

Analysis of Doubly Reinforced Beams

HW #5

4.28, 5.6, 5.26

5.28

Case 1 - All steel yields

$$f_s = f'_s = f_y$$

$$C = 0.85 f'_c a b$$

$$C_s = A'_s f_y$$

$$T = A_s f_y$$

$$T = C + C_s$$

- Doubly Reinforced Beams

- Cases

- Examples

Next: Shear

$$A_s f_y = 0.85 f'_c a b + A'_s f_y$$

$$(A_s - A'_s) f_y$$

$$a = \frac{(A_s - A'_s) f_y}{0.85 f'_c b}$$

$$M_n = C \left(d - \frac{a}{2} \right) + C_s (d - d')$$

Case 2 - Tensile Steel Yields

Compression steel has not yielded

$$f_s = f_y$$

$$f_s < f'_s$$

$$C_s = A'_s E'_s E_s$$

$$C = 0.85 f'_c a b$$

$$T = A_s f_y$$

$$T = C + C_s$$

$$A_s f_y = 0.85 f'_c a b + A'_s E'_s E_s$$

$$a = \frac{A_s f_y - E'_s E_s A'_s}{0.85 f'_c b}$$

$$\text{Let } E'_s = \frac{0.003 (c - d')}{c} ; c = a / \beta_1$$

Results in Quadratic

$$\underbrace{(0.85 f'_c b)}_A a^2 + \underbrace{(0.003 A'_s E_s - A_s f_y)}_B a + \underbrace{(A'_s E_s 0.003 d' \beta_1)}_C = 0$$

Solve for "a" (positive root answer)

$$M_n = A'_s E_s E_s (d - d') + (0.85 f'_c ab) \left(d - \frac{a}{2}\right)$$

Case 3 No steel yields

Go Back & redesign!!

Solution Procedure

① Assume Case 1

② Check assumption

if $\epsilon_s > f_y$, $\epsilon'_s > \epsilon_y \Rightarrow$ solve as case 1; done

if $\epsilon_s > f_y$, $\epsilon'_s < \epsilon_y \Rightarrow$ solve as case 2, recheck

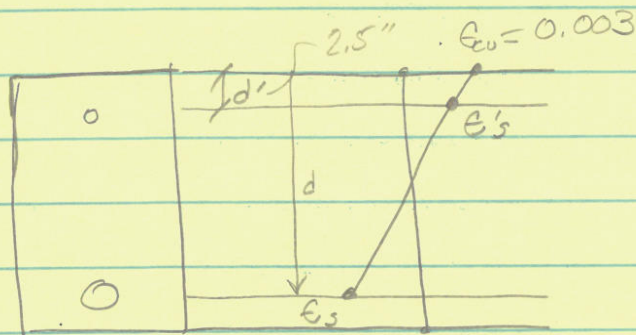
↑
tensile
steel

↑
compression
steel

assumptions $\epsilon_s > f_y$, $\epsilon'_s < \epsilon_y \Rightarrow$ done

③ If $\epsilon_s < \epsilon_y$, $\epsilon'_s < \epsilon_y \Rightarrow$ Stop Case 3 done

hand out



$$A_s = 10.12 \text{ in}^2$$

Sometimes $A_1 = A_s$

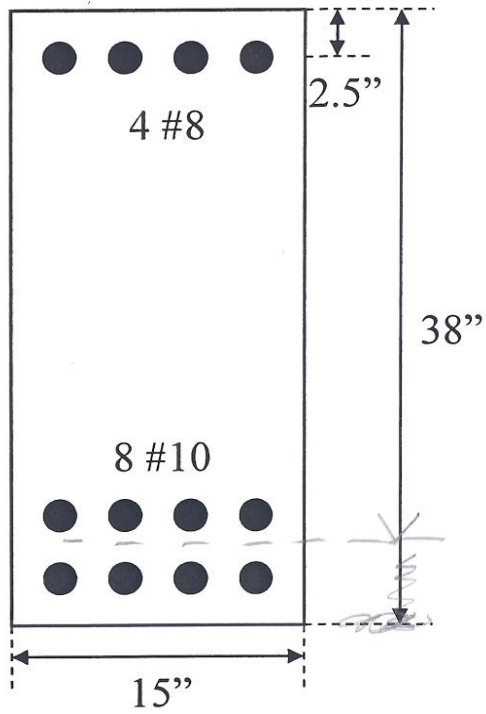
$$A'_s = 3.14 \text{ in}^2$$

$$A_2 = A_s - A'_s$$

① Assume all steel yields

$$f_s = f'_s = f_y$$

$$a = \frac{(A_s - A'_s)}{0.85 f'_c b} + \frac{A_2 f_y}{0.85 f'_c b} = \frac{6.98 \text{ in}^2 (60)}{(0.85)(4)(15)} = 8.21'' = a$$



Design Strength of Doubly Reinforced Beams
 ENCE 4359 Structural Concrete Design
 Dr. Lamanna

$$f_c = 4,000 \text{ psi}$$

$$f_y = 60,000 \text{ psi}$$

Compute the Design Strength.

Check the maximum permissible A_s to ensure tensile failure.

② check assumption that all steel yields

$$c = a/\beta_1 = \frac{8.21}{0.85} = 9.66''$$

$$E_s = \frac{0.003 d - c}{c} = \frac{(0.003)(38 - 9.66)}{9.66} = 0.0088 > 0.00207 \quad \checkmark$$

$$E_s' = \frac{0.003(c - d')}{c} = \frac{(0.003)(9.66 - 2.50)}{9.66} = 0.0022 > 0.00207 \quad \checkmark$$

∴ All Steel yields

E_y for Gr 60

③ Calculate M_n

$$\begin{aligned} \phi M_n &= (A_s - A_s') f_y (d - a/2) + A_s' f_y (d - d') \\ &= (10.12 - 3.14)(60) \left(38 - \frac{8.21}{2}\right) + (3.14)(60)(38 - 2.50) \\ &= 18,795 \text{ in}\cdot\text{k} = 1,566 \text{ ft}\cdot\text{k} \end{aligned}$$

④ Check tensile steel permitted

$$A_{s, \max} = \rho_{\max} b d + A_s' \frac{f_s'}{f_y}$$

$$\rho_{\max} = 0.0181 \text{ [Table A.2]}$$

$$A_{s, \max} = 0.0181(15)(38) + 3.14 \left(\frac{60}{60}\right) = 10.12 \text{ in}^2$$

✓ OK

$$M_y > 1,566 \text{ ft}\cdot\text{k}$$