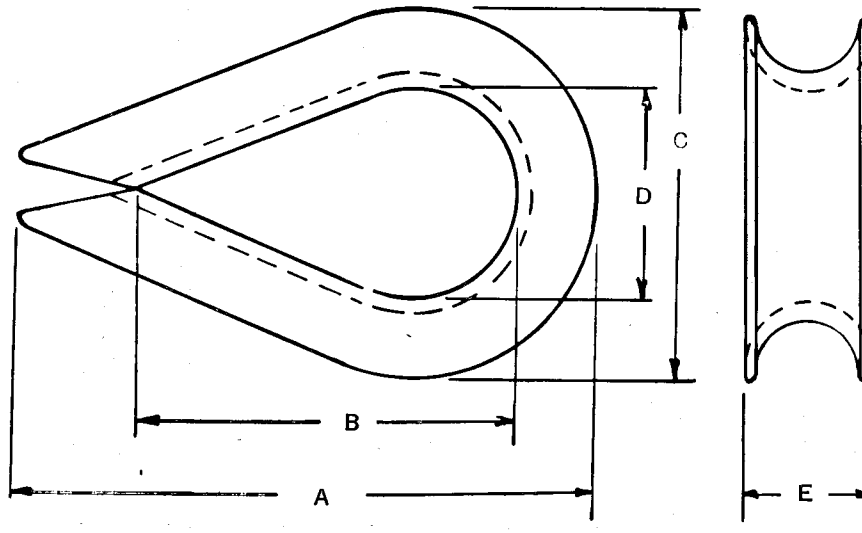
The clips should be attached to the rope ends as shown, with the base of the clip against the live or long end, and the U-Bolt bearing against the dead or short end of the rope. This is the only correct method of attaching wire rope clips.

The clips should be spaced at least six rope diameters apart to insure maximum holding power. After the rope has been placed in service and is under tension, the nuts should be tightened again to compensate for any decrease in rope diameter caused by the load.

GALVANIZED LIGHT WIRE ROPE THIMBLES

Galvanized Light Wire Rope Thimbles are recommended for use with Iron and Mild Plow Steel Ropes, Galvanized Guy Strands and Ropes, and stationary ropes.

For other ropes, the Galvanized Heavy Wire Rope Thimbles shown on the following page are recommended.



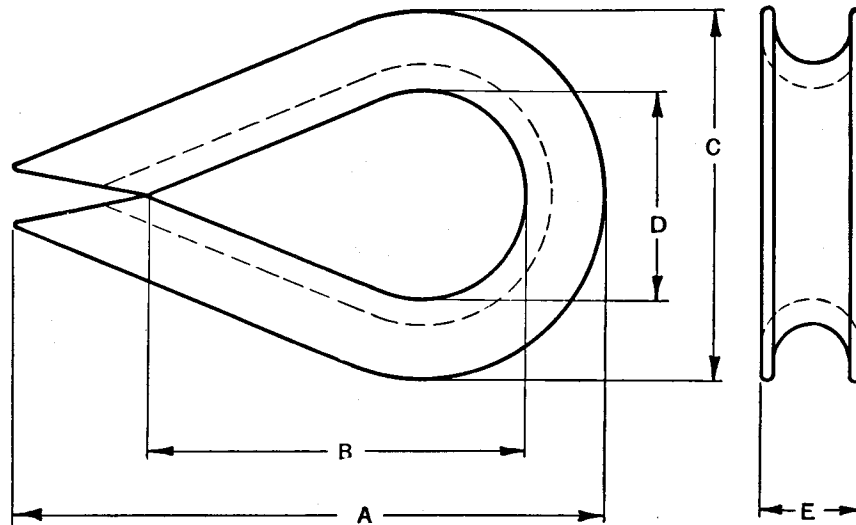
Dimensions of Thimbles

Rope Diam.	Length A	Length of Opening B	Width C	Width of Opening D	Depth E	Diam. of Largest Pin Thimble Will Take	Approx. Weight Pounds
$\frac{1}{8}$	$1\frac{5}{8}$	$1\frac{1}{8}$	$3\frac{1}{32}$	$\frac{5}{8}$	$\frac{1}{4}$	$\frac{9}{16}$	.025
$\frac{3}{16}$	$1\frac{3}{4}$	$1\frac{3}{16}$	$1\frac{3}{16}$	$1\frac{1}{16}$	$\frac{5}{16}$	$\frac{5}{8}$	.03
$\frac{1}{4}$	2	$1\frac{5}{16}$	$1\frac{5}{16}$	$\frac{3}{4}$	$\frac{3}{8}$	$1\frac{1}{16}$	.05
$\frac{5}{16}$	$2\frac{1}{4}$	$1\frac{1}{2}$	$1\frac{7}{16}$	$\frac{7}{8}$	$\frac{7}{16}$	$1\frac{3}{16}$	.07
$\frac{3}{8}$	$2\frac{3}{8}$	$1\frac{9}{16}$	$1\frac{5}{8}$	1	$\frac{1}{2}$	$1\frac{5}{16}$	.08
$\frac{7}{16}$	$2\frac{3}{4}$	$1\frac{5}{8}$	$1\frac{3}{4}$	$1\frac{1}{16}$	$\frac{9}{16}$	1	.14
$\frac{1}{2}$	3	$1\frac{7}{8}$	$1\frac{15}{16}$	$1\frac{1}{8}$	$\frac{5}{8}$	$1\frac{1}{16}$	.17
$\frac{5}{8}$	$3\frac{3}{4}$	$2\frac{1}{4}$	$2\frac{5}{16}$	$1\frac{3}{8}$	$\frac{7}{8}$	$1\frac{1}{4}$	.38
$\frac{3}{4}$	4	$2\frac{1}{2}$	$2\frac{5}{8}$	$1\frac{5}{8}$	1	$1\frac{1}{2}$	.50
$\frac{7}{8}$	$4\frac{1}{2}$	$2\frac{3}{4}$	$2\frac{7}{8}$	$1\frac{3}{4}$	$1\frac{1}{8}$	$1\frac{5}{8}$	.62
1	6	$4\frac{1}{4}$	$3\frac{1}{2}$	2	$1\frac{1}{4}$	$1\frac{7}{8}$	1.05
$1\frac{1}{8}$	$7\frac{1}{8}$	$4\frac{1}{2}$	$4\frac{1}{4}$	$2\frac{1}{2}$	$1\frac{3}{8}$	$2\frac{3}{8}$	2.81
$1\frac{1}{4}$	$7\frac{1}{8}$	$4\frac{3}{4}$	$4\frac{5}{8}$	$2\frac{3}{4}$	$1\frac{5}{8}$	$2\frac{5}{8}$	3.00
$1\frac{1}{2}$	$7\frac{3}{4}$	$4\frac{7}{8}$	$5\frac{1}{8}$	3	$1\frac{7}{8}$	$2\frac{7}{8}$	3.15
$1\frac{3}{4}$	$9\frac{1}{4}$	5	6	$3\frac{1}{2}$	$2\frac{1}{8}$	$3\frac{3}{8}$	5.50
2	11	7	$7\frac{1}{4}$	$4\frac{1}{2}$	$2\frac{3}{8}$	$4\frac{3}{8}$	7.50

GALVANIZED HEAVY WIRE ROPE THIMBLES

Galvanized Heavy Wire Rope Thimbles are recommended for use with all material handling

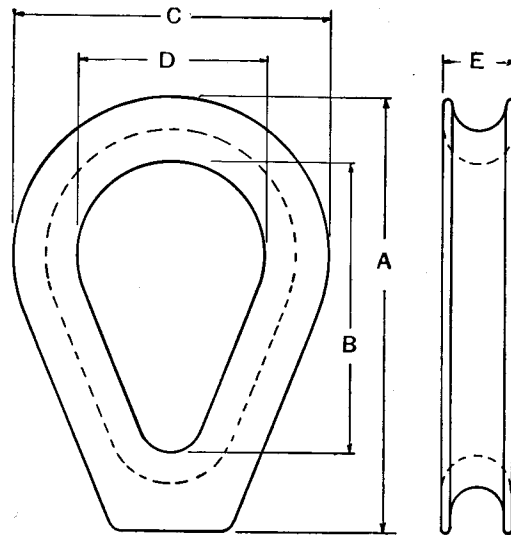
ropes, and all Plow Steel and Monitor Steel Wire Ropes.



Dimensions of Thimbles

Rope Diam.	Length A	Length of Opening B	Width C	Width of Opening D	Depth E	Diam. of Largest Pin Thimble Will Take	Approx. Weight Pounds
$\frac{1}{4}$	$2\frac{1}{2}$	$1\frac{5}{8}$	$1\frac{9}{16}$	$\frac{7}{8}$	$\frac{13}{32}$	$\frac{13}{16}$	.12
$\frac{5}{16}$	3	$1\frac{7}{8}$	$1\frac{13}{16}$	1	$\frac{15}{32}$	$\frac{15}{16}$	.15
$\frac{3}{8}$	$3\frac{3}{8}$	$2\frac{1}{8}$	$2\frac{1}{16}$	$1\frac{1}{8}$	$\frac{1}{2}$	$1\frac{1}{16}$	.22
$\frac{7}{16}$	$3\frac{3}{4}$	$2\frac{1}{2}$	$2\frac{1}{4}$	$1\frac{1}{4}$	$\frac{5}{8}$	$1\frac{3}{16}$	.30
$\frac{1}{2}$	$4\frac{1}{8}$	$2\frac{3}{4}$	$2\frac{5}{8}$	$1\frac{1}{2}$	$\frac{23}{32}$	$1\frac{7}{16}$	.50
$\frac{9}{16}$	$4\frac{1}{8}$	$2\frac{3}{4}$	$2\frac{5}{8}$	$1\frac{1}{2}$	$\frac{25}{32}$	$1\frac{7}{16}$	.50
$\frac{5}{8}$	$5\frac{1}{2}$	$3\frac{1}{4}$	$3\frac{3}{16}$	$1\frac{3}{4}$	$\frac{29}{32}$	$1\frac{5}{8}$	.75
$\frac{3}{4}$	$6\frac{1}{2}$	$3\frac{3}{4}$	$3\frac{3}{4}$	2	$1\frac{1}{16}$	$1\frac{7}{8}$	1.30
$\frac{7}{8}$	$7\frac{1}{8}$	$4\frac{1}{4}$	$4\frac{1}{16}$	$2\frac{1}{4}$	$\frac{13}{16}$	$2\frac{1}{8}$	1.60
1	$8\frac{1}{8}$	$4\frac{1}{2}$	$4\frac{11}{16}$	$2\frac{1}{2}$	$\frac{17}{16}$	$2\frac{3}{8}$	2.80
$1\frac{1}{8}$	$9\frac{1}{2}$	5	$5\frac{5}{8}$	$2\frac{7}{8}$	$\frac{15}{8}$	$2\frac{3}{4}$	4.50
$1\frac{1}{4}$	$9\frac{1}{2}$	$5\frac{1}{8}$	$5\frac{5}{8}$	$2\frac{7}{8}$	$1\frac{11}{16}$	$2\frac{3}{4}$	4.50
$1\frac{3}{8}$	$11\frac{5}{8}$	$6\frac{1}{4}$	$6\frac{7}{8}$	$3\frac{1}{2}$	$\frac{21}{16}$	$3\frac{3}{8}$	7.50
$1\frac{1}{2}$	$11\frac{5}{8}$	$6\frac{1}{4}$	$6\frac{7}{8}$	$3\frac{1}{2}$	$\frac{21}{8}$	$3\frac{3}{8}$	7.50
$1\frac{5}{8}$	$13\frac{3}{8}$	8	$7\frac{3}{4}$	4	$\frac{23}{8}$	$3\frac{7}{8}$	11.75
$1\frac{3}{4}$	14	9	$8\frac{1}{2}$	$4\frac{1}{2}$	$\frac{23}{4}$	$4\frac{3}{8}$	18.00
$1\frac{7}{8}$	15	10	$9\frac{1}{4}$	5	$\frac{27}{8}$	$4\frac{7}{8}$	22.00
2	17	12	$10\frac{1}{2}$	6	$\frac{31}{8}$	$5\frac{7}{8}$	30.00
$2\frac{1}{8}$	19	14	$12\frac{1}{4}$	7	$\frac{31}{2}$	$6\frac{7}{8}$	52.00
$2\frac{1}{4}$	19	14	$12\frac{1}{4}$	7	$\frac{31}{2}$	$6\frac{7}{8}$	52.00

GALVANIZED STANDARD HAWSER THIMBLES

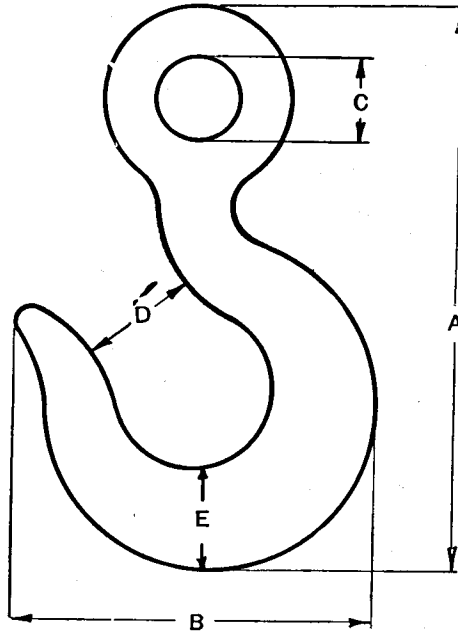


Galvanized Steel Castings

SIZE OF HAWSER		DIMENSIONS OF THIMBLES					Approx. Weight Pounds
		Length	Length of Opening	Width	Width of Opening	Depth	
Diameter	Circumference	A	B	C	D	E	
$\frac{5}{8}$	2	$6\frac{3}{4}$	$4\frac{1}{2}$	5	3	$1\frac{3}{16}$	4
$\frac{3}{4}$	$2\frac{3}{8}$	$6\frac{3}{4}$	$4\frac{1}{2}$	5	3	$1\frac{3}{16}$	4
$\frac{7}{8}$	$2\frac{3}{4}$	$8\frac{5}{8}$	$5\frac{3}{4}$	$6\frac{1}{4}$	$3\frac{3}{4}$	$1\frac{7}{16}$	8
1	$3\frac{1}{8}$	$8\frac{5}{8}$	$5\frac{3}{4}$	$6\frac{1}{4}$	$3\frac{3}{4}$	$1\frac{7}{16}$	8
$1\frac{1}{8}$	$3\frac{1}{2}$	$10\frac{1}{8}$	$6\frac{3}{4}$	$7\frac{1}{4}$	$4\frac{1}{4}$	$1\frac{11}{16}$	10
$1\frac{1}{4}$	$3\frac{7}{8}$	$10\frac{1}{8}$	$6\frac{3}{4}$	$7\frac{1}{4}$	$4\frac{1}{4}$	$1\frac{11}{16}$	10
$1\frac{3}{8}$	$4\frac{3}{8}$	$10\frac{7}{8}$	$7\frac{1}{8}$	$7\frac{3}{4}$	$4\frac{1}{2}$	$1\frac{15}{16}$	14
$1\frac{1}{2}$	$4\frac{3}{4}$	$12\frac{1}{4}$	8	9	5	$2\frac{3}{16}$	20
$1\frac{5}{8}$	$5\frac{1}{8}$	$12\frac{3}{4}$	8	$9\frac{1}{4}$	5	$2\frac{7}{16}$	28
$1\frac{3}{4}$	$5\frac{1}{2}$	$12\frac{3}{4}$	8	$9\frac{1}{4}$	5	$2\frac{7}{16}$	28
$1\frac{7}{8}$	$5\frac{7}{8}$	$14\frac{3}{4}$	$9\frac{1}{2}$	$10\frac{3}{4}$	6	$2\frac{11}{16}$	38
2	$6\frac{1}{4}$	$14\frac{3}{4}$	$9\frac{1}{2}$	$10\frac{3}{4}$	6	$2\frac{11}{16}$	38
$2\frac{1}{8}$	$6\frac{5}{8}$	$17\frac{1}{8}$	11	$12\frac{1}{2}$	7	$3\frac{1}{8}$	55
$2\frac{1}{4}$	$7\frac{1}{8}$	$17\frac{1}{8}$	11	$12\frac{1}{2}$	7	$3\frac{1}{8}$	55

AMERICAN TIGER BRAND

TIGER WIRE ROPE HOOK



Dimensions of Hooks

Hook Number	Approx. Strength In Pounds	Length A	Width B	Inside Dia. of Eye C	Opening D	Depth E	Approx. Weight Pounds
10	5,700	$4\frac{15}{16}$	$3\frac{3}{16}$	$\frac{7}{8}$	$1\frac{1}{16}$	$\frac{27}{32}$	0.75
15	7,500	$5\frac{13}{32}$	$3\frac{15}{32}$	1	$1\frac{1}{8}$	$\frac{29}{32}$	1.00
20	14,200	$6\frac{1}{4}$	$4\frac{3}{32}$	$1\frac{1}{8}$	$1\frac{1}{4}$	$1\frac{1}{8}$	1.50
30	17,500	$6\frac{7}{8}$	$4\frac{17}{32}$	$1\frac{1}{4}$	$1\frac{3}{8}$	$1\frac{5}{16}$	2.25
40	21,200	$7\frac{5}{8}$	$4\frac{7}{8}$	$1\frac{3}{8}$	$1\frac{1}{2}$	$1\frac{3}{8}$	3.00
50	25,000	$8\frac{19}{32}$	$5\frac{3}{4}$	$1\frac{1}{2}$	$1\frac{11}{16}$	$1\frac{9}{16}$	4.75
60	32,500	$9\frac{1}{2}$	$6\frac{3}{8}$	$1\frac{5}{8}$	$1\frac{7}{8}$	$1\frac{11}{16}$	6.00
75	42,500	$10\frac{11}{32}$	7	$1\frac{3}{4}$	$2\frac{1}{16}$	$1\frac{15}{16}$	8.50
90	47,500	$11\frac{27}{32}$	$7\frac{7}{16}$	2	$2\frac{1}{4}$	$2\frac{1}{8}$	11.25
120	65,000	$13\frac{9}{32}$	$8\frac{7}{16}$	$2\frac{3}{8}$	$2\frac{1}{2}$	$2\frac{15}{32}$	16.00
150	80,000	$14\frac{13}{16}$	$9\frac{3}{8}$	$2\frac{3}{4}$	3	$2\frac{11}{16}$	20.25
200	100,000	$15\frac{1}{2}$	$10\frac{1}{16}$	$2\frac{3}{4}$	$3\frac{1}{4}$	$3\frac{1}{8}$	27.00
240	120,000	$16\frac{1}{2}$	11	3	$3\frac{1}{2}$	$3\frac{3}{8}$	33.50
280	140,000	18	$12\frac{1}{8}$	$3\frac{1}{4}$	$3\frac{3}{4}$	$3\frac{3}{4}$	42.00
340	170,000	$19\frac{3}{4}$	$13\frac{3}{8}$	$3\frac{1}{2}$	4	4	50.00
400	200,000	$21\frac{1}{2}$	$14\frac{1}{2}$	$3\frac{3}{4}$	$4\frac{1}{4}$	$4\frac{3}{8}$	65.00
440	220,000	23	$15\frac{7}{8}$	4	$4\frac{1}{2}$	$4\frac{3}{4}$	82.00
500	250,000	$24\frac{1}{2}$	$16\frac{1}{2}$	$4\frac{1}{4}$	$4\frac{3}{4}$	$4\frac{7}{8}$	105.00
600	300,000	26	$18\frac{1}{2}$	$4\frac{1}{2}$	5	$5\frac{5}{8}$	130.00

See page 82 for size of hook to use with the various sizes and grades of wire rope.

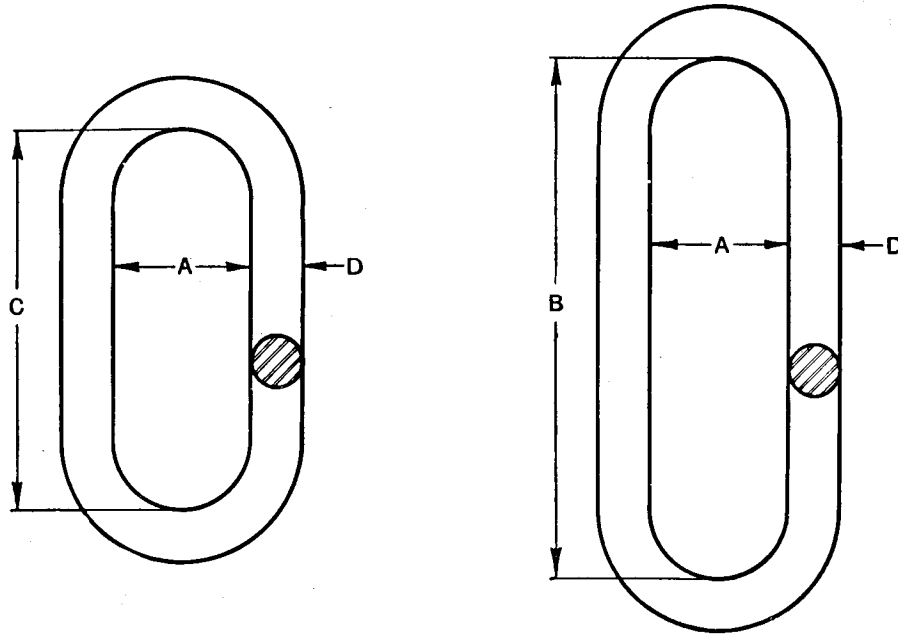
TIGER WIRE ROPE LINKS

The long link is the standard link used for wire rope fittings.

The short link is used to connect other fittings, as the long link to one end and hook to other

end of double pattern switch ropes.

See page 82 for sizes of links to use with the various sizes and grades of wire rope.



Link Number	Approx. Strength in Pounds	Long and Short Links		Long Links		Short Links	
		Inside Width A	Dia of Stock D	Inside Length B	Approx. Weight Pounds	Inside Length C	Approx. Weight Pounds
20	10,000	1	$\frac{3}{8}$	4	.3	3	.25
30	15,000	$1\frac{1}{4}$	$\frac{1}{2}$	6	1.0	4	.7
50	25,000	$1\frac{1}{2}$	$\frac{5}{8}$	8	2.0	4	1.0
75	37,500	$1\frac{3}{4}$	$\frac{3}{4}$	9	3.0	5	2.0
90	45,000	2	$\frac{7}{8}$	9	4.0	5	2.4
120	60,000	$2\frac{1}{4}$	1	10	6.0	6	4.0
150	75,000	$2\frac{1}{2}$	$1\frac{1}{8}$	10	8.0	6	5.5
200	100,000	$2\frac{7}{8}$	$1\frac{1}{4}$	10	10.0	6	7.0
240	120,000	$3\frac{1}{8}$	$1\frac{3}{8}$	11	12.5	8	10.0
280	140,000	$3\frac{5}{8}$	$1\frac{1}{2}$	11	16.0	8	13.0
340	170,000	$3\frac{7}{8}$	$1\frac{5}{8}$	12	20.0	8	15.0
400	200,000	4	$1\frac{3}{4}$	12	24.0	9	20.0
440	220,000	$4\frac{1}{4}$	$1\frac{7}{8}$	12	28.0	9	22.0
500	250,000	$4\frac{1}{2}$	2	12	32.0	9	26.5
600	300,000	5	$2\frac{1}{4}$	14	45.0	10	36.0
850	425,000	$5\frac{1}{2}$	$2\frac{1}{2}$	14	59.0	10	48.0

AMERICAN TIGER BRAND

SIZES OF HOOKS AND LINKS

For All Bright 6 Strand and 18 x 7 Non-Spinning Ropes



Link and Hook attached with Thimble Splices



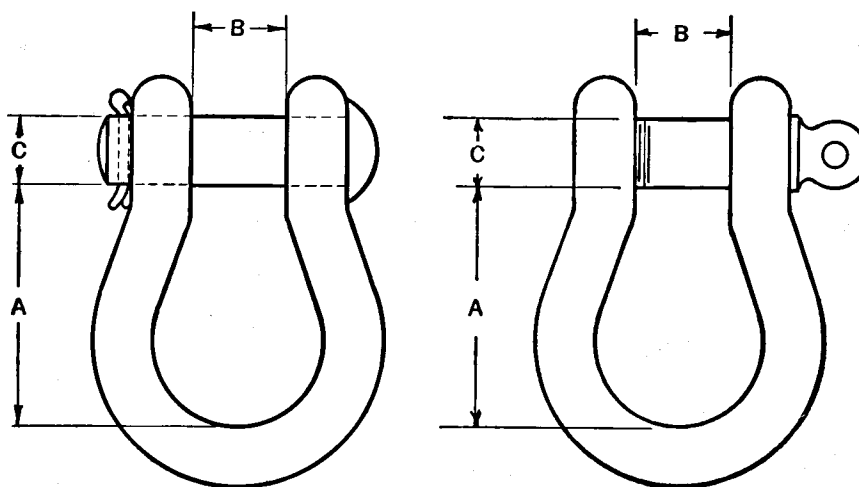
Link and Hook attached with Sockets

Rope Diameter Inches	When Attached to Rope with Socket			When Attached to Rope with Thimble Splice		
	Monitor Steel Rope	Plow Steel Rope	Mild Plow Steel Rope	Monitor Steel Rope	Plow Steel Rope	Mild Plow Steel Rope
$\frac{1}{4}$	15*	10*	10*	10*	10*	10*
$\frac{5}{16}$	20	15*	15*	20	15*	15*
$\frac{3}{8}$	30	20	20	20	20	20
$\frac{7}{16}$	40*	30	30	30	30	20
$\frac{1}{2}$	50	40*	40*	40*	40*	30
$\frac{9}{16}$	60*	50	50	50	50	40*
$\frac{5}{8}$	75	60*	60*	60*	60*	50
$\frac{3}{4}$	120	90	75	90	75	60*
$\frac{7}{8}$	150	120	120	120	120	90
1	200	150	150	150	150	120
$1\frac{1}{8}$	240	200	150	200	150	120
$1\frac{1}{4}$	280	240	200	240	200	150
$1\frac{3}{8}$	340	280	240	280	240	200
$1\frac{1}{2}$	400	340	280	340	280	240
$1\frac{5}{8}$	440	400	340	340	340	280
$1\frac{3}{4}$	500	440	400	400	340	340
$1\frac{7}{8}$	600	500	440	440	400	340
2	600 Sp.	600	500	500	440	400
$2\frac{1}{8}$	...	600 Sp.	...	600	500	...
$2\frac{1}{4}$	...	...	...	600 Sp.	600	...

*Use next larger size Link.

ANCHOR SHACKLES

Galvanized or Japanned



Size	Length of Opening A	Distance Between Eyes B	Diameter of Pin C	APPROXIMATE STRENGTH IN TONS OF 2,000 LBS.		Approx. Weight Pounds
				Round Pin	Screw Pin	
$\frac{1}{4}$	$1\frac{1}{8}$	$\frac{1}{2}$	$\frac{5}{16}$	2.6	2.2	.12
$\frac{5}{16}$	$1\frac{1}{4}$	$\frac{9}{16}$	$\frac{3}{8}$	4.0	3.4	.20
$\frac{3}{8}$	$1\frac{1}{2}$	$\frac{11}{16}$	$\frac{7}{16}$	5.2	4.4	.30
$\frac{7}{16}$	$1\frac{3}{4}$	$\frac{11}{16}$	$\frac{1}{2}$	7.8	6.6	.49
$\frac{1}{2}$	2	$\frac{7}{8}$	$\frac{5}{8}$	10.2	8.6	.75
$\frac{5}{8}$	$2\frac{3}{8}$	$1\frac{1}{16}$	$\frac{3}{4}$	17.2	14.5	1.45
$\frac{3}{4}$	$2\frac{7}{8}$	$1\frac{1}{4}$	$\frac{7}{8}$	23.2	19.5	2.2
$\frac{7}{8}$	$3\frac{1}{4}$	$1\frac{3}{8}$	1	31.3	26.5	3.4
1	$3\frac{5}{8}$	$1\frac{11}{16}$	$1\frac{1}{8}$	38.3	32.5	5.0
$1\frac{1}{8}$	$4\frac{1}{4}$	$1\frac{7}{8}$	$1\frac{1}{4}$	45.0	38.0	7.4
$1\frac{1}{4}$	$4\frac{3}{4}$	$1\frac{7}{8}$	$1\frac{3}{8}$	56.0	47.5	10.0
$1\frac{3}{8}$	$5\frac{1}{4}$	$2\frac{1}{4}$	$1\frac{1}{2}$	68.5	58.0	12.8
$1\frac{1}{2}$	$5\frac{1}{2}$	$2\frac{1}{4}$	$1\frac{5}{8}$	81.5	69.0	16.0
$1\frac{3}{4}$	7	$2\frac{7}{8}$	2	125.	106.	29.4
2	$7\frac{3}{4}$	$3\frac{1}{8}$	$2\frac{1}{4}$	151.	128.	41.6

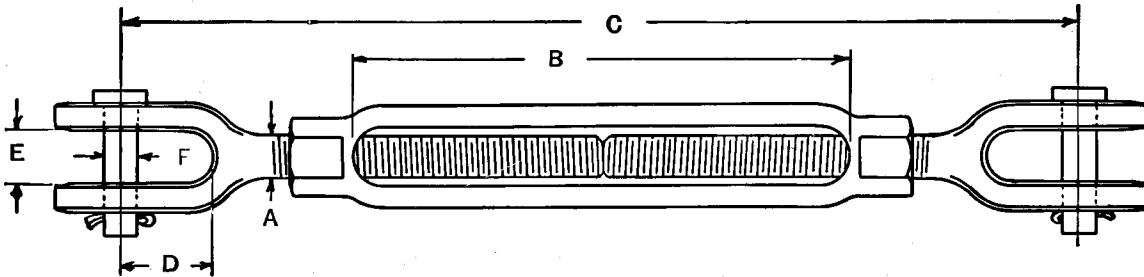
Inquiries and orders should state whether Round Pins or Screw Pins are desired.

AMERICAN TIGER BRAND

SHACKLE END TURNBUCKLES

Black or Galvanized

The dimensions given are for turnbuckles with standard lengths of takeup. For turnbuckles with other than Standard takeup see page 86.



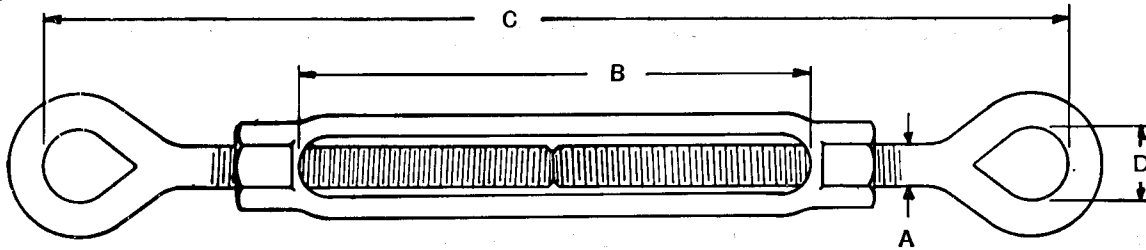
Dimensions of Turnbuckles

Size	Takeup	Minimum Length Center to Center of Pins	Depth of Throat	Distance Between Jaws	Diameter of Pin	Approx. Weight Pounds	Approx. Strength Pounds
A	B	C	D	E	F		
$\frac{1}{4}$	4	$7\frac{3}{8}$	$\frac{3}{4}$	$1\frac{13}{32}$	$\frac{1}{4}$	.4	1600
$\frac{5}{16}$	$4\frac{1}{2}$	$8\frac{1}{2}$	1	$1\frac{15}{32}$	$\frac{1}{4}$	.6	2700
$\frac{3}{8}$	6	$10\frac{3}{4}$	$1\frac{1}{32}$	$1\frac{1}{2}$	$\frac{5}{16}$	1.1	4000
$\frac{1}{2}$	6	12	$1\frac{1}{4}$	$\frac{5}{8}$	$\frac{3}{8}$	1.8	7500
$\frac{5}{8}$	6	$13\frac{1}{4}$	$1\frac{9}{16}$	$\frac{3}{4}$	$\frac{1}{2}$	3.1	12000
$\frac{3}{4}$	9	18	$1\frac{13}{16}$	$1\frac{15}{16}$	$\frac{5}{8}$	5.9	18000
$\frac{7}{8}$	12	$22\frac{1}{4}$	$2\frac{1}{8}$	$1\frac{1}{8}$	$\frac{3}{4}$	10	25000
1	12	24	$2\frac{1}{2}$	$1\frac{13}{16}$	$\frac{7}{8}$	13	33000
$1\frac{1}{4}$	12	$26\frac{3}{4}$	$3\frac{3}{8}$	$1\frac{3}{4}$	$1\frac{1}{8}$	23	53000
$1\frac{1}{2}$	18	$34\frac{7}{8}$	$3\frac{1}{2}$	$2\frac{1}{16}$	$1\frac{3}{8}$	44	78000
$1\frac{3}{4}$	18	38	$4\frac{3}{16}$	$2\frac{3}{8}$	$1\frac{5}{8}$	56	105000
2	24	47	$4\frac{11}{16}$	$2\frac{1}{2}$	$1\frac{7}{8}$	89	138000
$2\frac{1}{2}$	24	58	$5\frac{9}{16}$	$2\frac{7}{8}$	$2\frac{1}{4}$	160	223000

EYE END TURNBUCKLES

Black or Galvanized

The dimensions given are for turnbuckles with standard lengths of takeup. For turnbuckles with other than standard takeup see page 86.



Dimensions of Turnbuckles

Size	Takeup	Minimum Length Pull to Pull of Eyes	Width of Opening	Approx. Weight Pounds	Approx. Strength Pounds
A	B	C	D		
$\frac{1}{4}$	4	$7\frac{3}{8}$	$\frac{11}{32}$	.3	1600
$\frac{5}{16}$	$4\frac{1}{2}$	$8\frac{1}{2}$	$\frac{7}{16}$	.5	2700
$\frac{3}{8}$	6	$10\frac{3}{4}$	$\frac{17}{32}$	.9	4000
$\frac{1}{2}$	6	12	$\frac{23}{32}$	1.6	7500
$\frac{5}{8}$	6	$13\frac{1}{4}$	$\frac{7}{8}$	2.7	12000
$\frac{3}{4}$	9	18	1	5.1	18000
$\frac{7}{8}$	12	$22\frac{1}{4}$	$1\frac{1}{4}$	9.	25000
1	12	24	$1\frac{1}{16}$	11	33000
$1\frac{1}{4}$	12	$26\frac{3}{4}$	$1\frac{13}{16}$	20	53000
$1\frac{1}{2}$	18	$34\frac{7}{8}$	$2\frac{1}{8}$	40	78000
$1\frac{3}{4}$	18	38	$2\frac{3}{8}$	49	105000
2	24	47	$2\frac{11}{16}$	78	138000
$2\frac{1}{2}$	24	58	$3\frac{1}{8}$	140	223000

AMERICAN TIGER BRAND

TURNBUCKLES

The dimensions of turnbuckles given on the two preceding pages apply to turnbuckles with standard lengths of takeup as shown in columns marked "B."

The table shown below gives the various lengths of takeup of turnbuckles which can be supplied. Sizes shown in heavy type have standard lengths of takeup.

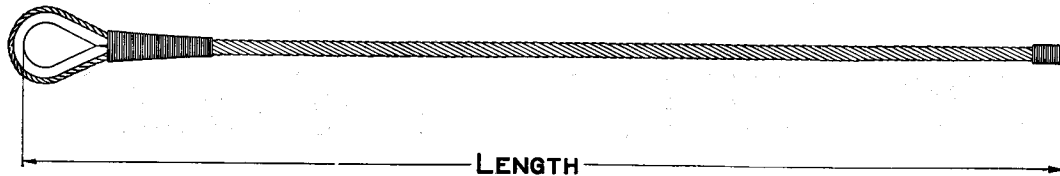
Lengths of Takeup in Inches

	4	4 1/2	6	9	12	18	24	36	48
Size of Turnbuckles	1/4								
		5/16							
			3/8						
			1/2	1/2	1/2				
			5/8	5/8	5/8	5/8			
			3/4	3/4	3/4	3/4	3/4		
					7/8	7/8	7/8		
					1	1	1	1	
					1 1/4	1 1/4	1 1/4	1 1/4	
					1 1/2	1 1/2	1 1/2	1 1/2	1 1/2
						1 3/4	1 3/4	1 3/4	1 3/4
						2	2	2	2
						2 1/2	2 1/2	2 1/2	2 1/2

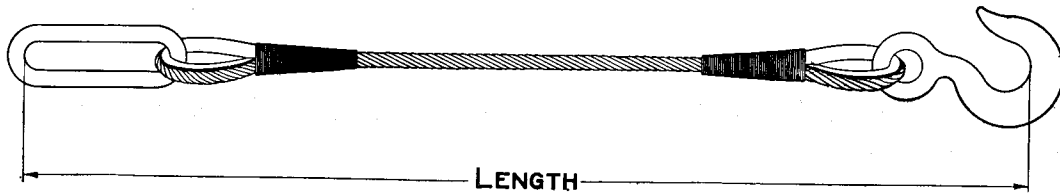
WIRE ROPE ENGINEERING HANDBOOK

When ORDERING WIRE ROPE WITH FITTINGS ATTACHED the lengths shown should be given. For combinations of fittings not illus-

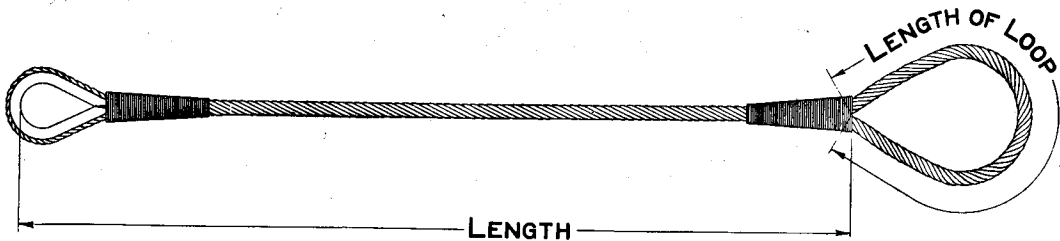
trated, the same methods of measuring should be followed.



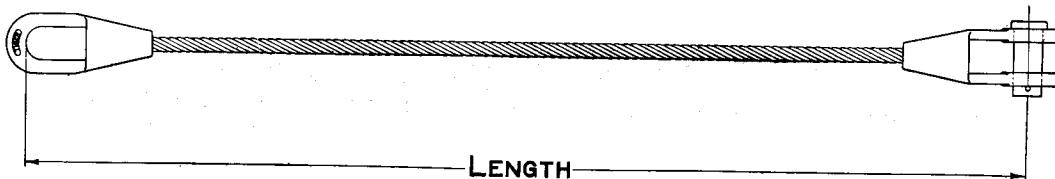
Thimble spliced in one end. Measurement: Pull of Thimble to End of Rope.



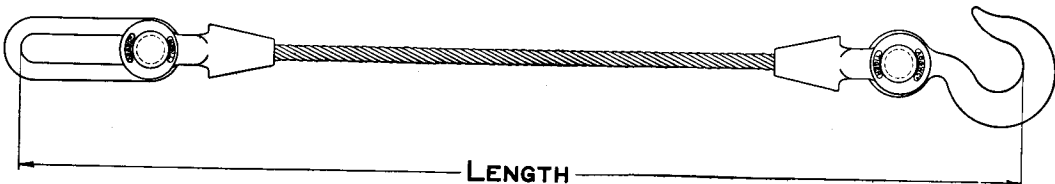
Link spliced in one end; Hook spliced in other end. Measurement: Pull of Link to Pull of Hook.



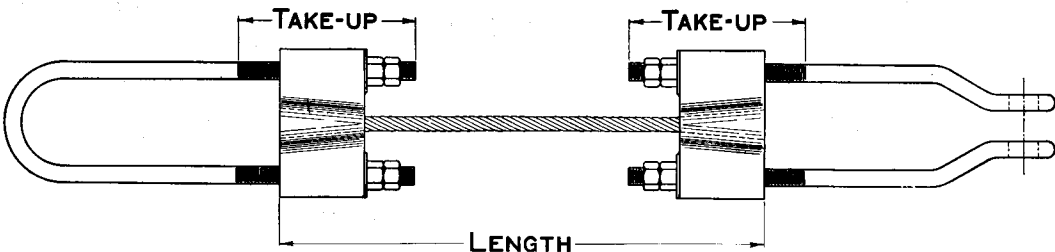
Thimble spliced in one end; Loop spliced in the other end. Measurements: Pull of Thimble to Base of Loop, and Circumference of Loop.



Tiger Closed Wire Rope Socket attached to one end; Tiger Open Wire Rope Socket attached to other end. Measurement: Pull of Closed Socket to Center Line of Pin of Open Socket.



Tiger Open Wire Rope Socket and Link attached to one end; Tiger Open Wire Rope Socket and Hook attached to other end. Measurement: Pull of Link to Pull of Hook.



Closed Bridge Socket attached to one end; Open Bridge Socket attached to other end. Measurements: Outer face of Socket Casting to Outer Face of Socket Casting, and Takeups required.

WIRE ROPE SLINGS

Properly designed and fabricated wire rope slings are the safest type of slings. They do not wear away as do slings made of fiber rope, nor do they lose their strength from exposure in the same rapid manner. Neither are they susceptible to the weakest link ailment of chains caused by the uncertainty of the strengths of the welds. Wire rope slings show by inspection their true condition. The appearance of broken wires clearly

indicates the fatigue of the metal, and the termination of the useful life of the sling.

There are six general types of wire rope slings. Each general type consists of numerous slings, each differing from the other in the fittings employed. The six general types and some of the numerous variations in which each general type may be furnished, are listed below:

TIGER EQUALIZING SLINGS

	TYPE
Two Part.....	2P-66-E
Two Part with Protective Saddles.....	2P-67-EP
Four Part.....	4P-68-E

TIGER SLIPNOOSE SLINGS

With Thimbles and Closed Sockets.....	S-71-TS
With Thimbles.....	S-72-T
With Thimbles, Open Sockets, and Hooks.....	S-73-TSH
With Loop Ends.....	S-75-L
Endless Grommet Type.....	S-76-G

TIGER BRIDLE SLINGS

With Spliced Loops.....	B-77-L
With Hooks.....	B-80-H
With Open Sockets and Hooks.....	B-81-SH
With Open Sockets.....	B-86-S
With Shackles.....	B-88-C
With Horizontal Plate Clamps.....	B-94-HP
With Vertical Plate Clamps.....	B-95-VP

TIGER DOUBLE BRIDLE SLINGS

With Loops (for Balanced Loads).....	DB-78-Lb
With Loops (for Unbalanced Loads).....	DB-79-Lu
With Open Sockets and Hooks.....	DB-82-SH

	TYPE
With Hooks and Rope Thimbles.....	DB-83-O
With Open Sockets.....	DB-87-S
With Closed Sockets and Shackles.....	DB-89-SC
With Thimbles and Shackles (for Balanced Loads).....	DB-90-TCb
With Thimbles and Shackles (for Unbalanced Loads).....	DB-91-TCu
With Thimbles (for Balanced Loads).....	DB-92-Tb
With Thimbles (for Unbalanced Loads).....	DB-93-Tu

TIGER SPECIAL BRIDLE SLINGS

Four End Bridle Slings with Hooks.....	4E-96-H
Three End Bridle Slings with Hooks.....	3E-00-H
Two End Bridle Slings with Round Ring and Open Sockets and Hooks.....	2E-01-SH

TIGER RAILROAD SLINGS

Wrecker Slings for Lifting Locomotive at Fire Door.....	W-05-LFD
Single Pattern Wrecking and Switching Ropes.....	W-07-SP
Double Pattern Wrecking and Switching Ropes.....	W-07-DP
Multiple Part Wire Rope Slings.....	M-08-P
Wrecker Slings for Lifting Box Cars.....	2EW-09-FC

Examples of commonly used types of slings are shown on the following pages. Details of these, and other types, and the fittings used, are shown in our Tiger Wire Rope Sling Catalogue.

Equalizing Slings, as well as some types of Slip-noose Slings and Double Bridle Slings, are made with grommets where endless slings are desired. Grommets are made from one continuous length

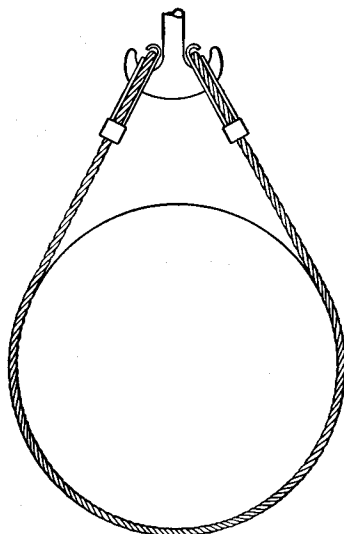
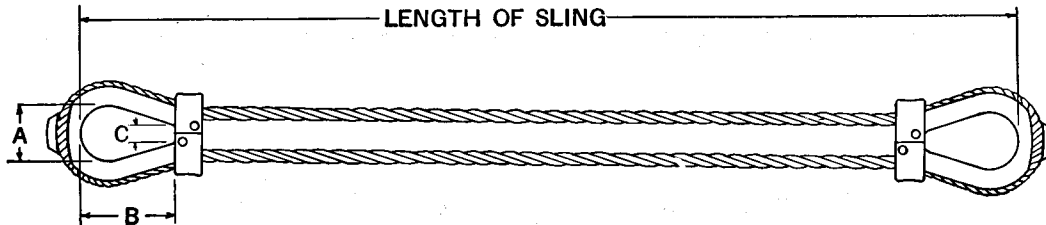
of strand. The advantage of a grommet over a length of rope spliced endless is that a grommet has but one point of tuck whereas the spliced six-strand rope has six points of tuck. This permits tucking the strand ends the entire length of the grommet, as compared to the much shorter length in the spliced rope.

TIGER TWO PART EQUALIZING SLINGS

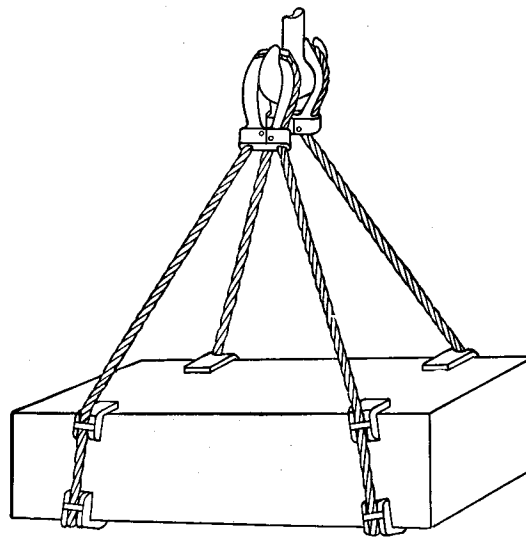
Monitor Steel

Tiger Two Part Equalizing Slings consist of one endless wire rope grommet and two equalizing thimbles. When used for lifting rectangular bodies or bodies with corner projections, protective saddles should be specified to protect the rope at the points it passes around the corners. Type 2P-67-

EP slings with four top protective saddles and four bottom protective saddles are especially useful in quarries and finishing mills for handling blocks of marble and granite. This type sling can be furnished with any form of protective saddles required.



Type 2P-66-E



Type 2P-67-EP

Safe Working Loads when used Doubled and Dimensions and Weights of Thimbles

Rope Diam.	SAFE LOADS IN TONS OF 2,000 POUNDS				EQUALIZING THIMBLES			Approx. Weight of Two Thimbles
	When Vertical	When Included Angle is 60°	When Included Angle is 90°	When Included Angle is 120°	Inside Width	Inside Length	Inside Width	
					A	B	C	
3/4	12	10	8	6	4	6	1 3/8	22
7/8	16	14	11	8	5	8	1 1/2	33
1	21	18	15	11	5	8	1 1/2	39
1 1/8	27	23	19	13	5 1/2	9	1 1/2	56
1 1/4	33	28	23	16	5 1/2	9	1 3/4	69
1 3/8	39	34	28	20	6	10	2	86
1 1/2	46	40	33	23	6	10	2	107
1 5/8	54	47	38	27	8 1/2	14	2 1/4	156
1 3/4	62	54	44	31	9	15	2 1/2	199
1 7/8	71	62	50	36	9 1/2	15	3	206
2	81	70	57	40	10	15	3	257
2 1/8	91	78	64	45	10	15 1/2	3	338
2 1/4	101	87	71	51	10	16	3	363

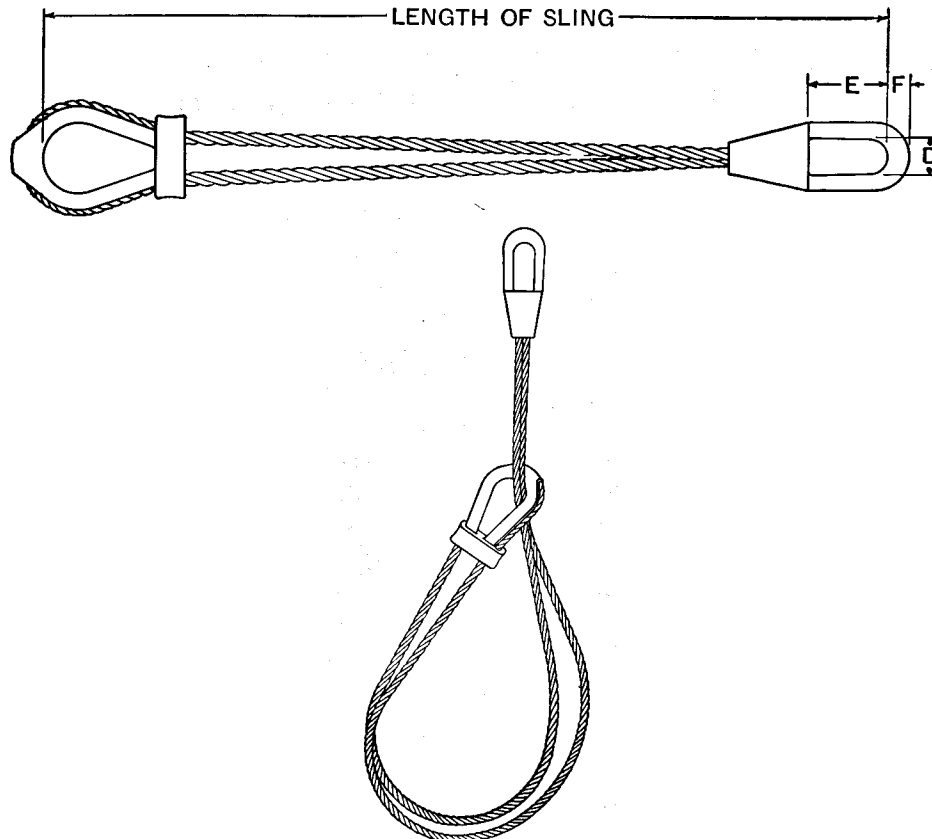
AMERICAN TIGER BRAND

TIGER SLIPNOOSE SLINGS WITH THIMBLE AND CLOSED SOCKET

Monitor Steel

An economical and practical slipnoose sling for use where the opening of the Closed Socket is large enough to engage the crane hook. When this

opening is not sufficient, Type S-72-T, Slipnoose Sling with Thimbles is recommended.



TYPE S-71-TS

Safe Working Loads, Dimensions of Sockets, and Weights of Fittings

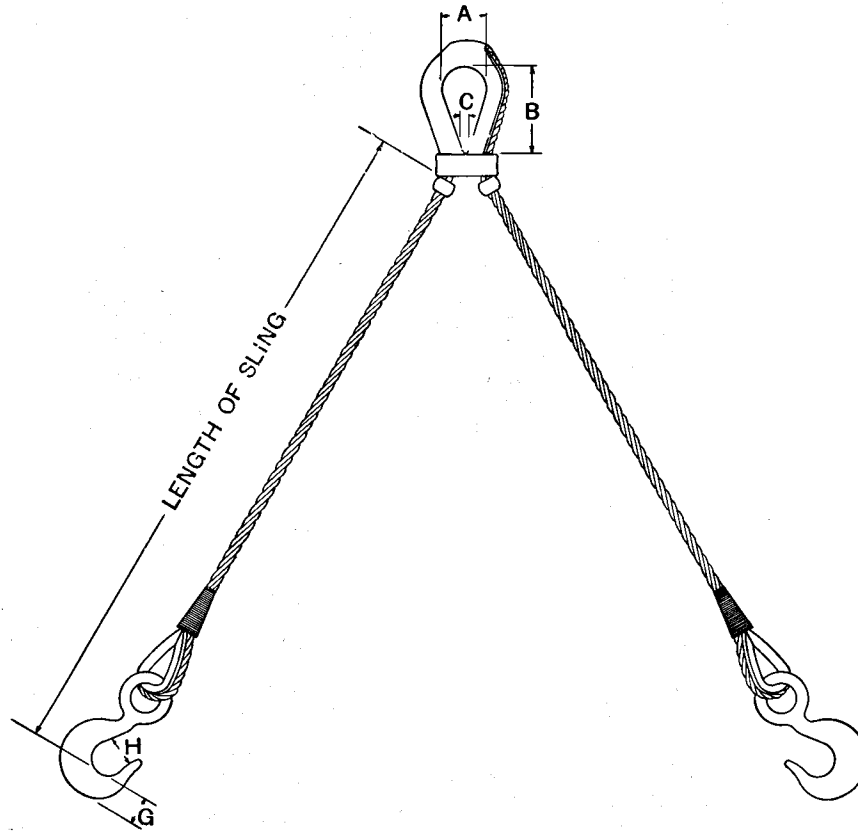
Rope Diam.	Safe Loads in Tons of 2,000 Pounds	TIGER CLOSED SOCKETS			Approx. Weight of Fittings
		Inside Width D	Inside Length E	Thickness of Loop—F	
$\frac{3}{8}$	1.6	$1\frac{3}{8}$	$2\frac{9}{16}$	$1\frac{3}{16}$	7
$\frac{1}{2}$	2.7	$1\frac{5}{8}$	$3\frac{1}{16}$	$1\frac{1}{16}$	10
$\frac{5}{8}$	4.2	$2\frac{1}{4}$	$4\frac{1}{8}$	$1\frac{3}{8}$	22
$\frac{3}{4}$	6	$2\frac{1}{2}$	$4\frac{5}{8}$	$1\frac{1}{2}$	31
$\frac{7}{8}$	8	$2\frac{3}{4}$	$5\frac{3}{16}$	$1\frac{5}{8}$	45
1	11	$3\frac{1}{8}$	$6\frac{3}{16}$	$1\frac{15}{16}$	57
$1\frac{1}{8}$	13	$3\frac{17}{32}$	$7\frac{13}{16}$	$2\frac{3}{16}$	95
$1\frac{1}{4}$	16	$3\frac{17}{32}$	$7\frac{13}{16}$	$2\frac{3}{16}$	105
$1\frac{3}{8}$	20	$3\frac{25}{32}$	$8\frac{13}{16}$	$2\frac{7}{16}$	143
$1\frac{1}{2}$	23	$4\frac{9}{32}$	$9\frac{3}{4}$	$2\frac{7}{8}$	187

Weight of zinc for attaching sockets is included in approximate weight of fittings.

TIGER BRIDLE SLINGS WITH HOOKS

Monitor Steel

A handy sling for many purposes. For slings of larger capacities, see Type DB-82-SH.



Type B-80-H

Safe Working Loads and Dimensions and Weights of Fittings

Rope Dia.	Safe Loads in Tons of 2,000 Lbs.			Equalizing Thimbles			Wire Rope Hooks			Approx. Weight of Fittings
	When Included Angle is 30°	When Included Angle is 60°	When Included Angle is 90°	Inside Width	Inside Length	Inside Width	Hook No.	Depth	Opening	
				A	B	C		G	H	
$\frac{3}{8}$	1.3	1.2	1.	$2\frac{1}{2}$	4	1	20	$1\frac{1}{8}$	$1\frac{1}{4}$	7
$\frac{1}{2}$	2.3	2.	1.7	$3\frac{3}{4}$	$5\frac{1}{2}$	$1\frac{1}{8}$	40	$1\frac{3}{8}$	$1\frac{1}{2}$	12
$\frac{5}{8}$	3.5	3.1	2.6	4	6	$1\frac{1}{4}$	60	$1\frac{11}{16}$	$1\frac{7}{8}$	21
$\frac{3}{4}$	4.9	4.4	3.6	4	6	$1\frac{3}{8}$	90	$2\frac{1}{8}$	$2\frac{1}{4}$	34
$\frac{7}{8}$	6.5	5.9	4.8	5	8	$1\frac{1}{2}$	120	$2\frac{15}{32}$	$2\frac{1}{2}$	50
1	8.5	7.5	6.1	5	8	$1\frac{1}{2}$	150	$2\frac{11}{16}$	3	62
$1\frac{1}{8}$	10.5	9.3	7.6	$5\frac{1}{2}$	9	$1\frac{1}{2}$	200	$3\frac{1}{8}$	$3\frac{1}{4}$	88
$1\frac{1}{4}$	12.	11.	9.	$5\frac{1}{2}$	9	$1\frac{3}{4}$	240	$3\frac{3}{8}$	$3\frac{1}{2}$	107
$1\frac{3}{8}$	14.	13.	10.5	6	10	2	280	$3\frac{3}{4}$	$3\frac{3}{4}$	137
$1\frac{1}{2}$	17.	15.	12.	6	10	2	340	4	4	163
$1\frac{5}{8}$	19.	17.	14.	$8\frac{1}{2}$	14	$2\frac{1}{4}$	340	4	4	225
$1\frac{3}{4}$	22.	20.	16.	9	15	$2\frac{1}{2}$	400	$4\frac{3}{8}$	$4\frac{1}{4}$	260

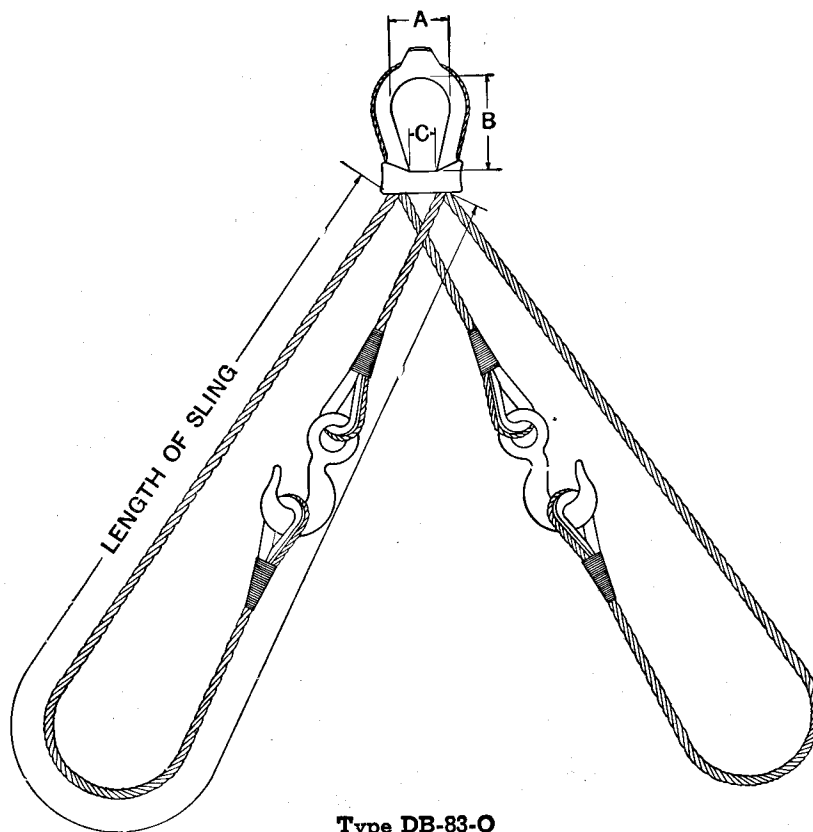
AMERICAN TIGER BRAND

TIGER DOUBLE BRIDLE SLINGS WITH HOOKS AND ROPE THIMBLES

Monitor Steel

The Hooks and Thimbles permit the opening of the legs so that the ends may be passed under the ends of housings, rolls, etc. This sling is de-

signed for use in steel and paper mills, and can also be used to advantage in other fields.



Type DB-83-O

Safe Working Loads, Dimensions of Equalizing Thimbles, and Weights of Fittings

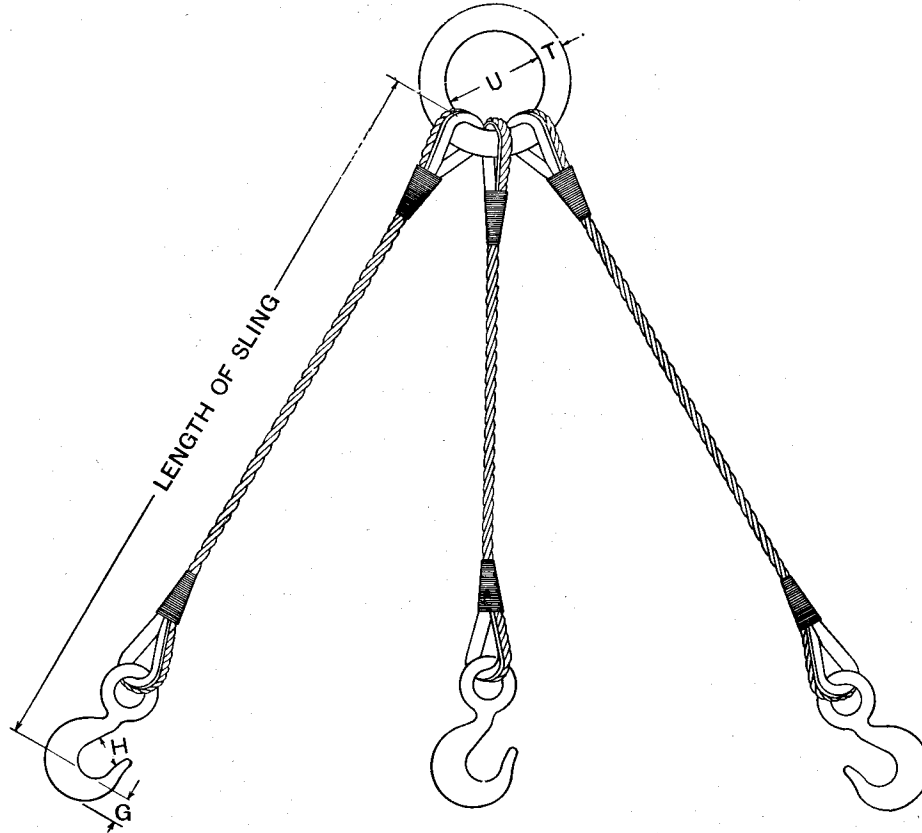
Rope Dia.	Safe Load in Tons of 2,000 Lbs.			Equalizing Thimbles			Approximate Weight of Fittings
	When Included Angle is 30°	When Included Angle is 60°	When Included Angle is 90°	Inside Width	Inside Length	Inside Width	
				A	B	C	
$\frac{3}{8}$	2.6	2.4	2.0	3	$4\frac{3}{4}$	$1\frac{1}{4}$	10.5
$\frac{1}{2}$	4.6	4.1	3.4	$4\frac{1}{2}$	$6\frac{7}{8}$	$1\frac{1}{4}$	19.
$\frac{5}{8}$	7.	6.2	5.2	5	$7\frac{1}{2}$	$1\frac{1}{2}$	32.
$\frac{3}{4}$	9.8	8.8	7.2	5	$7\frac{3}{4}$	$1\frac{1}{2}$	50.
$\frac{7}{8}$	13.	12.	10.	6	9	$2\frac{1}{8}$	71.
1	17.	15.	12.	$6\frac{1}{4}$	$9\frac{1}{2}$	$2\frac{3}{16}$	90.
$1\frac{1}{8}$	21.	19.	15.	7	11	$2\frac{1}{4}$	130.
$1\frac{1}{4}$	24.	22.	18.	$7\frac{1}{2}$	$11\frac{1}{2}$	$2\frac{7}{8}$	158.
$1\frac{3}{8}$	29.	26.	21.	8	$12\frac{1}{2}$	3	206.
$1\frac{1}{2}$	34.	30.	24.	$8\frac{1}{2}$	$13\frac{3}{4}$	3	250.

THREE END BRIDLE SLINGS WITH HOOKS

Monitor Steel

The Three End Bridle Sling with Hooks shown below is the type of Three End Bridle Sling in greatest demand. This type of sling can be furnished with Tiger Open Sockets, Tiger Closed

Sockets, Tiger Open Sockets and Hooks, Shackles, Tiger Closed Sockets and Shackles, Spliced Loops, or any form of wire rope fitting desired.



Type 3E-00-H

Safe Working Loads and Dimensions and Weights of Fittings

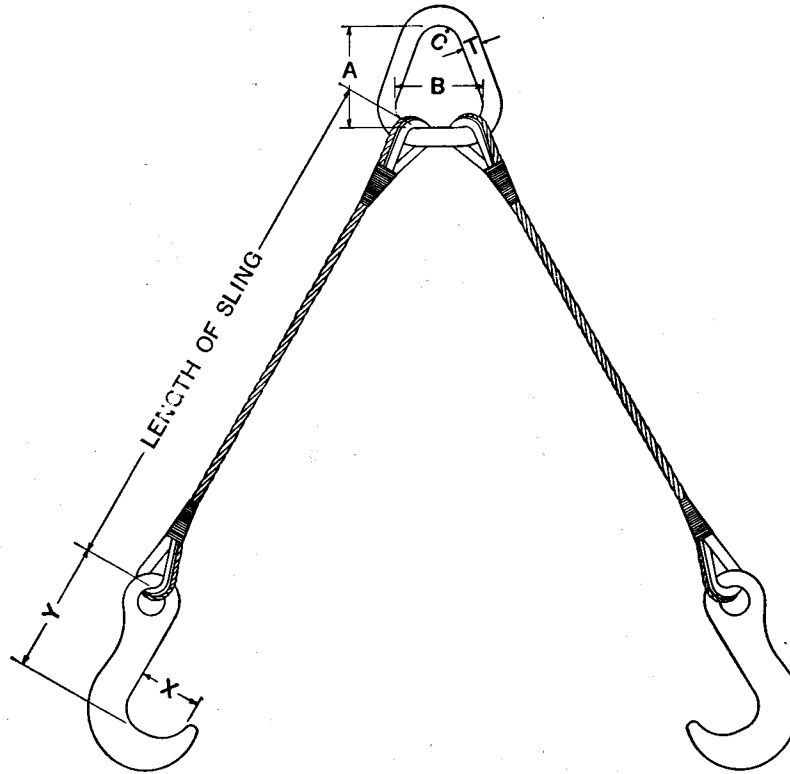
Safe Working Loads and Dimensions									
Rope Dia.	Safe Loads in Tons of 2,000 Lbs.			Hooks			Rings		Approx. Weight of Fittings
	When Included Angle is 30°	When Included Angle is 60°	When Included Angle is 90°	Hook No.	Depth	Opening	Dia. of Stock	Inside Opening	
									
	G	H	T						
3/8	2.	1.8	1.5	20	1 1/8	1 1/4	1 1/4	5	12
1/2	3.4	3.1	2.5	40	1 3/8	1 1/2	1 1/2	6	22
5/8	5.2	4.7	3.8	60	1 11/16	1 7/8	1 3/4	7	38
3/4	7.3	6.6	5.4	90	2 1/8	2 1/4	2	8	64
7/8	9.8	8.8	7.2	120	2 15/16	2 1/2	2 1/2	9	97
1	13.	11.	9.2	150	2 11/16	3	2 3/4	10	130
1 1/8	16.	14.	11.5	200	3 1/8	3 1/4	3	10	170
1 1/4	18.	16.	13.	240	3 3/8	3 1/2	3 1/4	11	200
1 3/8	22.	19.	16.	280	3 3/4	3 3/4	3 1/2	12	275
1 1/2	25.	23.	18.	340	4	4	3 3/4	12	315

TIGER WRECKING SLINGS FOR LIFTING BOX CARS

Monitor Steel

The two legs of this Wrecking Sling for Lifting Box and Caboose Cars are equipped with special forged steel Sill Hooks, and are connected by a

weldless steel Triangular Link for engaging the crane hook.



Type 2EW-09-FC

Safe Working Loads and Dimensions and Weights of Fittings

Rope Dia.	Safe Load in Tons of 2,000 Lbs. When Included Angle is 60° 	Triangular Link				Hooks		Approx. Weight of Fittings
		Inside Length	Inside Width	Radius	Dia. of Stock	Opening	Pull of Hook to Pull of Thimble	
		A	B	C	T	X	Y	
7/8	9.5	9 1/2	8	1 1/2	1 3/4	8	14	92
1	12.	9 1/2	8	1 1/2	2	8	14	135
1 1/8	15.	9 1/2	8	1 1/2	2	8	15	168
1 1/4	18.	9 1/2	8	2	2 1/4	8	16	225
1 1/2	24.	12	8	2 1/2	2 3/4	9	16	341
1 3/4	32.	12	9	3	3	10	18	500
2	40.	12	9	3	3 1/2	10	18	660

TIGER MULTIPLE PART WIRE ROPE SLINGS

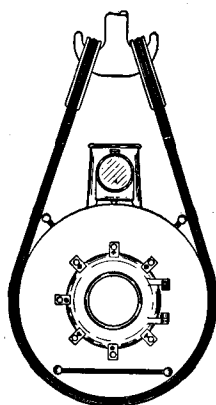
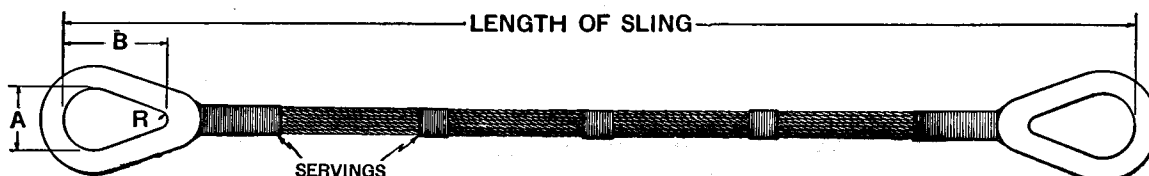
Monitor Steel

Tiger Multiple Part Slings are composed of one continuous length of $\frac{3}{8}$ " diameter Monitor Wire Rope wound around two Cast Steel Thimbles any required number of times and seized at regular intervals. The seizings hold the ropes in one compact cable.

This type of sling is adapted for lifting large circular bodies such as locomotive boilers. It

should not be used for lifting objects with sharp corners, as the multiple parts of rope cannot readily adjust themselves to sharp bends.

For lifting lighter loads than shown below, and for lifting objects which require bending the slings about corners, Tiger Two Part Equalizing Slings, Type 2P-66-E, and Tiger Four Part Equalizing Slings, Type 4P-68-E, are recommended.



Type M-08-P

Safe Working Loads when used Doubled and Dimensions and Weights of Thimbles

Number of Rope Parts	Approx. Diam. of Sling in Inches	SAFE LOADS IN TONS OF 2,000 POUNDS				CAST STEEL THIMBLES			Approx. Weight of Two Thimbles
		When Vertical	When Included Angle is 60°	When Included Angle is 90°	When Included Angle is 120°	Inside Width	Inside Length	Radius	
						A	B	R	
58	$3\frac{1}{8}$	100	87	71	50	$8\frac{1}{2}$	14	$1\frac{1}{8}$	220
64	$3\frac{3}{8}$	110	95	78	55	$8\frac{1}{2}$	14	$1\frac{1}{8}$	220
68	$3\frac{1}{2}$	117	101	83	59	$8\frac{1}{2}$	14	$1\frac{1}{8}$	220
72	$3\frac{5}{8}$	124	108	88	62	9	15	$1\frac{1}{4}$	300
76	$3\frac{3}{4}$	130	113	92	65	9	15	$1\frac{1}{4}$	300
80	$3\frac{7}{8}$	137	119	97	69	9	15	$1\frac{1}{4}$	300
84	$3\frac{15}{16}$	144	125	102	72	9	15	$1\frac{1}{4}$	300
88	4	151	131	107	76	9	15	$1\frac{1}{4}$	300
92	$4\frac{1}{8}$	158	137	112	79	9	15	$1\frac{1}{4}$	300
96	$4\frac{3}{16}$	165	143	117	83	10	16	$1\frac{3}{8}$	400
100	$4\frac{5}{16}$	172	149	122	86	10	16	$1\frac{3}{8}$	400
106	$4\frac{7}{16}$	182	158	129	91	10	16	$1\frac{3}{8}$	400
110	$4\frac{1}{2}$	189	164	134	95	10	16	$1\frac{3}{8}$	500
116	$4\frac{5}{8}$	199	173	141	100	10	16	$1\frac{3}{8}$	500
120	$4\frac{11}{16}$	205	178	145	103	10	16	$1\frac{3}{8}$	500

Dimensions of Thimbles shown are our standards. Thimbles of special dimensions can be furnished.

WIRE ROPE BRAIDED SLINGS



Tiger Brand Braided Slings offer certain advantages over the types outlined on the preceding pages. These advantages are secured by building the sling bodies of several pairs of Right Regular Lay and Left Regular Lay wire ropes.

A Tiger Brand Braided 8-Part Sling is composed of four parts of Right Regular Lay and four parts of Left Regular Lay Monitor Steel rope, so interwoven that opposite pairs are Right Lay and Left Lay, and special fittings. This braided construction gives maximum flexibility, prevents kinking and makes the sling non-rotating.

Some of the types of Tiger Braided Slings are listed below and three types are detailed on the following pages. Our catalog on Tiger Brand Braided Wire Rope Slings gives detailed information on these slings.

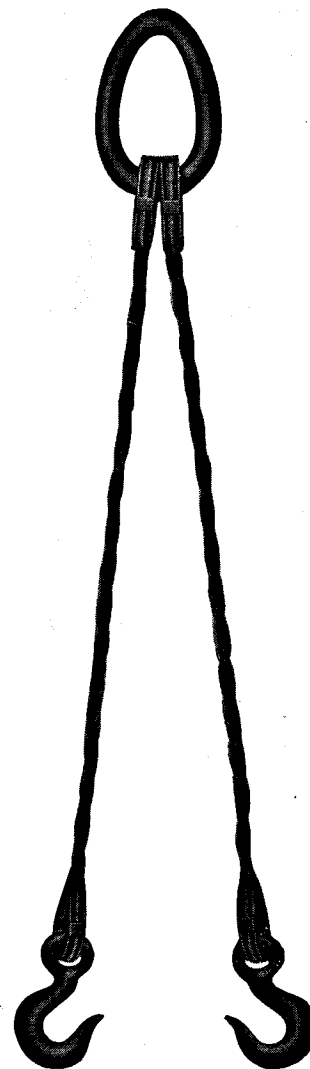
	Type
Braided 8-Part 2-End Bridle Slings with Hooks.....	K2E-20-H
Braided 8-Part 3-End Bridle Slings with Hooks.....	K3E-21-H
Braided 8-Part 4-End Bridle Slings with Hooks.....	K4E-22-H
Braided 8-Part Choker Slings with Thimble and Sliding Hook..	KC-26-TA
Braided 8-Part 2-Leg Choker Slings with Sliding Hooks.....	K2C-27-A
Braided 8-Part Slipnoose Slings with Loops.....	KS-30-L
Braided 8-Part Slipnoose Slings with Thimbles.....	KS-31-T
Braided 8-Part Slipnoose Slings with Loop and Protective Saddle.....	KS-32-LP
Braided 8-Part Slipnoose Slings with Thimble and Protective Saddle.....	KS-33-TP
Braided 16-Part Slipnoose Slings with Loop and Protective Saddle.....	K2S-34-LP
Braided 8-Part Draw Bar Slings with Loops.....	KWB-40-L
Braided 8-Part Draw Bar Slings with Thimbles.....	KWB-41-T
Braided 8-Part Utility Slings with Loop and Hook.....	KU-46-LH

BRAIDED 8-PART 2-END BRIDLE SLINGS WITH HOOKS

Type 2E-20-H

A light weight, flexible sling consisting of two legs with hooks attached by thimbles connected by thimbles to a pear shaped link of high carbon steel. The flexibility of this sling, together with the ease of handling, has made this type a favorite.

Diam. of Rope Used Inches	Safe Load in Tons			Pear Shaped Link				Hooks		
	When Included Angle is 30° 	When Included Angle is 60° 	When Included Angle is 90° 	Inside Length Inches	Radius at Top Inches	Radius at Bottom Inches	Diam. Stock Inches	Hook No.	Depth at Bite Inches	Throat Opening Inches
$\frac{3}{32}$	1	0.9	0.7	$3\frac{3}{4}$	$\frac{5}{8}$	$1\frac{1}{4}$	$\frac{5}{8}$	10	$2\frac{7}{32}$	$1\frac{1}{16}$
$\frac{1}{8}$	1.8	1.6	1.3	$4\frac{1}{2}$	$\frac{3}{4}$	$1\frac{1}{2}$	$\frac{3}{4}$	20	$1\frac{1}{8}$	$1\frac{1}{4}$
$\frac{3}{16}$	4	3.6	2.9	6	1	2	1	40	$1\frac{3}{8}$	$1\frac{1}{2}$
$\frac{1}{4}$	6.6	5.9	4.8	$8\frac{1}{4}$	$1\frac{3}{8}$	$2\frac{3}{4}$	$1\frac{3}{8}$	75	$1\frac{15}{16}$	$2\frac{1}{16}$
$\frac{5}{16}$	9.9	8.9	7.3	$9\frac{3}{4}$	$1\frac{5}{8}$	$3\frac{1}{4}$	$1\frac{5}{8}$	120	$2\frac{15}{32}$	$2\frac{1}{2}$
$\frac{3}{8}$	13	12	9.7	12	2	4	2	150	$2\frac{11}{16}$	3
$\frac{7}{16}$	18	16	13	$13\frac{1}{2}$	$2\frac{1}{4}$	$4\frac{1}{2}$	$2\frac{1}{4}$	200	$3\frac{1}{8}$	$3\frac{1}{4}$
$\frac{1}{2}$	23	21	17	16	$2\frac{5}{8}$	$5\frac{1}{4}$	$2\frac{5}{8}$	240	$3\frac{3}{8}$	$3\frac{1}{2}$
$\frac{9}{16}$	29	26	21	17	$2\frac{7}{8}$	$5\frac{3}{4}$	$2\frac{7}{8}$	340	4	4
$\frac{5}{8}$	36	32	26	$19\frac{1}{2}$	$3\frac{1}{4}$	$6\frac{1}{2}$	$3\frac{1}{4}$	400	$4\frac{3}{8}$	$4\frac{1}{4}$
$\frac{3}{4}$	52	46	38	24	4	8	4	500	$4\frac{7}{8}$	$4\frac{3}{4}$

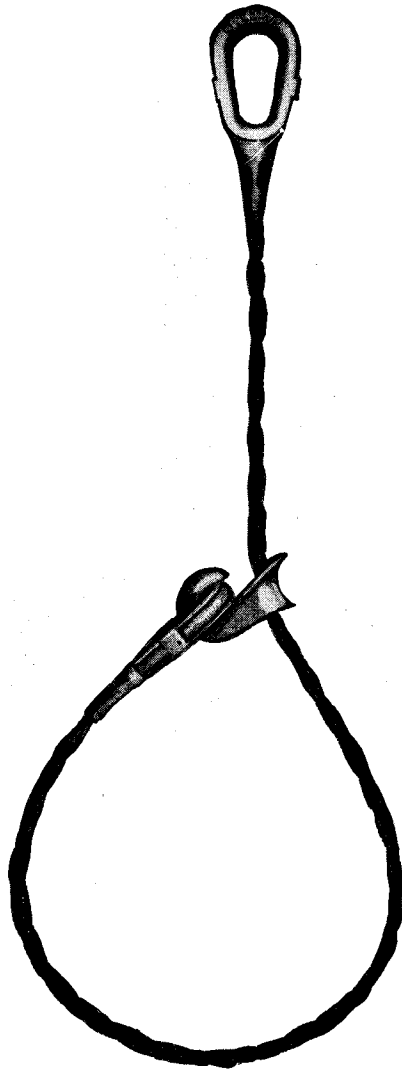


Type 2E-20-H

BRAIDED 8-PART CHOKER SLINGS WITH THIMBLE AND SLIDING HOOK

Type KC-26-TA

This sling is identical to Type KS-31-T except that a sliding hook has been added to provide a quick attachment for the thimble on the free end. The advantage of this Choker Sling over Slipnoose Slings is that it is not necessary to detach the thimble from the crane hook when handling loads.



Type KC-26-TA

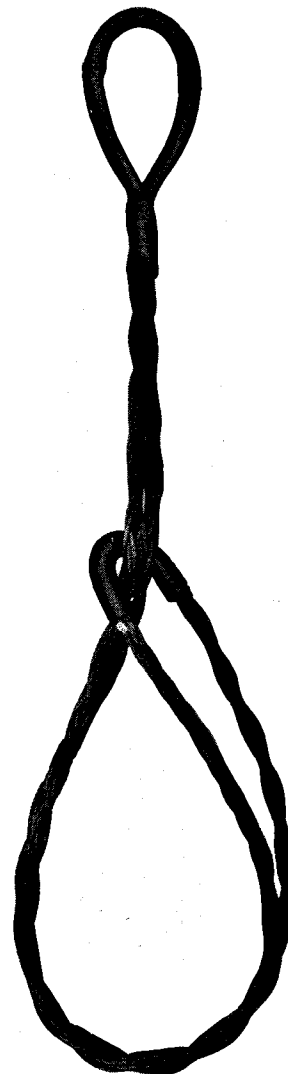
Diameter of Rope Used Inches	Safe Load in Tons Choker	Thimble		Sliding Hook No.
		Inside Width Inches	Inside Length Inches	
$\frac{3}{16}$	1.5	$2\frac{1}{2}$	5	3
$\frac{1}{4}$	2.6	3	$6\frac{1}{4}$	4
$\frac{5}{16}$	3.9	$3\frac{1}{2}$	$7\frac{1}{2}$	5
$\frac{3}{8}$	5.1	$4\frac{1}{2}$	$8\frac{1}{4}$	6
$\frac{7}{16}$	7	$5\frac{1}{2}$	$10\frac{1}{4}$	7
$\frac{1}{2}$	9	$6\frac{1}{4}$	$10\frac{3}{4}$	8
$\frac{9}{16}$	11	$6\frac{3}{4}$	12	9
$\frac{5}{8}$	14	8	$14\frac{3}{4}$	10

BRAIDED 16-PART SLIPNOOSE SLINGS WITH LOOP AND PROTECTIVE SADDLE

Type K2S-34-LP

The body of this sling is a length of 8-Part Braided Rope doubled back upon itself to double the number of parts to 16 and thus doubling its strength. The use of 16-Parts makes this sling extremely flexible. This flexibility, combined with its light weight, makes it very easy to handle.

Diameter of Rope Used Inches	Safe Load in Tons Choker Hitch	Loop	
		Inside Width Inches	Inside Length Inches
$\frac{1}{4}$	5.2	$4\frac{1}{4}$	14
$\frac{5}{16}$	7.8	$4\frac{1}{2}$	14
$\frac{3}{8}$	10	5	18
$\frac{7}{16}$	14	6	22
$\frac{1}{2}$	18	$7\frac{1}{2}$	24
$\frac{9}{16}$	22	8	26
$\frac{5}{8}$	28	10	34
$\frac{3}{4}$	40	10	34
$\frac{7}{8}$	54	14	44
1	70	14	44



Type K2S-34-LP

ATTACHING SOCKETS

The practice here detailed is recommended by the United States Bureau of Mines in Bulletin No. 75. It is the most satisfactory method in use today.

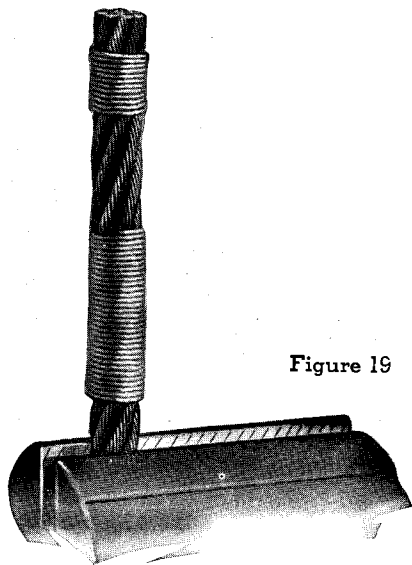


Figure 19

Place an additional seizing on the rope end to be socketed at a distance equal to the length of the basket of the socket from the end of the rope. It is important that this seizing be carefully applied

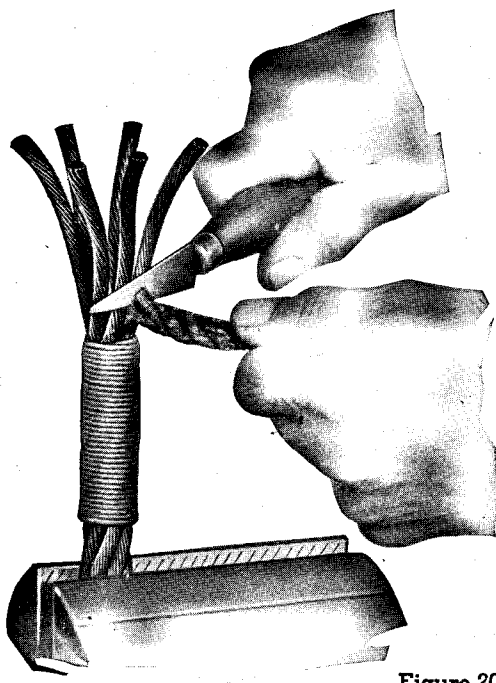


Figure 20

and of sufficient length to prevent any untwisting of the strands, which would result in unequal tension on the strands when socket is attached.

A seizing iron as shown in Fig. 46 page 111 is recommended for applying seizings to ropes one inch diameter and larger.

Place rope end upright in bench vise as shown in Fig. 19.

Remove any seizing above the one referred to in previous paragraph. Cut the fiber core at the seizing. See Fig. 20.

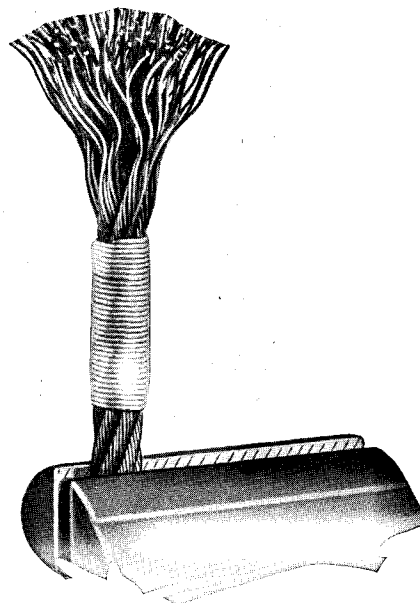


Figure 21

Untwist the strands and broom out the wires. The wires should be separated but not straightened. See Fig. 21.

The wires for the distance they are to be inserted in the socket should be carefully cleaned with benzine, naphtha, or gasoline, and then dipped in a bath of muriatic acid solution (50% commercial muriatic acid and 50% water) for about 30 seconds to one minute, or until the acid has thoroughly cleaned each wire. Care should be taken to prevent acid coming in contact with the fiber core or any portion of the rope other than the broomed wire ends. The acid should be neutralized by next dipping the wires into boiling water to which has been added a small amount of soda.

Draw the ends of the wires together with a piece of seizing wire so that the socket can be forced down over them. See Fig. 22.

Force the socket down over the rope end until it reaches the seizing on the wire rope. Remove the seizing wire from the wires and allow the wires to

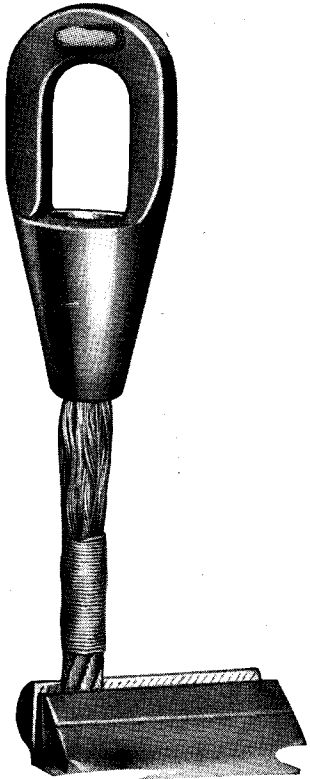


Figure 22

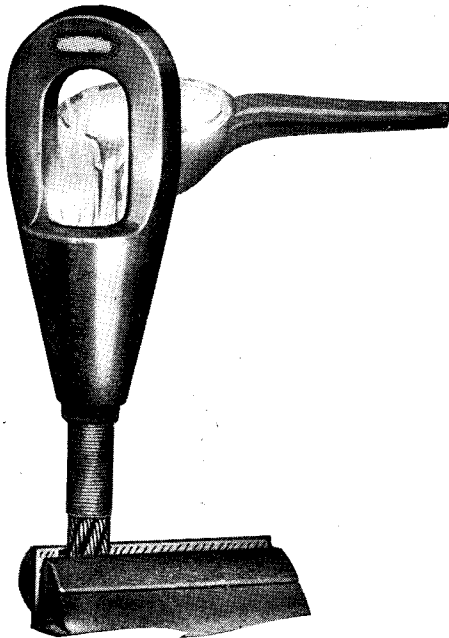


Figure 23

expand within the socket basket. The ends of the wires should be level with the upper end of the socket basket.

Care should be taken to see that the axis of the socket is in line with the axis of the rope.

Seal the base of the socket with putty, clay, or similar substance.

It is advisable to preheat the basket of the socket to expel any moisture and to prevent the molten zinc from congealing before it has completely filled the lower end of the basket.

Fill the socket basket with molten zinc. The zinc must not be too hot or it will anneal the wires, particularly on small ropes or ropes of small wires. From 800 to 875 degrees Fahrenheit is the correct temperature. See Fig. 23.

When the zinc has congealed the socket can be plunged into cold water to cool it.

The seizing can then be removed.

Fig. 24 shows a Tiger Wire Rope Socket applied by this method before the seizing was removed.

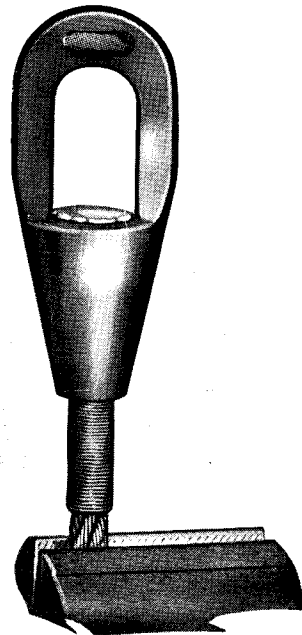


Figure 24

If the socketing is properly done, when tested to destruction, a wire rope will break before it will pull from the socket.

For directions for attaching sockets to strands we recommend that you confer with us stating size and grade of strand. We also suggest that you consult us regarding the socketing of stainless steel and bronze wire ropes and strands.

SPLICING WIRE ROPE

Directions for Splicing 6 Strand Ropes

When a rope is spliced endless, or two similar ropes are spliced together, a short length of each of the two ends is consumed in making the splice. This should be considered when ordering the lengths to be spliced.

There are two endless splices: the Standard Short Splice used for splicing most six strand ropes; and the Long Splice used for splicing Haulage Ropes and long lengths of wire rope operating under heavy loads. The Long Splice differs from

the Standard Short Splice in that the distance between tucks and length of tuck is greater and more rope is consumed in making the splice. Otherwise the two are the same.

The total amount of rope to allow for making endless splices is:

Diameter of Wire Rope in Inches	$\frac{1}{4}$ - $\frac{3}{8}$	$\frac{1}{2}$ - $\frac{5}{8}$	$\frac{3}{4}$ - $\frac{7}{8}$	1-1 $\frac{1}{8}$	1 $\frac{1}{4}$ -1 $\frac{3}{8}$	1 $\frac{1}{2}$
Length of Rope to Allow in Feet	(Standard Short Splice . . . Long Splice . . .					
	15	20	24	28	32	36
	30	40	50	60	70	80

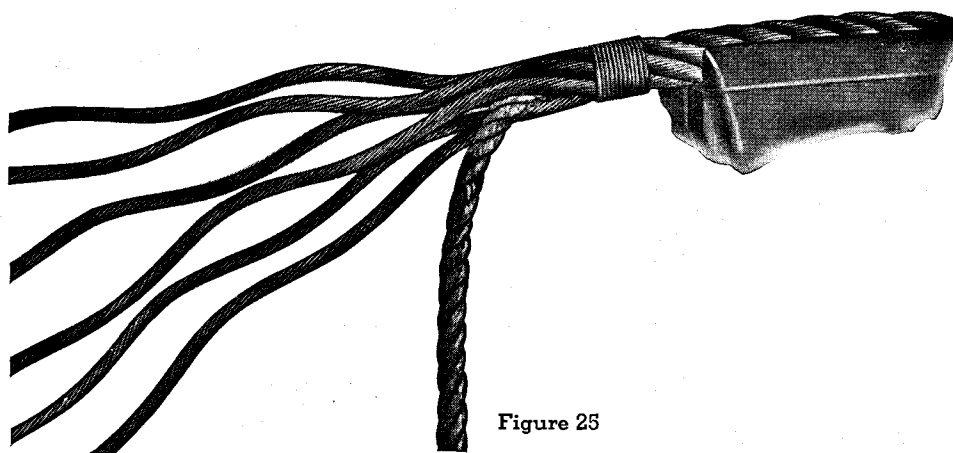


Figure 25

Place a seizing on each of the two rope ends to be spliced together at a distance from the end equal to one-half the allowance for splicing. As an example, if splicing two lengths of $\frac{1}{2}$ inch diameter rope together by the Standard Short Splice,

the seizings would be placed ten feet from the ends.

Unlay the strands of each end to these seizings. See Fig. 25.

Cut off the fiber cores as near the seizings as possible.

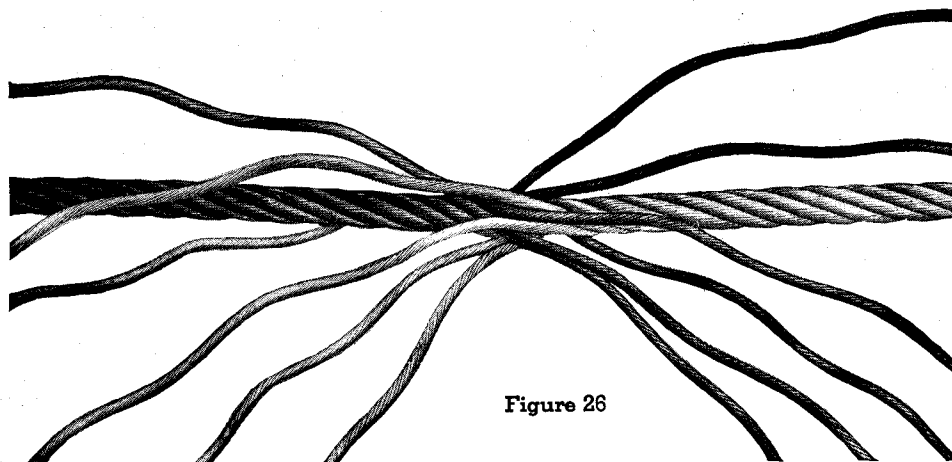


Figure 26

Interlock the six strands of each end in a finger lock position. Force the ends together so that seizings are as near each other as possible. Remove the seizings. See Fig. 26.

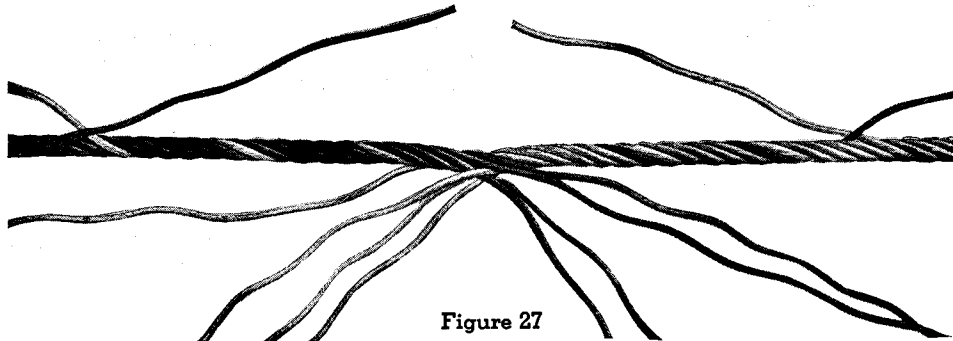


Figure 27

This process should be continued with the first strand from each rope end until only strand equal to the length of tuck remains.

The length of tuck is approximately one-twelfth the amount of rope allowed for the splice.

Diameter of Rope in Inches.....		$\frac{1}{4}$ - $\frac{3}{8}$	$\frac{1}{2}$ - $\frac{5}{8}$	$\frac{3}{4}$ - $\frac{7}{8}$	1- $1\frac{1}{8}$	$1\frac{1}{4}$ - $1\frac{3}{8}$	$1\frac{1}{2}$
Length of Tuck in Inches...	Standard Short Splice...	15	20	24	28	32	36
	Long Splice...	30	40	50	60	70	80

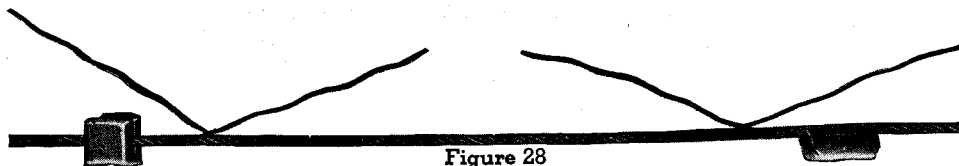


Figure 28

The strand ends of preformed wire ropes should be straightened. It is not necessary to straighten the strand ends of non-preformed ropes. With this exception the method of splicing is the same for both.

The strand ends should be wrapped with friction tape or twine. A layer of tape or twine helps hold the tucked ends in place as it makes them larger in diameter and increases the binding action of the outer strands. It is advisable to build up the diameter of the strand ends with tape or twine as much as possible without making the rope oversize when the strand ends are tucked.

The method of tucking the six pairs of strand ends is the same for each pair.

Unlay one strand, filling the groove vacated by this strand with a strand from the other rope end. Fig. 27 shows the first strand from each rope end being replaced by a strand from the other rope end.

The second strand from each rope end should be unlaid and replaced by a strand from the other rope end in the same manner, but stopped at a distance of twice the length of tuck from the point where the first pair of strands protrude. In a similar manner, the third strand from each end should be replaced by a strand from the other end for a distance equal to the length of tuck.

The twelve strands now protrude from the rope in pairs at points separated by twice the length of tuck.

The protruding strand ends should next be cut off leaving lengths equal to the length of tuck. Fig. 28 shows two of the six pairs of strand ends.

If a vise is available, it should be used as it facilitates the tucking operation. If a vise cannot be obtained, a manila rope sling and a short wooden lever may be used to untwist and open the rope.

Place the rope in the vise so that the vise grips the rope and one of the two strand ends just beyond the point where a pair of strand ends protrude from the rope. See Fig. 29. Drive marlin spike under three strands, opening the rope so that the fiber core may be cut and the end pulled through the opening made by the point of the marlin spike. Start the wrapped strand end into the space left vacant by the removal of the fiber core. Rotate the marlin spike so as to force out the fiber core and force the strand end into the center of the rope.

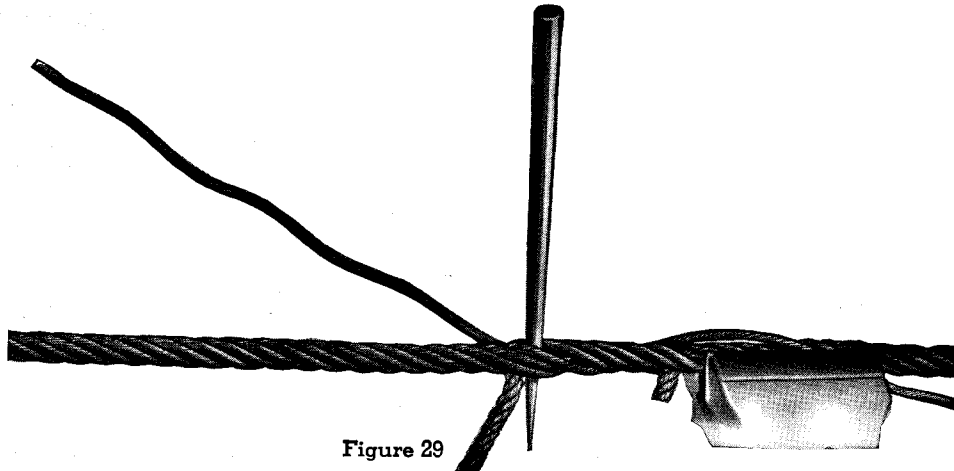


Figure 29

By rotating the spike, the strand end is tucked its entire length. See Fig. 30.

The rope is then regripped in the vise so that the second strand end can be tucked. See Fig. 31.

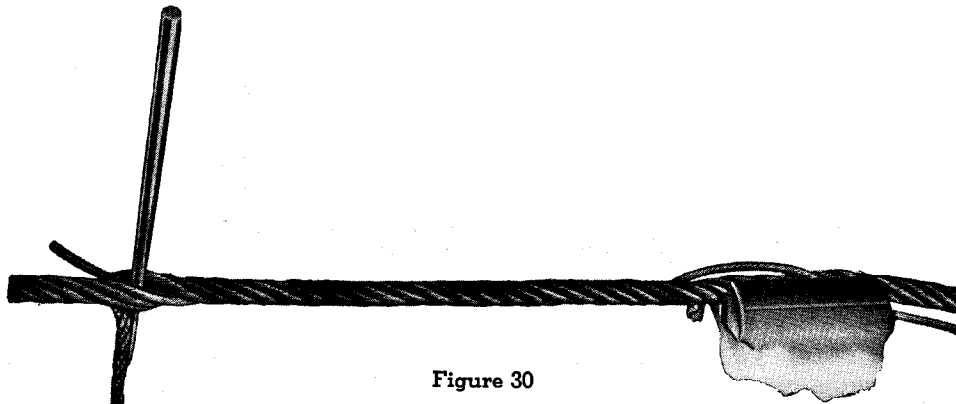


Figure 30

Drive the marlin spike under three strands as before.

In order to start the second strand end into the

rope without any slack, a pair of splicing tongs or some other form of clamp should be used to force this strand into its proper position. See Fig. 31.

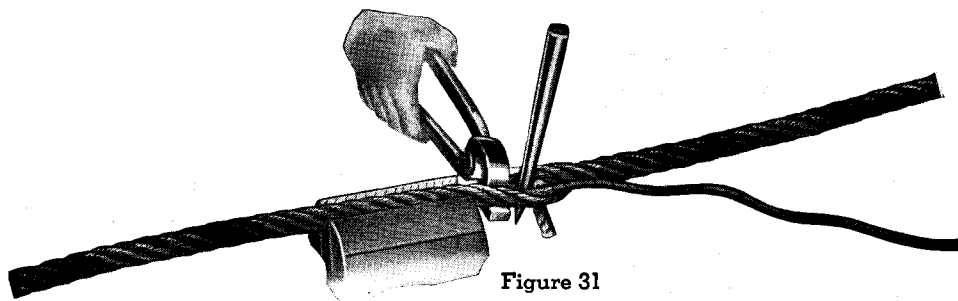


Figure 31

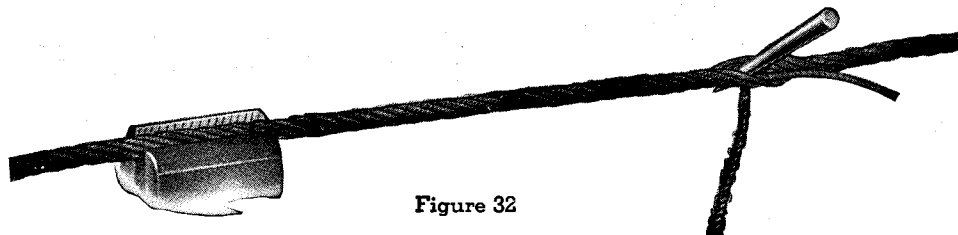


Figure 32

The marlin spike is then rotated forcing the fiber core from the rope and forcing the wrapped strand end into the space vacated by the fiber core. The strand end is tucked its entire length

in this manner. See Fig. 32.

When splicing regular lay ropes the strand ends should not cross at the point where the tucks begin. See Figs. 32, 33 and 34.

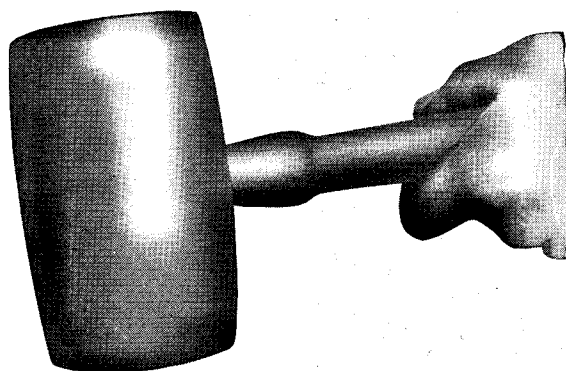
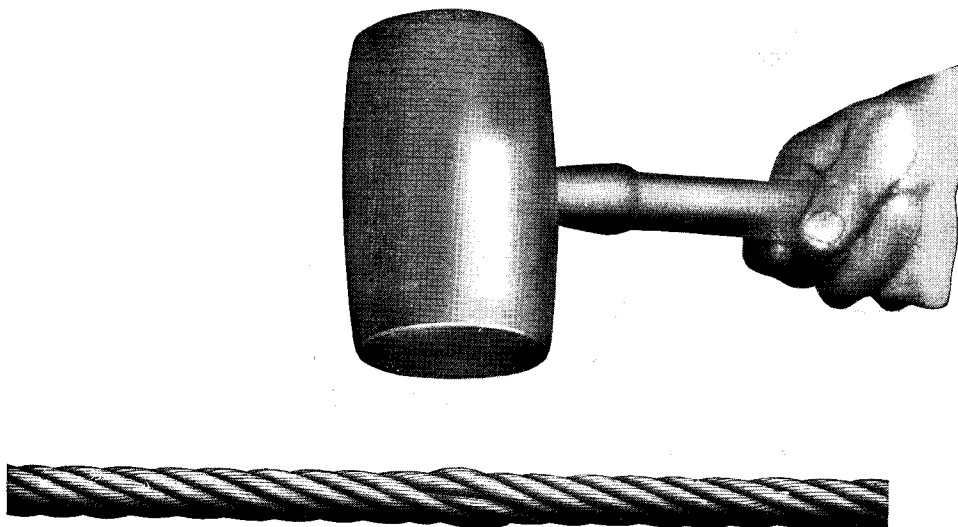


Figure 33

When splicing Lang lay ropes, it is advisable to have the strand ends cross at the points where the tucks begin, as this increases the holding power of the splice. This is accomplished by inserting the marlin spike under the strand end which has been

tucked when starting the tucking operation on the second strand end.

The rope will be somewhat deformed at the point where the tucks start. This can be remedied by hammering the rope at this point with wooden mallets. See Fig. 33.



Figure 34

Fig. 34 shows one of six similar points of the finished splice where one pair of tucked strands start. A rope spliced in this manner is nearly as

strong as the original rope. After running a few days, a well made splice cannot be detected except by a careful examination of the rope.

Directions for Splicing 8 Strand Ropes

Because the fiber core in an eight strand rope is so much greater in diameter than the strands, it is not practical to tuck the strand ends by the method outlined for splicing six strand ropes. The strand ends are secured by twisting or tying them together. This is known as the Nash Tuck.

The process for splicing together two similar eight strand ropes, or splicing an eight strand rope

endless, is similar to that for splicing a six strand rope up to the point where the strands are to be tucked. See Fig. 28. The only difference is that the length of tuck is approximately one sixteenth the amount of rope allowed for splicing.

The method of tucking the eight pairs of strand ends is the same for each pair.

Place seizings on rope each side of point where the strands project. Split the strand ends in two back to the seizings. See Fig. 35.

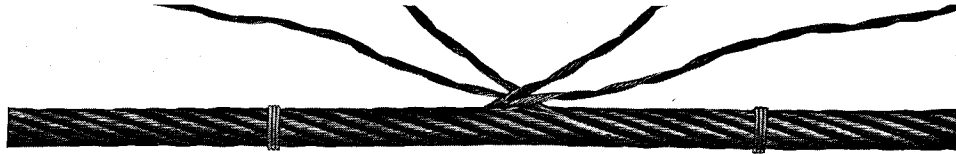


Figure 35

Take one-half of each strand end and tie a double knot. See Fig. 36.

Knot should be drawn down tight by a hand clamp or some similar tool.

Insert spike under the three strands beyond the knot and pull the half strands through. Fig. 37

shows one-half strand pulled through and the second half strand in the process of being pulled through.

The two half strands which have been tied and tucked are cut off close to the rope and each short end forced into the valley between the strands.

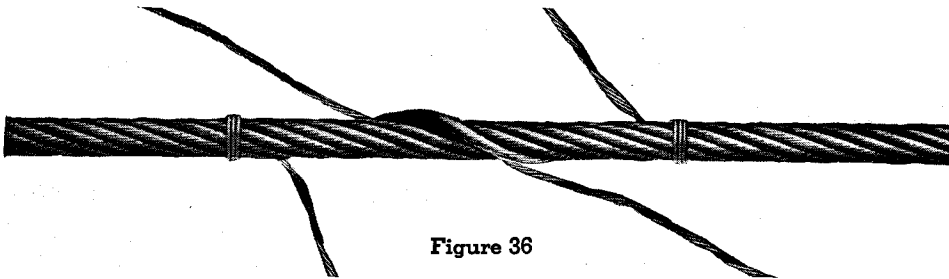


Figure 36

The other two half strands are tucked by inserting a marlin spike under the adjacent strand and pulling the half strand through. The ends are then cut off close to the rope and the short ends forced into the valleys between the strands.

Any unevenness in the rope should be removed by hammering with wooden mallets in the manner shown by Fig. 33.

Fig. 38 shows one of eight similar points of the finished splice.

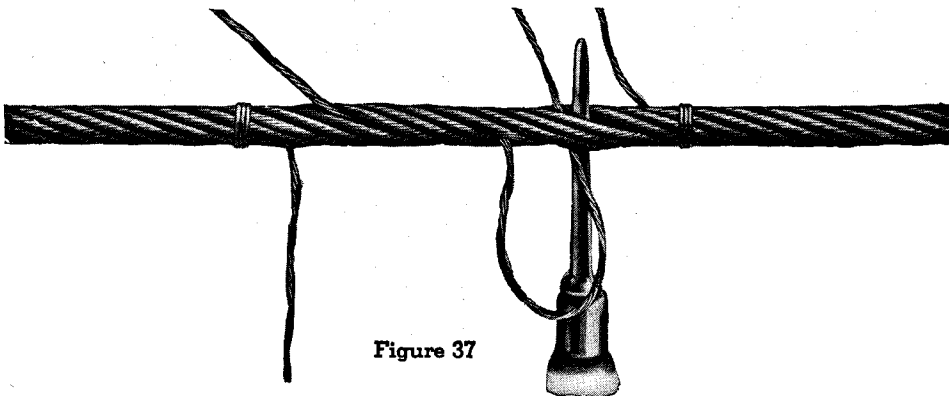


Figure 37



Figure 38

Directions for Eye Splicing Wire Ropes

While the following directions cover splicing a galvanized thimble into a six strand wire rope, the process is also used for eight strand ropes and for splicing eyes into ropes when thimbles are not used.

The process of splicing a thimble into a rope consists of bending the rope about the thimble and fastening the short end by tucking the individual strands under similar strands of the long end of the rope a sufficient number of times to hold them securely. Four tucks are usually sufficient for all ropes containing not more than nineteen wires to the strand. For ropes with more than 19 wires to the strand five tucks should be used.

A short length of wire rope is consumed in making an eye splice. The amounts required for Light and Heavy wire thimbles are shown in the opposite column. For larger thimbles, a proportionally greater amount of rope is required.

Diameter of Rope in Inches.....	$\frac{1}{4}$ – $\frac{3}{8}$	$\frac{1}{2}$	$\frac{5}{8}$ – $\frac{3}{4}$	$\frac{7}{8}$ –1	$1\frac{1}{8}$	$1\frac{1}{4}$	$1\frac{1}{2}$
Length to Allow in Feet.	1	$1\frac{1}{2}$	2	$2\frac{1}{2}$	3	$3\frac{1}{2}$	4

A riggers vise as shown in the following illustration is best adapted for eye splicing. A common bench vise can be used if a riggers vise is not available.

Measure off the amount of rope allowed for making the splice. Bend the rope about the thimble at this point and place rope and thimble in vise. See Fig. 39.

In these illustrations the strands of the short end of the rope have been numbered 1 to 6, inclusive, and the strands of the long end of the rope have been lettered A to F, inclusive. Strand 1 is to be tucked under Strand A; Strand 2 under Strand B; Strand 3 under Strand C; etc. Each strand of the short end of the rope is to be tucked under its corresponding strand of the long end of the rope four times.

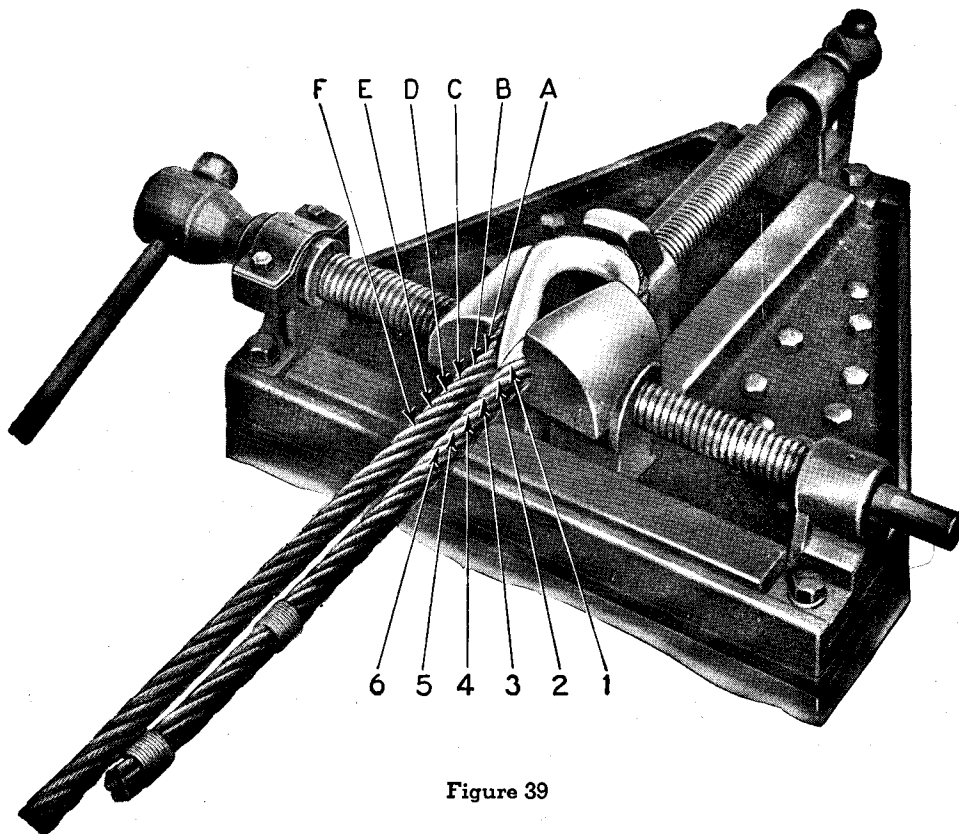


Figure 39

Remove seizings from the short end of the rope and separate the strands. Cut off the fiber core at the point where the strands separate. See Fig. 40.

Insert a marlin spike under the first two strands nearest the point of the thimble, Strands A and B,

and rotate the spike a half turn away from the thimble. Insert Strand 1 through the opening so formed and rotate spike back towards the thimble taking Strand 1 with it and pull Strand 1 tight. This gives Strand 1 one tuck. See Fig. 41.

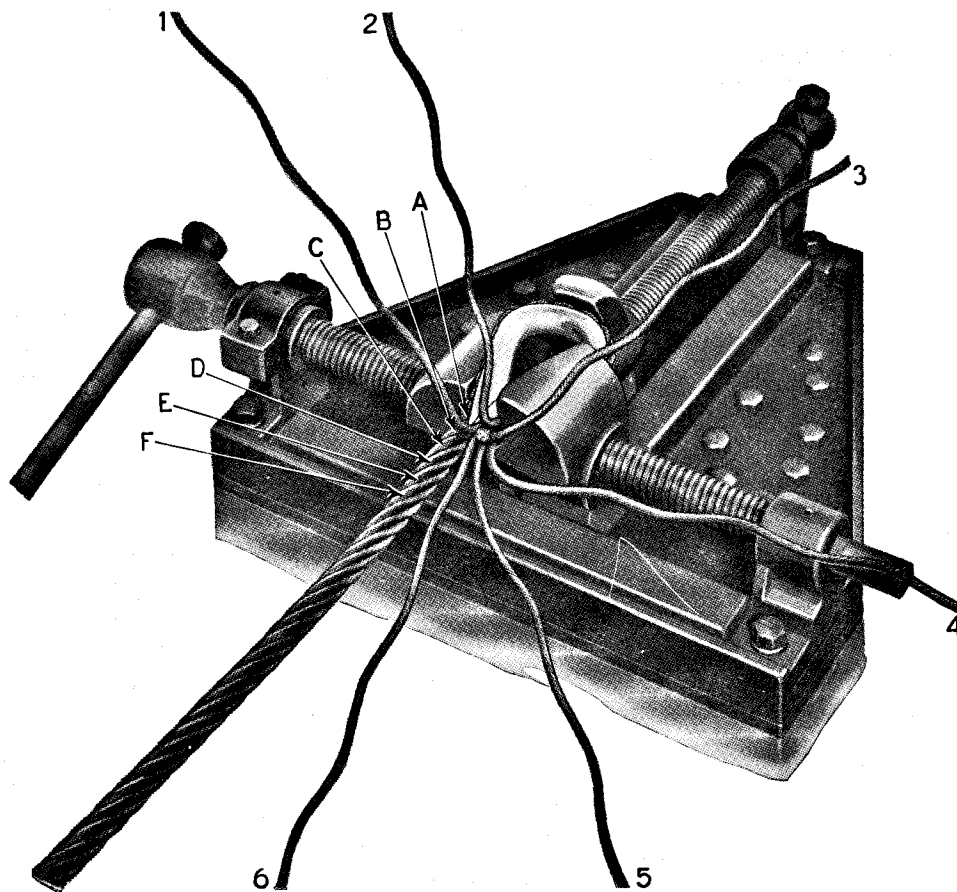


Figure 40

Insert marlin spike under next single strand, Strand B, and tuck Strand 2 by the same method.

Omit the next strand, Strand C, and insert marlin spike under the two strands beyond, Strands D and E, and tuck Strand 6 by inserting it through the opening in the direction opposite to

which Strands 1 and 2 were tucked. Rotate the marlin spike back to the point of the thimble, forcing Strand 6 with it, and pull Strand 6 tight. Figure 42 shows the splice at this point. Strands 6, 1, and 2 have been tucked once under Strands F, A, and B, respectively.

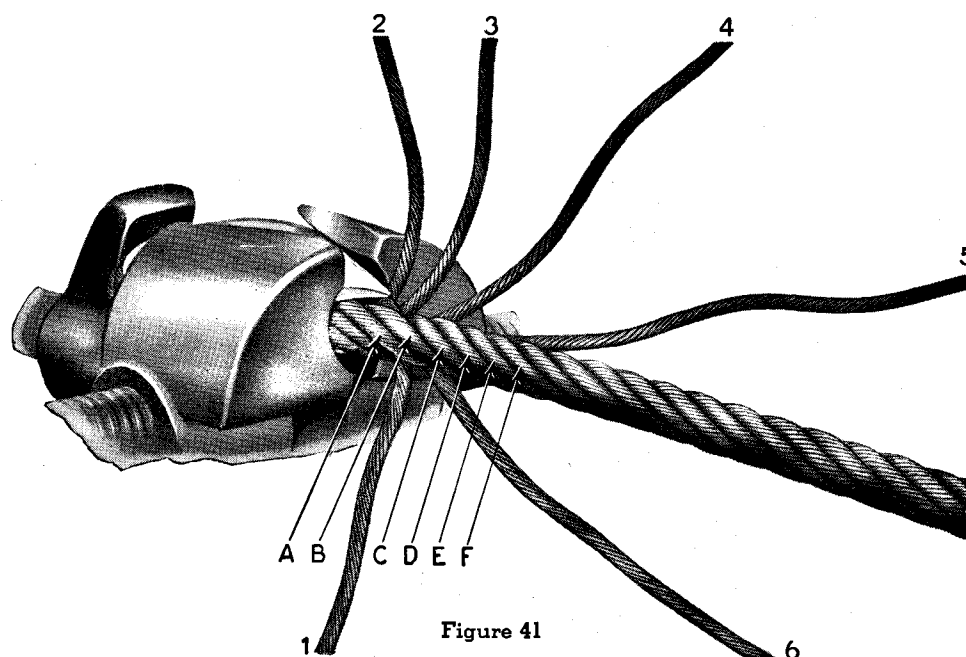


Figure 41

Insert marlin spike under Strand E and tuck Strand 5 in the same manner as Strand 2 was tucked. See Fig. 43.

Without removing the marlin spike give Strand 5 three additional tucks. This is accomplished by winding Strand 5 spirally around Strand E three

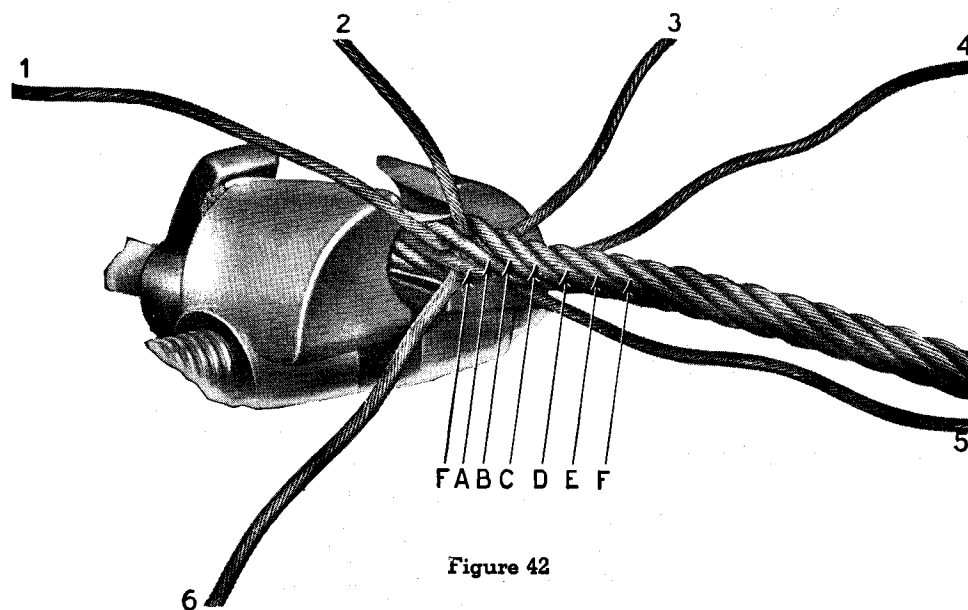


Figure 42

times. Each tuck is made by rotating the spike a half turn, pulling Strand 5 through the opening, and rotating the spike back toward the thimble to tighten the tuck.

Give Strand 4 four tucks by winding it about Strand D in the same manner.

Tuck Strand 3 four times about Strand C.

Fig. 44 shows Strands 3, 4, and 5 after these strands have been given four tucks.

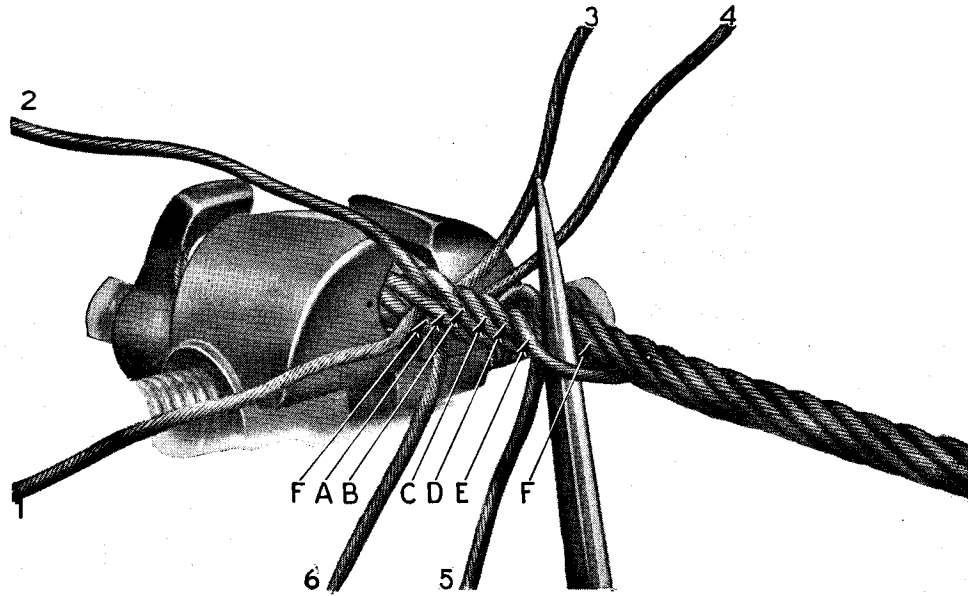


Figure 43

Strands 6, 1, and 2, should be given three additional tucks about Strands F, A, and B, respectively, in the manner outlined for Strand 5. Fig. 45 shows four completed tucks in each of the six

strands. If the rope contains more than nineteen wires per strand, each strand should be given an additional tuck.

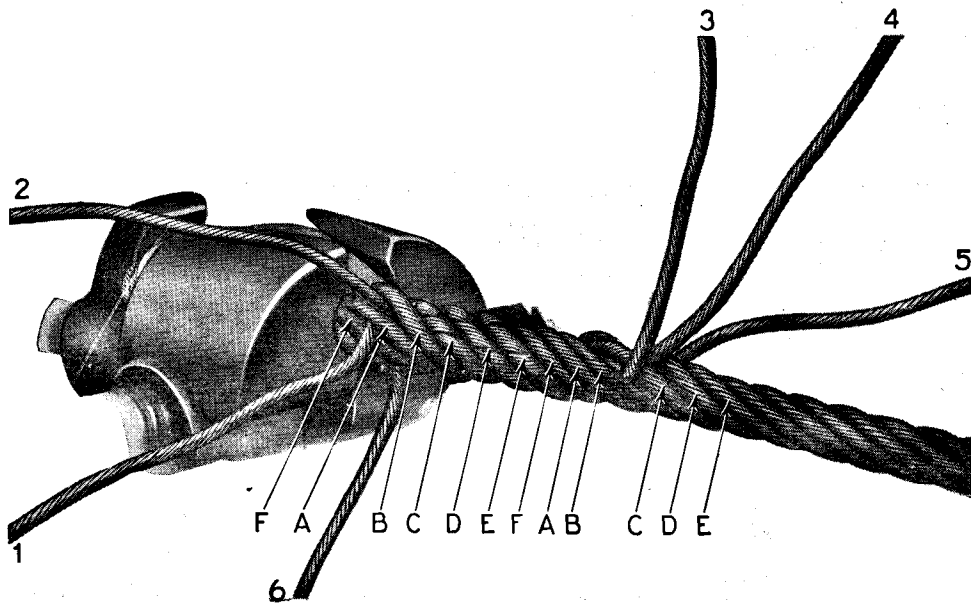


Figure 44

An eye splice made in this manner will have a slight taper as shown in Figs. 46 and 47. If a more pronounced taper is desired, this can be secured

by splitting each strand before the final tuck and cutting off a portion of the wires.

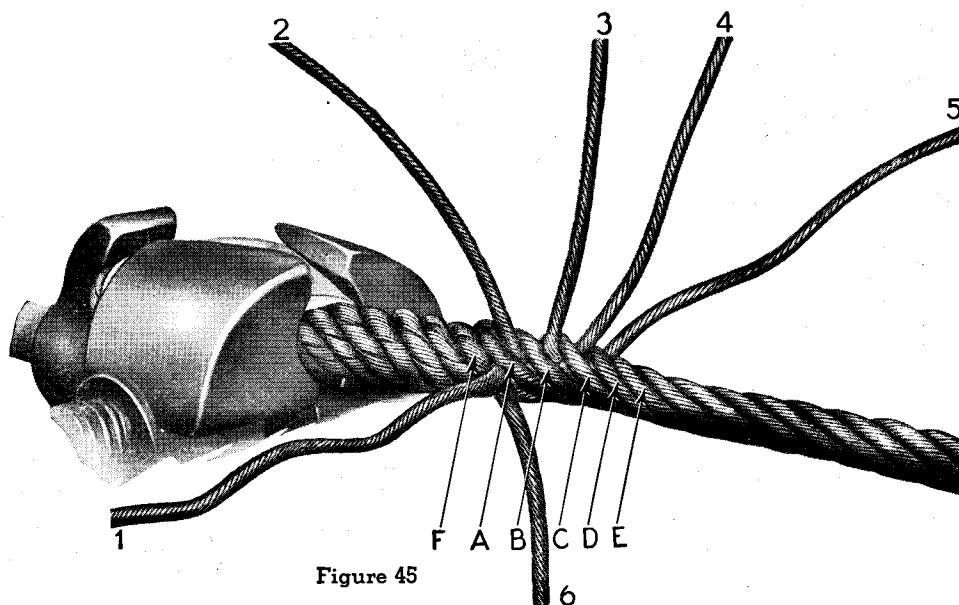


Figure 45

The protruding strand ends are cut off close to the rope.
Any inequalities in the splice should be removed

by hammering with wooden mallets as shown by Fig. 33, page 105.

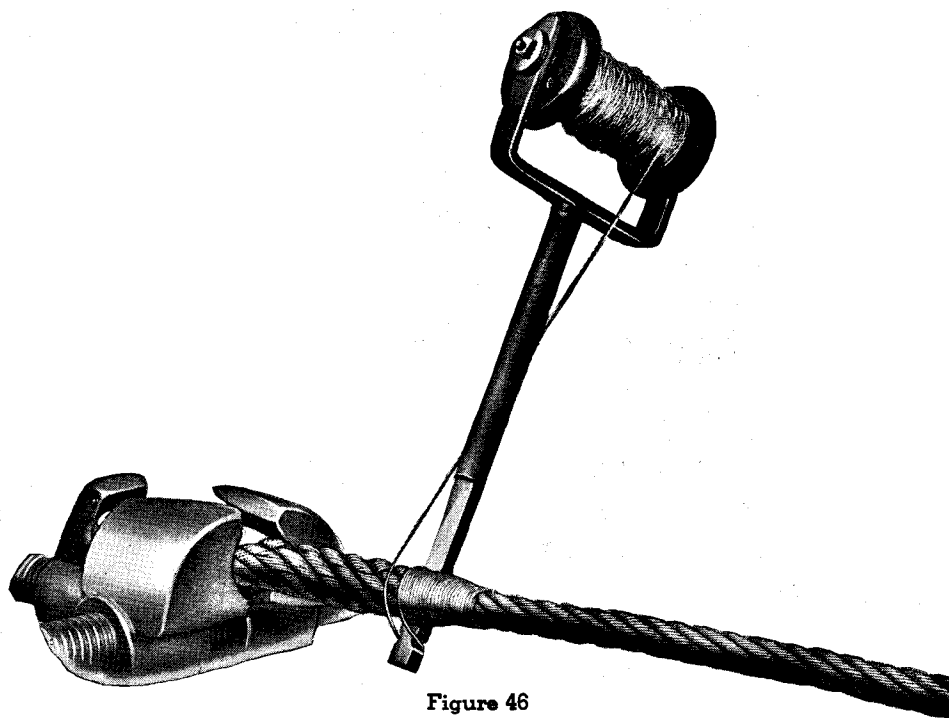


Figure 46

The splice should be wrapped with serving wire to protect the hands of workmen handling the rope. This is best accomplished by using a serving iron as shown in Fig. 46.

Fig. 47 shows a Galvanized Heavy Wire Rope Thimble spliced into the end of a 6x19 wire rope by the method here outlined.

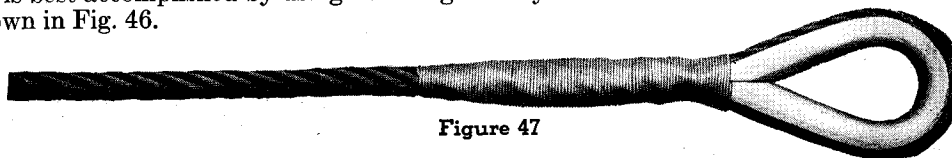
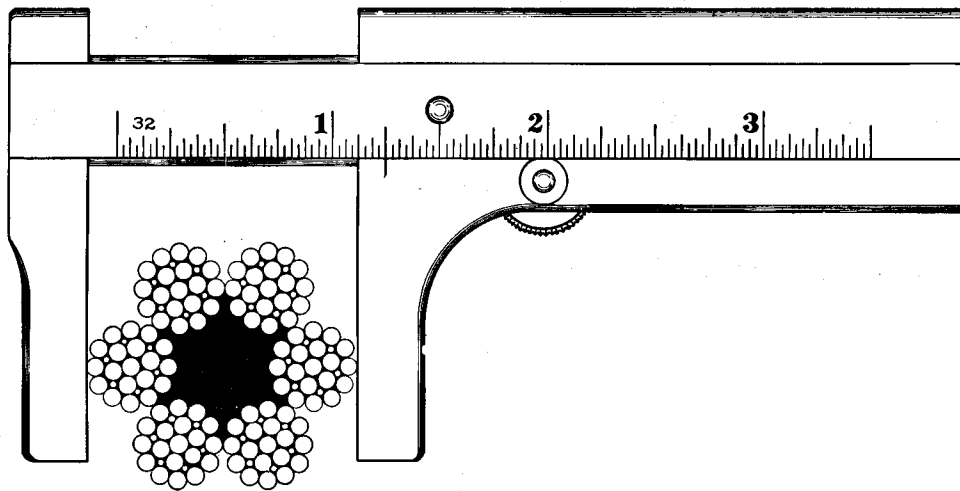


Figure 47

GAUGING WIRE ROPE

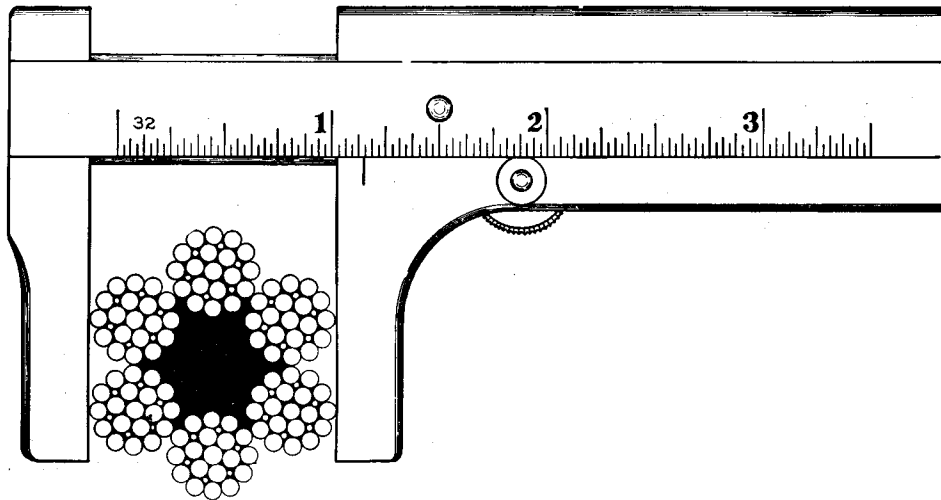
The diameter of a wire rope is the diameter of the circle which will just enclose all of the strands. In the case of strands, the diameter is that of the

circle which will just enclose all of the wires. The correct diameter is the greatest diameter of the rope or strand.



CORRECT WAY

This gives correct diameter.



INCORRECT WAY

This does not give correct diameter.

CARE OF WIRE ROPE

To obtain maximum useful rope life and greatest economy, there are several simple rules to observe.

Lubrication

Wire Ropes are lubricated during fabrication. The amount and grade of lubricant used depends upon the size and type of rope. As this initial lubrication is generally not sufficient to last the useful life of the rope, periodical applications of a good grade of oil or grease should be made. The lubricant should be free from acids and alkalies; should have sufficient adhesive strength to stay on the rope; should be able to penetrate between the wires and strands; should be non-soluble under the conditions prevailing where the rope operates; should have a high film strength; and should resist oxidation.

The importance of periodical lubrication is apparent from the fact that a wire rope is a machine with many moving parts. Each time a rope bends or straightens, the wires in the strands and the strands in the rope must slide on each other. This requires a film of lubricant on each moving part.

A second important reason for lubricating iron and steel wire ropes is to prevent corrosion of the wires and deterioration of the fiber core. There is no known means of inspection which will even approximate the strength of a corroded rope. A rusty rope is a liability.

Used ropes should be cleaned before they are lubricated. The cleaning may be accomplished by means of wire brushes or scrapers, or by compressed air or superheated steam. The object is to remove all foreign material and old lubricant from the valleys between the strands and from the spaces between the outer wires. The lubricant may be applied in any manner suitable to field conditions. It may be brushed onto the rope with a stiff brush; applied by passing the rope through saturated waste; or by passing the rope through a trough or box of lubricant; or the lubricant may drip onto the rope, preferably at a point where the rope opens slightly from bending. The object is to apply a uniform coating to the entire length of rope.

When a wire rope is taken out of service for an appreciable length of time, it should be cleaned

and lubricated. It should be stored in a dry place protected from the elements.

Cutting Back

The object of cutting short lengths of rope from the drum end is to change the position of the rope. Wear and fatigue are usually most severe at certain definite points on wire rope using equipment, and the removal of a short length of rope subjects different portions of it to these destructive forces. Cutting back the outer end removes that section next to the fitting where maximum localized fatigue from vibration often occurs.

In order to take advantage of this method of obtaining increased service, it is often advisable to use a length of rope slightly longer than normally required.

Reversing Ends

A rope is changed end for end to distribute the wear and fatigue from bending and vibration. If these destructive forces are uniform throughout the system, no economy is effected by such a change. On most installations these forces are more severe for one-half of the rope than for the other half, and reversing the ends increases the rope service.

Seizings

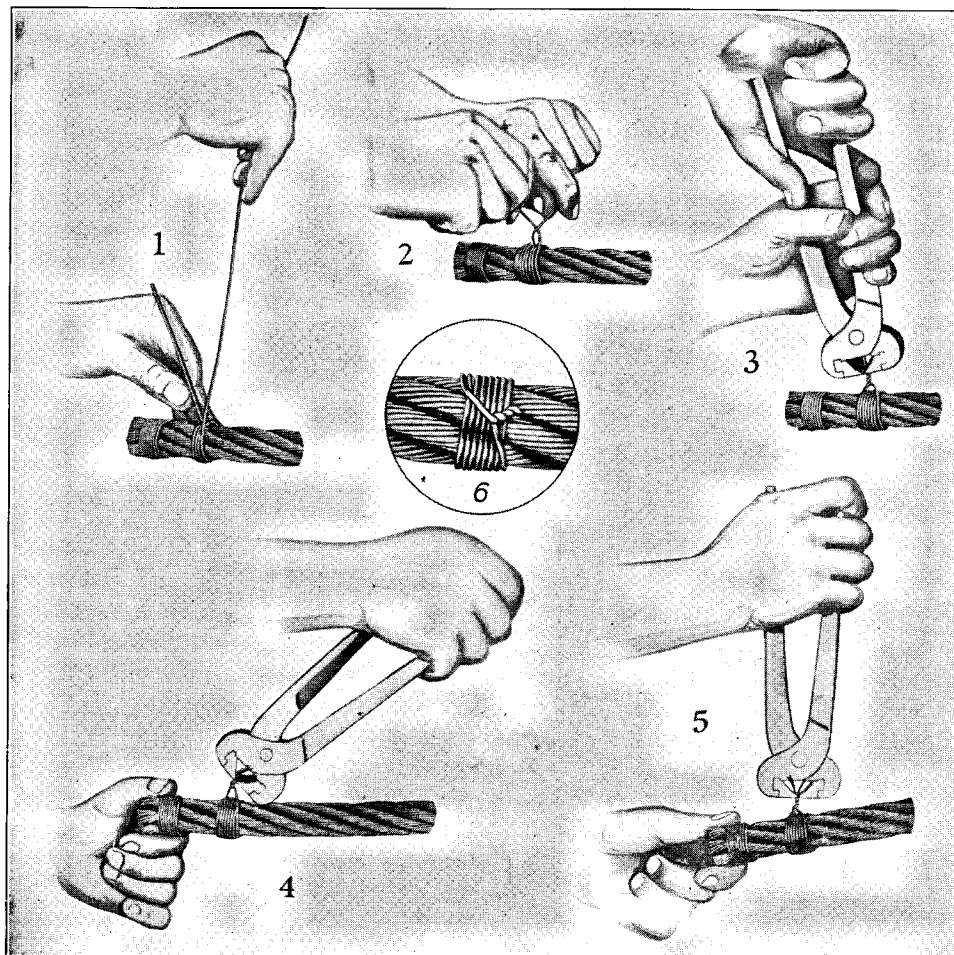
Seizings are required to prevent the untwisting of all non-preformed wire ropes unless the rope ends have been eye spliced, socketed, or attached to some other type of permanent fitting. Non-preformed rope should have seizings applied to both sides of the point where it is to be cut. Inadequate seizings which do not preserve the rope structure, but permit the strands to untwist, result in shortened service because of the unbalanced condition of the rope. One seizing applied to each side of the point of cutting a preformed wire rope is recommended in order to prevent distortion of the rope ends from the pressure applied during the cutting operation.

SEIZINGS RECOMMENDED

Rope Dia. in Inches	NUMBER OF SEIZINGS		Length of Seizings in Inches	Distance between Seizings in Inches	Approx. Size of Seizing Wire in inches
	Regular Lay Fiber Core Ropes	18 x 7 Non- Spinning; Lang Lay; and Wire Core Ropes			
1/2 and Smaller	2	3	1/2	1	.020-.030
9/16-7/8	3	3	1	2	.040-.060
1-1 1/4	3	4	1 1/2	2	.060-.090
1 3/8-1 5/8	4	4	2	2	.080-.125
1 3/4-2	4	4	3	2	.105-.125
2 1/8 and Larger	4	4	4	3	.105-.125

Seizing wire should be annealed iron grade. Galvanized annealed iron wire should be used for seizing galvanized ropes. Hand cutters may be used for applying seizings to ropes one inch diameter and smaller. For larger ropes a seizing iron as shown in Fig. 46, Page 111, or a round bar 1/2 to 5/8 inch diameter by 18 inches long is recommended.

The following illustrations and instructions for applying seizings are from the United States Government Master Specification for Wire Rope. A neater seizing made with hand cutters for ropes one inch diameter and smaller can be obtained by laying the seizing wire in the groove between two strands when starting the operation. See Fig. 49, page 115.



- (1) Wind the seizing wire on the rope by hand, keeping the coils together and considerable tension on the wire.
- (2) Twist the ends of the wire together *counter clockwise* by hand so that the twisted portion of the wires is near the middle of the seizing.
- (3) Using "Carew" cutters, tighten the twist just enough to take up the slack. Do not try to tighten the seizing by *twisting*.
- (4) Tighten the seizing by prying the twist away from the axis of the rope with the cutters.
- (5) Tighten the twist again as in (3). Repeat (4) and (5) as often as is necessary to make the seizing tight. Cut off the ends of the wires and pound the twist flat against the rope.
- (6) The appearance of the finished seizing.

Fig. 48. Method of applying seizings with hand cutters

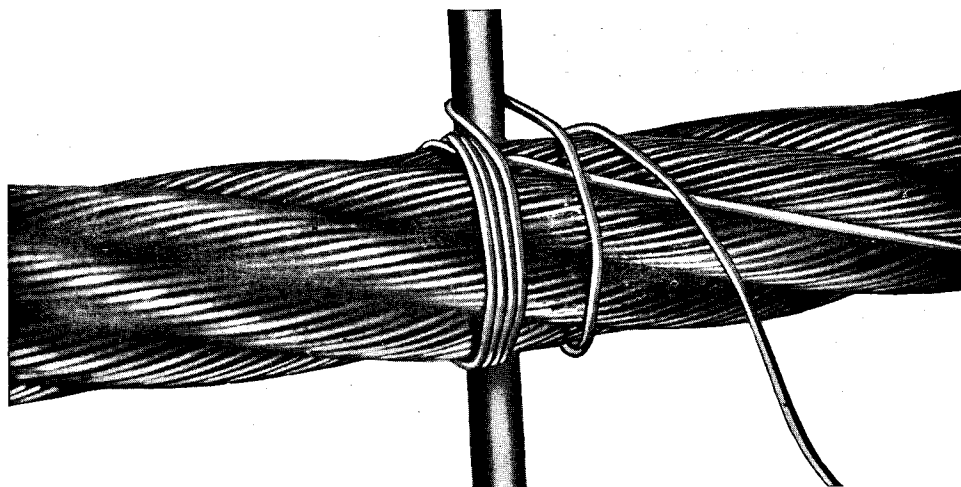


Fig. 49. Method of applying seizings with round bar

Lay one end of the seizing wire in the groove between two strands and wrap the long end back over this portion. If a seizing iron is used (See Fig. 46), tension of the seizing wire is obtained by adjusting the nuts on the shaft about which the spool rotates, or by wrapping the wire

around the shaft of the seizing iron. If a round bar is used, the necessary tension in the wire is secured by giving the free end one or two turns about the rope. The ends of the seizing wire are twisted together and tightened as in Fig. 48.

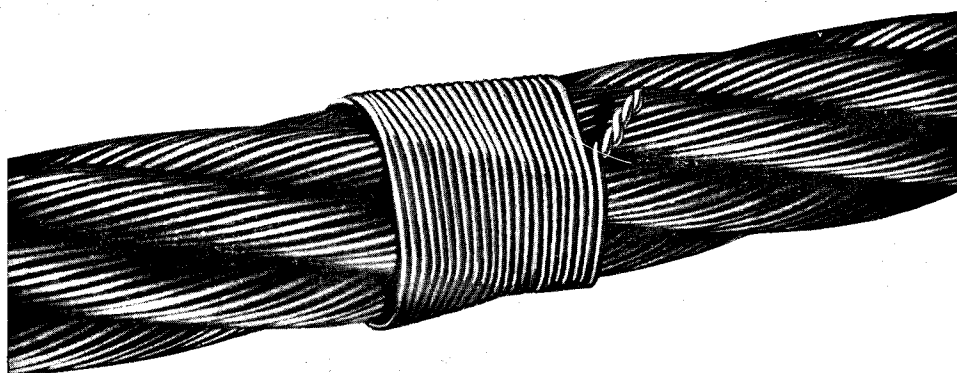


Fig. 50. A well made seizing

Sheaves

Sheaves should be aligned so that the axis of the rope is parallel to a line drawn from the center of the groove of one sheave to the center of the groove of the next sheave. This is important in order to prevent excessive wear on the rope caused by it bearing against flanges or dragging

across shoulders. This excessive and unnecessary wear will destroy the outer wires long before the rope has given nominal service. The use of sheaves with broken flanges produces the same undesirable results.

(See "Effects of Bending", page 32, "Grooves" page 64, and "Fleet Angle", page 66.)

Supporting Sheaves and Rollers

The use of supporting sheaves and rollers to prevent the rope dragging decreases the wear on the rope and results in increased service. Sheaves and grooved rollers should have grooves sufficiently large to prevent pinching the rope. See "Grooves", page 64. All sheaves and rollers should be machined or replaced when scored by the rope to prevent unnecessary wear on the outer wires of the rope. They should be free to rotate, large enough in diameter to avoid unnecessary bending of the rope, and to provide adequate support for the rope, and light in weight so as to readily start and stop as the rope starts and stops. Guide Sheaves and Rollers should be at least 6 times the rope diameter if grooved, and 9 times the rope diameter if flat faced. Heavy sheaves and rollers build up momentum when turning, which causes slippage when the rope stops. They are slow to pick up speed when the rope starts, and this produces additional slippage of the rope

on the sheaves or rollers. Slippage, in turn, produces abrasion of the outer wires of the rope.

The installation of supporting rollers at irregular intervals tends to dampen vibration. This is of particular benefit on long inclines operating at comparatively high speeds.

Handling

"Unreeling and Uncoiling", pages 67 and 68, gives directions for properly removing rope from reels and coils. Improper methods produce kinks. Kinks also result from improper handling of rope after it is unwound. A kink is formed by pulling a looped rope until the loop becomes so small that the rope cannot adjust itself by bending to the required arc. The rope is distorted at this point, damaging the individual wires and the rope structure.

Avoid kinks by not permitting loops to form in a rope.

Common Causes of Wire Rope Failures

Of the many forms of abuse of wire ropes, the most commonly encountered are:

Ropes of incorrect size, construction, or grade.

Ropes allowed to drag over obstacles.

Ropes not properly lubricated.

Ropes operating over sheaves and drums of inadequate size.

Ropes overwinding or crosswinding on drums.

Ropes operating over sheaves and drums out of alignment.

Ropes operating over sheaves and drums with improperly fitting grooves or broken flanges.

Ropes permitted to jump sheaves.

Ropes subjected to moisture or acid fumes.

Ropes with improperly attached fittings.

Ropes permitted to untwist.

Ropes subjected to excessive heat.

Ropes kinked.

Ropes subjected to severe overloads due to inefficient operation.

Ropes destroyed by internal wear caused by grit penetrating between strands and wires.

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AMERICAN TIGER BRAND

DECIMAL OF AN INCH AND OF A FOOT

Fractions of Inch or Foot		Inch Equivalents to Foot Fractions	Fractions of Inch or Foot		Inch Equivalents to Foot Fractions	Fractions of Inch or Foot		Inch Equivalents to Foot Fractions	Fractions of Inch or Foot		Inch Equivalents to Foot Fractions
	.0052	$\frac{1}{16}$		.2552	$\frac{3\frac{1}{16}}{16}$		.5052	$\frac{6\frac{1}{16}}{16}$		.7552	$\frac{9\frac{1}{16}}{16}$
	.0104	$\frac{1}{8}$		.2604	$\frac{3\frac{1}{8}}{16}$		.5104	$\frac{6\frac{1}{8}}{16}$		.7604	$\frac{9\frac{1}{8}}{16}$
$\frac{1}{64}$	.015625	$\frac{3}{16}$	$\frac{1}{64}$	.265625	$\frac{3\frac{3}{16}}{16}$	$\frac{3}{64}$	.515625	$\frac{6\frac{3}{16}}{16}$	$\frac{4}{64}$	.765625	$\frac{9\frac{3}{16}}{16}$
	.0208	$\frac{1}{4}$		.2708	$\frac{3\frac{1}{4}}{16}$		.5208	$\frac{6\frac{1}{4}}{16}$		.7708	$\frac{9\frac{1}{4}}{16}$
	.0260	$\frac{5}{16}$		.2760	$\frac{3\frac{5}{16}}{16}$		.5260	$\frac{6\frac{5}{16}}{16}$		.7760	$\frac{9\frac{5}{16}}{16}$
$\frac{1}{32}$	.03125	$\frac{3}{8}$	$\frac{9}{32}$	.28125	$\frac{3\frac{3}{8}}{16}$	$\frac{1}{32}$	.53125	$\frac{6\frac{3}{8}}{16}$	$\frac{25}{32}$	.78125	$\frac{9\frac{3}{8}}{16}$
	.0365	$\frac{7}{16}$		.2865	$\frac{3\frac{7}{16}}{16}$		.5365	$\frac{6\frac{7}{16}}{16}$		.7865	$\frac{9\frac{7}{16}}{16}$
	.0417	$\frac{1}{2}$		.2917	$\frac{3\frac{1}{2}}{16}$		.5417	$\frac{6\frac{1}{2}}{16}$		.7917	$\frac{9\frac{1}{2}}{16}$
$\frac{3}{64}$	.046875	$\frac{9}{16}$	$\frac{1}{64}$	.296875	$\frac{3\frac{9}{16}}{16}$	$\frac{3}{64}$	.546875	$\frac{6\frac{9}{16}}{16}$	$\frac{5}{64}$	.796875	$\frac{9\frac{9}{16}}{16}$
	.0521	$\frac{5}{8}$		.3021	$\frac{3\frac{5}{8}}{16}$		.5521	$\frac{6\frac{5}{8}}{16}$		.8021	$\frac{9\frac{5}{8}}{16}$
	.0573	$\frac{11}{16}$		.3073	$\frac{3\frac{11}{16}}{16}$		.5573	$\frac{6\frac{11}{16}}{16}$		.8073	$\frac{9\frac{11}{16}}{16}$
$\frac{1}{16}$	.0625	$\frac{3}{4}$	$\frac{5}{16}$	.3125	$\frac{3\frac{3}{4}}{16}$	$\frac{9}{16}$	.5625	$\frac{6\frac{3}{4}}{16}$	$\frac{13}{16}$	.8125	$\frac{9\frac{3}{4}}{16}$
	.0677	$\frac{13}{16}$		.3177	$\frac{3\frac{13}{16}}{16}$		.5677	$\frac{6\frac{13}{16}}{16}$		.8177	$\frac{9\frac{13}{16}}{16}$
	.0729	$\frac{7}{8}$		.3229	$\frac{3\frac{7}{8}}{16}$		.5729	$\frac{6\frac{7}{8}}{16}$		.8229	$\frac{9\frac{7}{8}}{16}$
$\frac{5}{64}$	.078125	$\frac{15}{16}$	$\frac{2}{64}$	.328125	$\frac{3\frac{15}{16}}{16}$	$\frac{3}{64}$	.578125	$\frac{6\frac{15}{16}}{16}$	$\frac{5}{64}$	.828125	$\frac{9\frac{15}{16}}{16}$
	.0833	1		.3333	4		.5833	7		.8333	10
	.0885	$\frac{1}{16}$		.3385	$\frac{4\frac{1}{16}}{16}$		.5885	$\frac{7\frac{1}{16}}{16}$		.8385	$\frac{10\frac{1}{16}}{16}$
$\frac{3}{32}$	.09375	$\frac{1}{8}$	$\frac{1}{32}$	.34375	$\frac{4\frac{1}{8}}{16}$	$\frac{1}{32}$	.59375	$\frac{7\frac{1}{8}}{16}$	$\frac{2}{32}$	.84375	$\frac{10\frac{1}{8}}{16}$
	.0990	$\frac{13}{16}$		.3490	$\frac{4\frac{13}{16}}{16}$		.5990	$\frac{7\frac{13}{16}}{16}$		.8490	$\frac{10\frac{13}{16}}{16}$
	.1042	$\frac{1}{4}$		.3542	$\frac{4\frac{1}{4}}{16}$		.6042	$\frac{7\frac{1}{4}}{16}$		.8542	$\frac{10\frac{1}{4}}{16}$
$\frac{7}{64}$	.109375	$\frac{15}{16}$	$\frac{2}{64}$	.359375	$\frac{4\frac{15}{16}}{16}$	$\frac{3}{64}$	.609375	$\frac{7\frac{15}{16}}{16}$	$\frac{5}{64}$	.859375	$\frac{10\frac{15}{16}}{16}$
	.1146	$\frac{1}{8}$		.3646	$\frac{4\frac{1}{8}}{16}$		.6146	$\frac{7\frac{1}{8}}{16}$		.8646	$\frac{10\frac{1}{8}}{16}$
	.1198	$\frac{1}{16}$		.3698	$\frac{4\frac{1}{16}}{16}$		.6198	$\frac{7\frac{1}{16}}{16}$		.8698	$\frac{10\frac{1}{16}}{16}$
$\frac{1}{8}$	.1250	$\frac{1}{2}$	$\frac{3}{8}$	.3750	$\frac{4\frac{1}{2}}{16}$	$\frac{5}{8}$	.6250	$\frac{7\frac{1}{2}}{16}$	$\frac{7}{8}$	.8750	$\frac{10\frac{1}{2}}{16}$
	.1302	$\frac{19}{16}$		.3802	$\frac{4\frac{19}{16}}{16}$		.6302	$\frac{7\frac{19}{16}}{16}$		.8802	$\frac{10\frac{19}{16}}{16}$
	.1354	$\frac{1}{8}$		.3854	$\frac{4\frac{1}{8}}{16}$		.6354	$\frac{7\frac{1}{8}}{16}$		.8854	$\frac{10\frac{1}{8}}{16}$
$\frac{9}{64}$	.140625	$\frac{11}{16}$	$\frac{2}{64}$	.390625	$\frac{4\frac{11}{16}}{16}$	$\frac{4}{64}$	.640625	$\frac{7\frac{11}{16}}{16}$	$\frac{5}{64}$	.890625	$\frac{10\frac{11}{16}}{16}$
	.1458	$\frac{1}{4}$		.3958	$\frac{4\frac{1}{4}}{16}$		.6458	$\frac{7\frac{1}{4}}{16}$		.8958	$\frac{10\frac{1}{4}}{16}$
	.1510	$\frac{13}{16}$		.4010	$\frac{4\frac{13}{16}}{16}$		.6510	$\frac{7\frac{13}{16}}{16}$		.9010	$\frac{10\frac{13}{16}}{16}$
$\frac{5}{32}$	.15625	$\frac{1}{8}$	$\frac{1}{32}$	.40625	$\frac{4\frac{1}{8}}{16}$	$\frac{2}{32}$	.65625	$\frac{7\frac{1}{8}}{16}$	$\frac{2}{32}$	.90625	$\frac{10\frac{1}{8}}{16}$
	.1615	$\frac{115}{16}$		.4115	$\frac{4\frac{115}{16}}{16}$		.6615	$\frac{7\frac{115}{16}}{16}$		.9115	$\frac{10\frac{115}{16}}{16}$
	.1667	2		.4167	5		.6667	8		.9167	11
$\frac{1}{64}$	.171875	$\frac{2}{16}$	$\frac{2}{64}$	.421875	$\frac{5\frac{1}{16}}{16}$	$\frac{4}{64}$	.671875	$\frac{8\frac{1}{16}}{16}$	$\frac{5}{64}$	.921875	$\frac{11\frac{1}{16}}{16}$
	.1771	$\frac{2}{8}$		.4271	$\frac{5\frac{1}{8}}{16}$		.6771	$\frac{8\frac{1}{8}}{16}$		.9271	$\frac{11\frac{1}{8}}{16}$
	.1823	$\frac{2}{16}$		.4323	$\frac{5\frac{1}{16}}{16}$		.6823	$\frac{8\frac{1}{16}}{16}$		.9323	$\frac{11\frac{1}{16}}{16}$
$\frac{3}{16}$	.1875	$\frac{2}{4}$	$\frac{7}{16}$	.4375	$\frac{5\frac{1}{4}}{16}$	$\frac{1}{16}$	.6875	$\frac{8\frac{1}{4}}{16}$	$\frac{15}{16}$	.9375	$\frac{11\frac{1}{4}}{16}$
	.1927	$\frac{25}{16}$		.4427	$\frac{5\frac{25}{16}}{16}$		.6927	$\frac{8\frac{25}{16}}{16}$		.9427	$\frac{11\frac{25}{16}}{16}$
	.1979	$\frac{2}{8}$		.4479	$\frac{5\frac{1}{8}}{16}$		.6979	$\frac{8\frac{1}{8}}{16}$		.9479	$\frac{11\frac{1}{8}}{16}$
$\frac{13}{64}$	.203125	$\frac{2}{16}$	$\frac{2}{64}$	.453125	$\frac{5\frac{1}{16}}{16}$	$\frac{4}{64}$	.703125	$\frac{8\frac{1}{16}}{16}$	$\frac{6}{64}$	.953125	$\frac{11\frac{1}{16}}{16}$
	.2083	$\frac{2}{12}$		.4583	$\frac{5\frac{1}{2}}{16}$		.7083	$\frac{8\frac{1}{2}}{16}$		.9583	$\frac{11\frac{1}{2}}{16}$
	.2135	$\frac{2}{16}$		.4635	$\frac{5\frac{1}{16}}{16}$		.7135	$\frac{8\frac{1}{16}}{16}$		.9635	$\frac{11\frac{1}{16}}{16}$
$\frac{7}{32}$	.21875	$\frac{2}{8}$	$\frac{15}{32}$	.46875	$\frac{5\frac{1}{8}}{16}$	$\frac{2}{32}$	.71875	$\frac{8\frac{1}{8}}{16}$	$\frac{3}{32}$	.96875	$\frac{11\frac{1}{8}}{16}$
	.2240	$\frac{2}{16}$		.4740	$\frac{5\frac{1}{16}}{16}$		.7240	$\frac{8\frac{1}{16}}{16}$		.9740	$\frac{11\frac{1}{16}}{16}$
	.2292	$\frac{2}{4}$		.4792	$\frac{5\frac{1}{4}}{16}$		.7292	$\frac{8\frac{1}{4}}{16}$		.9792	$\frac{11\frac{1}{4}}{16}$
$\frac{15}{64}$	.234375	$\frac{2}{16}$	$\frac{3}{64}$	.484375	$\frac{5\frac{13}{16}}{16}$	$\frac{4}{64}$	.734375	$\frac{8\frac{13}{16}}{16}$	$\frac{6}{64}$	.984375	$\frac{11\frac{13}{16}}{16}$
	.2396	$\frac{2}{8}$		.4896	$\frac{5\frac{1}{8}}{16}$		.7396	$\frac{8\frac{1}{8}}{16}$		.9896	$\frac{11\frac{1}{8}}{16}$
	.2448	$\frac{2}{15}$		.4948	$\frac{5\frac{15}{16}}{16}$		.7448	$\frac{8\frac{15}{16}}{16}$		.9948	$\frac{11\frac{15}{16}}{16}$
$\frac{1}{4}$	.2500	3	$\frac{1}{2}$	.5000	6	$\frac{3}{4}$	.7500	9	1	1.0000	12

WIRE ROPE ENGINEERING HANDBOOK

AREAS OF ROUND WIRES

Areas in Square Inches of Round Wires

Dia.	Area	Dia.	Area	Dia.	Area	Dia.	Area
.005	.00001963	.030	.0007068	.070	.003848	.110	.009503
.0055	.00002375	.031	.0007547	.071	.003959	.111	.009676
.006	.00002827	.032	.0008042	.072	.004071	.112	.009852
.0065	.00003318	.033	.0008552	.073	.004185	.113	.01002
.007	.00003848	.034	.0009079	.074	.004300	.114	.01022
.0075	.00004417	.035	.000962	.075	.004417	.115	.01038
.008	.00005026	.036	.001017	.076	.004536	.116	.01056
.0085	.00005674	.037	.001075	.077	.004656	.117	.01075
.009	.00006361	.038	.001134	.078	.004778	.118	.01093
.0095	.00007088	.039	.001194	.079	.004901	.119	.01112
.010	.00007854	.040	.001256	.080	.005026	.120	.01131
.0105	.00008659	.041	.001320	.081	.005153	.121	.01150
.011	.00009503	.042	.001385	.082	.005281	.122	.01169
.0115	.0001038	.043	.001452	.083	.005410	.123	.01188
.012	.0001131	.044	.001520	.084	.005541	.124	.01207
.0125	.0001227	.045	.001590	.085	.005674	.125	.01227
.013	.0001327	.046	.001661	.086	.005808	.126	.01246
.0135	.0001431	.047	.001734	.087	.005944	.127	.01266
.014	.0001539	.048	.001809	.088	.006082	.128	.01286
.0145	.0001651	.049	.001885	.089	.006221	.129	.01307
.015	.0001767	.050	.001963	.090	.006361	.130	.01327
.0155	.0001887	.051	.002042	.091	.006503	.131	.01348
.016	.0002010	.052	.002123	.092	.006647	.132	.01368
.0165	.0002138	.053	.002206	.093	.006792	.133	.01389
.017	.0002269	.054	.002290	.094	.006939	.134	.01410
.0175	.0002405	.055	.002375	.095	.007088	.135	.01431
.018	.0002544	.056	.002463	.096	.007238	.136	.01453
.0185	.0002688	.057	.002551	.097	.007390	.137	.01474
.019	.0002835	.058	.002642	.098	.007542	.138	.01496
.0195	.0002986	.059	.002734	.099	.007697	.139	.01517
.020	.0003141	.060	.002827	.100	.007854	.140	.01539
.021	.0003463	.061	.002922	.101	.008011	.141	.01561
.022	.0003801	.062	.003019	.102	.008171	.142	.01584
.023	.0004154	.063	.003117	.103	.008332	.143	.01606
.024	.0004523	.064	.003217	.104	.008494	.144	.01629
.025	.0004908	.065	.003318	.105	.008659	.145	.01651
.026	.0005309	.066	.003421	.106	.008824	.146	.01674
.027	.0005725	.067	.003525	.107	.008992	.147	.01697
.028	.0006157	.068	.003632	.108	.009160	.148	.01720
.029	.0006605	.069	.003739	.109	.009331	.149	.01744

AMERICAN TIGER BRAND

AREAS OF ROUND WIRES

Areas in Square Inches of Round Wires

Dia.	Area	Dia.	Area	Dia.	Area	Dia.	Area
.150	.01767	.183	.02630	.216	.03664	.249	.04869
.151	.01791	.184	.02660	.217	.03698	.250	.04909
.152	.01815	.185	.02688	.218	.03732	.251	.04949
.153	.01839	.186	.02717	.219	.03767	.252	.04988
.154	.01863	.187	.02747	.220	.03801	.253	.05027
.155	.01887	.188	.02776	.221	.03836	.254	.05067
.156	.01911	.189	.02806	.222	.03871	.255	.05107
.157	.01936	.190	.02835	.223	.03906	.256	.05147
.158	.01961	.191	.02865	.224	.03941	.257	.05187
.159	.01986	.192	.02895	.225	.03976	.258	.05228
.160	.02011	.193	.02926	.226	.04011	.259	.05268
.161	.02036	.194	.02956	.227	.04047	.260	.05309
.162	.02061	.195	.02986	.228	.04083	.261	.05350
.163	.02086	.196	.03017	.229	.04119	.262	.05391
.164	.02112	.197	.03048	.230	.04156	.263	.05432
.165	.02138	.198	.03079	.231	.04191	.264	.05474
.166	.02164	.199	.03110	.232	.04227	.265	.05515
.167	.02190	.200	.03141	.233	.04264	.266	.05557
.168	.02217	.201	.03173	.234	.04301	.267	.05599
.169	.02243	.202	.03205	.235	.04337	.268	.05641
.170	.02270	.203	.03237	.236	.04374	.269	.05683
.171	.02297	.204	.03269	.237	.04412	.270	.05726
.172	.02324	.205	.03301	.238	.04449	.271	.05768
.173	.02351	.206	.03333	.239	.04486	.272	.05811
.174	.02378	.207	.03365	.240	.04524	.273	.05853
.175	.02405	.208	.03398	.241	.04562	.274	.05896
.176	.02433	.209	.03431	.242	.04600	.275	.05939
.177	.02461	.210	.03464	.243	.04638		
.178	.02488	.211	.03497	.244	.04676		
.179	.02517	.212	.03530	.245	.04714		
.180	.02545	.213	.03563	.246	.04753		
.181	.02573	.214	.03597	.247	.04792		
.182	.02602	.215	.03631	.248	.04831		

WEIGHTS OF STEEL WIRES

Weights in Pounds per 1,000 Feet of Round Steel Wires

Dia.	Weight	Dia.	Weight	Dia.	Weight	Dia.	Weight
.005	0.0667	.032	2.731	.074	14.61	.116	35.89
.0055	0.0807	.033	2.905	.075	15.00	.117	36.52
.006	0.0960	.034	3.083	.076	15.41	.118	37.14
.0065	0.1127	.035	3.267	.077	15.82	.119	37.77
.007	0.1307	.036	3.457	.078	16.24	.120	38.41
.0075	0.1500	.037	3.652	.079	16.65	.121	39.06
.008	0.1707	.038	3.853	.080	17.07	.122	39.70
.0085	0.1927	.039	4.057	.081	17.50	.123	40.35
.009	0.2160	.040	4.268	.082	17.93	.124	41.00
.0095	0.2407	.041	4.484	.083	18.37	.125	41.68
.010	0.2667	.042	4.705	.084	18.82	.126	42.34
.0105	0.2941	.043	4.931	.085	19.27	.127	43.02
.011	0.3228	.044	5.164	.086	19.72	.128	43.70
.0115	0.3527	.045	5.401	.087	20.18	.129	44.39
.012	0.3841	.046	5.644	.088	20.65	.130	45.08
.0125	0.4168	.047	5.894	.089	21.13	.131	45.78
.013	0.4508	.048	6.145	.090	21.60	.132	46.47
.0135	0.4861	.049	6.404	.091	22.09	.133	47.18
.014	0.5228	.050	6.668	.092	22.58	.134	47.89
.0145	0.5608	.051	6.939	.093	23.07	.135	48.61
.015	0.6001	.052	7.212	.094	23.57	.136	49.33
.0155	0.6408	.053	7.493	.095	24.07	.137	50.07
.016	0.6828	.054	7.778	.096	24.58	.138	50.80
.0165	0.7261	.055	8.068	.097	25.09	.139	51.54
.017	0.7708	.056	8.364	.098	25.61	.140	52.28
.0175	0.8168	.057	8.665	.099	26.14	.141	53.03
.018	0.8642	.058	8.972	.100	26.67	.142	53.78
.0185	0.9128	.059	9.286	.101	27.21	.143	54.55
.019	0.9630	.060	9.602	.102	27.75	.144	55.31
.0195	1.014	.061	9.925	.103	28.30	.145	56.08
.020	1.067	.062	10.25	.104	28.85	.146	56.86
.021	1.176	.063	10.59	.105	29.41	.147	57.64
.022	1.291	.064	10.92	.106	29.97	.148	58.42
.023	1.411	.065	11.27	.107	30.54	.149	59.22
.024	1.536	.066	11.62	.108	31.11	.150	60.01
.025	1.667	.067	11.97	.109	31.69	.151	60.80
.026	1.804	.068	12.33	.110	32.27	.152	61.61
.027	1.945	.069	12.70	.111	32.86	.153	62.44
.028	2.091	.070	13.07	.112	33.45	.154	63.26
.029	2.243	.071	13.44	.113	34.06	.155	64.08
.030	2.400	.072	13.82	.114	34.66	.156	64.91
.031	2.562	.073	14.21	.115	35.27		

AMERICAN TIGER BRAND

WEIGHTS OF STEEL WIRES

Weights in Pounds per 1,000 Feet of Round Steel Wires

Dia.	Weight	Dia.	Weight	Dia.	Weight	Dia.	Weight
.157	65.74	.188	94.28	.219	127.9	.250	166.7
.158	66.59	.189	95.28	.220	129.1	.251	168.0
.159	67.43	.190	96.28	.221	130.2	.252	169.4
.160	68.28	.191	97.30	.222	131.4	.253	170.7
.161	69.14	.192	98.32	.223	132.6	.254	172.1
.162	70.00	.193	99.34	.224	133.8	.255	173.4
.163	70.87	.194	100.4	.225	135.0	.256	174.8
.164	71.74	.195	101.4	.226	136.2	.257	176.2
.165	72.61	.196	102.4	.227	137.4	.258	177.5
.166	73.48	.197	103.5	.228	138.6	.259	178.9
.167	74.38	.198	104.6	.229	139.8	.260	180.3
.168	75.28	.199	105.6	.230	141.1	.261	181.7
.169	76.18	.200	106.7	.231	142.4	.262	183.1
.170	77.08	.201	107.8	.232	143.6	.263	184.5
.171	77.98	.202	108.9	.233	144.8	.264	185.9
.172	78.91	.203	109.9	.234	146.1	.265	187.3
.173	79.83	.204	111.0	.235	147.3	.266	188.7
.174	80.75	.205	112.1	.236	148.6	.267	190.1
.175	81.68	.206	113.2	.237	149.8	.268	191.6
.176	82.62	.207	114.3	.238	151.1	.269	193.0
.177	83.56	.208	115.4	.239	152.4	.270	194.4
.178	84.51	.209	116.5	.240	153.6	.271	195.8
.179	85.46	.210	117.6	.241	154.9	.272	197.3
.180	86.42	.211	118.7	.242	156.2	.273	198.8
.181	87.38	.212	119.9	.243	157.5	.274	200.3
.182	88.36	.213	121.1	.244	158.8	.275	201.7
.183	89.34	.214	122.2	.245	160.1		
.184	90.32	.215	123.3	.246	161.4		
.185	91.30	.216	124.4	.247	162.7		
.186	92.28	.217	125.6	.248	164.0		
.187	93.28	.218	126.7	.249	165.4		

MEASURES AND WEIGHTS

Linear Measure

1000 mils	= 1 inch
12 inches	= 1 foot
3 feet	= 1 yard
2 yards	= $\begin{cases} 1 \text{ fathom} \\ 6 \text{ feet} \end{cases}$
$5\frac{1}{2}$ yards	= $\begin{cases} 1 \text{ rod} \\ 16\frac{1}{2} \text{ feet} \end{cases}$
40 rods	= $\begin{cases} 1 \text{ furlong} \\ 660 \text{ feet} \end{cases}$
8 furlongs	= $\begin{cases} 1 \text{ mile} \\ 5280 \text{ feet} \end{cases}$
1.15156 miles	= $\begin{cases} 1 \text{ nautical mile, or knot} \\ 6080.26 \text{ feet} \end{cases}$
3 nautical miles	= $\begin{cases} 1 \text{ league} \\ 18,240.78 \text{ feet} \end{cases}$

Square Measure

144 square inches	= 1 square foot
9 square feet	= 1 square yard
$30\frac{1}{4}$ square yds.	= $\begin{cases} 1 \text{ square rod} \\ 272\frac{1}{4} \text{ square feet} \end{cases}$
160 square rods	= $\begin{cases} 1 \text{ acre} \\ 43,560 \text{ square feet} \end{cases}$
640 acres	= $\begin{cases} 1 \text{ square mile} \\ 27,878,400 \text{ square feet} \end{cases}$
A circular mil is the area of a circle 1 mil, or 0.001 inch in diameter.	
1 square inch	= 1,273,239 circular mils
A circular inch is the area of a circle 1 inch in diameter = 0.7854 square inches.	
1 square inch	= 1.2732 circular inches

Cubic Measure

1728 cubic inches	= $\begin{cases} 1 \text{ cubic foot} \\ 7.4805 \text{ gallons} \end{cases}$
27 cubic feet	= 1 cubic yard
128 cubic feet	= 1 cord

Liquid Measure

4 gills	= 1 pint
2 pints	= 1 quart
4 quarts	= $\begin{cases} 1 \text{ gallon} \\ 231 \text{ cubic inches} \\ .134 \text{ cubic feet} \end{cases}$
$31\frac{1}{2}$ gallons	= 1 barrel
2 barrels	= 1 hogshead

Dry Measure

2 pints	= 1 quart
8 quarts	= 1 peck
4 pecks	= $\begin{cases} 1 \text{ bushel} \\ 2150.42 \text{ cubic inches} \\ 1.2445 \text{ cubic feet} \end{cases}$

Weight—Avoirdupois or Commercial

437.5 grains	= 1 ounce
16 ounces	= 1 pound
112 pounds	= 1 hundredweight
20 hundredweight	= $\begin{cases} 1 \text{ gross, or long ton} \\ 2240 \text{ pounds} \end{cases}$
2000 pounds	= 1 net, or short ton
2204.6 pounds	= 1 metric ton

METRIC SYSTEM OF MEASURES AND WEIGHTS

Linear Measure

1 millimeter	=	0.03937 inches
1 centimeter	=	0.3937 inches
1 decimeter	=	3.937 inches
1 meter	=	39.37 inches
	=	3.28083 feet
	=	1.09361 yards
	=	3280.83 feet
1 kilometer	=	1093.61 yards
	=	0.62137 miles
1 inch	=	25.4 millimeters
1 inch	=	2.54 centimeters
1 inch	=	.254 decimeters
1 inch	=	.0254 meters
1 foot	=	.3048 meters
1 yard	=	.9144 meters
1 mile	=	1.60935 kilometers

Square Measure

1 square millimeter	=	0.00155 square inches
	=	1973.5 circular mils
1 square centimeter	=	0.155 square inches
1 square decimeter	=	15.5 square inches
1 square meter	=	1550 square inches
	=	10.7639 square feet
	=	1.196 square yards
1 square kilometer	=	0.386109 square miles
	=	247.11 acres
1 square myriameter	=	38.6109 square miles
1 square inch	=	645.2 square millimeters
1 square inch	=	6.452 square centimeters
1 square inch	=	0.06452 square decimeters
1 square foot	=	0.0929 square meters
1 square yard	=	0.836 square meters

Cubic Measure

1 cubic centimeter	=	0.061 cubic inches
1 cubic decimeter	=	61.0234 cubic inches
	=	0.035314 cubic feet
1 cubic meter	=	35.314 cubic feet
	=	1.308 cubic yards

Weight

1 gram	=	15.432 grains
1 kilogram	=	2.204622 pounds
1 metric ton 1000 kilograms	=	2204.6 pounds
	=	0.9842 gross tons
	=	1.1023 net tons
1 grain	=	0.0648 gram
1 pound	=	0.4536 kilogram

Capacity

1 Liter	=	61.0234 cubic inches
	=	0.03531 cubic feet
	=	0.2642 gallons
1 cubic foot	=	28.317 liters
1 gallon	=	3.785 liters

Compound Units

1 gram per square millimeter	=	1.422 pounds per square inch
1 kilogram per square millimeter	=	1422.32 pounds per square inch
1 kilogram per square centimeter	=	14.2232 pounds per square inch
1 kilogram per square meter	=	0.2048 pounds per square foot
	=	1.8433 pounds per square yard
1 kilogram meter	=	7.2330 foot pounds
1 kilogram per meter	=	0.6720 pounds per foot
1 pound per square inch	=	0.07031 kilogram per square centimeter
1 pound per square foot	=	0.0004882 kilogram per square centimeter
1 pound per square foot	=	0.006944 pounds per square inch
1 pound per cubic inch	=	27679.7 kilograms per cubic meter
1 pound per cubic foot	=	16.0184 kilograms per cubic meter
1 kilogram per cubic meter	=	0.06243 pounds per cubic foot
1 foot per second	=	0.30480 meters per second
1 meter per second	=	3.28083 feet per second
1 meter per second	=	2.23693 miles per hour

TABLE OF MULTIPLES

Circumference of Circle	= Diameter $\times$ 3.1416
Area of Circle	= { Square of Diameter $\times$ 0.7854, or, Square of Radius $\times$ 3.1416, or, Square of Circumference $\times$ 0.07958
Area of Triangle	= Base $\times$ one-half altitude
Surface of Sphere	= { Circumference $\times$ diameter, or, Square of diameter $\times$ 3.1416
Volume of Sphere	= { Surface $\times$ one-sixth diameter, or, Cube of diameter $\times$ 0.5236
Area of Hexagon	= Square of Diameter of Inscribed Circle $\times$ 0.866
Area of Octagon	= Square of Diameter of Inscribed Circle $\times$ 0.828

ENGINEERING UNITS

1 Horsepower	= 33,000 foot pounds per minute 550 foot pounds per second 746 watts .746 kilowatts
1 Horsepower Hour	= .746 kilowatt hours 1,980,000 foot pounds 2,545 heat units (B.T.U.)
1 Kilowatt	= 1,000 watts 1.34 horsepower 737.3 foot pounds per second 44,240 foot pounds per minute 56.9 heat units (B.T.U.) per minute
1 Kilowatt Hour	= 1,000 watt hours 1.34 horsepower hours 2,654,200 foot pounds 3,412 heat units (B.T.U.)
1 British Thermal Unit	= 1,055 watt seconds 778 foot pounds .000293 kilowatt hour .000393 horsepower hour
1 Watt	= 1 joule per second .00134 horsepower 3,412 heat units (B.T.U.) per hour .7373 foot pounds per second 44.24 foot pounds per minute

SPECIFIC GRAVITIES AND WEIGHTS

Substance	Specific Gravity	Weight, Lbs. per Cu. Ft.	Substance	Specific Gravity	Weight, Lbs. per Cu. Ft.
METALS, ALLOYS, ORES			Sulphur, Amorphous.....	2.05	128
Aluminum, cast-hammered.....	2.55-2.75	165	Wax.....	0.95-0.98	60
Aluminum, bronze.....	7.7	481	TIMBER, U. S. SEASONED		
Antimony.....	6.62-6.72	416	Ash, white.....	0.65	41
Arsenic.....	5.73	358	Beech.....	0.70	44
Bismuth.....	9.70-9.78	608	Birch, yellow.....	0.68	43
Brass, cast-rolled.....	8.4-8.7	534	Cedar, Port Orford.....	0.47	29
Bronze (gun metal)— copper 88, tin 10, zinc 2%....	8.7	544	Cedar, white, red.....	0.35-0.37	22-23
Bronze (Phosphor)— copper 80, tin 10, lead 10%...	9.0	562	Chestnut.....	0.48	30
Chromium.....	6.80-6.92	428	Cypress, southern.....	0.51	32
Cobalt.....	8.72-8.95	552	Douglas Fir, coast type.....	0.54	34
Copper, cast-rolled.....	8.8-9.0	556	Douglas Fir, mountain.....	0.48	30
Copper, ore, pyrites.....	4.1-4.3	262	Elm, American.....	0.56	35
Gold, cast-hammered.....	19.25-19.35	1205	Hemlock, eastern, western.....	0.45	28
Iron, cast, pig.....	7.2	450	Hickory, bigleaf.....	0.77	48
Iron, wrought.....	7.6-7.9	485	Hickory, pignut.....	0.85	53
Iron, Spiegel-eisen.....	7.5	468	Larch, western.....	0.58	36
Iron, ferro-silicon.....	6.7-7.3	437	Maple, red, black.....	0.61-0.64	38-40
Iron, ore, hematite.....	5.2	325	Maple, silver.....	0.53	33
Iron, ore, hematite in bank.....		160-180	Oak, Oregon white.....	0.82	51
Iron, ore, hematite loose.....		130-160	Oak, red.....	0.71	44
Iron, ore, limonite.....	3.6-4.0	237	Pine, red.....	0.53	33
Iron, ore, magnetite.....	4.9-5.2	315	Pine, white, yellow, western.....	0.43-0.45	27-28
Iron, slag.....	2.5-3.0	172	Poplar, yellow.....	0.45	28
Lead.....	11.28-11.35	706	Redwood.....	0.48	30
Lead ore, galena.....	7.3-7.6	465	Spruce, black, red.....	0.45	28
Magnesium.....	1.74	109	Spruce, Engelmann.....	0.37	23
Manganese.....	7.20-7.42	456	Tamarack.....	0.60	37
Manganese ore, pyrolusite.....	3.7-4.6	259	Walnut.....	0.63-0.64	39-40
Mercury.....	13.59	848	Moisture Contents:		
Molybdenum.....	9.01	562	Seasoned timber 12%.....		
Nickel.....	8.57-8.90	545	Green timber up to 50%.....		
Nickel monel metal.....	8.8-9.0	556	VARIOUS LIQUIDS		
Platinum, cast-hammered.....	21.1-21.5	1330	Alcohol, 100%.....	0.79	49
Silver, cast-hammered.....	10.4-10.6	656	Acids, Muriatic 40%.....	1.20	75
Steel.....	7.8-7.9	490	Acids, nitric 91%.....	1.50	94
Tin, cast-hammered.....	7.2-7.5	459	Acids, sulphuric 87%.....	1.80	112
Tin, babbitt metal.....	7.1	443	Lye, soda.....66%.....	1.70	106
Tin, ore, cassiterite.....	6.4-7.0	418	Oils, vegetable.....	0.91-0.94	58
Tungsten.....	18.7-19.1	1180	Oils, mineral, lubricants.....	0.90-0.93	57
Vanadium.....	5.5-5.7	350	Petroleum.....	0.88	55
Zinc, cast-rolled.....	6.9-7.2	440	Gasoline.....	0.66-0.69	42
Zinc, ore, blende.....	3.9-4.2	253	Water, 4° C, max. density.....	1.0	62.428
VARIOUS SOLIDS			Water, 100° C.....	0.9584	59.830
Carbon, amorphous, graphitic...	1.88-2.25	129	Water, ice.....	0.88-0.92	56
Cork.....	0.24	15	Water, snow, fresh fallen.....	.125	8
Ebony.....	1.22	76	Water, sea water.....	1.02-1.03	64
Fats.....	0.92-0.94	58	GASES		
Glass, common, plate.....	2.40-2.72	160	Air, 0° C, 760 mm.....	1.0	.08071
Glass, crystal.....	2.90-3.00	184	Ammonia.....	0.5920	.0478
Glass, flint.....	3.15-3.90	220	Carbon dioxide.....	1.5291	.1234
Phosphorous, white.....	1.83	114	Carbon monoxide.....	0.9673	.0781
Porcelain, china.....	2.30-2.50	150	Gas, illuminating.....	0.35-0.45	.028-.036
Resins, Rosin, Amber.....	1.07	67	Gas, natural.....	0.47-0.48	.038-.039
Rubber, caoutchouc.....	0.93	58	Hydrogen.....	0.0693	.00559
Silicon.....	2.49	155	Nitrogen.....	0.9714	.0784
			Oxygen.....	1.1056	.0892

The specific gravities of solids and liquids refer to water at 4°C., those of gases to air at 0°C. and 760 mm pressure. The weights per cubic foot are derived from average specific gravities, except where stated that weights are for bulk, heaped or loose material, etc.

SPECIFIC GRAVITIES AND WEIGHTS

Substance	Specific Gravity	Weight, Lbs. per Cu. Ft.	Substance	Specific Gravity	Weight, Lbs. per Cu. Ft.
MINERALS			ASHLAR MASONRY		
Asbestos.....	2.1-2.8	153	Granite, gneiss.....	2.6-2.9	172
Barytes.....	4.50	281	Limestone, crystalline.....	2.4-2.7	160
Basalt.....	2.7-3.2	184	Limestone, oolitic.....	2.3	144
Bauxite.....	2.55	159	Marble.....	2.6-2.8	168
Borax.....	1.7-1.8	109	Sandstone, bluestone.....	2.2-2.5	147
Chalk.....	1.8-2.6	137	MORTAR RUBBLE MASONRY		
Clay, marl.....	1.8-2.6	137	Granite, gneiss.....	2.5-2.8	165
Dolomite.....	2.9	181	Limestone, crystalline.....	2.3-2.6	156
Feldspar, orthoclase.....	2.5-2.6	159	Limestone, oolitic.....	2.2	138
Granite, gneiss.....	2.6-2.9	172	Marble.....	2.5-2.7	162
Greenstone, trap.....	2.8-3.2	187	Sandstone, bluestone.....	2.1-2.4	140
Gypsum, alabaster.....	2.3-2.8	159	BRICK MASONRY		
Hornblende.....	3.0	187	Pressed brick.....	2.2-2.3	140
Limestone, crystalline.....	2.4-2.7	160	Common brick.....	1.8-2.0	120
Limestone, oolitic.....	2.3	144	Soft brick.....	1.5-1.7	100
Magnesite.....	3.0	187	CONCRETE		
Marble.....	2.7-2.8	168	Cement, stone, sand.....	2.2-2.4	144
Phosphate rock, apatite.....	3.2	200	Cement, slag, etc.....	1.9-2.3	130
Porphyry.....	2.6-2.9	172	Cement, cinder, etc.....	1.5-1.7	100
Pumice, natural.....	0.37-0.90	40	VARIOUS BUILDING MATERIAL		
Quartz, flint.....	2.5-2.8	165	Ashes, cinders.....		40-45
Sandstone, bluestone.....	2.2-2.5	147	Cement, Portland, loose.....		90
Slate, shale.....	2.7-2.8	172	Cement, Portland, set.....	2.7-3.2	183
Soapstone, talc.....	2.6-2.8	169	Lime, gypsum, loose.....		65-75
STONE, QUARRIED, PILED			Mortar, set.....	1.4-1.9	103
Basalt, granite, gneiss.....		96	Slags, bank slag.....		67-72
Limestone, marble, quartz.....		95	Slags, bank, screenings.....		98-117
Sandstone.....		82	Slags, machine slag.....		96
Shale.....		92	Slags, slag sand.....		49-55
Greenstone, hornblende.....		107	EARTH, ETC., EXCAVATED		
BITUMINOUS SUBSTANCES			Clay, dry.....		63
Asphaltum.....	1.1-1.5	81	Clay, damp, plastic.....		110
Coal, anthracite.....	1.4-1.7	97	Clay and gravel, dry.....		100
Coal, bituminous.....	1.2-1.5	84	Earth, dry, loose.....		76
Coal, lignite.....	1.1-1.4	78	Earth, dry, packed.....		95
Coal, peat, turf, dry.....	0.65-0.85	47	Earth, moist, loose.....		78
Coal, charcoal, pine.....	0.28-0.44	23	Earth, moist, packed.....		96
Coal, charcoal, oak.....	0.47-0.57	33	Earth, mud, flowing.....		108
Coal, coke.....	1.0-1.4	75	Earth, mud, packed.....		115
Graphite.....	1.9-2.3	131	Riprap, limestone.....		80-85
Paraffine.....	0.87-0.91	56	Riprap, sandstone.....		90
Petroleum, crude.....	0.88	55	Riprap, shale.....		105
Petroleum, refined.....	0.79-0.82	50	Sand, gravel, dry, loose.....		90-105
Petroleum, benzine.....	0.73-0.75	46	Sand, gravel, dry, packed.....		100-120
Petroleum, gasoline.....	0.66-0.69	42	Sand, gravel, wet.....		118-120
Pitch.....	1.07-1.15	69	EXCAVATIONS IN WATER		
Tar, bituminous.....	1.20	75	Sand or gravel.....		60
COAL AND COKE, PILED			Sand or gravel and clay.....		65
Coal, anthracite.....		47-58	Clay.....		80
Coal, bituminous, lignite.....		40-54	River mud.....		90
Coal, peat, turf.....		20-26	Soil.....		70
Coal, charcoal.....		10-14	Stone riprap.....		65
Coal, coke.....		23-32			

The specific gravities of solids and liquids refer to water at 4°C., those of gases to air at 0°C. and 760 mm pressure. The weights per cubic foot are derived from average specific gravities, except where stated that weights are for bulk, heaped or loose material, etc.

AMERICAN TIGER BRAND

STRENGTH OF NEW MANILA ROPES

Circumference in Inches	Nominal Diameter in Inches	Minimum Tensile Strength in Pounds
$\frac{9}{16}$	$\frac{3}{16}$	420
$\frac{3}{4}$	$\frac{1}{4}$	550
1	$\frac{5}{16}$	950
$1\frac{1}{8}$	$\frac{3}{8}$	1,275
$1\frac{1}{4}$	$\frac{7}{16}$	1,750
$1\frac{3}{8}$	$\frac{15}{32}$	2,250
$1\frac{1}{2}$	$\frac{1}{2}$	2,650
$1\frac{3}{4}$	$\frac{9}{16}$	3,450
2	$\frac{5}{8}$	4,400
$2\frac{1}{4}$	$\frac{3}{4}$	5,400
$2\frac{1}{2}$	$\frac{13}{16}$	6,500
$2\frac{3}{4}$	$\frac{7}{8}$	7,700
3	1	9,000
$3\frac{1}{4}$	$1\frac{1}{16}$	10,500
$3\frac{1}{2}$	$1\frac{1}{8}$	12,000
$3\frac{3}{4}$	$1\frac{1}{4}$	13,500
4	$1\frac{5}{16}$	15,000
$4\frac{1}{2}$	$1\frac{1}{2}$	18,500
5	$1\frac{5}{8}$	22,500
$5\frac{1}{2}$	$1\frac{3}{4}$	26,500
6	2	31,000
$6\frac{1}{2}$	$2\frac{1}{8}$	36,000
7	$2\frac{1}{4}$	41,000
$7\frac{1}{2}$	$2\frac{1}{2}$	46,500
8	$2\frac{5}{8}$	52,000
$8\frac{1}{2}$	$2\frac{7}{8}$	58,000
9	3	64,000
$9\frac{1}{2}$	$3\frac{1}{8}$	71,000
10	$3\frac{1}{4}$	77,000
11	$3\frac{1}{2}$	91,000
12	4	105,000

STRENGTH OF MATERIALS

Stresses in Pounds per Square Inch

Building Materials	ULTIMATE AVERAGE STRESSES			Modulus of Elasticity	SAFE WORKING STRESSES		
	Compress.	Tension	Bending		Compress.	Bearing	Shearing
STONE							
Granite, gneiss, bluestone.....	12,000	1,200	1,600	7,000,000	1,200	1,200	200
Limestone, marble.....	8,000	800	1,500	7,000,000	800	800	150
Sandstone.....	5,000	150	1,200	3,000,000	500	500	150
Slate.....	10,000	3,000	5,000	14,000,000	1,000	1,000	175
MASONRY							
Granite.....					420	600	
Limestone, bluestone.....					350	500	
Sandstone.....					280	400	
Rubble.....					140	250	
Rubble, coursed.....					170	250	
Brick, medium burned.....	10,000				170	300	
Brick, hard burned.....	15,000				210	300	
Brick, pressed, paving brick.....	6,000						
Terra Cotta.....	5,000						
CEMENT, PORTLAND							
Neat, 28 days.....	7,040	740					
Neat, 90 days.....	7,350	740					
1:3 sand, 28 days.....	1,290	320					
1:3 sand, 90 days.....	1,490	340					
CONCRETE, P. C.				REINFORCED CONCRETE			
1:1:2 { Granite, trap rock.....	3,300	Modulus of Elasticity	{ 3,000,000 for ult. compression over... 2,900.				
1:1:2 { Furnace Slag.....	3,000		{ 2,500,000 for ult. compression up to... 2,900.				
1:1:2 { Lime and Sandstone, hard..	3,000		{ 2,000,000 for ult. compression up to... 2,200.				
1:1:2 { Lime and Sandstone, soft...	2,200		{ 750,000 for ult. compression under... 800.				
1:1:2 { Cinders.....	800						
1:1½:3 { Granite, trap rock.....	2,800	Safe Working Stresses in Percent of Ultimate Compression					
1:1½:3 { Furnace Slag.....	2,500						
1:1½:3 { Lime and Sandstone, hard..	2,500	Compression	{ Plain Concrete Piers, length 4 dia.... 22.5%				
1:1½:3 { Lime and Sandstone, soft...	1,800		{ Reinforced Columns, length 12 dia.... 22.5%				
1:1½:3 { Cinders.....	700	{ Reinforced Beams..... 32.5%					
1:2:4 { Granite, trap rock.....	2,200	Bearing	Surface twice the loaded area..... 35.0%				
1:2:4 { Furnace Slag.....	2,000						
1:2:4 { Lime and Sandstone, hard..	2,000	Shear and Diag. Tension	{ Horizontal Bars, no web reinforcement 2.0%				
1:2:4 { Lime and Sandstone, soft...	1,500		{ Horizontal Bars, vertical stirrups.... 4.5%				
1:2½:5 { Granite, trap rock.....	1,800	{ Bent Bars and vertical stirrups..... 5.0%					
1:2½:5 { Furnace Slag.....	1,600	{ Same, securely attached..... 6.0%					
1:2½:5 { Lime and Sandstone, hard..	1,600	Bond Stress	{ Drawn Wire..... 2.0%				
1:2½:5 { Lime and Sandstone, soft...	1,200		{ Plain reinforcing bars..... 4.0%				
1:3:6 { Granite, trap rock.....	1,400	{ Deformed Bars, best type..... 5.0%					
1:3:6 { Furnace Slag.....	1,300	For complete data see Transactions of the American Society of Civil Engineers, Vol. LXXXI-No. 1398, Dec. 1917					
1:3:6 { Lime and Sandstone, hard..	1,300						
1:3:6 { Lime and Sandstone, soft...	1,000						
1:3:6 { Cinders.....	400						
MISCELLANEOUS							
Glass, common.....	30,000	3,000					
Plaster.....	700	70	3,000	8,000,000			

AMERICAN TIGER BRAND

STRENGTH OF MATERIALS

Stresses per Square Inch

Metals and Alloys	STRESSES IN KIPS					Modulus of Elasticity	Elongation, %
	Tension, Ultimate	Elastic Limit	Compression, Ultimate	Bending, Ultimate	Shearing, Ultimate		
Aluminum, cast	15	6.5	12		12	11,000,000	
Aluminum, bars, sheets	24-28	12-14					
Aluminum, wire, hard	30-65	16-30					
Aluminum, wire, annealed	20-35	14					
Aluminum, 2% to 7% Ni, Cu, Fe, etc.	40-50	25					
Aluminum Bronze, 5% to 7½% Al	75	40	120				
Aluminum Bronze, 10% Al	85-100	60					
Copper, cast	25	6	40	22	30	10,000,000	
Copper, plates, rods, bolts	32-35	10	32				
Copper, wire, hard	55-65					18,000,000	
Copper, wire, annealed	36	10				15,000,000	
Brass, 17% Zn	32.6	8.2		23.2			26.7
Brass, 23% Zn		7.6	42	22.3			35.8
Brass, 30% Zn	28.1	8.6		26.9			20.7
Brass, 39% Zn	41.1	17.4	75	39			20.7
Brass, 50% Zn	31	17.9	117	33.5			5.0
Brass, cast, common	18-24	6	30	20	36	9,000,000	
Brass, wire, hard	80						
Brass, wire, annealed	50	16				14,000,000	
Bronze 8% Sn	28.5	19	42	43.7		10,000,000	5.5
Bronze 13% Sn	29.4	20	53	34.5			3.3
Bronze 20% Sn	33		78	56.7			0.04
Bronze 24% Sn	22	22	114	32			0
Bronze 30% Sn	5.6	5.6	147	12.1			0
Bronze gun metal, 9 Cu, 1 Sn	25-55	10		52		10,000,000	
Bronze Manganese, cast } 10% Sn	60	30	125				
Bronze Manganese, rolled } 2% Mn	100	80					
Bronze Phosphorus, cast } 9% Sn	50	24					
Bronze Phosphorus, wire } 1% P	100						
Bronze Silicon, cast, 3% Si	55						
Bronze Silicon, cast, 5% Si	75						
Bronze Silicon, wire	108						
Bronze Tobin, cast } 38% Zn	66						
Bronze Tobin, rolled } 1½% Sn	80	40				4,500,000	
Bronze Tobin, cold rolled } ½% Pb	100						
Delta Metal, cast } 55% to 60% Cu	45						
Delta Metal, plates } 38% to 40% Zn	68						
Delta Metal, bars } 2% to 4% Fe	85						
Delta Metal, wire } 1% to 2% Sn	100						
German Silver, 25% Zn, 20% Ni							
Iron, see next page							
Gold, cast	20	4				8,000,000	
Gold, wire	30						
Gold, copper, 5 Au, 1 Cu	50						
Lead, cast	1.8					1,000,000	
Lead, pipe, wire	2.2-2.5					1,000,000	
Lead, rolled sheets	3.3					720,000	
Platinum, wire, unannealed	53						
Platinum, wire, annealed	32						
Silver, cast	40						
Steel, see next page							
Tin, cast	3.5-4.6	1.5-1.8	6	4		4,000,000	
Tin, antimony, 10 Sn, 1 Sb	11						
Zinc, cast	4-6	4	18	7		13,000,000	
Zinc, rolled sheets	7-16						

STRENGTH OF MATERIALS

Stresses per Square Inch

Metals and Alloys	STRESSES IN KIPS					Modulus of Elasticity	Elongation, %
	Tension, Ultimate	Elastic Limit	Com- pression, Ultimate	Bending, Ultimate	Shearing, Ultimate		
STEEL							
Shapes, Plates, Bars*							
Bridges.....	55-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	27.3-23.1
Buildings.....	55-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	25.5-21.5
Cars.....	50-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	30.0-23.1
Locomotives.....	55-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	27.3-23.1
Ships.....	60-72	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	25.0-21.1
Boiler Plates*							
Flange plates.....	55-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	27.3-23.1
Fire box.....	52-62	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	28.8-24.2
Rivets*							
Boilers.....	45-55	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	33.3-27.3
Bridges.....	52-62	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	28.8-24.2
Buildings.....	52-62	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	28.8-24.2
Cars.....	52-62	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	28.8-24.2
Ships.....	55-65	1/2 tens.	tensile	tensile	3/4 tens.	29,000,000	27.3-23.0
Concrete Bars*							
Plain, structural grade.....	55-70	33	tensile	tensile	3/4 tens.	29,000,000	25.5-20.0
Plain, intermediate.....	70-90	40	tensile	tensile	3/4 tens.	29,000,000	18.6-14.3
Plain hard.....	80	50	tensile	tensile	3/4 tens.	29,000,000	15.0
Deformed, structural grade.....	55-70	33	tensile	tensile	3/4 tens.	29,000,000	22.7-17.9
Deformed, intermediate.....	70-90	40	tensile	tensile	3/4 tens.	29,000,000	16.1-11.3
Deformed, hard.....	80	50	tensile	tensile	3/4 tens.	29,000,000	12.5
Cold twisted.....		55	tensile	tensile	3/4 tens.	29,000,000	5.0
Castings*							
Soft.....	60	30	tensile	tensile	3/4 tens.	29,000,000	26
Medium.....	70	38	tensile	tensile	3/4 tens.	29,000,000	24
Hard.....	80	43	tensile	tensile	3/4 tens.	29,000,000	17
Forgings*							
STEEL ALLOYS							
Nickel Steel,* 3.25% Ni.							
Shapes, plates, bars.....	85-100	50	tensile	tensile	3/4 tens.	29,000,000	17.6-15.0
Rivets.....	70-80	45	tensile	tensile	3/4 tens.	29,000,000	21.4-18.8
Eye bars, unannealed.....	95-110	55	tensile	tensile	3/4 tens.	29,000,000	15.8-13.6
Eye bars, annealed.....	90-105	52	tensile	tensile	3/4 tens.	29,000,000	20.0
STEEL SPRINGS AND WIRE							
Springs, untempered.....	65-110	40-70					
Wire, unannealed.....	120	60					
Wire, annealed.....	80	40					
Wire, bridge cable.....	220	150					
WROUGHT IRON							
Shapes.....	48	26	tensile	tensile	5/6 tens.	28,000,000	
Bars.....	50	27	tensile	tensile	5/6 tens.	28,000,000	
Wire, unannealed.....	80					15,000,000	
Wire, annealed.....	60	27				25,000,000	
CAST IRON							
Common.....	15-18	6	80	30	18-20	12,000,000	
Gray.....	18-24			25-33			
Malleable.....	27-35	15-20	46	30	40		

*See Specifications of the American Society for Testing Materials.

AMERICAN TIGER BRAND

SINES AND COSINES

Natural Trigonometric Functions

Degrees	SINES							Cosines
	0'	10'	20'	30'	40'	50'	60'	
0	0.00000	0.00291	0.00582	0.00873	0.01164	0.01454	0.01745	89
1	0.01745	0.02036	0.02327	0.02618	0.02908	0.03199	0.03490	88
2	0.03490	0.03781	0.04071	0.04362	0.04653	0.04943	0.05234	87
3	0.05234	0.05524	0.05814	0.06105	0.06395	0.06685	0.06976	86
4	0.06976	0.07266	0.07556	0.07846	0.08136	0.08426	0.08716	85
5	0.08716	0.09005	0.09295	0.09585	0.09874	0.10164	0.10453	84
6	0.10453	0.10742	0.11031	0.11320	0.11609	0.11898	0.12187	83
7	0.12187	0.12476	0.12764	0.13053	0.13341	0.13629	0.13917	82
8	0.13917	0.14205	0.14493	0.14781	0.15069	0.15356	0.15643	81
9	0.15643	0.15931	0.16218	0.16505	0.16792	0.17078	0.17365	80
10	0.17365	0.17651	0.17937	0.18224	0.18509	0.18795	0.19081	79
11	0.19081	0.19366	0.19652	0.19937	0.20222	0.20507	0.20791	78
12	0.20791	0.21076	0.21360	0.21644	0.21928	0.22212	0.22495	77
13	0.22495	0.22778	0.23062	0.23345	0.23627	0.23910	0.24192	76
14	0.24192	0.24474	0.24756	0.25038	0.25320	0.25601	0.25882	75
15	0.25882	0.26163	0.26443	0.26724	0.27004	0.27284	0.27564	74
16	0.27564	0.27843	0.28123	0.28402	0.28680	0.28959	0.29237	73
17	0.29237	0.29515	0.29793	0.30071	0.30348	0.30625	0.30902	72
18	0.30902	0.31178	0.31454	0.31730	0.32006	0.32282	0.32557	71
19	0.32557	0.32832	0.33106	0.33381	0.33655	0.33929	0.34202	70
20	0.34202	0.34475	0.34748	0.35021	0.35293	0.35565	0.35837	69
21	0.35837	0.36108	0.36379	0.36650	0.36921	0.37191	0.37461	68
22	0.37461	0.37730	0.37999	0.38268	0.38537	0.38805	0.39073	67
23	0.39073	0.39341	0.39608	0.39875	0.40142	0.40408	0.40674	66
24	0.40674	0.40939	0.41204	0.41469	0.41734	0.41998	0.42262	65
25	0.42262	0.42525	0.42788	0.43051	0.43313	0.43575	0.43837	64
26	0.43837	0.44098	0.44359	0.44620	0.44880	0.45140	0.45399	63
27	0.45399	0.45658	0.45917	0.46175	0.46433	0.46690	0.46947	62
28	0.46947	0.47204	0.47460	0.47716	0.47971	0.48226	0.48481	61
29	0.48481	0.48735	0.48989	0.49242	0.49495	0.49748	0.50000	60
30	0.50000	0.50252	0.50503	0.50754	0.51004	0.51254	0.51504	59
31	0.51504	0.51753	0.52002	0.52250	0.52498	0.52745	0.52992	58
32	0.52992	0.53238	0.53484	0.53730	0.53975	0.54220	0.54464	57
33	0.54464	0.54708	0.54951	0.55194	0.55436	0.55678	0.55919	56
34	0.55919	0.56160	0.56401	0.56641	0.56880	0.57119	0.57358	55
35	0.57358	0.57596	0.57833	0.58070	0.58307	0.58543	0.58779	54
36	0.58779	0.59014	0.59248	0.59482	0.59716	0.59949	0.60182	53
37	0.60182	0.60414	0.60645	0.60876	0.61107	0.61337	0.61566	52
38	0.61566	0.61795	0.62024	0.62251	0.62479	0.62706	0.62932	51
39	0.62932	0.63158	0.63383	0.63608	0.63832	0.64056	0.64279	50
40	0.64279	0.64501	0.64723	0.64945	0.65166	0.65386	0.65606	49
41	0.65606	0.65825	0.66044	0.66262	0.66480	0.66697	0.66913	48
42	0.66913	0.67129	0.67344	0.67559	0.67773	0.67987	0.68200	47
43	0.68200	0.68412	0.68624	0.68835	0.69046	0.69256	0.69466	46
44	0.69466	0.69675	0.69883	0.70091	0.70298	0.70505	0.70711	45
Sines	60'	50'	40'	30'	20'	10'	0'	Degrees
COSINES								

SINES AND COSINES

Natural Trigonometric Functions

Degrees	COSINES							Sines
	0'	10'	20'	30'	40'	50'	60'	
0	1.00000	1.00000	0.99998	0.99996	0.99993	0.99989	0.99985	89
1	0.99985	0.99979	0.99973	0.99966	0.99958	0.99949	0.99939	88
2	0.99939	0.99929	0.99917	0.99905	0.99892	0.99878	0.99863	87
3	0.99863	0.99847	0.99831	0.99813	0.99795	0.99776	0.99756	86
4	0.99756	0.99736	0.99714	0.99692	0.99668	0.99644	0.99619	85
5	0.99619	0.99594	0.99567	0.99540	0.99511	0.99482	0.99452	84
6	0.99452	0.99421	0.99390	0.99357	0.99324	0.99290	0.99255	83
7	0.99255	0.99219	0.99182	0.99144	0.99106	0.99067	0.99027	82
8	0.99027	0.98986	0.98944	0.98902	0.98858	0.98814	0.98769	81
9	0.98769	0.98723	0.98676	0.98629	0.98580	0.98531	0.98481	80
10	0.98481	0.98430	0.98378	0.98325	0.98272	0.98218	0.98163	79
11	0.98163	0.98107	0.98050	0.97992	0.97934	0.97875	0.97815	78
12	0.97815	0.97754	0.97692	0.97630	0.97566	0.97502	0.97437	77
13	0.97437	0.97371	0.97304	0.97237	0.97169	0.97100	0.97030	76
14	0.97030	0.96959	0.96887	0.96815	0.96742	0.96667	0.96593	75
15	0.96593	0.96517	0.96440	0.96363	0.96285	0.96206	0.96126	74
16	0.96126	0.96046	0.95964	0.95882	0.95799	0.95715	0.95630	73
17	0.95630	0.95545	0.95459	0.95372	0.95284	0.95195	0.95106	72
18	0.95106	0.95015	0.94924	0.94832	0.94740	0.94646	0.94552	71
19	0.94552	0.94457	0.94361	0.94264	0.94167	0.94068	0.93969	70
20	0.93969	0.93869	0.93769	0.93667	0.93565	0.93462	0.93358	69
21	0.93358	0.93253	0.93148	0.93042	0.92935	0.92827	0.92718	68
22	0.92718	0.92609	0.92499	0.92388	0.92276	0.92164	0.92050	67
23	0.92050	0.91936	0.91822	0.91706	0.91590	0.91472	0.91355	66
24	0.91355	0.91236	0.91116	0.90996	0.90875	0.90753	0.90631	65
25	0.90631	0.90507	0.90383	0.90259	0.90133	0.90007	0.89879	64
26	0.89879	0.89752	0.89623	0.89493	0.89363	0.89232	0.89101	63
27	0.89101	0.88968	0.88835	0.88701	0.88566	0.88431	0.88295	62
28	0.88295	0.88158	0.88020	0.87882	0.87743	0.87603	0.87462	61
29	0.87462	0.87321	0.87178	0.87036	0.86892	0.86748	0.86603	60
30	0.86603	0.86457	0.86310	0.86163	0.86015	0.85866	0.85717	59
31	0.85717	0.85567	0.85416	0.85264	0.85112	0.84959	0.84805	58
32	0.84805	0.84650	0.84495	0.84339	0.84182	0.84025	0.83867	57
33	0.83867	0.83708	0.83549	0.83389	0.83228	0.83066	0.82904	56
34	0.82904	0.82741	0.82577	0.82413	0.82248	0.82082	0.81915	55
35	0.81915	0.81748	0.81580	0.81412	0.81242	0.81072	0.80902	54
36	0.80902	0.80730	0.80558	0.80386	0.80212	0.80038	0.79864	53
37	0.79864	0.79688	0.79512	0.79335	0.79158	0.78980	0.78801	52
38	0.78801	0.78622	0.78442	0.78261	0.78079	0.77897	0.77715	51
39	0.77715	0.77531	0.77347	0.77162	0.76977	0.76791	0.76604	50
40	0.76604	0.76417	0.76229	0.76041	0.75851	0.75661	0.75471	49
41	0.75471	0.75280	0.75088	0.74896	0.74703	0.74509	0.74314	48
42	0.74314	0.74120	0.73924	0.73728	0.73531	0.73333	0.73135	47
43	0.73135	0.72937	0.72737	0.72537	0.72337	0.72136	0.71934	46
44	0.71934	0.71732	0.71529	0.71325	0.71121	0.70916	0.70711	45
Cosines	60'	50'	40'	30'	20'	10'	0'	Degrees
SINES								

AMERICAN TIGER BRAND

TANGENTS AND COTANGENTS

Natural Trigonometric Functions

Degrees	TANGENTS							Cotangents
	0'	10'	20'	30'	40'	50'	60'	
0	0.00000	0.00291	0.00582	0.00873	0.01164	0.01455	0.01746	89
1	0.01746	0.02036	0.02328	0.02619	0.02910	0.03201	0.03492	88
2	0.03492	0.03783	0.04075	0.04366	0.04658	0.04949	0.05241	87
3	0.05241	0.05533	0.05824	0.06116	0.06408	0.06700	0.06993	86
4	0.06993	0.07285	0.07578	0.07870	0.08163	0.08456	0.08749	85
5	0.08749	0.09042	0.09335	0.09629	0.09923	0.10216	0.10510	84
6	0.10510	0.10805	0.11099	0.11394	0.11688	0.11983	0.12278	83
7	0.12278	0.12574	0.12869	0.13165	0.13461	0.13758	0.14054	82
8	0.14054	0.14351	0.14648	0.14945	0.15243	0.15540	0.15838	81
9	0.15838	0.16137	0.16435	0.16734	0.17033	0.17333	0.17633	80
10	0.17633	0.17933	0.18233	0.18534	0.18835	0.19136	0.19438	79
11	0.19438	0.19740	0.20042	0.20345	0.20648	0.20952	0.21256	78
12	0.21256	0.21560	0.21864	0.22169	0.22475	0.22781	0.23087	77
13	0.23087	0.23393	0.23700	0.24008	0.24316	0.24624	0.24933	76
14	0.24933	0.25242	0.25552	0.25862	0.26172	0.26483	0.26795	75
15	0.26795	0.27107	0.27419	0.27732	0.28046	0.28360	0.28675	74
16	0.28675	0.28990	0.29305	0.29621	0.29938	0.30255	0.30573	73
17	0.30573	0.30891	0.31210	0.31530	0.31850	0.32171	0.32492	72
18	0.32492	0.32814	0.33136	0.33460	0.33783	0.34108	0.34433	71
19	0.34433	0.34758	0.35085	0.35412	0.35740	0.36068	0.36397	70
20	0.36397	0.36727	0.37057	0.37388	0.37720	0.38053	0.38386	69
21	0.38386	0.38721	0.39055	0.39391	0.39727	0.40065	0.40403	68
22	0.40403	0.40741	0.41081	0.41421	0.41763	0.42105	0.42447	67
23	0.42447	0.42791	0.43136	0.43481	0.43828	0.44175	0.44523	66
24	0.44523	0.44872	0.45222	0.45573	0.45924	0.46277	0.46631	65
25	0.46631	0.46985	0.47341	0.47698	0.48055	0.48414	0.48773	64
26	0.48773	0.49134	0.49495	0.49858	0.50222	0.50587	0.50953	63
27	0.50953	0.51320	0.51688	0.52057	0.52427	0.52798	0.53171	62
28	0.53171	0.53545	0.53920	0.54296	0.54674	0.55051	0.55431	61
29	0.55431	0.55812	0.56194	0.56577	0.56962	0.57348	0.57735	60
30	0.57735	0.58124	0.58513	0.58905	0.59297	0.59691	0.60086	59
31	0.60086	0.60483	0.60881	0.61280	0.61681	0.62083	0.62487	58
32	0.62487	0.62892	0.63299	0.63707	0.64117	0.64528	0.64941	57
33	0.64941	0.65355	0.65771	0.66189	0.66608	0.67028	0.67451	56
34	0.67451	0.67875	0.68301	0.68728	0.69157	0.69588	0.70021	55
35	0.70021	0.70455	0.70891	0.71329	0.71769	0.72211	0.72654	54
36	0.72654	0.73100	0.73547	0.73996	0.74447	0.74900	0.75355	53
37	0.75355	0.75812	0.76272	0.76733	0.77196	0.77661	0.78129	52
38	0.78129	0.78598	0.79070	0.79544	0.80020	0.80498	0.80978	51
39	0.80978	0.81461	0.81946	0.82434	0.82923	0.83415	0.83910	50
40	0.83910	0.84407	0.84906	0.85408	0.85912	0.86419	0.86929	49
41	0.86929	0.87441	0.87955	0.88473	0.88992	0.89515	0.90040	48
42	0.90040	0.90569	0.91099	0.91633	0.92170	0.92709	0.93252	47
43	0.93252	0.93797	0.94345	0.94896	0.95451	0.96008	0.96569	46
44	0.96569	0.97133	0.97700	0.98270	0.98843	0.99420	1.00000	45
Tangents	60'	50'	40'	30'	20'	10'	0'	Degrees
COTANGENTS								

TANGENTS AND COTANGENTS

Natural Trigonometric Functions

Degrees	COTANGENTS							Tangents
	0'	10'	20'	30'	40'	50'	60'	
0	∞	343.77371	171.88540	114.58865	85.93979	68.75009	57.28996	89
1	57.28996	49.10388	42.96408	38.18846	34.36777	31.24158	28.63625	88
2	28.63625	26.43160	24.54176	22.90377	21.47040	20.20555	19.08114	87
3	19.08114	18.07498	17.16934	16.34986	15.60478	14.92442	14.30067	86
4	14.30067	13.72674	13.19688	12.70621	12.25051	11.82617	11.43005	85
5	11.43005	11.05943	10.71191	10.38540	10.07803	9.78817	9.51436	84
6	9.51436	9.25530	9.00983	8.77689	8.55555	8.34496	8.14435	83
7	8.14435	7.95302	7.77035	7.59575	7.42871	7.26873	7.11537	82
8	7.11537	6.96823	6.82694	6.69116	6.56055	6.43484	6.31375	81
9	6.31375	6.19703	6.08444	5.97576	5.87080	5.76937	5.67128	80
10	5.67128	5.57638	5.48451	5.39552	5.30928	5.22566	5.14455	79
11	5.14455	5.06584	4.98940	4.91516	4.84300	4.77286	4.70463	78
12	4.70463	4.63825	4.57363	4.51071	4.44942	4.38969	4.33148	77
13	4.33148	4.27471	4.21933	4.16530	4.11256	4.06107	4.01078	76
14	4.01078	3.96165	3.91364	3.86671	3.82083	3.77595	3.73205	75
15	3.73205	3.68909	3.64705	3.60588	3.56557	3.52609	3.48741	74
16	3.48741	3.44951	3.41236	3.37594	3.34023	3.30521	3.27085	73
17	3.27085	3.23714	3.20406	3.17159	3.13972	3.10842	3.07768	72
18	3.07768	3.04749	3.01783	2.98869	2.96004	2.93189	2.90421	71
19	2.90421	2.87700	2.85023	2.82391	2.79802	2.77254	2.74748	70
20	2.74748	2.72281	2.69853	2.67462	2.65109	2.62791	2.60509	69
21	2.60509	2.58261	2.56046	2.53865	2.51715	2.49597	2.47509	68
22	2.47509	2.45451	2.43422	2.41421	2.39449	2.37504	2.35585	67
23	2.35585	2.33693	2.31826	2.29984	2.28167	2.26374	2.24604	66
24	2.24604	2.22857	2.21132	2.19430	2.17749	2.16090	2.14451	65
25	2.14451	2.12832	2.11233	2.09654	2.08094	2.06553	2.05030	64
26	2.05030	2.03526	2.02039	2.00569	1.99116	1.97680	1.96261	63
27	1.96261	1.94858	1.93470	1.92098	1.90741	1.89400	1.88073	62
28	1.88073	1.86760	1.85462	1.84177	1.82907	1.81649	1.80405	61
29	1.80405	1.79174	1.77955	1.76749	1.75556	1.74375	1.73205	60
30	1.73205	1.72047	1.70901	1.69766	1.68643	1.67530	1.66428	59
31	1.66428	1.65337	1.64256	1.63185	1.62125	1.61074	1.60033	58
32	1.60033	1.59002	1.57981	1.56969	1.55966	1.54972	1.53987	57
33	1.53987	1.53010	1.52043	1.51084	1.50133	1.49190	1.48256	56
34	1.48256	1.47330	1.46411	1.45501	1.44598	1.43703	1.42815	55
35	1.42815	1.41934	1.41061	1.40195	1.39336	1.38484	1.37638	54
36	1.37638	1.36800	1.35968	1.35142	1.34323	1.33511	1.32704	53
37	1.32704	1.31904	1.31110	1.30323	1.29541	1.28764	1.27994	52
38	1.27994	1.27230	1.26471	1.25717	1.24969	1.24227	1.23490	51
39	1.23490	1.22758	1.22031	1.21310	1.20593	1.19882	1.19175	50
40	1.19175	1.18474	1.17777	1.17085	1.16398	1.15715	1.15037	49
41	1.15037	1.14363	1.13694	1.13029	1.12369	1.11713	1.11061	48
42	1.11061	1.10414	1.09770	1.09131	1.08496	1.07864	1.07237	47
43	1.07237	1.06613	1.05994	1.05378	1.04766	1.04158	1.03553	46
44	1.03553	1.02952	1.02355	1.01761	1.01170	1.00583	1.00000	45
Cotangents	60'	50'	40'	30'	20'	10'	0'	Degrees

TANGENTS

AMERICAN TIGER BRAND

SECANTS AND COSECANTS

Natural Trigonometric Functions

Degrees	SECANTS							Cosecants
	0'	10'	20'	30'	40'	50'	60'	
0	1.00000	1.00000	1.00002	1.00004	1.00007	1.00011	1.00015	89
1	1.00015	1.00021	1.00027	1.00034	1.00042	1.00051	1.00061	88
2	1.00061	1.00072	1.00083	1.00095	1.00108	1.00122	1.00137	87
3	1.00137	1.00153	1.00169	1.00187	1.00205	1.00224	1.00244	86
4	1.00244	1.00265	1.00287	1.00309	1.00333	1.00357	1.00382	85
5	1.00382	1.00408	1.00435	1.00463	1.00491	1.00521	1.00551	84
6	1.00551	1.00582	1.00614	1.00647	1.00681	1.00715	1.00751	83
7	1.00751	1.00787	1.00825	1.00863	1.00902	1.00942	1.00983	82
8	1.00983	1.01024	1.01067	1.01111	1.01155	1.01200	1.01247	81
9	1.01247	1.01294	1.01342	1.01391	1.01440	1.01491	1.01543	80
10	1.01543	1.01595	1.01649	1.01703	1.01758	1.01815	1.01872	79
11	1.01872	1.01930	1.01989	1.02049	1.02110	1.02171	1.02234	78
12	1.02234	1.02298	1.02362	1.02428	1.02494	1.02562	1.02630	77
13	1.02630	1.02700	1.02770	1.02842	1.02914	1.02987	1.03061	76
14	1.03061	1.03137	1.03213	1.03290	1.03368	1.03447	1.03528	75
15	1.03528	1.03609	1.03691	1.03774	1.03858	1.03944	1.04030	74
16	1.04030	1.04117	1.04206	1.04295	1.04385	1.04477	1.04569	73
17	1.04569	1.04663	1.04757	1.04853	1.04950	1.05047	1.05146	72
18	1.05146	1.05246	1.05347	1.05449	1.05552	1.05657	1.05762	71
19	1.05762	1.05869	1.05976	1.06085	1.06195	1.06306	1.06418	70
20	1.06418	1.06531	1.06645	1.06761	1.06878	1.06995	1.07115	69
21	1.07115	1.07235	1.07356	1.07479	1.07602	1.07727	1.07853	68
22	1.07853	1.07981	1.08109	1.08239	1.08370	1.08503	1.08636	67
23	1.08636	1.08771	1.08907	1.09044	1.09183	1.09323	1.09464	66
24	1.09464	1.09606	1.09750	1.09895	1.10041	1.10189	1.10338	65
25	1.10338	1.10488	1.10640	1.10793	1.10947	1.11103	1.11260	64
26	1.11260	1.11419	1.11579	1.11740	1.11903	1.12067	1.12233	63
27	1.12233	1.12400	1.12568	1.12738	1.12910	1.13083	1.13257	62
28	1.13257	1.13433	1.13610	1.13789	1.13970	1.14152	1.14335	61
29	1.14335	1.14521	1.14707	1.14896	1.15085	1.15277	1.15470	60
30	1.15470	1.15665	1.15861	1.16059	1.16259	1.16460	1.16663	59
31	1.16663	1.16868	1.17075	1.17283	1.17493	1.17704	1.17918	58
32	1.17918	1.18133	1.18350	1.18569	1.18790	1.19012	1.19236	57
33	1.19236	1.19463	1.19691	1.19920	1.20152	1.20386	1.20622	56
34	1.20622	1.20859	1.21099	1.21341	1.21584	1.21830	1.22077	55
35	1.22077	1.22327	1.22579	1.22833	1.23089	1.23347	1.23607	54
36	1.23607	1.23869	1.24134	1.24400	1.24669	1.24940	1.25214	53
37	1.25214	1.25489	1.25767	1.26047	1.26330	1.26615	1.26902	52
38	1.26902	1.27191	1.27483	1.27778	1.28075	1.28374	1.28676	51
39	1.28676	1.28980	1.29287	1.29597	1.29909	1.30223	1.30541	50
40	1.30541	1.30861	1.31183	1.31509	1.31837	1.32168	1.32501	49
41	1.32501	1.32838	1.33177	1.33519	1.33864	1.34212	1.34563	48
42	1.34563	1.34917	1.35274	1.35634	1.35997	1.36363	1.36733	47
43	1.36733	1.37105	1.37481	1.37860	1.38242	1.38628	1.39016	46
44	1.39016	1.39409	1.39804	1.40203	1.40606	1.41012	1.41421	45
Secants	60'	50'	40'	30'	20'	10'	0'	Degrees
COSECANTS								

SECANTS AND COSECANTS

Natural Trigonometric Functions

Degrees	COSECANTS							Secants
	0'	10'	20'	30'	40'	50'	60'	
0	∞	343.77516	171.88831	114.59301	85.94561	68.75736	57.29869	89
1	57.29869	49.11406	42.97571	38.20155	34.38232	31.25758	28.65371	88
2	28.65371	26.45051	24.56212	22.92559	21.49368	20.23028	19.10732	87
3	19.10732	18.10262	17.19843	16.38041	15.63679	14.95788	14.33559	86
4	14.33559	13.76312	13.23472	12.74550	12.29125	11.86837	11.47371	85
5	11.47371	11.10455	10.75849	10.43343	10.12752	9.83912	9.56677	84
6	9.56677	9.30917	9.06515	8.83367	8.61379	8.40466	8.20551	83
7	8.20551	8.01565	7.83443	7.66130	7.49571	7.33719	7.18530	82
8	7.18530	7.03962	6.89979	6.76547	6.63633	6.51208	6.39245	81
9	6.39245	6.27719	6.16607	6.05886	5.95536	5.85539	5.75877	80
10	5.75877	5.66533	5.57493	5.48740	5.40263	5.32049	5.24084	79
11	5.24084	5.16359	5.08863	5.01585	4.94517	4.87649	4.80973	78
12	4.80973	4.74482	4.68167	4.62023	4.56041	4.50216	4.44541	77
13	4.44541	4.39012	4.33622	4.28366	4.23239	4.18238	4.13357	76
14	4.13357	4.08591	4.03938	3.99393	3.94952	3.90613	3.86370	75
15	3.86370	3.82223	3.78166	3.74198	3.70315	3.66515	3.62796	74
16	3.62796	3.59154	3.55587	3.52094	3.48671	3.45317	3.42030	73
17	3.42030	3.38808	3.35649	3.32551	3.29512	3.26531	3.23607	72
18	3.23607	3.20737	3.17920	3.15155	3.12440	3.09774	3.07155	71
19	3.07155	3.04584	3.02057	2.99574	2.97135	2.94737	2.92380	70
20	2.92380	2.90063	2.87785	2.85545	2.83342	2.81175	2.79043	69
21	2.79043	2.76945	2.74881	2.72850	2.70851	2.68884	2.66947	68
22	2.66947	2.65040	2.63162	2.61313	2.59491	2.57698	2.55930	67
23	2.55930	2.54190	2.52474	2.50784	2.49119	2.47477	2.45859	66
24	2.45859	2.44264	2.42692	2.41142	2.39614	2.38107	2.36620	65
25	2.36620	2.35154	2.33708	2.32282	2.30875	2.29487	2.28117	64
26	2.28117	2.26766	2.25432	2.24116	2.22817	2.21535	2.20269	63
27	2.20269	2.19019	2.17786	2.16568	2.15366	2.14178	2.13005	62
28	2.13005	2.11847	2.10704	2.09574	2.08458	2.07356	2.06267	61
29	2.06267	2.05191	2.04128	2.03077	2.02039	2.01014	2.00000	60
30	2.00000	1.98998	1.98008	1.97029	1.96062	1.95106	1.94160	59
31	1.94160	1.93226	1.92302	1.91388	1.90485	1.89591	1.88709	58
32	1.88708	1.87834	1.86970	1.86116	1.85271	1.84435	1.83608	57
33	1.83608	1.82790	1.81981	1.81180	1.80388	1.79604	1.78829	56
34	1.78829	1.78062	1.77303	1.76552	1.75808	1.75073	1.74345	55
35	1.74345	1.73624	1.72911	1.72205	1.71506	1.70815	1.70130	54
36	1.70130	1.69452	1.68782	1.68117	1.67460	1.66809	1.66164	53
37	1.66164	1.65526	1.64894	1.64268	1.63648	1.63035	1.62427	52
38	1.62427	1.61825	1.61229	1.60639	1.60054	1.59475	1.58902	51
39	1.58902	1.58333	1.57771	1.57213	1.56661	1.56114	1.55572	50
40	1.55572	1.55036	1.54504	1.53977	1.53455	1.52938	1.52425	49
41	1.52425	1.51918	1.51415	1.50916	1.50422	1.49933	1.49448	48
42	1.49448	1.48967	1.48491	1.48019	1.47551	1.47087	1.46628	47
43	1.46628	1.46173	1.45721	1.45274	1.44831	1.44391	1.43956	46
44	1.43956	1.43524	1.43096	1.42672	1.42251	1.41835	1.41421	45
Cosecants	60'	50'	40'	30'	20'	10'	0'	Degrees

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