

STRUCTURAL SHEET LIST	
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S0-02	GENERAL NOTES
S0-03	GENERAL NOTES
S0-04	GENERAL NOTES
S0-05	MASTER SUBMITTAL LIST
S0-10	SCHEDULE OF SPECIAL INSPECTION
S0-11	SCHEDULE OF SPECIAL INSPECTION
S0-20	STRUCTURAL LOADING INFORMATION
S1-1.1	FOUNDATION PLAN - BUILDING A
S1-1.2	LEVEL 2 FRAMING PLAN - BUILDING A
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S1-1.3	LEVEL 3 FRAMING PLAN - BUILDING A
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S1-2.2B	LEVEL 2 FRAMING PLAN - BUILDING B
S1-2.2C	LEVEL 2 FRAMING PLAN - BUILDING B
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S3-01	FOUNDATION SECTIONS AND DETAILS
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S4-20	CMU FRAMING SECTIONS AND DETAILS
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S4-91	WOOD ROOF FRAMING SECTIONS AND DETAILS
S4-92	WOOD ROOF FRAMING SECTIONS AND DETAILS
S5-00A	WOOD WALL SCHEDULE AND DETAILS
S5-00B	WOOD WALL SCHEDULE AND DETAILS
S5-00C	WOOD WALL SCHEDULE AND DETAILS
S6-00A	WOOD SCHEDULES AND DETAILS - LATERAL
S6-00B	WOOD SCHEDULES AND DETAILS - LATERAL
S6-00C	WOOD SCHEDULES AND DETAILS - LATERAL
S6-00D	BRACING SECTIONS AND DETAILS
S6-01	BRACING PLAN - BUILDING A
S6-02	BRACING PLAN - BUILDING B
S6-03	BRACING PLAN - CLUBHOUSE
S6-04	BRACING PLANS - ANCILLARY BUILDINGS



**PHILLIPS**

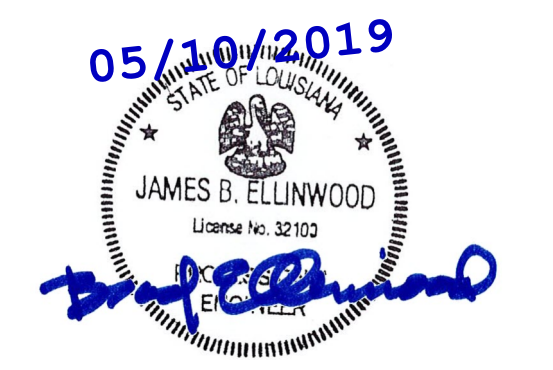
CONSULTANT



**CONSULTING ENGINEERS**

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SEAL



ISSUE & REVISION RECORD

#	DATE	DESCRIPTION
	09/24/2018	PERMIT SET
D	12/05/18	COORDINATION REVIEW
F	02/07/2019	CONSTRUCTION REVIEW SET
G	03/22/2019	CONSTRUCTION RELEASE
H	05/10/2019	ISSUED FOR CONSTRUCTION

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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

DRAWING TITLE

COVER SHEET

SHEET NUMBER

**S0-00**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

PHILLIPSPART.COM 770-394-1616

**FOR CONSTRUCTION**

1. DESIGN CRITERIA

- A. GENERAL BUILDING CODE
1. THE CONTRACT DOCUMENTS ARE BASED ON THE REQUIREMENTS OF THE 2015 INTERNATIONAL BUILDING CODE...
B. SUPPLEMENTARY BUILDING CODE REFERENCES INCLUDE THOSE REFERENCES LISTED BELOW...
C. DEAD LOADS:
1. RESIDENTIAL UNITS (TYPE V CONSTRUCTION):
a. ACTUAL DEAD LOADS...
b. AN ALLOWANCE OF 5 PSF HAS BEEN APPLIED AS A UNIFORMLY-DISTRIBUTED SUPERIMPOSED DEAD LOAD...
C. LIVE LOADS
1. LIVE LOADS PRODUCED BY THE USE OR OCCUPANCY OF THE STRUCTURE HAVE BEEN INCLUDED IN THE DESIGN...
D. WIND LOADS
WIND LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE GENERAL BUILDING CODE REFERENCED ABOVE...

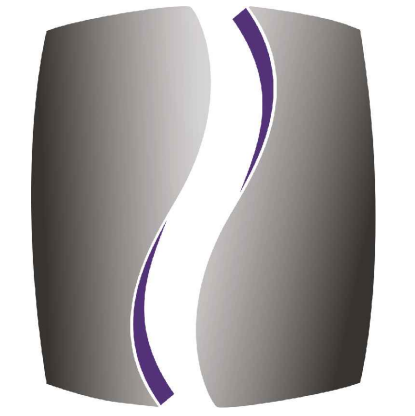
- E. SEISMIC LOADS
SEISMIC LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE GENERAL BUILDING CODE REFERENCED ABOVE...
ANALYSIS PROCEDURE:
EQUIVALENT LATERAL FORCE PROCEDURE PER ASCE 7, §12.8
RESPONSE MODIFICATION FACTOR, R..... 6.5
DEFLECTION AMPLIFICATION FACTOR, Cd..... 4.0
SEISMIC RESPONSE COEFFICIENT, Cs
IL BUILDING (R=6.5)..... 0.0165
DESIGN BASE SHEAR
CLUBHOUSE
DESIGN BASE SHEAR (X) TRANSVERSE E/W..... 2.57K
DESIGN BASE SHEAR (Y) LONGITUDINAL N/S..... 2.57K
BUILDING A
DESIGN BASE SHEAR (X) TRANSVERSE E/W..... 14.3K
DESIGN BASE SHEAR (Y) LONGITUDINAL N/S..... 14.3K
BUILDING B
DESIGN BASE SHEAR (X) TRANSVERSE E/W..... 16.5K
DESIGN BASE SHEAR (Y) LONGITUDINAL N/S..... 16.5K
F. SNOW LOADS
SNOW LOADS HAVE BEEN DETERMINED IN ACCORDANCE WITH THE GENERAL BUILDING CODE REFERENCED ABOVE...
GROUND SNOW LOAD, P..... 5.0 PSF
FLAT ROOF SNOW LOAD, P..... 3.2 PSF
SNOW EXPOSURE COEFFICIENT, Ce..... 0.9
IMPORTANCE FACTOR, I..... 1.0
THERMAL FACTOR, Ct..... 1.0
RAIN ON SNOW SURCHARGE (1/2" PER FOOT SLOPE)..... 5.0 PSF
DRIFT CONDITIONS CONSIDERED PER ASCE 7, §7.7.
G. FLOOD LOADS
BASE FLOOD ELEVATION, BFE..... 9 FT
DESIGN FLOOD ELEVATION, DFE..... 11 FT
ELEVATION OF PROPOSED LOWEST FLOOR..... 13 FT
H. DESIGN LOAD COMBINATIONS
1. BASIC LOAD COMBINATIONS PER SECTION 1605.2 FOR LOAD AND RESISTANCE FACTOR DESIGN (LRFD) ARE USED.
2. BASIC LOAD COMBINATIONS PER SECTION 1605.3 FOR ALLOWABLE STRESS DESIGN (ASD) ARE USED.
I. CONSTRUCTION SEQUENCING
THE CONTRACTOR SHALL COORDINATE THE SEQUENCE AND SCHEDULE OF CONSTRUCTION WITH THE ENGINEER.
J. BUILDING DEFLECTIONS AND DRIFTS - ANTICIPATED DIFFERENTIAL MOVEMENTS FOR CLADDING DESIGN AND INSTALLATION
1. UNDER NORMAL LOADING CONDITIONS, CONCRETE SLABS AND BEAMS WILL DEFLECT, WHETHER REINFORCED OR POST-TENSIONED...
2. PROPER ALLOWANCE MUST BE MADE IN FINISHES AND ATTACHED ELEMENTS TO PERMIT THE STRUCTURE TO DEFLECT WITHOUT DAMAGE...
3. THE FOLLOWING ANTICIPATED DIFFERENTIAL MOVEMENTS BETWEEN ANY TWO FLOORS ARE BASED ON THE ASSUMPTION THAT CLADDING AND GLAZING INSTALLATION OCCURS NO EARLIER THAN 3 MONTHS AFTER SHORING IS REMOVED...
K. SEE MECHANICAL, PLUMBING, ELECTRICAL AND/OR CIVIL DRAWINGS FOR THE FOLLOWING INFORMATION:
a. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC.
b. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS
c. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES
d. UNDERGROUND CONCRETE DUCTS, TRENCHES, PITS OR MANHOLES
e. CONCRETE AND ASPHALT PAVEMENT
L. CONTRACTOR TO REFERENCE THE GEOTECHNICAL ENGINEERING REPORT FOR TEMPORARY SITE DE-WATERING REQUIREMENTS.
M. UNDER-SLAB DRAINAGE SYSTEMS, IF REQUIRED, ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS...
N. UNDER-SLAB VAPOR MITIGATION SYSTEMS (VMS), IF REQUIRED, ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS...
O. UNLESS DIRECTION PROVIDED BY THE GEOTECHNICAL ENGINEER IN THE REFERENCED GEOTECHNICAL ENGINEERING REPORT STATES THAT MORE STRENUOUS REQUIREMENTS MUST BE MET, SOIL BELOW INTERIOR CONCRETE SLABS ON GRADE AND ANY FILL WITHIN 10'-0" OF BUILDING LIMIT SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR (ASTM D698) IN FINAL TWO (2) FEET OF FILL AND 95% OF STANDARD PROCTOR BELOW.
P. SUBGRADE PREPARATION FOR SLABS ON GRADE SHALL BE PERFORMED IN ACCORDANCE WITH THE REFERENCED GEOTECHNICAL ENGINEERING REPORT...
Q. WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL...
R. GRADE BEAMS
1. BOTH VERTICAL FACES OF ALL GRADE BEAMS SHALL BE FORMED FOR FULL BEAM DEPTH.
2. GRADE BEAM REINFORCING SHALL BE SUPPORTED WITH A MINIMUM OF 3 INCHES CLEAR COVER USING BEAM BOLSTERS DESIGNED AND MANUFACTURED FOR SUPPORT ON SOIL.
S. CONSTRUCTION COORDINATION
1. SLABS ON GRADE HAVE NOT BEEN DESIGNED FOR CRANE LOADS...
2. SHOULD CONSTRUCTION SEQUENCING REQUIRE THAT ONE OR MORE SLABS ON GRADE BE DESIGNED TO RESIST CRANE LOADS...
3. THE CONCRETE RETAINING WALLS HAVE NOT BEEN DESIGNED TO RESIST LATERAL LOADS RESULTING FROM CRANE LOADS...
T. REINFORCED CONCRETE
A. THE DESIGN OF ALL REINFORCED CONCRETE SHALL CONFORM TO ACI 318, IN ACCORDANCE WITH THE GENERAL BUILDING CODE NOTED ABOVE...
B. CONCRETE TEST REPORTS SHALL BE FURNISHED TO THE DESIGN TEAM AS THEY ARE MADE AVAILABLE BY THE TESTING AGENCY...
C. CONCRETE MIX DESIGNS SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318, CHAPTER 5...
D. CLASSES OF CONCRETE
ALL CONCRETE SHALL CONFORM TO THE REQUIREMENTS SPECIFIED IN THE TABLE BELOW UNLESS NOTED OTHERWISE ON THESE DRAWINGS:
USAGE COMPRESSIVE STRENGTH (F'c) AGGREGATE SIZE (IN.) CONCRETE EXPOSURE CLASSES
SPREAD FOOTINGS 3,000 1.00 F0, S0, P0, C1
CONTINUOUS FOOTINGS 3,000 1.50 F0, S0, P0, C1
GRADE BEAMS 3,000 1.00 F0, S0, P0, C1
PILASTERS 4,000 1.00 F0, S0, P0, C1
PEDESTALS 4,000 1.00 F0, S0, P0, C1
RETAINING WALLS 4,000 1.00 F0, S0, P0, C1
RETAINING WALLS (EXTERIOR, EXPOSED) 4,500 1.00 F2, S0, P0, C1
INTERIOR SLABS ON GRADE 3,000 1.00 F0, S0, P0, C1
EXTERIOR SLABS ON GRADE 4,500 1.00 F2, S0, P0, C1
NON-STRUCTURAL TOPPING 3,000 0.75 F0, S0, P0, C0
NON-STRUCTURAL TOPPING AT CORRIDORS (WOOD-FRAMED STRUCTURE) 2,000 -- F0, S0, P0, C0
1. COMPRESSIVE STRENGTH NOTED ABOVE SHALL BE THE 28-DAY COMPRESSIVE STRENGTH FOR ALL CONCRETE MIXES WITH A REQUIRED STRENGTH OF 6,500 PSI OR LESS...
2. IN ADDITION TO THE MINIMUM COMPRESSIVE STRENGTH REQUIREMENT, CONCRETE MIX DESIGNS FOR ELEVATED HORIZONTAL FRAMING (SLABS, BEAMS, GIRDERS, ETC.), COLUMNS, AND WALLS SHALL BE PROPORTIONED FOR A MAXIMUM WATER-CEMENT RATIO OF 0.45...
3. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE, UNLESS OTHERWISE NOTED...
4. CONCRETE SLUMP SHALL BE 4", ±1", AT LOCATION OF DISCHARGE, UNLESS OTHERWISE NOTED...
5. ALL CONCRETE MIX DESIGNS SHALL BE PROPORTIONED FOR A MAXIMUM ALLOWABLE UNIT SHRINKAGE OF 0.05% AS DETERMINED BY ASTM C157 (MEASURED AT 28 DAYS AFTER CURING IN LIME WATER WITH AIR STORAGE)...
6. ALL CONCRETE EXPOSED TO FREEZE-THAW CYCLES (ANY CONCRETE EXPOSED TO THE ELEMENTS) SHALL BE AIR ENTRAINED. THE PERCENTAGE OF ENTRAINED AIR SHALL BE AS REQUIRED PER ACI 318, TABLE 19.3.3.1...
7. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED...
8. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I...
9. SUPPLEMENTARY CEMENTICIOUS MATERIALS, AS LISTED BELOW, MAY BE USED TO REPLACE A PORTION OF THE PORTLAND CEMENT IN A CONCRETE MIX DESIGN...
a. FLY ASH MEETING THE REQUIREMENTS OF EITHER TYPE "C" OR TYPE "F" PER ASTM C618.
b. GROUND GRANULATED BLAST-FURNACE SLAB MEETING THE REQUIREMENTS OF ASTM C989.
c. SILICA FUME MEETING THE REQUIREMENTS OF ASTM C1240.
U. FOUNDATION DESIGN IS BASED ON "ADDITIONAL GEOTECHNICAL ENGINEERING REPORT PROPOSED MULTI-FAMILY RESIDENTIAL DEVELOPMENT, FREMAUX TOWN CENTER, SLIDELL, LOUISIANA, BY STRATUM ENGINEERING, LLC, DATED JUNE 11, 2018 (STRATUM ENGINEERING PROJECT No. G18-010, Addendum No. 1).
V. CONTRACTOR SHALL OBTAIN A COPY OF THE ABOVE REFERENCED SOILS REPORT AND COMPLY WITH ITS RECOMMENDATIONS.
W. GEOTECHNICAL RECOMMENDATIONS AND FINDINGS FOR SHALLOW FOUNDATIONS:
1. ALLOWABLE SOIL BEARING PRESSURE:
CONCRETE SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,500 PSF
CONTINUOUS CONCRETE WALL FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,000 PSF
2. MINIMUM FOOTING BEARING DEPTH (AS MEASURED FROM TOP OF EXTERIOR GRADE TO FOOTING BEARING ELEVATION)..... 24"
3. MINIMUM FOOTING WIDTH..... 18" (STRIP FTGS)
24" (SPREAD FTGS)
4. LATERAL EARTH PRESSURES
ACTIVE (Ka)..... ## PSF/FT AT REST (Kc)..... ## PSF/FT
PASSIVE (Kp)..... ## PSF/FT
COEFFICIENT OF FRICTION..... 350 PSF/FT
(BETWEEN BOTTOM OF FOOTING AND SOIL)..... 0.38
INTERNAL ANGLE OF FRICTION..... ##
SOIL MOIST UNIT WEIGHT..... 115 PCF
D. THE SOILS ENGINEER OF RECORD SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED AND COMPLETED AS SPECIFIED.
E. SOIL-SUPPORTED CONCRETE SLABS ON GRADE SHALL BE AS FOLLOWS:
1. IN AREAS OF RESIDENTIAL UNITS (BUILDING A, BUILDING B) - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE.
2. IN THE CLUBHOUSE - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE...
WHERE THE STRUCTURAL FOUNDATION PLANS INDICATE THAT SLABS ON GRADE ARE TO BE POST-TENSIONED, REFERENCE §5 OF THESE GENERAL NOTES FOR ADDITIONAL INFORMATION.
F. GEOTECHNICAL ENGINEER SHALL VERIFY THE ASSUMED BEARING PRESSURE(S) NOTED ABOVE, CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACKFILLS, ETC.
G. SIDES OF FOUNDATION ELEMENTS (FOOTINGS, GRADE BEAMS, ETC.) SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS POURED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS:
1. SLOPE SIDES OF EXCAVATIONS AS APPROVED BY GEOTECHNICAL ENGINEER.
2. CLEAR UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT.
H. CONTRACTOR SHALL PROVIDE AND INSTALL ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY RETAIN EARTH BANKS.
I. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
J. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED, BUT NOT BEFORE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH. NO BACKFILL SHALL BE PLACED AGAINST CONCRETE WALLS UNTIL CONCRETE HAS ATTAINED FULL 28-DAY STRENGTH.
K. FOUNDATION WALLS THAT HAVE BEEN DESIGNED AS CANTILEVER WALLS SHALL NOT REQUIRE LATERAL BRACING DURING OR AFTER BACKFILLING, PROVIDED THAT ALL COMPACTION OF SOILS/FILL BEHIND THE WALL IS PERFORMED ONLY BY HAND OPERATED EQUIPMENT IN THE ZONE OCCURRING WITHIN 5'-0" OF THE BACK FACE OF THE WALL.
1. SHOULD THE CONTRACTOR OPT TO USE A REMOTE CONTROLLED COMPACTOR TO COMPACT SOILS IN THE ZONE OCCURRING WITHIN 5'-0" OF THE BACK FACE OF THE WALL, THAT IS ACCEPTABLE PROVIDED THAT THE COMPACTOR IS ONE OF THOSE LISTED BELOW:
a. WACKER NEUSON RT SERIES OR RTX SERIES TRENCH ROLLER
b. ALTERNATE EQUIPMENT APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.

2. GENERAL

- I. SEE MECHANICAL, PLUMBING, ELECTRICAL AND/OR CIVIL DRAWINGS FOR THE FOLLOWING INFORMATION:
a. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC.
b. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS
c. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL OR PLUMBING FIXTURES
d. UNDERGROUND CONCRETE DUCTS, TRENCHES, PITS OR MANHOLES
e. CONCRETE AND ASPHALT PAVEMENT
J. THE CONTRACT STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE INDICATED. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION...
K. FIELD VERIFY ALL EXISTING ABOVE AND BELOW GROUND CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTION...
L. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
M. THE STRUCTURAL ENGINEER OF RECORD (ELLINWOOD + MACHADO, LLC) IS NOT RESPONSIBLE FOR THE DESIGN OF THE FOLLOWING ITEMS:
1. SITE RETAINING WALLS (RETAINING WALLS THAT OCCUR OUTSIDE OF BUILDING FOOTPRINT, EXCEPT WHERE SPECIFICALLY SHOWN/DETAILED IN THESE DRAWINGS)
2. SPECIALTY FOUNDATION SYSTEMS, INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING:
a. RAMMED AGGREGATE PIERS
b. UNDER-PINNING
c. SHEET PILING
d. MECHANICALLY STABILIZED EARTH (MSE) WALLS
e. LAGGING WALLS
f. SOIL NAILS
4. STEEL STAIRS
5. PRECAST STAIRS
6. WOOD STAIRS
7. HANDRAILS AND/OR GUARDS (REGARDLESS OF MATERIAL(S) USED)
8. STRUCTURAL STEEL MEMBER END CONNECTIONS (UNLESS SPECIFICALLY SHOWN AND FULLY DETAILED IN THE STRUCTURAL DRAWINGS)
9. CURTAIN WALL SYSTEMS
10. WINDOW WALL SYSTEMS
11. COLD-FORMED METAL STUD (CFMS) FRAMING (TO INCLUDE LOAD-BEARING WALLS, NON-LOAD-BEARING WALLS, CEILING JOISTS, SOFFITS, ETC.)
12. WOOD FLOOR TRUSSES
13. WOOD ROOF TRUSSES
14. STAND-ALONE STRUCTURES SHOWN ON THE LANDSCAPE ARCHITECTURE DRAWINGS (EXCEPT WHERE SPECIFICALLY SHOWN/DETAILED IN THESE DRAWINGS)
15. ALL OTHER SYSTEMS NOT SPECIFICALLY SHOWN IN THE STRUCTURAL DOCUMENTS
THE ITEMS ABOVE SHALL BE CONSIDERED TO BE DELEGATED DESIGN ITEMS, WHICH REQUIRE DESIGN TO BE COMPLETED BY A SPECIALTY ENGINEER...
DELEGATED DESIGN ITEMS MUST BE SUBMITTED FOR REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD AND SIGNATURE BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF LOUISIANA...
3. FOUNDATION
A. FOUNDATION DESIGN IS BASED ON "ADDITIONAL GEOTECHNICAL ENGINEERING REPORT PROPOSED MULTI-FAMILY RESIDENTIAL DEVELOPMENT, FREMAUX TOWN CENTER, SLIDELL, LOUISIANA, BY STRATUM ENGINEERING, LLC, DATED JUNE 11, 2018 (STRATUM ENGINEERING PROJECT No. G18-010, Addendum No. 1).
B. CONTRACTOR SHALL OBTAIN A COPY OF THE ABOVE REFERENCED SOILS REPORT AND COMPLY WITH ITS RECOMMENDATIONS.
C. GEOTECHNICAL RECOMMENDATIONS AND FINDINGS FOR SHALLOW FOUNDATIONS:
1. ALLOWABLE SOIL BEARING PRESSURE:
CONCRETE SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,500 PSF
CONTINUOUS CONCRETE WALL FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,000 PSF
2. MINIMUM FOOTING BEARING DEPTH (AS MEASURED FROM TOP OF EXTERIOR GRADE TO FOOTING BEARING ELEVATION)..... 24"
3. MINIMUM FOOTING WIDTH..... 18" (STRIP FTGS)
24" (SPREAD FTGS)
4. LATERAL EARTH PRESSURES
ACTIVE (Ka)..... ## PSF/FT AT REST (Kc)..... ## PSF/FT
PASSIVE (Kp)..... ## PSF/FT
COEFFICIENT OF FRICTION..... 350 PSF/FT
(BETWEEN BOTTOM OF FOOTING AND SOIL)..... 0.38
INTERNAL ANGLE OF FRICTION..... ##
SOIL MOIST UNIT WEIGHT..... 115 PCF
D. THE SOILS ENGINEER OF RECORD SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED AND COMPLETED AS SPECIFIED.
E. SOIL-SUPPORTED CONCRETE SLABS ON GRADE SHALL BE AS FOLLOWS:
1. IN AREAS OF RESIDENTIAL UNITS (BUILDING A, BUILDING B) - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE.
2. IN THE CLUBHOUSE - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE...
WHERE THE STRUCTURAL FOUNDATION PLANS INDICATE THAT SLABS ON GRADE ARE TO BE POST-TENSIONED, REFERENCE §5 OF THESE GENERAL NOTES FOR ADDITIONAL INFORMATION.
F. GEOTECHNICAL ENGINEER SHALL VERIFY THE ASSUMED BEARING PRESSURE(S) NOTED ABOVE, CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACKFILLS, ETC.
G. SIDES OF FOUNDATION ELEMENTS (FOOTINGS, GRADE BEAMS, ETC.) SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING...
H. CONTRACTOR SHALL PROVIDE AND INSTALL ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY RETAIN EARTH BANKS.
I. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
J. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED, BUT NOT BEFORE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH...
K. FOUNDATION WALLS THAT HAVE BEEN DESIGNED AS CANTILEVER WALLS SHALL NOT REQUIRE LATERAL BRACING DURING OR AFTER BACKFILLING...
L. SHOULD THE CONTRACTOR OPT TO USE A REMOTE CONTROLLED COMPACTOR TO COMPACT SOILS IN THE ZONE OCCURRING WITHIN 5'-0" OF THE BACK FACE OF THE WALL, THAT IS ACCEPTABLE PROVIDED THAT THE COMPACTOR IS ONE OF THOSE LISTED BELOW:
a. WACKER NEUSON RT SERIES OR RTX SERIES TRENCH ROLLER
b. ALTERNATE EQUIPMENT APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.

- L. CONTRACTOR TO REFERENCE THE GEOTECHNICAL ENGINEERING REPORT FOR TEMPORARY SITE DE-WATERING REQUIREMENTS.
M. UNDER-SLAB DRAINAGE SYSTEMS, IF REQUIRED, ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS...
N. UNDER-SLAB VAPOR MITIGATION SYSTEMS (VMS), IF REQUIRED, ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS...
O. UNLESS DIRECTION PROVIDED BY THE GEOTECHNICAL ENGINEER IN THE REFERENCED GEOTECHNICAL ENGINEERING REPORT STATES THAT MORE STRENUOUS REQUIREMENTS MUST BE MET, SOIL BELOW INTERIOR CONCRETE SLABS ON GRADE AND ANY FILL WITHIN 10'-0" OF BUILDING LIMIT SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR (ASTM D698) IN FINAL TWO (2) FEET OF FILL AND 95% OF STANDARD PROCTOR BELOW.
P. SUBGRADE PREPARATION FOR SLABS ON GRADE SHALL BE PERFORMED IN ACCORDANCE WITH THE REFERENCED GEOTECHNICAL ENGINEERING REPORT...
Q. WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL...
R. GRADE BEAMS
1. BOTH VERTICAL FACES OF ALL GRADE BEAMS SHALL BE FORMED FOR FULL BEAM DEPTH.
2. GRADE BEAM REINFORCING SHALL BE SUPPORTED WITH A MINIMUM OF 3 INCHES CLEAR COVER USING BEAM BOLSTERS DESIGNED AND MANUFACTURED FOR SUPPORT ON SOIL.
S. CONSTRUCTION COORDINATION
1. SLABS ON GRADE HAVE NOT BEEN DESIGNED FOR CRANE LOADS...
2. SHOULD CONSTRUCTION SEQUENCING REQUIRE THAT ONE OR MORE SLABS ON GRADE BE DESIGNED TO RESIST CRANE LOADS...
3. THE CONCRETE RETAINING WALLS HAVE NOT BEEN DESIGNED TO RESIST LATERAL LOADS RESULTING FROM CRANE LOADS...
4. REINFORCED CONCRETE
A. THE DESIGN OF ALL REINFORCED CONCRETE SHALL CONFORM TO ACI 318, IN ACCORDANCE WITH THE GENERAL BUILDING CODE NOTED ABOVE...
B. CONCRETE TEST REPORTS SHALL BE FURNISHED TO THE DESIGN TEAM AS THEY ARE MADE AVAILABLE BY THE TESTING AGENCY...
C. CONCRETE MIX DESIGNS SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318, CHAPTER 5...
D. CLASSES OF CONCRETE
ALL CONCRETE SHALL CONFORM TO THE REQUIREMENTS SPECIFIED IN THE TABLE BELOW UNLESS NOTED OTHERWISE ON THESE DRAWINGS:
USAGE COMPRESSIVE STRENGTH (F'c) AGGREGATE SIZE (IN.) CONCRETE EXPOSURE CLASSES
SPREAD FOOTINGS 3,000 1.00 F0, S0, P0, C1
CONTINUOUS FOOTINGS 3,000 1.50 F0, S0, P0, C1
GRADE BEAMS 3,000 1.00 F0, S0, P0, C1
PILASTERS 4,000 1.00 F0, S0, P0, C1
PEDESTALS 4,000 1.00 F0, S0, P0, C1
RETAINING WALLS 4,000 1.00 F0, S0, P0, C1
RETAINING WALLS (EXTERIOR, EXPOSED) 4,500 1.00 F2, S0, P0, C1
INTERIOR SLABS ON GRADE 3,000 1.00 F0, S0, P0, C1
EXTERIOR SLABS ON GRADE 4,500 1.00 F2, S0, P0, C1
NON-STRUCTURAL TOPPING 3,000 0.75 F0, S0, P0, C0
NON-STRUCTURAL TOPPING AT CORRIDORS (WOOD-FRAMED STRUCTURE) 2,000 -- F0, S0, P0, C0
1. COMPRESSIVE STRENGTH NOTED ABOVE SHALL BE THE 28-DAY COMPRESSIVE STRENGTH FOR ALL CONCRETE MIXES WITH A REQUIRED STRENGTH OF 6,500 PSI OR LESS...
2. IN ADDITION TO THE MINIMUM COMPRESSIVE STRENGTH REQUIREMENT, CONCRETE MIX DESIGNS FOR ELEVATED HORIZONTAL FRAMING (SLABS, BEAMS, GIRDERS, ETC.), COLUMNS, AND WALLS SHALL BE PROPORTIONED FOR A MAXIMUM WATER-CEMENT RATIO OF 0.45...
3. ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE, UNLESS OTHERWISE NOTED...
4. CONCRETE SLUMP SHALL BE 4", ±1", AT LOCATION OF DISCHARGE, UNLESS OTHERWISE NOTED...
5. ALL CONCRETE MIX DESIGNS SHALL BE PROPORTIONED FOR A MAXIMUM ALLOWABLE UNIT SHRINKAGE OF 0.05% AS DETERMINED BY ASTM C157 (MEASURED AT 28 DAYS AFTER CURING IN LIME WATER WITH AIR STORAGE)...
6. ALL CONCRETE EXPOSED TO FREEZE-THAW CYCLES (ANY CONCRETE EXPOSED TO THE ELEMENTS) SHALL BE AIR ENTRAINED...
7. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED...
8. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I...
9. SUPPLEMENTARY CEMENTICIOUS MATERIALS, AS LISTED BELOW, MAY BE USED TO REPLACE A PORTION OF THE PORTLAND CEMENT...
a. FLY ASH MEETING THE REQUIREMENTS OF EITHER TYPE "C" OR TYPE "F" PER ASTM C618.
b. GROUND GRANULATED BLAST-FURNACE SLAB MEETING THE REQUIREMENTS OF ASTM C989.
c. SILICA FUME MEETING THE REQUIREMENTS OF ASTM C1240.
U. FOUNDATION DESIGN IS BASED ON "ADDITIONAL GEOTECHNICAL ENGINEERING REPORT PROPOSED MULTI-FAMILY RESIDENTIAL DEVELOPMENT, FREMAUX TOWN CENTER, SLIDELL, LOUISIANA, BY STRATUM ENGINEERING, LLC, DATED JUNE 11, 2018 (STRATUM ENGINEERING PROJECT No. G18-010, Addendum No. 1).
V. CONTRACTOR SHALL OBTAIN A COPY OF THE ABOVE REFERENCED SOILS REPORT AND COMPLY WITH ITS RECOMMENDATIONS.
W. GEOTECHNICAL RECOMMENDATIONS AND FINDINGS FOR SHALLOW FOUNDATIONS:
1. ALLOWABLE SOIL BEARING PRESSURE:
CONCRETE SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,500 PSF
CONTINUOUS CONCRETE WALL FOOTINGS BEARING ON RESIDUAL SOILS OR ENGINEERED, COMPACTED FILL MATERIAL(S)..... 2,000 PSF
2. MINIMUM FOOTING BEARING DEPTH (AS MEASURED FROM TOP OF EXTERIOR GRADE TO FOOTING BEARING ELEVATION)..... 24"
3. MINIMUM FOOTING WIDTH..... 18" (STRIP FTGS)
24" (SPREAD FTGS)
4. LATERAL EARTH PRESSURES
ACTIVE (Ka)..... ## PSF/FT AT REST (Kc)..... ## PSF/FT
PASSIVE (Kp)..... ## PSF/FT
COEFFICIENT OF FRICTION..... 350 PSF/FT
(BETWEEN BOTTOM OF FOOTING AND SOIL)..... 0.38
INTERNAL ANGLE OF FRICTION..... ##
SOIL MOIST UNIT WEIGHT..... 115 PCF
D. THE SOILS ENGINEER OF RECORD SHALL CERTIFY IN WRITING THAT ALL FOUNDATIONS WERE PLACED AND COMPLETED AS SPECIFIED.
E. SOIL-SUPPORTED CONCRETE SLABS ON GRADE SHALL BE AS FOLLOWS:
1. IN AREAS OF RESIDENTIAL UNITS (BUILDING A, BUILDING B) - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE.
2. IN THE CLUBHOUSE - 4" THICK, POST-TENSIONED, ON PREPARED SUBGRADE...
WHERE THE STRUCTURAL FOUNDATION PLANS INDICATE THAT SLABS ON GRADE ARE TO BE POST-TENSIONED, REFERENCE §5 OF THESE GENERAL NOTES FOR ADDITIONAL INFORMATION.
F. GEOTECHNICAL ENGINEER SHALL VERIFY THE ASSUMED BEARING PRESSURE(S) NOTED ABOVE, CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, WALLS, FILLS, BACKFILLS, ETC.
G. SIDES OF FOUNDATION ELEMENTS (FOOTINGS, GRADE BEAMS, ETC.) SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING...
H. CONTRACTOR SHALL PROVIDE AND INSTALL ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY RETAIN EARTH BANKS.
I. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC. ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
J. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED, BUT NOT BEFORE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH...
K. FOUNDATION WALLS THAT HAVE BEEN DESIGNED AS CANTILEVER WALLS SHALL NOT REQUIRE LATERAL BRACING DURING OR AFTER BACKFILLING...
L. SHOULD THE CONTRACTOR OPT TO USE A REMOTE CONTROLLED COMPACTOR TO COMPACT SOILS IN THE ZONE OCCURRING WITHIN 5'-0" OF THE BACK FACE OF THE WALL, THAT IS ACCEPTABLE PROVIDED THAT THE COMPACTOR IS ONE OF THOSE LISTED BELOW:
a. WACKER NEUSON RT SERIES OR RTX SERIES TRENCH ROLLER
b. ALTERNATE EQUIPMENT APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.

FOR CONSTRUCTION



PHILLIPS

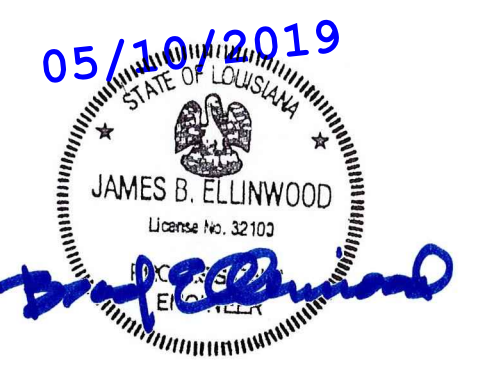
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SEAL



ISSUE & REVISION RECORD

Table with columns: #, DATE, DESCRIPTION. Includes entries for design development, permit set, coordination review, construction review, construction release, and issued for construction.

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PROJECT

FREMAUX PARK APARTMENTS

CLIENT SLIDELL, LA



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

DRAWING TITLE

GENERAL NOTES

SHEET NUMBER

S0-01

5901 PEACHTREE DUNWOODY RD. BUILDING A, SUITE 450 ATLANTA, GEORGIA 30328

PHILLIPSPART.COM 770-394-1616

RECOMMENDED LIMITS ON CEMENTICIOUS MATERIALS FOR CONCRETE MIX DESIGNS	
CEMENTICIOUS MATERIAL(S)	MAXIMUM PERCENT OF REPLACEMENT, BY MASS
FLY ASH CONFORMING TO ASTM C618	25%
SLAG CEMENT CONFORMING TO ASTM C989	50%
SILICA FUME CONFORMING TO ASTM C1240	10%
TOTAL COMBINED AMOUNT OF FLY ASH (ASTM C618) AND SILICA FUME (ASTM C1240)	35%
TOTAL COMBINED AMOUNT OF FLY ASH (ASTM C618), SLAG CEMENT (ASTM C989) AND SILICA FUME (ASTM C1240)	50%

10. WHERE SUPPLEMENTARY CEMENTICIOUS MATERIALS (FLY ASH, SLAG AND/OR SILICA FUME) ARE TO BE USED IN CONCRETE MIX DESIGNS INTENDED FOR POST-TENSIONED CONCRETE CONSTRUCTION, IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT MIX DESIGN INFORMATION THAT IS SUBMITTED FOR REVIEW AND APPROVAL CONTAINS HISTORICAL BREAK DATA FOR EARLY AGE CYLINDER BREAKS. EARLY AGE SHALL BE DEFINED AS 1-DAY, 2-DAY OR 3-DAY BREAKS. WHILE NOT A CODE REQUIREMENT, THE HISTORY IS NEEDED TO EVALUATE THE ABILITY OF THE MIX TO ACHIEVE ADEQUATE STRENGTH GAIN TO ENSURE THAT POST-TENSIONING TENDONS CAN BE STRESSED WITHIN 72 HOURS OF CONCRETE BEING CAST.

E. HORIZONTAL CONSTRUCTION JOINTS  
1. THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN THE PLACEMENT OF CONCRETE SLABS ON GRADE.  
2. HORIZONTAL CONSTRUCTION JOINTS THRU COLUMNS, PEDESTALS AND/OR PILASTERS SHALL OCCUR AT THE SOFFIT OF SLABS, DROP PANELS, OR BEAMS, UNLESS SPECIFICALLY SHOWN OTHERWISE IN THIS SET OF STRUCTURAL DRAWINGS. IF ADDITIONAL HORIZONTAL CONSTRUCTION JOINTS ARE REQUIRED AS A RESULT OF CONSTRUCTION SEQUENCING, IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE THE ENGINEER OF RECORD AWARE OF THE LOCATIONS WHERE THIS CONDITION EXISTS. SHOP DRAWINGS SHALL BE PREPARED SUCH THAT THESE CONDITIONS ARE NOTED AND ADDITIONAL REINFORCING IS PROVIDED, AS REQUIRED.  
3. HORIZONTAL CONSTRUCTION JOINTS THRU CONCRETE CANTILEVER RETAINING WALLS AND/OR CONCRETE BASEMENT WALLS ARE ACCEPTABLE, PROVIDED THAT WALL REINFORCING IS DETAILED AND PROVIDED PER THE DETAILS SHOWN IN THIS SET OF STRUCTURAL DRAWINGS.

F. VERTICAL CONSTRUCTION JOINTS  
1. VERTICAL CONSTRUCTION JOINTS THRU COLUMNS, PEDESTALS AND/OR PILASTERS SHALL NOT OCCUR.  
2. VERTICAL CONSTRUCTION JOINT LOCATIONS IN CONCRETE CANTILEVER RETAINING WALLS AND CONCRETE BASEMENT WALLS SHALL BE DETERMINED BY THE GENERAL CONTRACTOR, AND SHALL BE IN ACCORDANCE WITH THE DETAILS PROVIDED IN THE SET OF STRUCTURAL DRAWINGS. SPACING OF VERTICAL CONSTRUCTION JOINTS SHALL NOT EXCEED 60'-0". VERTICAL CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS AND CLEANED.

G. REFER TO ARCHITECTURAL DRAWINGS FOR CLIPS, GROOVES, ROUNDS, ETC., TO BE CAST IN CONCRETE AND CONCRETE FINISHES. CHAMFER OR ROUND ALL EXPOSED CORNERS, MINIMUM 3/4".

H. VERTICAL SLEEVED OPENINGS THRU SOIL-SUPPORTED CONCRETE SLABS ON GRADE  
1. REINFORCE AROUND SLEEVES AS SHOWN IN THE DETAILS IN THIS SET OF STRUCTURAL DRAWINGS.  
2. PROVIDE CLEARANCE BETWEEN SLEEVES AS SHOWN IN THE DETAILS IN THIS SET OF STRUCTURAL DRAWINGS.  
3. PROVIDE CLEARANCE BETWEEN SLEEVES AND REBAR OF NO LESS THAN 2".  
4. IF SLEEVES ARE OMITTED AND THE GENERAL CONTRACTOR MUST LATER CORE CONCRETE SLAB ON GRADE, THE GENERAL CONTRACTOR MUST PROCEED WITH CORING WITHOUT WRITTEN DIRECTION FROM THE ENGINEER OF RECORD  
a. IF THE SOIL-SUPPORTED CONCRETE SLAB ON GRADE IS POST-TENSIONED, CORING SHOULD NOT COMMENCE WITHOUT REVIEW AND WRITTEN APPROVAL FROM THE ENGINEER OF RECORD, AS CORING MAY RESULT IN DAMAGE TO POST-TENSIONING TENDONS THAT MAY REQUIRE FURTHER REMEDIAL WORK.

I. CONDUIT LOCATED WITHIN CONCRETE SLABS, JOISTS, BEAMS, GIRDERS, ETC.  
1. HORIZONTAL CONDUIT IS NOT ALLOWED IN CONCRETE SLABS ON GRADE. CONDUIT SHOULD BE ROUTED BELOW THE SLAB.  
2. REFER TO THE MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR CONDUIT SIZE, TYPE, MATERIAL, LAYOUTS, ETC.

J. REINFORCING STEEL  
1. ALL REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS, UNLESS NOTED OTHERWISE ON THESE DRAWINGS OR IN THE NOTES BELOW.  
a. BAR SIZES #10 THROUGH #18 (INCLUSIVE) - ASTM A615, GRADE 60  
b. BAR SIZES #11, #14, #18 - ASTM A615, GRADE 75  
2. PROVIDE REINFORCING STEEL CONFORMING TO ASTM A706 FOR ALL REINFORCING STEEL REQUIRED TO BE WELDED AND WHERE NOTED ON THESE DRAWINGS.  
3. PROVIDE GALVANIZED REINFORCING STEEL IN ACCORDANCE WITH ASTM A767 CLASS II (2.0 OZ ZINC PER SQUARE FOOT), WHERE NOTED ON THESE DRAWINGS.  
4. PROVIDE EPOXY-COATED REINFORCING STEEL CONFORMING TO ASTM A775 WHERE NOTED ON THESE DRAWINGS.  
5. PROVIDE DEFORMED BAR ANCHORS CONFORMING TO ASTM A496 (75,000 PSI YIELD STRENGTH) WHERE NOTED ON THESE DRAWINGS. REINFORCING BARS SHALL NOT BE SUBSTITUTED FOR DEFORMED BAR ANCHORS. ALL REINFORCING BAR BENDS TO BE MADE COLD.  
6. PROVIDE WELDED SMOOTH WIRE REINFORCEMENT CONFORMING TO ASTM A1064 (65,000 PSI YIELD STRENGTH) AND SHALL BE PROVIDED IN FLAT SHEETS WHERE NOTED ON THESE DRAWINGS.  
7. PROVIDE WELDED DEFORMED WIRE REINFORCEMENT CONFORMING TO ASTM A497 (70,000 PSI YIELD STRENGTH) WHERE NOTED ON THESE DRAWINGS.

K. WHERE WELDED WIRE REINFORCEMENT IS SPECIFIED, IT SHALL BE CONTINUOUS ACROSS THE ENTIRE CONCRETE SURFACE WITHOUT INTERRUPTION BY BEAMS, GIRDERS, OR COLUMNS. SPLICES SHALL BE LAPPED ONE CROSS WIRE SPACING PLUS 21 INCHES.

L. PROVIDE WELDED SMOOTH WIRE REINFORCEMENT (VWR 6x6-W2.9xW2.9, MINIMUM) IN ALL TOPPING SLABS AND HOUSEKEEPING SLABS, UNLESS SPECIFIED OTHERWISE ON THESE DRAWINGS.

M. CONCRETE PROTECTION FOR REINFORCEMENT OF CAST-IN-PLACE MEMBERS SHALL BE IN ACCORDANCE WITH ACI 318, §20.6.1.3. UNLESS NOTED OTHERWISE, MINIMUM CLEAR COVER OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE AS FOLLOWS.  
1. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH..... 3.00"  
2. CONCRETE EXPOSED TO EARTH AND/OR WEATHER  
NO. 6 THROUGH NO. 11 BARS..... 2.00"  
NO. 5 BAR AND SMALLER..... 1.50"  
POST TENSIONED SLABS..... 1.00"  
POST TENSIONED BEAMS..... 1.50"  
3. CONCRETE NOT EXPOSED TO WEATHER AND NOT IN CONTACT WITH GROUND SLABS, WALLS, JOISTS W/ NO. 11 BAR AND SMALLER..... 0.75"  
ADDITIONALS, TIES, SPIRALS, STIRRUPS..... 1.50"  
POST TENSIONED BEAMS (PRIMARY REINFORCEMENT)..... 1.50"  
POST TENSIONED BEAMS (TIES/STIRRUPS)..... 1.00"  
POST TENSIONED SLABS (REBAR BOTTOM COVER AT END SPANS)..... 1.50"

N. THE FOLLOWING NOTATION IS USED ON THESE DRAWINGS TO DENOTE REINFORCING STEEL DEVELOPMENT LENGTHS AND SPLICE TYPES:

CD	COMPRESSION DEVELOPMENT LENGTH
TD	TENSION DEVELOPMENT LENGTH
CS	COMPRESSION LAP SPLICE
EB	END-BEARING COMPRESSION SPLICE
M	MECHANICAL SPLICE (MINIMUM 125% YIELD STRENGTH)
A	CLASS A TENSION LAP SPLICE
B	CLASS B TENSION LAP SPLICE

REFER TO TABLES ON THESE DRAWINGS FOR MINIMUM DEVELOPMENT AND SPLICE LENGTHS.

O. UNSCHEDULED BEAMS AND SLABS, INCLUDING GRADE BEAMS, SHALL HAVE CONTINUOUS TOP BARS LAPPED AT MIDSPAN BETWEEN SUPPORTS WITH A CLASS "A" TENSION SPLICE. BOTTOM BARS SHALL BE LAPPED AT THE SUPPORTS WITH A CLASS "A" TENSION SPLICE.

P. BARS SHALL BE IN CONTACT WHEN FORMING A LAP SPLICE, UNLESS NOTED OTHERWISE.

Q. PROVIDE CORNER BARS AT ALL TURN-DOWN SLAB CORNERS AND CAST-IN-PLACE CONCRETE WALL CORNERS. PROVIDE CLASS "B" LAP SPLICE BETWEEN CORNER BARS AND MAIN REINFORCING.

R. REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED WITH A CLASS "B" LAP SPLICE, UNLESS SPECIFICALLY DETAILED OTHERWISE. PROVIDE CONTINUOUS REINFORCEMENT WHERE EVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE TENSION SPLICE (CLASS "B"), UNLESS NOTED OTHERWISE.

S. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE SPECIFIED WALL OR COLUMN REINFORCEMENT AND SHALL BE LAPPED WITH TENSION SPLICES (CLASS "B"), UNLESS NOTED OTHERWISE.

T. HORIZONTAL REINFORCEMENT IN FOOTINGS, TURNDOWN SLABS AND WALLS SHALL BE CONTINUOUS AROUND CORNERS. HORIZONTAL REINFORCEMENT SHALL CONTINUE AT ENDS AND CORNERS WITH BEND TO FAR FACE OF INTERSECTING ELEMENT IN EACH DIRECTION. ADDITIONAL HORIZONTAL CORNER BARS OF SAME SIZE AND SPACING MAY BE PROVIDED. PROVIDE CORNER BARS AT ALL TURN-DOWN SLAB CORNERS AND CAST-IN-PLACE CONCRETE ALL CORNERS. PROVIDE CLASS "B" LAP BETWEEN CORNER BARS AND MAIN REINFORCING.

U. DETAILING, FABRICATION AND PLACING REINFORCING STEEL AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" DETAILING MANUAL. SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.

V. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT, WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS, ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE. THE GENERAL CONTRACTOR SHALL NOT PLACE ANY REINFORCING UNTIL APPROVED SHOP DRAWINGS ARE RECEIVED AT THE JOB SITE.

W. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING CONCRETE. TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES.

X. DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS DIRECTED TO DO SO IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.

Y. ALL CONCRETE CONSTRUCTION SHALL BE INSPECTED BY OR BE UNDER SUPERVISION OF A LICENSED DESIGN PROFESSIONAL OR BY A QUALIFIED SPECIAL INSPECTOR.

Z. WHERE INDICATED ON THE PLANS, REINFORCING BAR STANDARD HOOKS MAY BE REPLACED WITH LENTON TERMINATORS PER ICC ER 3967.  
AA. WHERE INDICATED ON THE PLANS, CLASS "B" TENSION LAP SPLICES MAY BE REPLACED WITH TYPE 2 MECHANICAL SPLICES PER ICC ER 5064 OR ICC ER 5461.  
AB. THE GENERAL CONTRACTOR SHALL INCLUDE IN THEIR PRICE 25 YARDS OF 3,000 PSI CONCRETE TO BE PLACED AT THE DIRECTION OF THE ENGINEER OF RECORD. A CREDIT SHALL BE GIVEN BACK TO THE OWNER AFTER PROJECT COMPLETION FOR CONCRETE NOT PLACED.  
AC. PROVIDE FOR AN ALLOWANCE OF 1.00% OF REINFORCING BAR TONNAGE TO BE FABRICATED AND PLACED DURING PROGRESS OF WORK AS MAY BE DIRECTED BY THE STRUCTURAL ENGINEER, IN ADDITION TO ALL THE STEEL INDICATED ON THE DRAWINGS.

**5. SOIL-SUPPORTED POST-TENSIONED SLAB ON GRADE**

A. DESIGN  
1. FOUNDATIONS SHOWN ON THESE DRAWINGS HAVE BEEN DESIGNED USING ACCEPTED ENGINEERING PRACTICES AND THE AMERICAN CONCRETE INSTITUTE'S "CONCRETE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND THE POST-TENSIONING INSTITUTE'S "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF POST-TENSIONED SLABS-ON-GROUND."  
2. THE FINAL TENDON LAYOUT SHALL BE SHOWN IN ITS ENTIRETY ON THE SHOP DRAWINGS THAT ARE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD FOR REVIEW AND APPROVAL, AND SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF LOUISIANA.  
3. THE DESIGN OF THE POST-TENSIONED SLABS ON GRADE DO NOT ALLOW FOR IMPROPER DRAINAGE. TREES LOCATED TOO CLOSE TO THE FOUNDATION OR IMPROPER MAINTENANCE.  
4. CALCULATIONS FOR TENDON LOSSES SHALL BE FURNISHED TO THE DESIGN TEAM AS A PART OF THE POST-TENSION SHOP DRAWINGS.

B. COORDINATION  
1. THE BUILDING CONTRACTOR SHALL VERIFY ALL DROPS, OFF-SETS, BRICK LEDGES AND BLOCKOUTS ON ARCHITECTURAL PLANS AND NOTIFY THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES THAT MAY EXIST BETWEEN THE STRUCTURAL DRAWINGS AND ARCHITECTURAL DRAWINGS.  
2. THE GENERAL CONTRACTOR SHALL COORDINATE STRUCTURAL PLANS WITH THE ARCHITECTURAL AND M/E/P DRAWINGS FOR ALL SLEEVES, OPENINGS, INSERTS AND OTHER RELATED ITEMS REQUIRED TO COMPLETE THE FOUNDATION.

C. MATERIALS  
1. CONCRETE IN FOUNDATION BEAMS AND SLABS MUST ATTAIN A MINIMUM COMPRESSIVE STRENGTH AS NOTED IN THE CONCRETE SECTION OF THE GENERAL NOTES AND AT LEAST 2,000 PSI AT THE TIME OF STRESSING. CONCRETE DESIGN MIX SHALL BE IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS (ACI 318) TO ENSURE QUALITY CONCRETE.  
2. PLACE A 10 MIL VAPOR BARRIER OF POLYETHYLENE UNDER ALL CONCRETE SLABS.  
3. TENDONS SHALL BE LOW RELAXATION CABLE WITH PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING CONFORMING TO ASTM A-416. END ANCHORAGE DEVICES SHALL CONFORM TO P.T.I. DESIGN SPECIFICATIONS. ALL DEAD END ANCHORAGE MUST BE FACTORY SEATED OR POWER SEATED.  
4. MILD STEEL REINFORCING (IF REQUIRED) SHALL BE REBAR IN ACCORDANCE WITH ASTM A-615-60 OR WELDED WIRE FABRIC IN ACCORDANCE WITH ASTM 188.  
5. SEE CONCRETE SECTION OF GENERAL NOTES FOR ADDITIONAL INFORMATION.  
6. EXTRA CORROSION PROTECTION OF THE ANCHORAGES AND SHEATHING SHALL BE PROVIDED BY ENCAPSULATION AS SHOWN ON STRUCTURAL PLANS. ENCAPSULATED ANCHORAGES SHALL INCLUDE THE HARDWARE AND MATERIALS NECESSARY TO PROVIDE A WATERTIGHT CONNECTION OF THE SHEATHING TO THE ANCHORAGE AND WATERTIGHT CLOSING OF THE WEDGE CAVITY. THE METHOD FOR PROVIDING WATERTIGHT ANCHORAGES AND CONNECTIONS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO THE APPROVAL OF ANY POST-TENSIONING STEEL SHOP DRAWINGS.  
a. SHEATHING THICKNESS FOR TENDONS SHALL NOT BE LESS THAN 0.040 IN. FOR MEDIUM OR HIGH DENSITY POLYETHYLENE OR POLYPROPYLENE.  
b. CORROSION PREVENTIVE COATING AT THE EXPOSED TENDON AND REPAIRING THE DAMAGED PORTIONS OF SHEATHING SHALL BE REAPPLIED BY RE-APPLYING THE SHEATHING IN A WATERTIGHT FASHION WITH AT LEAST ONE NEW LAYER OF SHEATHING COMPLETELY SPIRITALLY WRAPPED WITH AT LEAST TWO LAYERS OF ADHESIVE MOISTURE-PROOF TAPE.  
c. PRIOR TO GROUTING THE ANCHOR RECESSES, THE INSIDE CONCRETE SURFACES OF THE POCKET SHALL BE COATED OR SPRAYED WITH A RESIN, CORROSION RESISTANT, BONDING AGENT.

D. CONSTRUCTION  
1. FOUNDATION BEAM DIMENSIONS SHOWN ARE THE MINIMUM SIZE RECOMMENDED BY ELLINWOOD & MACHADO ARCHITECTS, PC. NOT BE REDUCED OR ENLARGED WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.  
2. PLACE FILL UNDER SLABS IN COMPLIANCE WITH SITE PREPARATION RECOMMENDATIONS FROM THE REFERENCED GEOTECHNICAL ENGINEERING REPORT (SEE §3 OF THE GENERAL NOTES).  
3. CONCRETE SHALL BE WELL CONSOLIDATED, PARTICULARLY IN THE VICINITY OF THE TENDON ANCHORAGE.  
4. SUPPORT TENDONS AND BARS SECURELY TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING PLACING OF CONCRETE.  
5. ALLOW FOR A CLEARANCE AREA OF 8" DIAMETER, CENTERED ON TENDON AXIS, BY 36" LENGTH FOR STRESSING EQUIPMENT.  
6. IF TENDON SHEATHING IS DAMAGED FOR 5" OR MORE IT SHALL BE RE-SHEATHED TO PREVENT CONCRETE FROM BONDING TO THE CABLE.  
7. NO PLUMBING LINES SHALL BE RUNNING PARALLEL TO AND WITHIN OR UNDER ANY FOUNDATION BEAM.

E. QUALIFICATION  
1. POST-TENSIONING CONTRACTOR SHALL, UPON REQUEST, FURNISH THE FOLLOWING:  
a. LABORATORY TEST ON ANCHORAGE SYSTEM.  
b. LATEST CALIBRATION DATE OF EQUIPMENT USED.  
c. LABORATORY TEST COEFFICIENT OF FRICTION ON STRANDS.

d. MILL TEST CERTIFICATE ON STRANDS.

F. STRESSING  
1. 1/2" DIAMETER TENDONS SHALL BE ANCHORED AT 28.9K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33.0K PER STRAND.  
2. TO REDUCE OR ELIMINATE POSSIBLE SHRINKAGE CRACKS, IT IS ALLOWABLE FOR EACH TENDON SHALL BE STRESSED TO APPROXIMATELY 16 HOURS AFTER THE CONCRETE STRENGTH REACHES 1,000 PSI (OR APPROXIMATELY 16 HOURS AFTER CONCRETE PLACEMENT).

G. THE CONTRACTOR SHALL SUBMIT TO THE STRUCTURAL ENGINEER OF RECORD A SUMMARY OF ALL STRESSING OPERATIONS FOR EACH STRESSING SEQUENCE. THE SUBMITTED SUMMARY SHALL INCLUDE THE CALCULATED AND ACTUAL MEASURED ELONGATIONS FOR EACH JACKING POINT FOR EACH TENDON. THE STRESSING RAM NUMBER AND JACKING PRESSURE OR FORCE FOR EACH TENDON, AND THE DATE OF STRESSING AND SIGNATURE OF CONTRACTOR'S STRESSING PERSONNEL.

H. TENDON ENDS (TAILS) SHALL NOT BE CUT OFF UNTIL ALL TENDONS HAVE BEEN SATISFACTORILY STRESSED AND THE ELONGATION RECORDS REVIEWED. STRESSING END ANCHORAGES AND POCKETS SHALL BE SEALED AS FOLLOWS:  
1. CUT STRAND OFF INSIDE FACE OF CONCRETE. CARE SHALL BE TAKEN TO AVOID HEATING THE WEDGES.  
2. COAT ANCHORAGE WITH RUST-O-LEUM, Z.R.C. COLD GALVANIZING COMPOUND, OR OTHER EQUIVALENT ENGINEER-APPROVED CORROSION PROTECTION. CARE SHALL BE TAKEN TO PREVENT CONTAMINATION OF THE ANCHORAGE RECESS SURFACE, WHICH MAY REDUCE THE BONDING CAPACITY OF THE NON-SHRINK GROUT.  
3. PATCH POCKET WITH MASTER BLDGERS MASTERFLOW 713, L&M CONSTRUCTION CHEMICALS CRYSTEX, SIMPSON STRONG-TIE FX-228, OR OTHER EQUIVALENT ENGINEER-APPROVED NON-METALLIC NON-SHRINK GROUT. CARE SHALL BE TAKEN TO PROPERLY PREPARE THE CONCRETE SURFACE AND CURE THE PATCHING GROUT ACCORDING TO GROUT MANUFACTURER'S INSTRUCTIONS.

I. WELDING IN THE VICINITY OF POST-TENSIONING TENDONS IS NOT ALLOWED. TENDONS SHALL NOT BE USED AS AN ELECTRICAL GROUND FOR ANY WELDING OPERATIONS.

J. CONSTRUCTION JOINTS LOCATIONS IN POST-TENSIONED MEMBERS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL (TYPICALLY AS A PART OF THE POST-TENSIONED REINFORCING SHOP DRAWINGS).  
1. CONSTRUCTION JOINTS SHALL BE LOCATED WHERE THE C.G.S. (CENTER OF GRAVITY - STEEL) COINCIDES WITH THE C.G.C. (CENTER OF GRAVITY - CONCRETE SECTION).  
2. CALCULATED ELONGATIONS (COMPLETED BY THE TENDON SUPPLIER AND SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW) MUST BE COORDINATED WITH CONSTRUCTION JOINT LOCATIONS. ELONGATION CALCULATIONS MUST REFLECT THE NEED FOR TENDONS TO BE STRESSED MORE THAN ONCE, AND THE REPORTED ELONGATIONS SHOULD BE SHOWN FOR EACH STRESSING OF EACH TENDON.  
3. IT SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL CONSTRUCTION JOINT LOCATIONS ARE COORDINATED WITH FORMWORK AND REBAR SHOP DRAWINGS.

K. SPECIAL NOTES  
1. DO NOT SCALE FROM THE FOUNDATION PLANS. IF THE GENERAL CONTRACTOR FEELS THAT DIMENSIONS ARE MISSING, THE STRUCTURAL ENGINEER OF RECORD SHALL BE CONTACTED TO PROVIDE CLARIFICATION.  
2. ALL EDGE OF SLAB (EOS) DIMENSIONS SHALL BE AS SHOWN IN THE ARCHITECTURAL DRAWINGS.  
3. POST-TENSIONED SLAB-ON-GRADE SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO PLACEMENT OF THE SLAB. SHOP DRAWINGS SHALL INCLUDE EXACT LOCATION, QUANTITY AND TYPE OF POST-TENSIONING STRANDS, FOUNDATION BEAM (TURN DOWN) AND THICKENED SLAB SIZE AND LOCATION, REQUIRED MILD STEEL REINFORCEMENT, LOCATIONS OF CONSTRUCTION JOINTS, DESIGNATION OF DEAD AND LIVE TENDON ENDS, APPLICABLE DETAILS AND SECTIONS, AND REQUIRED ANCHORAGES AND ACCESSORIES.

L. ALL CMU SHALL BE PLACED IN RUNNING BOND.  
M. ALL MASONRY CONSTRUCTION AND INSPECTION SHALL COMPLY WITH ACI 530 AND ACI 530.1.  
N. ALL CONCRETE MASONRY CONSTRUCTION SHALL BE INSPECTED AND TESTED PER THE REQUIREMENTS OF ACI 530.1. COSTS OF THE SERVICES OF AN INDEPENDENT TESTING LABORATORY TO PERFORM TESTING AND INSPECTION SERVICES SHALL BE BORNE BY THE CONTRACTOR.  
O. CMU GROUT FILL SHALL ARRIVE AT THE JOB SITE WITH A SLUMP BETWEEN 3" TO 5". CMU GROUT FILL SHALL ARRIVE AT THE JOB SITE WITH A SLUMP OF 3" - 5". PRIOR TO DEPOSITING GROUT, SUPERPLASTICIZER SHALL BE ADDED TO THE GROUT AT THE JOB SITE, INCREASING THE SLUMP TO 8" - 10".  
P. CMU WALL REINFORCING SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO FABRICATION. DRAWINGS SHALL SHOW ALL WALL AND PILASTER REINFORCING IN PLAN AND IN ELEVATION.  
Q. PROVIDE CORNER BARS AT ALL BOND BEAMS TO ENSURE CONTINUITY AT CORNERS. LAP CORNER BARS WITH A CLASS "B" TENSION LAP SPLICE.  
R. PROVIDE BAR SUPPORTS AND POSITIONERS AS REQUIRED TO ENSURE THAT FINAL IN-PLACE LOCATION OF REINFORCING IS AS INDICATED ON THE DRAWINGS.  
S. MASONRY SHALL BE PROTECTED FROM FREEZING DURING PLACEMENT & CURING. COLD WEATHER MASONRY PROCEDURES SHALL COMPLY W/ACI 530 AND ACI 530.1.  
T. THE GENERAL CONTRACTOR SHALL PROVIDE AND INSTALL BRACING AND SHORING FOR ALL MASONRY WALLS AS REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.  
U. SIDE AND TOP OF MASONRY WALL PANELS SHALL BE ANCHORED TO STRUCTURE BY DOVETAIL ANCHORS, METAL STRAPS OR STEEL ANGLES. SEE DETAILS ON SHEETS S4-2#.  
V. BRICK TIES SHALL BE LOCATED/SPACED BOTH VERTICALLY AND HORIZONTALLY AS SPECIFIED BY THE ARCHITECT AND SHALL BE CAPABLE OF SUPPORTING THE BRICK FOR OUT-OF-PLANE LOADING DUE TO WIND AND SEISMIC LOADS. SEE DETAILS ON SHEET S0-20 FOR COMPONENT AND CLADDING WIND LOADS.  
W. BRICK AND STONE VENEER SHALL HAVE VERTICAL CONTROL JOINTS AT 20'-0" MAXIMUM SPACING. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LAYOUT. AS MORE CLOSELY SPACED CONTROL JOINTS MAY BE NECESSARY AT WINDOWS, DOORS, ETC.

**6. EXPANSION ANCHORS, ADHESIVE ANCHORS AND POWDER ACTUATED FASTENERS**

A. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW AND APPROVAL.  
1. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A CODE REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION.  
2. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATIONS OR TABLE(S) OF PUBLISHED, TABULATED VALUES OBTAINED THROUGH EMPIRICAL TEST PROCEDURES THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT.  
B. CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR PRODUCT INSTALLATION TRAINING AND A LETTER SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD INDICATING TRAINING HAS TAKEN PLACE.  
C. SPECIAL INSPECTIONS ARE REQUIRED PER THE 2015 INTERNATIONAL BUILDING CODE AND PRODUCT EVALUATION REPORTS.  
D. THE BELOW PRODUCTS ARE THE DESIGN BASIS FOR THIS PROJECT.

1. FOR ANCHORING INTO CONCRETE:  
a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 308.2 AND ICC-ES AC1093. PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)  
b. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308. PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "AT-XP" (APMO UES ER-263)  
2. FOR ANCHORING INTO SOLID-GROUTED CONCRETE MASONRY:  
a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC106. PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)  
b. ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH CC-ES AC58. PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "AT-XP" (APMO UES ER-281)  
3. FOR ANCHORING INTO HOLLOW CONCRETE MASONRY:  
a. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC106. PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)  
b. ADHESIVE ANCHORS WITH SCREEN TUBES SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH CC-ES AC58. THE APPROPRIATE PRE-APPROVED ANCHORS INCLUDE: - SIMPSON STRONG-TIE "SET" (ICC-ES ESR-1772)

A. ALL LUMBER (EXCLUDING WALL STUDS) SHALL BE VISUALLY GRADED, SOUTHERN YELLOW PINE (SYP) DIMENSIONAL LUMBER, KD19 (19% MAXIMUM MOISTURE CONTENT AFTER DRYING), UNLESS NOTED OTHERWISE, AND IN ACCORDANCE WITH THE FOLLOWING MINIMUM GRADE REQUIREMENTS (GRADING PER SPIB):  
JOISTS..... STRUCT. GRADE NO. 2  
BEAMS (2"-4" THICK)..... STRUCT. GRADE NO. 2  
POSTS..... STRUCT. GRADE NO. 2  
PLATE STOCK..... STRUCT. GRADE NO. 3

B. ALL WALL STUDS SHALL BE VISUALLY GRADED, FINGER JOINTED SYP, STRUCTURAL GRADE NO. 2, KD19 (19% MAXIMUM MOISTURE CONTENT AFTER DRYING), UNLESS NOTED OTHERWISE. GRADING SHALL BE PER PS 20-70 (AMERICAN SOFTWOOD LUMBER STANDARD) OR NLGA.

C. MATERIALS FOR LAMINATED BEAMS (LVL AND PSL) SHALL CONFORM TO THE FOLLOWING MINIMUM ALLOWABLE STRESSES AND MATERIAL PROPERTIES.  
TIMBERSTRAND 1.3E (LSL) PLANK:  
F<sub>b</sub>..... 1,900 PSI  
F<sub>v</sub>..... 150 PSI  
E..... 1.3x10<sup>6</sup> PSI  
F<sub>L</sub>..... 1,075 PSI  
F<sub>c</sub>-perpendicular..... 670 PSI  
F<sub>c</sub>-parallel..... 1,835 PSI  
(\*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d)<sup>3</sup>(0.892)

MICROLAM 2.0E (LVL) BEAM:  
F<sub>b</sub>..... 2,600 PSI  
F<sub>v</sub>..... 285 PSI  
E..... 2.0x10<sup>6</sup> PSI  
F<sub>L</sub>..... 1,555 PSI  
F<sub>c</sub>-perpendicular..... 750 PSI  
F<sub>c</sub>-parallel..... 2,510 PSI  
(\*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d)<sup>3</sup>(0.136)

E. WHERE REQUIRED FOR ANCHORAGE TO STEEL, CONCRETE AND/OR CONCRETE MASONRY, POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70.

a. PRE-APPROVED FASTENERS INCLUDE:  
- SIMPSON STRONG-TIE "GAS ACTUATED PINS" (ICC-ES ESR-2811)  
- SIMPSON STRONG-TIE "POWDER ACTUATED PINS" (ICC-ES ESR-2138)

**7. CONCRETE MASONRY**

A. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS (F'<sub>m</sub>) SHALL BE 1,500 PSI.

B. MATERIAL SHALL BE AS FOLLOWS:  
CMU..... GRADE N, ASTM C-90 (UNIT STRENGTH = 1900 PSI)  
MORTAR..... TYPE S FOR WALLS NOT IN CONTACT WITH EARTH  
TYPE M FOR WALLS IN CONTACT WITH EARTH

C. GROUT FOR CONCRETE MASONRY WALL SHALL CONFORM TO ASTM C476, F<sub>c</sub> = 3,000 PSI, MIN. GROUT SHALL BE CONSOLIDATED BY THOROUGHLY RODDING ALL CELLS.

D. GROUT PLACEMENT SHALL BE LOW-LIFT. THE CONSTRUCTION JOINTS ARE CREATED BY THE LEVEL OF GROUT STOPPING AT 1-1/2" FROM TOP OF MASONRY AND THE STEEL REINFORCING PROJECTING ABOVE THE TOP COURSE FOR A SUFFICIENT HEIGHT TO PROVIDE A LAP AT THE SPLICE OF 48 BAR DIAMETERS. THE CONSTRUCTION JOINT SHALL BE LOCATED 3'-0" MINIMUM FROM TOP AND BOTTOM OF STRUCTURAL ELEMENTS SUCH AS SLABS, ROOFS, ETC.

E. CONCRETE MASONRY WALLS SHALL BE TEMPORARILY BRACED DURING ERECTION. REMOVE TEMPORARY BRACING ONLY AFTER WALLS ARE CONNECTED TO SUPPORTING ELEMENTS.

F. WHERE APPLICABLE, ALL CONCRETE BLOCK BELOW THE DESIGN FLOOR ELEVATION SHALL HAVE ALL CELLS FILLED WITH GROUT. SEE ARCH. AND CIVIL DRAWINGS FOR ADDITIONAL INFO.

G. ALL CELLS CONTAINING REINFORCEMENT SHALL BE GROUTED SOLID.  
H. MAXIMUM CONTROL JOINT SPACING IN MASONRY WALL = 30'-0" UNLESS NOTED. SEE ARCHITECTURAL DRAWINGS FOR LOCATION.

I. UNLESS SPECIFICALLY NOTED OTHERWISE, ALL NON-LOAD-BEARING CMU WALLS SHALL BE REINFORCED AS SHOWN IN DETAILS ON SHEET S4-20. ADDITIONALLY:  
1. PROVIDE CONTINUOUS BOND BEAM REINFORCING W/2#(5) CONTINUOUS AT ALL FLOOR LEVELS. WHERE FLOOR-TO-FLOOR HEIGHTS EXCEED 11'-0", PROVIDE INTERMEDIATE BOND BEAMS AT 10'-0" MAXIMUM SPACING.  
2. PROVIDE CONTINUOUS 6 GAGE TRUSS-TYPE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY (EVERY OTHER COURSE), TYP.

J. DOWEL ALL CMU WALLS INTO GRADE BEAMS, SPREAD FOOTINGS, CONTINUOUS FOOTINGS, THICKENED SLAB ELEMENTS, CONCRETE FOUNDATION WALLS AND/OR ELEVATED CONCRETE FRAMING UPON WHICH IT BEARS. DOWELS SHALL HAVE STANDARD HOOKS INTO THE CONCRETE FOUNDATION, WALL OR FRAMING, TYP. DOWELS SHALL BE OF SUFFICIENT LENGTH TO PROVIDE A CLASS "B" TENSION LAP SPLICE. DOWELS SHALL BE OF SAME SIZE AND SPACING AS VERTICAL WALL REINFORCING.

K. SEE ARCHITECTURAL DRAWINGS FOR ALL CMU WALL OPENING SIZES AND LOCATIONS.

L. ALL CMU SHALL BE PLACED IN RUNNING BOND.

M. ALL MASONRY CONSTRUCTION AND INSPECTION SHALL COMPLY WITH ACI 530 AND ACI 530.1.

N. ALL CONCRETE MASONRY CONSTRUCTION SHALL BE INSPECTED AND TESTED PER THE REQUIREMENTS OF ACI 530.1. COSTS OF THE SERVICES OF AN INDEPENDENT TESTING LABORATORY TO PERFORM TESTING AND INSPECTION SERVICES SHALL BE BORNE BY THE CONTRACTOR.  
O. CMU GROUT FILL SHALL ARRIVE AT THE JOB SITE WITH A SLUMP BETWEEN 3" TO 5". CMU GROUT FILL SHALL ARRIVE AT THE JOB SITE WITH A SLUMP OF 3" - 5". PRIOR TO DEPOSITING GROUT, SUPERPLASTICIZER SHALL BE ADDED TO THE GROUT AT THE JOB SITE, INCREASING THE SLUMP TO 8" - 10".  
P. CMU WALL REINFORCING SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO FABRICATION. DRAWINGS SHALL SHOW ALL WALL AND PILASTER REINFORCING IN PLAN AND IN ELEVATION.  
Q. PROVIDE CORNER BARS AT ALL BOND BEAMS TO ENSURE CONTINUITY AT CORNERS. LAP CORNER BARS WITH A CLASS "B" TENSION LAP SPLICE.  
R. PROVIDE BAR SUPPORTS AND POSITIONERS AS REQUIRED TO ENSURE THAT FINAL IN-PLACE LOCATION OF REINFORCING IS AS INDICATED ON THE DRAWINGS.  
S. MASONRY SHALL BE PROTECTED FROM FREEZING DURING PLACEMENT & CURING. COLD WEATHER MASONRY PROCEDURES SHALL COMPLY W/ACI 530 AND ACI 530.1.  
T. THE GENERAL CONTRACTOR SHALL PROVIDE AND INSTALL BRACING AND SHORING FOR ALL MASONRY WALLS AS REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.  
U. SIDE AND TOP OF MASONRY WALL PANELS SHALL BE ANCHORED TO STRUCTURE BY DOVETAIL ANCHORS, METAL STRAPS OR STEEL ANGLES. SEE DETAILS ON SHEETS S4-2#.  
V. BRICK TIES SHALL BE LOCATED/SPACED BOTH VERTICALLY AND HORIZONTALLY AS SPECIFIED BY THE ARCHITECT AND SHALL BE CAPABLE OF SUPPORTING THE BRICK FOR OUT-OF-PLANE LOADING DUE TO WIND AND SEISMIC LOADS. SEE DETAILS ON SHEET S0-20 FOR COMPONENT AND CLADDING WIND LOADS.  
W. BRICK AND STONE VENEER SHALL HAVE VERTICAL CONTROL JOINTS AT 20'-0" MAXIMUM SPACING. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LAYOUT. AS MORE CLOSELY SPACED CONTROL JOINTS MAY BE NECESSARY AT WINDOWS, DOORS, ETC.

Q. PROVIDE CORNER BARS AT ALL BOND BEAMS TO ENSURE CONTINUITY AT CORNERS. LAP CORNER BARS WITH A CLASS "B" TENSION LAP SPLICE.

R. PROVIDE BAR SUPPORTS AND POSITIONERS AS REQUIRED TO ENSURE THAT FINAL IN-PLACE LOCATION OF REINFORCING IS AS INDICATED ON THE DRAWINGS.

S. MASONRY SHALL BE PROTECTED FROM FREEZING DURING PLACEMENT & CURING. COLD WEATHER MASONRY PROCEDURES SHALL COMPLY W/ACI 530 AND ACI 530.1.

T. THE GENERAL CONTRACTOR SHALL PROVIDE AND INSTALL BRACING AND SHORING FOR ALL MASONRY WALLS AS REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.

U. SIDE AND TOP OF MASONRY WALL PANELS SHALL BE ANCHORED TO STRUCTURE BY DOVETAIL ANCHORS, METAL STRAPS OR STEEL ANGLES. SEE DETAILS ON SHEETS S4-2#.

V. BRICK TIES SHALL BE LOCATED/SPACED BOTH VERTICALLY AND HORIZONTALLY AS SPECIFIED BY THE ARCHITECT AND SHALL BE CAPABLE OF SUPPORTING THE BRICK FOR OUT-OF-PLANE LOADING DUE TO WIND AND SEISMIC LOADS. SEE DETAILS ON SHEET S0-20 FOR COMPONENT AND CLADDING WIND LOADS.  
W. BRICK AND STONE VENEER SHALL HAVE VERTICAL CONTROL JOINTS AT 20'-0" MAXIMUM SPACING. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LAYOUT. AS MORE CLOSELY SPACED CONTROL JOINTS MAY BE NECESSARY AT WINDOWS, DOORS, ETC.

**8. SOLID SAWN & LAMINATED LUMBER**

A. ALL LUMBER (EXCLUDING WALL STUDS) SHALL BE VISUALLY GRADED, SOUTHERN YELLOW PINE (SYP) DIMENSIONAL LUMBER, KD19 (19% MAXIMUM MOISTURE CONTENT AFTER DRYING), UNLESS NOTED OTHERWISE, AND IN ACCORDANCE WITH THE FOLLOWING MINIMUM GRADE REQUIREMENTS (GRADING PER SPIB):  
JOISTS..... STRUCT. GRADE NO. 2  
BEAMS (2"-4" THICK)..... STRUCT. GRADE NO. 2  
POSTS..... STRUCT. GRADE NO. 2  
PLATE STOCK..... STRUCT. GRADE NO. 3</

PARALLAM 1.8E (PSL) COLUMN:	
Fb	2,400 PSI
Fv	190 PSI
E	1.8x10 <sup>6</sup> PSI
Ft	1,755 PSI
Fc-perpendicular	545 PSI
Fc-parallel	2,500 PSI
(*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d) <sup>2</sup> (0.111)	

PARALLAM 2.0E (PSL) BEAM:	
Fb*	2,900 PSI
Fv	290 PSI
E	2.0x10 <sup>6</sup> PSI
Ft	2,025 PSI
Fc-perpendicular	625 PSI
Fc-parallel	2,900 PSI
(*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d) <sup>2</sup> (0.111)	

PARALLAM PLUS (PSL+) BEAM:	
Fb*	1,827 PSI
Fv	197 PSI
E	1.46x10 <sup>6</sup> PSI
Ft	1,397 PSI
Fc-perpendicular	338 PSI
Fc-parallel	1,508 PSI
(*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d) <sup>2</sup> (0.111)	
NOTE: PSL+ DESIGN VALUES PROVIDED FOR A SERVICE LEVEL 2, REPEATED EXPOSURE TO WET AND DRY CYCLES, APPLICATION ENVIRONMENT.	

PARALLAM PLUS (PSL+) COLUMN:	
Fb	1,440 PSI
Fv	120 PSI
E	1.224x10 <sup>6</sup> PSI
Ft	1,211 PSI
Fc-perpendicular	196 PSI
Fc-parallel	1,300 PSI
(*) - FOR 12-INCH DEPTH FOR ALL OTHER DEPTHS MULTIPLY BY (12/d) <sup>2</sup> (0.111)	
NOTE: PSL+ DESIGN VALUES PROVIDED FOR A SERVICE LEVEL 2, REPEATED EXPOSURE TO WET AND DRY CYCLES, APPLICATION ENVIRONMENT.	

GLULAM BEAM (SOUTHERN PINE 24F-V3):	
F <sub>y</sub> **	2,400 PSI
F <sub>y</sub> **	1,950 PSI
F <sub>v</sub>	300 PSI
E	1.700x10 <sup>6</sup> PSI
F <sub>t</sub>	1,150 PSI
F <sub>c-perpendicular</sub>	740 PSI
F <sub>c-parallel</sub>	2,300 PSI

(\*\*) THE MATERIALS USED IN THE MANUFACTURE OF GLULAM BEAM MEMBERS DIFFERS FROM THE EXTREMITY TOP AND BOTTOM OF THE BEAM, AS A RESULT, IT IS OF GREAT IMPORTANCE THAT THE BEAM IS INSTALLED WITH THE CORRECT SIDE UP. GENERAL CONTRACTOR AND FRAMER TO ENSURE THAT ALL GLULAM BEAMS ARE BEING INSTALLED WITH THE CORRECT ORIENTATION PRIOR TO FRAMING ABOVE.

GLULAM COLUMN (SOUTHERN PINE N1D14):	
F <sub>y</sub> (f <sub>c</sub> )	2,300 PSI
F <sub>y</sub> (f <sub>c</sub> )	2,100 PSI
F <sub>v</sub> (f <sub>c</sub> )	1,750 PSI
F <sub>w</sub> (f <sub>c</sub> )	2,100 PSI
F <sub>w</sub>	300 PSI
E	1.900x10 <sup>6</sup> PSI
F <sub>t</sub>	1,550 PSI
F <sub>t</sub>	740 PSI
F <sub>c-perpendicular</sub>	2,300 PSI
F <sub>c-parallel</sub>	1,700 PSI

- F<sub>y</sub> SHALL INDICATE BENDING ABOUT THE Y-Y AXIS AS A RESULT OF LOADING APPLIED PARALLEL TO THE WIDE FACES OF THE LAMINATIONS
- F<sub>v</sub> SHALL INDICATE BENDING ABOUT THE X-X AXIS AS A RESULT OF LOADING APPLIED PERPENDICULAR TO THE WIDE FACES OF THE LAMINATIONS
- "Q<sub>4</sub>" INDICATES 4 OR MORE LAMINATIONS
- "Q<sub>3</sub>" INDICATES 3 LAMINATIONS
- "Q<sub>2</sub>" INDICATES 2 LAMINATIONS
- "Q<sub>4</sub>" INDICATES 4 OR MORE LAMINATIONS
- "Q<sub>2,3</sub>" INDICATES 2 OR 3 LAMINATIONS

- D. ALL CONNECTOR TYPES REFER TO SIMPSON STRONG-TIE SPECIFICATIONS. ANY CHANGE, MODIFICATION OR SUBSTITUTION MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.
- E. INSTALL TEMPORARY BRACING DURING CONSTRUCTION AT STUD WALLS UNTIL ALL FLOOR AND ROOF SHEATHING, ROOF TRUSSES, AND SHEAR PANELS HAVE BEEN INSTALLED. LET-IN METAL OR WOOD BRACING THAT REQUIRES NOTCHING OF THE STUDS IS NOT PERMITTED.
- F. ALL LUMBER USED IN EXTERIOR CONSTRUCTION AND ALL LUMBER IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE-TREATED TO MEET THE REQUIREMENTS OF AWPA U1. PRESERVATIVE TREATMENTS SHALL BE SELECTED TO MEET THE APPROPRIATE ASSIGNED USE CATEGORIES AS SHOWN BELOW:
- SILL PLATES USE CATEGORY UC2
  - CMU/CONCRETE USE CATEGORY UC2
  - BALCONY POSTS USE CATEGORY UC3B
  - BALCONY DECK BOARDS USE CATEGORY UC3B
  - BALCONY LEDGER BOARDS USE CATEGORY UC3B
  - BALCONY JOISTS USE CATEGORY UC3B
  - BALCONY PLYWOOD (SHEATHING) USE CATEGORY UC3B
  - LUMBER IN DIRECT CONTACT WITH THE GROUND USE CATEGORY UC4B
- CONTRACTOR SHALL IDENTIFY AND SUBMIT THE PROPOSED PRESERVATIVE TREATMENT AND RETENTION LEVELS FOR EACH LUMBER APPLICATION. PRESERVATIVE SHALL BE APPLICABLE TO THE USE CATEGORY, COMMODITY SPECIFICATION AND LUMBER SPECIES AS DEFINED BY AWPA U1. CCA (CHROMATED COPPER ARSENATE) PRESERVATIVE TREATMENTS ARE NOT PERMITTED.

- G. ALL FASTENERS (INCLUDING NUTS AND WASHERS) IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIP GALVANIZED PER ASTM A153. ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE HOT-DIP GALVANIZED PER ASTM A663 AND MADE FROM CLASS G185 SHEET WITH 1.85 OUNCES (MINIMUM) OF ZINC COATING PER SQUARE FOOT. FASTENERS AND CONNECTORS IN CONTACT WITH PRESERVATIVE-TREATED LUMBER WITH A USE CATEGORY OF UC4 SHALL BE STAINLESS STEEL. FASTENERS AND ANCHORS SHALL COMPLY WITH TYPE 304 OR 305. CONNECTORS SHALL COMPLY WITH TYPE 316L.

- H. SEE SHEAR WALL SCHEDULE ON S6-00A FOR SILL PLATE ANCHORAGE AT ALL SHEAR WALLS. SHEAR WALLS SHALL BE AS DESIGNATED IN THE BRACING PLANS (SEE S6-## SERIES OF DRAWINGS).

- I. ANCHORAGE OF SILL PLATES AT STUD WALLS SHALL BE AS FOLLOWS:
- EXTERIOR WALLS:**
- a) LOAD-BEARING, NON-SHEAR WALL
    - SIMPSON MASA @ 48" O.C. (MAXIMUM SPACING), OR
    - ½"Ø A307 THREADED ROD @ 48" O.C. (WITH CHUBBY SMACK CHAIR)
  - b) NON-LOAD-BEARING, NON-SHEAR WALL
    - SIMPSON MASA @ 48" O.C. (MAXIMUM SPACING), OR
    - ½"Ø A307 THREADED ROD @ 48" O.C. (WITH CHUBBY SMACK CHAIR)
  - c) SHEAR WALL
    - PER THE SILL PLATE ANCHORAGE TYPE SHOWN IN THE SHEAR WALL SCHEDULE IN DETAIL S/6-00A
- INTERIOR WALLS:**
- a) LOAD-BEARING, NON-SHEAR WALL
    - ½"Ø A307 THREADED ROD @ 48" O.C. (WITH CHUBBY SMACK CHAIR), OR
    - HILTI X-CP POWDER ACTUATED FASTENERS (P.A.F.) @ 18" O.C. AND TWO (2) ADDITIONAL FASTENERS @ 6" O.C. AT EACH END OF EACH WALL
  - b) NON-LOAD-BEARING, NON-SHEAR WALL
    - ½"Ø A307 THREADED ROD @ 48" O.C. (WITH CHUBBY SMACK CHAIR), OR
    - HILTI X-CP POWDER ACTUATED FASTENERS (P.A.F.) @ 18" O.C. AND TWO (2) ADDITIONAL FASTENERS AT EACH END OF EACH WALL
  - c) SHEAR WALL
    - PER THE SILL PLATE ANCHORAGE TYPE SHOWN IN THE SHEAR WALL SCHEDULE IN DETAIL S/6-00A

- J. WHERE DESIGN DELEGATED ITEMS SUCH AS HANDRAILS, GUARDRAILS AND STAIR STRUCTURAL ELEMENTS ARE ATTACHED TO THE WOOD-FRAMED SUPERSTRUCTURE, THE CONTRACTOR'S ENGINEER SHALL BE RESPONSIBLE FOR SUBMITTING SIGNED/SEALED CALCULATIONS FOR THE CONNECTIONS OF THE HANDRAIL(S), GUARDRAIL(S) AND/OR STAIR STRUCTURAL ELEMENT(S) TO THE BUILDING SUPERSTRUCTURE, IN ACCORDANCE WITH §15 (STAIRS, HANDRAILS AND GUARDRAILS) OF THESE GENERAL NOTES.

- K. INSTALL BEAMS WITH CROWN UP, TYP.

- L. THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2x MEMBER BEAMS SHALL MATCH THE NUMBER OF MEMBERS IN THE BEAM, UNLESS NOTED OTHERWISE. ALL ENGINEERED LUMBER BEAMS (LVL, LSL, PSL, GLULAM) SHALL HAVE A THREE (3) STUD MINIMUM BEARING, UNLESS NOTED OTHERWISE. THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS.

- M. ALL TIMBER FRAMING CONNECTIONS SHALL BE MADE WITH JOIST HANGERS, TIE DOWNS, FRAMING ANCHORS, POST CAPS, ETC., UNLESS NOTED OTHERWISE.

- N. DOUBLE PLY 2x MEMBERS AND DOUBLE PLY 2x WITH PLYWOOD FLITCH BEAMS SHALL BE SPIKED TOGETHER W/ 12d @ 12" TOP AND BOTTOM OF BEAM. DOUBLE PLY MICROLAM AND TRIPLE PLY-2x BEAMS AND WIDER SHALL BE BOLTED TOGETHER W/ ½" LAG BOLTS IN 5/16" PILOT HOLES OR ½" THROUGH BOLTS IN 9/16" HOLES. 2"-6" O.C. MAX @ TOP AND BOTTOM STAGGERED.

- O. WHEN NAILING IS NOT SHOWN IN PLANS, NAIL PER TABLE 2304.9.1 FROM THE 2015 INTERNATIONAL BUILDING CODE.

- P. UNLESS WOOD COLUMNS/STUD PACKS HAVE BEEN CALLED OUT AT HEADER BEAM ENDS IN THE FRAMING PLANS, ALL WALL OPENINGS SHALL BE FRAMED PER THE INFORMATION IN THE KING AND JAMB SCHEDULE (SEE DETAIL S/S-00B). ALL WOOD COLUMNS/STUD PACKS SHALL BE CONTINUOUS TO CONCRETE SLAB ON GRADE, CONCRETE PODIUM, OR OTHER IDENTIFIED TRANSFER CONDITION BELOW, TYP. ALL KING STUDS SHALL BE CONTINUOUS TO CONCRETE SLAB ON GRADE, CONCRETE PODIUM, OR OTHER IDENTIFIED TRANSFER CONDITION BELOW, TYP.

- Q. ALL FREE-STANDING WOOD COLUMNS AND/OR WOOD STUD PACKS SHALL HAVE A SIMPSON STRONG-TIE PB, PBS, CB, CBS OR LCB TYPE POST BASE. GENERAL CONTRACTOR TO COORDINATE TYPE AND SIZE WITH SCHEDULED WOOD COLUMN AND/OR STUD PACK. STANDOFF POST BASES SHALL BE FULLY GROUTED WITHIN THE BASE CAVITY, UNLESS NOTED OTHERWISE ON THESE DRAWINGS.

- R. IF WALL PANELS ARE USED, LET-IN WOOD OR METAL BRACES ARE NOT ALLOWED. ADJACENT EXTERIOR PANELS AND SHEAR WALL PANELS SHALL BE ATTACHED ALONG VERTICAL EDGES WITH 12d @ 8" O.C. MAX FROM ONE PANEL TO THE ADJACENT PANEL.

- S. DRAFT STOP AND/OR FIRE STOP AT UNIT SEPARATION WALLS, CHASE WALLS, ETC., IN FLOOR SPACE AS INDICATED ON ARCHITECTURAL DRAWINGS SHALL BE ATTACHED TO FRAMING USING THE NAILING INDICATED IN THE SHEAR WALL SCHEDULE FOR THE APPROPRIATE SHEATHING MATERIAL.

- T. WHERE SHEAR WALLS HAVE NOT BEEN SHOWN AT THE EXTERIOR OF BUILDING (SEE S6-## SHEETS FOR SHEAR WALL LOCATIONS), ALL EXTERIOR SHEATHING SHALL AS REQUIRED IN THE SPECIFIED ARCHITECTURAL WALL ASSEMBLIES SHOWN IN THE ARCHITECTURAL DRAWINGS. WHERE THE ARCHITECTURAL WALL ASSEMBLY SHOWS PLYWOOD OR OSB MATERIAL, IT SHALL BE ¾" THICK (MINIMUM) EXTERIOR C-C APA RATED SHEATHING (PLYWOOD OR OSB) IN MIN. 4x8 SHEETS, UNLESS NOTED OTHERWISE. ATTACH SHEATHING TO VERTICAL WALL STUDS WITH 8d @ 8" O.C. @ EDGES AND 8d @ 12" O.C. AT INTERMEDIATE FRAMING MEMBERS. SEE SHEET S6-00A FOR SHEAR WALL SHEATHING REQUIREMENTS.

- U. UNLESS NOTED OTHERWISE, ALL EXTERIOR SHEATHING SHALL BE MADE CONTINUOUS FROM FOUNDATION TO ROOF AND HORIZONTALLY CONTINUOUS FROM BUILDING CORNER TO BUILDING CORNER AND AROUND OPENINGS BY NAILING TO STUDS, BLOCKING OR DEADWOOD IN WALLS. DEADWOOD TO BE FACE NAILED W/ 16d NAILS @ 8" O.C.

- V. UNLESS NOTED OTHERWISE, ALL EXTERIOR BREEZEWAY WALLS SHALL BE SHEATHED WITH 5/8" GYPSUM SHEATHING BOARD (RATED FOR EXTERIOR EXPOSURE). SHEATHING AT ALL EXTERIOR BREEZEWAY WALLS, INCLUDING BREEZEWAY SHEAR WALLS SHALL BE CONTINUOUS FOR THE FULL HEIGHT OF THE BREEZEWAY WALL. ALL BREEZEWAY WALLS, NOT INCLUDING BREEZEWAY SHEAR WALLS, SHALL BE ATTACHED (AT A MINIMUM) W/ 11 GA. GALVANIZED STEEL W/ 1-3/4" LEGS @ 7" O.C. TO EDGES AND TO INTERMEDIATE FRAMING. SEE SHEET S6-00A FOR SHEAR WALL SHEATHING REQUIREMENTS.

- W. ATTACH CHIMNEY TO ROOF BY EXTENDING ADDITIONAL 2-2x4 STUDS CONTINUOUS FROM CHIMNEY MID-HEIGHT (+/-) AT FOUR (4) CORNERS OF CHIMNEY DOWN TO ROOF TRUSS BOTTOM CHORD. ATTACH STUDS TO TOP AND BOTTOM TRUSS CHORDS WITH (3)-16d AND ONE (1) 1½"-16 GAGE GALVANIZED METAL STRAP. ADD 2-2x6 BLOCKING BETWEEN CHORDS AS REQUIRED FOR ATTACHMENT.

- X. WHERE LISTED WALL ASSEMBLIES OR LISTED CEILING ASSEMBLIES, SPECIFIED BY THE ARCHITECT, REQUIRE SPECIFIC FASTENER TYPES AND/OR FASTENING SPACINGS(S), THE GENERAL CONTRACTOR SHALL ENSURE THAT CONSTRUCTED ASSEMBLIES MEET BOTH THE REQUIREMENTS OF THE LISTED ASSEMBLY TO ENSURE THE LISTED ASSEMBLY HAS THE STATED FIRE RATING) AND THE STRUCTURAL REQUIREMENTS STATED IN THIS SET OF DRAWINGS.

- Y. WHERE COMPOSITE DECK BOARDS ARE UTILIZED AT BALCONIES, MAXIMUM JOIST SPACING SHALL BE 16" O.C. FOR DECK BOARDS RUNNING PERPENDICULAR TO JOISTS, AND 12" O.C. FOR DECK BOARDS RUNNING AT A 45° ANGLE TO JOISTS. GENERAL CONTRACTOR TO ALSO REFER TO MANUFACTURER REQUIREMENTS.

**9. BUILDING COMPRESSION MANAGEMENT**

- A. IT IS RECOMMENDED THAT THE CONTRACTOR INSTITUTE A SHRINKAGE MANAGEMENT AND MONITORING PROGRAM TO MANAGE BUILDING MOVEMENT AND MINIMIZE POTENTIAL DAMAGE TO BRITTLE FINISHES. REGULARLY SCHEDULED MOISTURE MONITORING SHOULD BE INSTITUTED TO TRACK THE MOISTURE CONTENT IN THE WOOD STRUCTURE, SPECIFICALLY IN THE WALL PLATES IN THE EXTERIOR WALLS. INTERIOR GYPSUM PANELS AND EXTERIOR MASONRY FINISHES SHOULD BE INSTALLED WHEN THE MOISTURE CONTENT OF THE WOOD HAS DROPPED BELOW 12% IF POSSIBLE.
- B. DIFFERENTIAL MOVEMENT WILL OCCUR BETWEEN THE PRIMARY WOOD STRUCTURE AND THE EXTERIOR MASONRY FACADE. SUFFICIENT GAPPING SHOULD BE PROVIDED AT EXTERIOR FENESTRATIONS TO ALLOW SUFFICIENT SPACE FOR MOVEMENT.
- C. BRICK TIES SHALL BE AS INDICATED IN SECTION 7 OF THE GENERAL NOTES. SLOTTED VENEER ANCHORS SHALL ACCOMMODATE A MINIMUM OF 4" OF VERTICAL ADJUSTABILITY. BRICK TIES SHALL BE INSTALLED NO CLOSER TO 1½" FROM THE TOP OF THE ANCHOR SLOT. ACCEPTABLE PRODUCTS ARE "DW-110" BY HOHMANN & BARNARD, INC. OR EQUIVALENT.

- D. IT IS RECOMMENDED THAT THE CONTRACTOR "PRE-LOAD" THE EXTERIOR NON-LOAD BEARING WALLS BY PLACING STACKS OF GYPSUM BOARDS, LAID FLAT, WITHIN THE UNITS, LOCATED NEAR THE EXTERIOR WALLS, PRIOR TO INSTALLING THE EXTERIOR MASONRY FACADE. GYPSUM SHALL NOT BE STACKED WITH GREATER THAN (12)¾" THICK PANELS AT EACH LOCATION.
- E. WHERE SHEAR WALL CHORD STRAPPING IS REQUIRED PER S/6-00A, IT IS RECOMMENDED THAT THE STRAPS BE INSTALLED AFTER THE WOOD SUPERSTRUCTURE HAS BEEN FULLY ERECTED INCLUDING THE ROOF STRUCTURE, TO ACCOMMODATE BUILDING MOVEMENT. AN ACCEPTABLE ALTERNATIVE WOULD BE TO INSTALL THE STRAP TO THE UPPER WALL CHORD POST DURING INITIAL FRAMING AND DELAY FASTENING TO THE LOWER WALL CHORD UNTIL THE WOOD SUPERSTRUCTURE HAS BEEN FULLY ERECTED.
- F. WHERE A CONTINUOUS LOAD PATH (CLP), ANCHOR TIEDOWN SYSTEM (ATS), OR SIMILAR UPLIFT RESTRAINT SYSTEM IS IMPLEMENTED ON THE PROJECT, TAKE-UP DEVICES SHALL BE INCORPORATED TO ACCOMMODATE ESTIMATED BUILDING MOVEMENT AS INDICATED IN THE TABLE BELOW.

ESTIMATED BUILDING MOVEMENT		
ROOF		0'-0 3/4"
3RD FLOOR		0'-0 1/2"
2ND FLOOR		0'-0 1/4"

**10. WOOD TRUSSES**

- A. ALL TRUSS CONNECTOR PLATES SHALL BE MANUFACTURED FROM ASTM A446-72 GRADE A GALVANIZED STEEL OF NO LESS THAN 20 GAGE THICKNESS WITH A MINIMUM YIELD OF 33,000 PSI AND AN ULTIMATE TENSILE STRENGTH OF 45,000 PSI. CONNECTOR PLATE GAUGES SHALL BE AS REQUIRED BY MANUFACTURERS DESIGN CALCULATIONS.

- B. TRUSS SHOP DRAWINGS SHALL BE SUBMITTED FOR THE ARCHITECTS REVIEW PRIOR TO FABRICATION AND SHALL INCLUDE THE FOLLOWING, PER THE REQUIREMENTS OF THE 2015 INTERNATIONAL BUILDING CODE, §2303.4.1.1:
1. SLOPE OR DEPTH, SPAN AND SPACING;
  2. LOCATION OF ALL JOINTS, LOCATIONS OF ALL SUPPORT POINTS;
  3. NUMBER OF PILES REQUIRED, IF MORE THAN ONE (1);
  4. REQUIRED BEARING WIDTH;
  5. DESIGN LOADS, AS APPLICABLE, TO INCLUDE THE FOLLOWING AT A MINIMUM:
    - a. TOP CHORD LIVE LOAD;
    - b. TOP CHORD DEAD LOAD;
    - c. BOTTOM CHORD LIVE LOAD;
    - d. BOTTOM CHORD DEAD LOAD;
    - e. ADDITIONAL LOADS AND LOADINGS;
    - f. ENVIRONMENTAL DESIGN CRITERIA AND LOADS (WIND, RAIN, SNOW, SEISMIC, ETC.);
  6. OTHER LATERAL LOADS, INCLUDING DRAG STRUT LOADS;
  7. ADJUSTMENTS TO WOOD MEMBER AND METAL CONNECTOR PLATE DESIGN VALUES FOR CONDITIONS OF USE;
  8. MAXIMUM REACTION FORCE AND DIRECTION, INCLUDING MAXIMUM UPLIFT REACTION FORCES (WHERE APPLICABLE);
  9. METAL-CONNECTOR-PLATE TYPE, SIZE, THICKNESS/GAGE, AND DIMENSIONED LOCATION OF EACH METAL CONNECTOR PLATE;
    - a. WHERE CONNECTOR PLATES ARE SYMMETRIC RELATIVE TO A JOINT INTERFACE, DIMENSIONS NEED ONLY BE SUPPLIED TO PLATES ON ONE SIDE OF THE LINE OF SYMMETRY;
    10. SIZE, SPECIES AND GRADE FOR EACH WOOD MEMBER;
    11. TRUSS-TO-TRUSS CONNECTIONS, TRUSS-TO-GIRDER-TRUSS CONNECTIONS, TRUSS-TO-FLUSH BEAM CONNECTIONS, AND TRUSS FIELD ASSEMBLY REQUIREMENTS;
    12. CALCULATED SPAN-TO-DEFLECTION RATIO AND MAXIMUM VERTICAL AND HORIZONTAL DEFLECTION FOR LIVE LOAD AND TOTAL LOAD CONDITIONS;
    13. MAXIMUM AXIAL TENSION/COMPRESSION FORCES IN EACH TRUSS MEMBER;
    14. REQUIRED PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT LOCATION AND THE METHOD AND DETAILS OF RESTRAINT/BRACING TO BE USED IN ACCORDANCE WITH 2015 INTERNATIONAL BUILDING CODE, §2303.4.1.2.

- C. THE FOLLOWING ITEMS WILL ALSO BE INCLUDED AS PART OF THE TRUSS SHOP DRAWINGS, AS REQUESTED BY THE ENGINEER OF RECORD:
1. SEAL AND SIGNATURE OF TRUSS COMPANY ENGINEER OF RESPONSIBLE CHARGE ON ALL TRUSS ENGINEERED SHEETS OR DRAWINGS
  2. NAME AND TRADEMARK OF PLATE MANUFACTURER AND TRUSS FABRICATOR AS WELL AS PROJECT NAME AND LOCATION
  3. ALLOWABLE LOADS FOR STRESS GRADE LUMBER AND PLATES AS ALLOWED BY I.C.B.O. AND INCLUDING I.C.B.O. REPORT NUMBER

- D. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR OTHERWISE ALTERED IN ANY WAY WITHOUT WRITTEN APPROVAL FROM THE TRUSS ENGINEER. WHERE FIELD REPAIRS OF DAMAGED TRUSSES ARE REQUIRED, THE TRUSS ENGINEER SHALL SUBMIT A SIGNED/SEALED PROFILE TO THE GENERAL CONTRACTOR PRIOR TO REPAIR INSTALLATION. THE GENERAL CONTRACTOR SHALL SUBMIT ALL SIGNED/SEALED TRUSS REPAIRS TO THE STRUCTURAL ENGINEER OF RECORD ON A MONTHLY BASIS FOR PROJECT RECORD (INFORMATIONAL PURPOSES ONLY).

- E. THE DESIGN, MANUFACTURE AND QUALITY ASSURANCE OF METAL-PLATE-CONNECTED WOOD TRUSSES SHALL BE IN ACCORDANCE WITH TPI 1.

- F. ALL ROOF TRUSSES SHALL BE MECHANICALLY FASTENED AT EACH BEARING POINT WITH UPLIFT CONNECTOR TO RESIST NET UPLIFT FORCE RESULTING FROM WIND.
1. REFERENCE SHEET S4-90 FOR INFORMATION RELATED TO LOAD PATH FOR WIND UPLIFT.
  2. REFERENCE COMPONENT AND CLADDING WIND FORCE INFORMATION ON SHEET S0-20.

- G. TRUSS SUPPLIER IS TO PROVIDE PLAN AND PROCEDURES FOR INSTALLING, SECURING AND BRACING OF ALL TRUSSES. THE GENERAL CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR THE TRUSSES DURING ERECTION. ALL FLOOR AND ROOF TRUSS CONSTRUCTION PRACTICES SHALL COMPLY WITH THE BCSI DOCUMENT, "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL-PLATE-CONNECTED WOOD TRUSSES."

- H. TRUSS SUPPLIER SHALL PROVIDE SHEAR PANELS CAPABLE OF TRANSFERRING LATERAL LOADS AS NOTED ON THE STRUCTURAL PLANS AND/OR IN THE STRUCTURAL DETAILS.

- I. APPROVED TRUSS PLANS SHALL BE AVAILABLE ON JOB SITE DURING TIMES OF INSPECTION.

- J. WHERE ROOF TRUSSES RUN PARALLEL TO SHEAR WALLS, THE TRUSS MANUFACTURER SHALL PROVIDE A ROOF TRUSS ABOVE ALL SHEAR WALLS, AS DETAILED IN THIS SET OF STRUCTURAL DRAWINGS. WHERE A ROOF TRUSS CANNOT BE PROVIDED DIRECTLY ABOVE THE SHEAR WALL, FIELD CONNECTION OF THE TRUSS TO THE SHEAR WALL WILL BE REQUIRED. REFERENCE THE S6-## SERIES OF DRAWINGS FOR SHEAR WALL LOCATIONS, EXTENTS, ORIENTATION, ETC.

- K. TRUSS MANUFACTURER SHALL COORDINATE TRUSS LAYOUT WITH MECH/PLUMBING DRAWINGS TO ALLOW ALL PIPES AND DUCTS ADEQUATE SPACE FOR PROPER INSTALLATION. THE MANUFACTURER SHALL COORDINATE THE WEB MEMBER CONFIGURATION WITH THE MECHANICAL DRAWINGS AND ARCHITECTURAL DRAWINGS SUCH THAT ADEQUATE OPENING IS PROVIDED FOR ANY MECHANICAL UNITS AND DUCTS AND ACCESS CATWALKS. HARD DUCT LOCATIONS, AS INDICATED ON THE MECHANICAL DRAWINGS, SHALL BE SHOWN ON THE SUBMITTED TRUSS LAYOUT SHOP DRAWINGS FOR DESIGN TEAM REVIEW.

- L. TRUSSES SHALL BE SPACED AS SHOWN ON THE PLANS.

1. LESSER SPACING MAY BE USED IF REQUIRED BY THE TRUSS DESIGNER. HOWEVER, THE USE OF LESS TRUSS SPACINGS THAN INDICATED IN THE DESIGN DOCUMENTS SHALL NOT BE A JUSTIFICATION FOR AN OWNER CHANGE ORDER. IT IS THE RESPONSIBILITY OF THE FRAMER AND THE TRUSS DESIGNER TO REVIEW PROJECT LOADING, REQUIRED SPANS AND TRUSS CONFIGURATIONS DURING THE BIDDING PROCESS.
2. ACTUAL TRUSS SPACING SHALL BE USED TO DETERMINE UNIFORM LOADS PER FOOT IN THE TRUSS PROFILE DESIGN.
3. ROOF TRUSSES SHALL BE DESIGNED FOR APPLICABLE WIND LOADS AT THE PROJECT SITE IN COMBINATION WITH THE DEAD LOADS SHOWN IN THE TABLE BELOW, IN ACCORDANCE WITH THE LOAD COMBINATIONS AND PRESSURE/SUCTION FACTORS INDICATED IN THE 2015 INTERNATIONAL BUILDING CODE. UPLIFT INDICATED IS ONLY A MINIMUM.

- M. SEE PLANS FOR TRUSS LOCATIONS AND SPANS.

- N. TRUSS DESIGN LOADS SHALL BE AS FOLLOWS:

LOAD TYPE	TRUSS TYPE (ALL LOADS ARE POUNDS PER SQUARE FOOT, PSF)				
	ROOF	UNIT FLOOR	PUBLIC ASSEMBLY, MECH, ELEC, TELECOM	STORAGE	BALCONY
TOP CHORD LIVE LOAD	20	40	100	125	60
TOP CHORD DEAD LOAD	15	25	25	25	30
TOP CHORD UPLIFT LOAD (*)	10	0	0	0	0
BOTTOM CHORD LIVE LOAD	0	0	0	0	0
BOTTOM CHORD DEAD LOAD	5	5	5	5	5

- THE ABOVE NOTED VALUES ARE MINIMUM REQUIREMENTS FOR DEAD AND LIVE LOADS. SEE SHEET S0-20 FOR COMPONENT AND CLADDING WIND FORCE INFORMATION.
- TOP CHORD DEAD LOAD SHOWN ABOVE INCLUDES 3 PSF FOR TRUSS SELF-WEIGHT. TRUSS DESIGNER SHALL INCREASE DEAD LOAD, AS REQUIRED.

- FOR ROOF TRUSS TOP CHORD LIVE LOAD, THE LOADS SHOWN (2014) HAVE BEEN MADE WITH THE ASSUMPTION THAT THERE IS A 0'-3" MAXIMUM BETWEEN PRIMARY ROOF DRAINS AND THE SECONDARY DRAINAGE SYSTEM ENGAGED IN THE EVENT THAT THE PRIMARY DRAINS BECOME CLOGGED.

- O. TRUSS MANUFACTURER TO COORDINATE WITH MECHANICAL DRAWINGS AND PLUMBING DRAWINGS FOR ADDITIONAL CONCENTRATED LOADS DUE TO DOMESTIC WATER AND SPRINKLER PIPE SUPPORTS.
1. IN ADDITION TO THE DEAD LOADS PRESCRIBED IN THE TABLE ABOVE (WHICH INCLUDE WATER-FILLED SPRINKLER PIPING), THE TRUSS DESIGNER SHALL APPLY A MINIMUM 250 POUND CONCENTRATED LIVE LOAD TO ANY SINGLE FIRE SPRINKLER SUPPORT POINT TO ACCOMMODATE INSTALLATION PERSONNEL PER NFPA 13. THE 250 POUND POINT LOAD IS NOT REQUIRED TO BE ANALYZED CONCURRENT WITH OTHER LIVE LOADS. IF MULTIPLE SPRINKLER LINES ARE ATTACHED TO THE SAME TRUSS, THE 250 POUND POINT LOAD SHOULD BE APPLIED AT ONE (1) LOCATION AT A TIME.
  2. SPRINKLER PIPE SUPPORTS SHALL BE SPACED OR DISTRIBUTED SO THAT THE MAXIMUM POINT LOAD (CONSIDERING A FULLY FILLED PIPE) APPLIED AT ANY SINGLE SPRINKLER SUPPORT DOES NOT EXCEED 50 POUNDS.

- P. MEMBERS OF THE GABLE END WALL TRUSSES SHALL BE DESIGNED FOR COMPONENT WIND FORCES AGAINST THE EXPOSED FACE OF THE TRUSS.

- Q. MAXIMUM DEFLECTIONS FOR FLOOR TRUSSES SHALL BE AS FOLLOWS:
1. FOR LIVE LOADS = L/480
  2. FOR TOTAL LOAD = L/360, BUT NOT TO EXCEED 1.00"

- R. MAXIMUM DEFLECTIONS FOR ROOF TRUSSES SHALL BE AS FOLLOWS:
1. FOR LIVE LOADS = L/240
  2. FOR TOTAL LOAD = L/180, BUT NOT TO EXCEED 1.00"

- S. MAXIMUM LIVE LOAD DEFLECTION FOR ALL CANTILEVERED TRUSSES = CANTILEVER LENGTH/180.

- T. ALL TRUSS FRAMING GRAVITY LOAD CONNECTIONS TO WALLS, GIRDER TRUSSES, LEDGERS, BEAMS OR OTHER SUPPORTS SHALL BE MADE WITH JOIST HANGERS, NAILING, FRAMING ANCHORS, POST CAPS, ETC., AND SHALL BE DESIGNED BY AND SPECIFIED BY THE TRUSS DESIGNER/MANUFACTURER AND SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW TO THE ARCHITECT AND STRUCTURAL ENGINEER. STRUCTURAL ENGINEER WILL SPECIFY ALL UPLIFT AND DRAG LOAD CONNECTIONS.

- U. ALL HEADERS, BEARING WALLS AND POSTS SHOWN ON THE PLANS SHALL BE USED FOR BEARING IN DESIGN OF THE FLOOR AND ROOF TRUSSES. MODIFICATION BY THE TRUSS DESIGNER/MANUFACTURER OF TRUSS FRAMING DIRECTION OR BEARING ELEMENT LOCATIONS SHOWN ON THE STRUCTURAL DRAWINGS SHALL NOT BE PERMITTED UNLESS PREVIOUSLY APPROVED IN WRITING BY THE ENGINEER OF RECORD. SUCH CHANGES MAY RESULT IN ADDITIONAL MODIFICATIONS TO THE STRUCTURAL DRAWINGS THAT MAY HAVE A SCHEDULE OR COST IMPACT ON THE PROJECT.

- V. WHERE DRAFT STOPS ARE REQUIRED BY THE ARCHITECTURAL DRAWING, PROVIDE A MINIMUM OF 2x4 VERTICALS AT 24" ON CENTER BETWEEN TRUSS TOP AND BOTTOM CHORDS AND WEB MEMBERS, AS REQUIRED, FOR DRAFT STOP NAILING.

- W. ALL TRUSS-TO-TRUSS, TRUSS-TO-GIRDER-TRUSS, TRUSS-TO-BEAM AND TRUSS-TO-LEDGER CONNECTIONS SHALL BE DESIGNED BY TRUSS MANUFACTURER.

- X. LOADS INDICATED HEREIN SHALL BE UTILIZED IN THE DESIGN OF ROOF GIRDER, HIP AND JACK TRUSSES, AS REQUIRED.

- Y. STRONGBACKS SHALL BE INSTALLED AT A MAXIMUM SPACING OF 10'-0" ON CENTER. STRONGBACKS SHALL BE INSTALLED IN ACCORDANCE WITH THE TYPICAL STRONGBACK INSTALLATION DETAIL PROVIDED BY THE TRUSS MANUFACTURER. SUCCESSIVE STRONGBACK MEMBERS SHALL BE LAPPED A MINIMUM OF 4'-0" SUCH THAT THE STRONGBACKS ARE ATTACHED TO A MINIMUM OF THREE (3) COMMON TRUSS MEMBERS. WHERE STRONGBACKS ARE INSTALLED IN WRITING BY THE ARCHITECT (MECHANICAL, ELECTRICAL, ETC.), IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE ADDITIONAL STRONGBACKS ARE INSTALLED TO MEET THE SPLICE REQUIREMENTS OUTLINED HERE.

- Z. IT IS THE RESPONSIBILITY OF THE TRUSS DESIGNER AND TRUSS MANUFACTURER TO PROVIDE PROPER CONSIDERATION TO THE POSSIBLE EFFECT OF CROSS-GRAIN DIMENSIONAL CHANGES WHICH MAY OCCUR AS A RESULT OF SHRINKAGE EFFECTS DUE TO THE VARIABLE MOISTURE CONTENT OF THE LUMBER USED DURING TRUSS FABRICATION, STORAGE AND CONDITIONS DURING CONSTRUCTION.

**11. LIGHT-FRAMED LATERAL LOAD RESISTING SYSTEM**

- A. ALL ROOF DECK AND SUBFLOORS HAVE BEEN DESIGNED IN ACCORDANCE WITH SECTIONS 2305 AND 2306 OF THE 2015 INTERNATIONAL BUILDING CODE AND UNLESS NOTED OTHERWISE ON OTHER WOOD ASSEMBLIES, SHALL BE CONSIDERED UNBLOCKED DIAPHRAGMS. SHEATHING PROVIDED FOR ROOF AND SUBFLOORS SHALL MEET THE FOLLOWING CRITERIA:
1. ROOF SHEATHING SHALL BE 23/32" THICK EXPOSURE 1 RATED ORIENTED STRAND BOARD (O.S.B.) OR PLYWOOD WITH A 48/24 PANEL SPAN INDEX (U.S.) BEARING THE TRADEMARK STAMP OF THE AMERICAN PLYWOOD ASSOCIATION (APA), UNLESS NOTED OTHERWISE ON PLAN. PANELS SHALL BE NAILED WITH 8d NAILS @ 8" O.C. AT ALL PANEL EDGES AND 12" O.C. AT ALL INTERIOR SUPPORTS AND INSTALLED W/ GALVANIZED STEEL PANEL EDGE CLIPS PER MANUFACTURER RECOMMENDATIONS AND CODE REQUIREMENTS. ROOF SHEATHING SHALL BE APPLIED WITH STRENGTH AXIS PERPENDICULAR TO SUPPORT OVER A MINIMUM OF THREE (3) SUPPORTS.
  2. FLOOR SHEATHING SHALL BE GRADE 23/32" THICK (TONGUE AND GROOVE) EXPOSURE 1 RATED ORIENTED STRAND BOARD (O.S.B.) OR PLYWOOD WITH A 48/24 PANEL SPAN INDEX (U.S.) BEARING THE TRADEMARK STAMP OF THE AMERICAN PLYWOOD ASSOCIATION (APA), UNLESS NOTED OTHERWISE ON PLAN. PANELS SHALL BE NAILED WITH 10d NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. AT ALL INTERIOR SUPPORTS.

- B. ALL SHEAR WALLS HAVE BEEN DESIGNED IN ACCORDANCE WITH SECTIONS 2305 AND 2306 OF THE 2015 INTERNATIONAL BUILDING CODE. ALL SHEAR WALL SHEATHING SHALL BE APPLIED, AND FASTENED, DIRECTLY TO THE STUDS, WHICH SHALL NOT BE SPACED GREATER THAN 16" O.C. ALL SHEAR WALLS SHALL BE SHEATHED WITH THE FOLLOWING MATERIALS ONLY:
1. STRUCTURAL PANEL SHEAR WALLS SHALL BE SHEATHED WITH 15/32" THICK EXPOSURE 1 RATED ORIENTED STRAND BOARD (O.S.B.) OR PLYWOOD WITH A 32/16 PANEL SPAN INDEX (U.S.) BEARING THE TRADEMARK STAMP OF THE AMERICAN PLYWOOD ASSOCIATION (APA). PANELS SHALL BE NAILED IN ACCORDANCE WITH THE SHEAR WALL SCHEDULE ON SHEET S6-00A.

- C. REFER TO BRACING PLANS (S6-##) FOR TYPE AND LOCATION OF ALL SHEAR WALLS AND HOLD DOWN ANCHORS. REFER TO SHEET S6-00A FOR EXPLANATION OF, AND MINIMUM FASTENER REQUIREMENTS FOR, ALL SHEAR WALL TYPES AND HOLD DOWN ANCHORS.

## 12. NAILING

A. ALL NAILS LISTED BELOW SHALL COMPLY WITH THE FOLLOWING DIMENSIONAL REQUIREMENTS:

NAIL DIMENSIONS, INCHES						
NAIL TYPE	FEATURE	PENNY WEIGHT				
		6d	8d	10d	12d	16d
COMMON	LENGTH	2	2½	3	3¼	3½
	DIAMETER	0.113	0.131	0.148	0.148	0.162
	HEAD	0.226	0.281	0.312	0.312	0.344
BOX	LENGTH	2	2½	3	3¼	3½
	DIAMETER	0.099	0.113	0.128	0.128	0.135
	HEAD	0.266	0.297	0.312	0.312	0.344
SINKER	LENGTH	1½	2¼	2½	3¼	3½
	DIAMETER	0.092	0.113	0.120	0.135	0.148
	HEAD	0.234	0.266	0.281	0.312	0.341

### B. FASTENING SCHEDULE

CONNECTION:	FASTENER	NUMBER OR SPACING
BAND JOIST TO SILL OR TOP PLATE, TOE NAIL	8d COMMON	6" O.C.
JOIST TO BAND JOIST, FACE NAIL	16d COMMON	3
JOIST TO SILL OR GIRDER, TOE NAIL	8d COMMON	3
BRIDGING TO JOIST, TOE NAIL EACH END	8d COMMON	2
LEDBER STRIP	16d COMMON	3 AT EACH JOIST
1X6 SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	8d COMMON	2
OVER 1X6 SUBFLOOR TO EACH JOIST, FACE NAIL	8d COMMON	3
2 INCH SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	16d COMMON	2
SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d COMMON	16" O.C.
TOP OR SOLE PLATE TO STUD, END NAILED	16d COMMON	2
STUD TO SOLE PLATE, TOE NAIL	8d COMMON	4
DOUBLED STUDS, FACE NAIL	16d COMMON	24" O.C.
DOUBLED TOP PLATES, FACE NAIL	16d COMMON	16" O.C.
TOP PLATES, LAP AND INTERSECTIONS FACE NAIL	2-16d OR 3-10d COMMON	EACH EDGE
CONTINUOUS HEADER, TWO PIECES	16d COMMON	4
CEILING JOISTS TO PLATE, TOE NAIL	8d COMMON	3- 16d OR 4- 10d COMMON
CONTINUOUS HEADER TO STUD, TOE NAIL	8d COMMON	3- 16d OR 4- 10d COMMON
CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	-	3- 16d OR 4- 10d COMMON
CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	-	4- 10d COMMON
RAFTER TO PLATE, TOE NAIL	8d COMMON	3
1 INCH BRACE TO EACH STUD AND PLATE, FACE NAIL	8d COMMON	2
1X8 SHEATHING OR LESS TO EA. BEARING, FACE NAIL	8d COMMON	3
OVER 1X8 SHEATHING TO EACH BEARING, FACE NAIL	8d COMMON	3
BUILT-UP CORNER STUDS	16d COMMON	24" O.C.
BUILT-UP GIRDERS AND BEAMS, OF THREE MEMBERS	20d COMMON	32" O.C. AT TOP AND BOTTOM AND STAGGERED 2 ENDS AND AT EACH SPICE.
2 INCH PLANKS	16d COMMON	2 EACH BEARING
STUDS TO SOLE PLATE, END NAIL	16d COMMON	2 EACH END

WOOD STRUCTURAL PANEL SUBFLOORING	FASTENER	NUMBER OR SPACING
15/32", 1/2", 7/16"	6d COMMON ANNULAR OR SPIRAL THREAD	6" O.C. EDGES AND 12" O.C. INTERMEDIATE
19/32" - 3/4"	8d COMMON OR 6d ANNULAR OR SPIRAL THREAD	6" O.C. EDGES AND 12" O.C. INTERMEDIATE
1", 1 1/8"	10d COMMON OR 8d ANNULAR OR SPIRAL THREAD	6" O.C. EDGES AND 8" O.C. INTERMEDIATE
15/32", 1/2"	16 GA GALV WIRE STAPLES, 3/8" MIN. CROWN	4" O.C. EDGES AND 7" O.C. INTERMEDIATE
19/32", 5/8"	1 5/8" LENGTH EDGES 4" O.C. INTERMEDIATE	2 1/2" O.C.

### WOOD STRUCTURAL PANEL ROOF AND WALL SHEATHING AND PARTICLE BOARDWALL SHEATHING<sup>11</sup>

1/2" OR LESS	6d COMMON	6" O.C. EDGES AND 12" O.C. INTERMEDIATE
19/32" OR GREATER	8d COMMON	6" O.C. EDGES AND 12" O.C. INTERMEDIATE
5/16" - 1/2"	16 GA GALV WIRE STAPLES, 3/8" MIN. CROWN LENGTH OF 1" PLUS WOOD STRUCTURAL PANEL OR PARTICLE BOARD THICKNESS	4" O.C. EDGES AND 8" O.C. INTERMEDIATE
19/32" - 3/4"	16 GA GALV WIRE STAPLES, 3/8" MIN. CROWN LENGTH OF 1" PLUS WOOD STRUCTURAL PANEL OR PARTICLE BOARD THICKNESS	2" O.C. EDGES AND 5" O.C. INTERMEDIATE

### FIBERBOARD SHEATHING

1/2" REGULAR	6d COMMON NAIL OR 11 GA. GALV. ROOFING NAIL	6" O.C. AT EDGES AND 12" O.C. AT OTHER BEARINGS
1/2" STRUCTURAL	8d COMMON NAIL OR 11 GA. GALV. ROOFING NAIL	3" O.C. AT EDGES AND 8" O.C. AT OTHER BEARINGS
23/32" STRUCTURAL	8d COMMON NAIL OR 11 GA. GALV. ROOFING NAIL	3" O.C. AT EDGES AND 8" O.C. AT OTHER BEARINGS

GYPSUM SHEATHING	FASTENER	NUMBER OR SPACING
1/2" GYPSUM SHEATHING	11 GA 1 1/2" GALV. 7/16" HEAD	4" O.C. AT EDGES AND 7" O.C. AT OTHER BEARINGS
5/8" GYPSUM SHEATHING	11 GA 1 3/4" GALV. 7/16" HEAD	4" O.C. AT EDGES AND 7" O.C. AT OTHER BEARINGS
GYPSUM WALLBOARD	1 3/8" DRYWALL NAIL 1 1/2" DRYWALL NAIL	7" O.C. ON CEILINGS 7" O.C. ON WALLS 7" O.C. ON CEILINGS 7" O.C. WALLS

PARTICLE BOARD SIDING	FASTENER	NUMBER OR SPACING
5/16" - 1/2" <sup>3</sup>	6d 4	
5/8" <sup>4</sup>	8d 4	
6 3/4" <sup>5</sup>	8d 4	

- NOTES:
- FIBERBOARD SHEATHING MAY BE STAPLED USING 16 GA GALVANIZED STAPLES 1 1/8" LONG FOR 1/2" SHEATHING AND 1 1/2" LONG FOR 25/32" SHEATHING. STAPLES TO HAVE MINIMUM CROWN OF 7/16" AND SPACED 3" O.C. AT EDGES AND 6" O.C. AT OTHER BEARINGS.
  - DRYWALL NAILS SHALL CONFORM TO ASTM C514.
  - SIDING APPLIED TO 5/8" NET WOOD SHEATHING, 15/32" STRUCTURAL PANEL OR 1/2" PARTICLE BOARD SHEATHING.
  - CORROSION RESISTANT NAILS SHALL BE USED AND SPACED 6" O.C. AT EDGE AND 8" O.C. AT INTERMEDIATE SUPPORTS. NAILS SHALL HAVE A MINIMUM EDGE DISTANCE OF 3/8".
  - SIDING APPLIED TO STUDS SPACED 16" O.C. MAXIMUM.
  - SIDING APPLIED DIRECTLY TO STUDS SPACED 24" O.C. MAXIMUM.
  - USE ANNULAR OR SPIRAL THREAD NAILS FOR COMBINATION SUBFLOOR/UNDERLAYMENT.
  - FOR 1-INCH WOOD STRUCTURAL PANELS, 12" O.C. INTERMEDIATE NAILING SHALL BE PERMITTED.
  - ALL FASTENING REQUIREMENTS SHALL COMPLY WITH THE SCHEDULE ABOVE OR PER U.L. DESIGNS AS SPECIFIED BY THE ARCHITECT, WHICHEVER IS STRICTER.
  - WHERE NAIL SCHEDULE ABOVE CONFLICTS WITH OTHER NOTES AND/OR SCHEDULES WITHIN CONSTRUCTION DOCUMENTS, THE MORE STRICT NAIL SIZE AND SPACING SHALL CONTROL.
  - AT SHEAR WALLS, NAIL SIZE AND SPACING PRESCRIBED IN SHEAR WALL SCHEDULE ON SHEET S6-00A SHALL CONTROL.

## 13. STRUCTURAL STEEL

### A. MATERIAL

- ALL HOT ROLLED STEEL PLATES, SHAPES, SHEET PILING, AND BARS SHALL BE NEW STEEL CONFORMING TO ASTM SPECIFICATION A6-98A.
- CLEARLY MARK THE GRADE OF STEEL ON EACH PIECE, WITH A DISTINGUISHING MARK VISIBLE FROM FLOOR SURFACES, FOR THE PURPOSE OF FIELD INSPECTION OF PROPER GRADE OF STEEL. UNLESS NOTED OTHERWISE ON THE DRAWINGS, STRUCTURAL STEEL SHALL BE AS FOLLOWS:
  - ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. (ASTM A572-50 IS ACCEPTABLE AS A SUBSTITUTE FOR A992).
  - ALL CHANNEL SHAPES SHALL CONFORM TO ASTM A36.
  - ALL ROLLED ANGLE SHAPES SHALL CONFORM TO ASTM A36.
  - ALL PIPE SHAPES SHALL CONFORM TO ASTM A53 (TYPES E OR S), GRADE B OR ASTM A501.
  - ALL RECTANGULAR HSS SHALL CONFORM TO ASTM A500, GRADE B (FY=46 KSI).
  - ALL ROUND HSS SHALL CONFORM TO ASTM A500, GRADE B (FY=42 KSI).
  - ALL BASE PLATES SHALL CONFORM TO ASTM A572 AS FOLLOWS:
    - ALL CONNECTION MATERIAL, EXCEPT AS NOTED OTHERWISE HEREIN OR ON THE DRAWINGS, INCLUDING BEARING PLATES, GUSSET PLATES, STIFFENER PLATES, FILLER PLATES, ANGLES, ETC. SHALL CONFORM TO ASTM A36 UNLESS A HIGHER GRADE OF STEEL IS REQUIRED BY STRENGTH AND PROVIDED THE RESULTING SIZE ARE COMPATIBLE WITH THE CONNECTED MEMBERS.
    - ANY OTHER STEEL NOT INDICATED OTHERWISE SHALL CONFORM TO ASTM A992 OR ASTM A572-50, EXCEPT PLATES AND ANGLES WHICH SHALL CONFORM TO ASTM A36.

### B. CONNECTIONS

- STRUCTURAL STEEL DETAILING, FABRICATION AND ERECTION SHALL BE DONE IN ACCORDANCE WITH AISC 360-10, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," AND AISC 303-05, "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES." ALL CONNECTIONS SHALL BE SHOP WELDED AND FIELD BOLTED EXCEPT AS NOTED ON DRAWINGS.
- ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR. DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE STEEL CONNECTION DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL ONLY AND ARE NOT INTENDED TO SPECIFICALLY INDICATE THE REQUIRED QUANTITY, SPACING OR SIZE OF BOLTS OR WELDS, UNLESS SPECIFICALLY NOTED.
- CONNECTION DETAILS NOT COMPLETELY DETAILED ON THE DRAWINGS (INCLUDING MATERIAL GRADE AND SIZES; NUMBER, SIZE, AND GRADE OF BOLTS; AND SIZE OF WELDS) SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. CONNECTION DESIGN REACTIONS NOTED ON THESE DRAWINGS ARE BASED ON FACTORED (ULTIMATE) LOADS AND ARE INTENDED FOR USE WITH THE AISC LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD.
- ALL BOLTS IN STRUCTURAL CONNECTIONS SHALL CONFORM TO ASTM A325 TYPE 1, UNLESS INDICATED OTHERWISE ON THESE DRAWINGS. FIELD BOLTS SHALL BE 3/4" DIAMETER ASTM A325 BEARING TYPE BOLTS WITH THREADS INCLUDED IN THE SHEAR PLANE, UNLESS NOTED OTHERWISE.
- ALL SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION AS PER THE REQUIREMENTS OF THE AISC 360-05, SECTION J1.2.
- SEE DRAWINGS FOR LOCATIONS REQUIRING SLIP CRITICAL BOLTS AND/OR BOLTS CONFORMING TO ASTM A490.
- MINIMUM NUMBER OF BOLT ROWS BASED ON MEMBER DEPTH FOR W & C SHAPES ARE AS FOLLOWS:
 

UP TO 12" DEEP	2 ROWS
14" TO 16" DEEP	3 ROWS
18" TO 21" DEEP	4 ROWS
24" TO 27" DEEP	5 ROWS
- THE GENERAL CONTRACTOR SHALL SUBMIT TO THE ARCHITECT, FOR REVIEW, ENGINEERED AND CHECKED SHOP DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS AND ERECTION DRAWINGS FOR ALL STRUCTURAL STEEL. SUBMIT SHOP DRAWINGS PREPARED IN ACCORDANCE WITH AISC MANUAL "DETAILING FOR STEEL CONSTRUCTION". DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.
- NON-COMPOSITE BEAM CONNECTIONS SHALL BE DESIGNED FOR THE REACTION DUE TO MAXIMUM ALLOWABLE LOAD FOR THE APPROPRIATE SPAN AND SHAPE BASED ON THE BEAM TABLES OF THE AISC MANUAL OF STEEL CONSTRUCTION (14TH EDITION).

### C. WELDING

- UNLESS NOTED OTHERWISE, ELECTRODES FOR WELDING SHALL CONFORM TO E70XX (SMAW), E70X - EXXX (SAW), E70S-X (GMAW), OR E7X-X (FCAW). ALL FIELD WELDING SHALL BE DONE WITH E-70XX ELECTRODES.
- ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS AND SHALL CONFORM TO THE "CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION", LATEST EDITION.
- ALL WELDED JOINTS SHALL COMPLY W/ THE PROVISIONS OF AWS D1.1 STRUCTURAL WELDING CODE BY AMERICAN WELDING SOCIETY (SECTION 2207). THE GENERAL CONTRACTOR SHALL MAKE PROOF OF WELDER CERTIFICATION AVAILABLE AT THE JOB SITE.

### D. ANCHOR RODS

- UNLESS INDICATED OTHERWISE IN THE COLUMN SCHEDULE OR ON THE DRAWINGS, ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36. SEE ANCHOR BOLT SCHEDULE FOR ANCHOR BOLT DIAMETERS, QUANTITY AND TYPE OF NUTS, AND REQUIRED EMBEDMENT LENGTH. DAMAGE THREADS AT TWO PLACES BELOW THE NUT TO PREVENT LOOSENING.

### E. GROUT

- GROUT BENEATH STRUCTURAL STEEL BASE PLATES SHALL BE SIMPSON STRONG-TIE FX-228 NON-METALLIC, NON-SHRINK GROUT, OR AN APPROVED EQUAL. THE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AS FOLLOWS:
 

BEARING MATERIAL GROUT STRENGTH	
3,000 PSI CONCRETE	6,000 PSI
4,000 PSI CONCRETE	8,000 PSI
5,000 PSI CONCRETE	8,000 PSI

### F. MISCELLANEOUS

- ALL BEAMS AND GIRDERS SHALL BE FABRICATED WITH NATURAL CAMBER UP.
- AFTER FABRICATION, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS.
- THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBER FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE ARCHITECT.
- ALL ADDITIONAL STEEL REQUIRED BY THE CONTRACTOR FOR ERECTION PURPOSES AND SITE ACCESS OF STOCKPILED MATERIALS SHALL BE PROVIDED AT NO COST TO THE OWNER. ALL SUCH ADDITIONAL STEEL SHALL BE REMOVED BY THE CONTRACTOR.
- ALL STEEL EXPOSED TO EARTH SHALL BE PAINTED WITH BITUMINOUS COATING. ALL STRUCTURAL STEEL EXPOSED TO THE WEATHER SHALL BE SHOP PRIMED & FIELD PAINTED W/ TWO COATS OF PAINT. (EXCEPT FOR SURFACES TO RECEIVE WELD OR TO BE ENCASED IN CONCRETE. TOUCH UP ALL STRUCTURAL STEEL AFTER ERECTION WITH SAME PRIMER.)
- CONTRACTOR SHALL PROVIDE TEMPORARY BRACING OF STRUCTURAL FRAMING UNTIL ALL PERMANENT BRACING AND MOMENT CONNECTIONS ARE COMPLETELY INSTALLED.
- STEEL COLUMNS ARE TO BE CONTINUOUS WITH NO SPLICES UNLESS NOTED OTHERWISE. ALL STEEL COLUMNS TO HAVE 1/2" CAP PLATE UNLESS NOTED.

## 14. STAIRS, HANDRAILS, AND GUARDRAILS

- COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF STEEL STAIRS, STEEL AND/OR ALUMINUM HANDRAILS AND GUARDRAILS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS CONSTRUCTED AND SHALL BE AVAILABLE AT THE JOB SITE DURING THE TIMES OF INSPECTION.
- SHOP DRAWINGS SHALL INCLUDE DESIGN LOAD CRITERIA, DESIGN LOADS, LOCATIONS OF ANCHORAGE SUPPORT POINTS, FRAMING DETAILS, STRINGERS, STAIR TREADS, POSTS, PICKETS, TOP RAILS, INTERMEDIATE RAILS, HANGERS, POSTS AND CONNECTION DETAILS TO THE SUPPORTING STRUCTURE.
- COMPLETE STRUCTURAL CALCULATIONS SHALL BE SUBMITTED WITH SHOP DRAWINGS. CALCULATIONS SHALL INCLUDE, BUT NOT BE LIMITED TO CONNECTION DESIGN TO THE SUPPORTING STRUCTURE. CALCULATIONS SHALL BE SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS CONSTRUCTED.
- DESIGN LOAD CRITERIA SHALL MEET OR EXCEED THE MINIMUM REQUIREMENTS OF THE 2015 INTERNATIONAL BUILDING CODE, AS WELL AS ANY SPECIFIC REQUIREMENTS IDENTIFIED ON THE ARCHITECTURAL PLANS. IN PARTICULAR, SECTION 1607 REQUIRES THAT STAIRS AND EXITS BE DESIGNED FOR A 100 PSF UNIFORM LIVE LOAD WITH STAIR TREADS ALSO BEING CHECKED FOR A MINIMUM CONCENTRATED LOAD OF 300 LBS OVER AN AREA OF 4 SQ. IN.
- SECTION 1607.7.1 REQUIRES THAT HANDRAILS AND GUARDRAILS BE DESIGNED TO RESIST 50 PLF APPLIED IN ANY DIRECTION AT THE TOP AND TO TRANSFER THIS LOAD THROUGH THE SUPPORTS TO THE STRUCTURE. HANDRAILS AND GUARDRAILS SHALL ALSO BE ABLE TO RESIST A SINGLE CONCENTRATED LOAD OF 200 LBS APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP AND HAVE ATTACHMENT DEVICES AND SUPPORTING STRUCTURE TO TRANSFER THIS LOAD TO THE APPROPRIATE STRUCTURAL ELEMENTS OF THE BUILDING. INTERMEDIATE RAILS, BALUSTERS, PICKETS, AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A NORMAL LOAD OF 50 LBS ON AN AREA NOT TO EXCEED 1 SQ. FT. INCLUDING OPENINGS AND SPACE BETWEEN THE RAILS.
- COMPLY WITH NAAMM, NATIONAL ASSOCIATION OF ARCHITECTURAL METAL MANUFACTURERS, "METAL STAIR MANUAL".
- MINIMUM SIZES SHALL BE AS FOLLOWS:
 

14 GA. (UNLESS NOTED OTHERWISE)	
STRINGERS:	MC12x10.6 (UNLESS NOTED OTHERWISE)
- SEE ARCHITECTURAL DRAWINGS FOR EXACT LAYOUT AND DIMENSIONS.

## 15. MISCELLANEOUS

- CONTRACT DOCUMENTS:
  - IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO OBTAIN ALL CONTRACT DOCUMENTS AND LATEST ADDENDA AND TO SUBMIT SUCH DOCUMENTS TO ALL SUBCONTRACTORS AND SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD.
- DRAWING CONFLICTS:
  - THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
- EXISTING CONDITIONS:
  - THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS OF THE EXISTING BUILDING AT THE JOB SITE AND REPORT ANY DISCREPANCIES FROM ASSUMED CONDITIONS SHOWN ON THE DRAWINGS TO THE ARCHITECT AND ENGINEER PRIOR TO THE FABRICATION AND ERECTION OF ANY MEMBERS.
- RESPONSIBILITY OF THE CONTRACTOR FOR STABILITY OF THE STRUCTURE DURING CONSTRUCTION:
  - ALL STRUCTURAL ELEMENTS OF THE PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL REQUIRED BRACING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE LATERAL-LOAD RESISTING OR STABILITY-PROVIDING SYSTEM IS COMPLETELY INSTALLED AND THE STRUCTURE IS COMPLETELY TIED TOGETHER.
- CONFLICTS IN STRUCTURAL REQUIREMENTS:
  - WHERE CONFLICT EXISTS AMONG THE VARIOUS PARTS OF THE STRUCTURAL CONTRACT DOCUMENTS, STRUCTURAL DRAWINGS, GENERAL NOTES, AND SPECIFICATIONS, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE ENGINEER, SHALL GOVERN.
- STABILITY AND BRACING OF MASONRY WALLS DURING CONSTRUCTION:
  - ALL MASONRY WALLS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES APPLIED TO THEM IN THE FINAL CONSTRUCTED CONFIGURATION ONLY ASSUMING FULL BRACING TOP, BOTTOM, AND/OR SIDE OF WALL AS SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROPERLY AND ADEQUATELY BRACE ALL MASONRY WALLS AT ALL STAGES DURING CONSTRUCTION TO RESIST ERECTION LOADS AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.
- CONTRACTOR SUBSTITUTIONS:
  - ANY MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL THAT ARE DIFFERENT FROM THE MATERIAL OR PRODUCTS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS WILL BE APPROVED ONLY IF THE FOLLOWING CRITERIA ARE SATISFIED:
    - A COST SAVINGS TO THE OWNER IS DOCUMENTED AND SUBMITTED WITH THE REQUEST.
    - THE MATERIAL OR PRODUCT HAS BEEN APPROVED BY THE INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS (ICBO) AND THE ICBO REPORT IS SUBMITTED WITH THE REQUEST. SUBMITTALS NOT SATISFYING THE ABOVE CRITERIA WILL NOT BE CONSIDERED.
- SITE OBSERVATION BY THE STRUCTURAL ENGINEER:
  - THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION.
  - CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE.
  - THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
  - PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF ELLINWOOD + MACHADO, LLC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

- MAINTENANCE STATEMENT:
  - ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFE SPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT.
  - A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE SUCH ITEMS SUCH AS BUT NOT LIMITED TO PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTRA JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.


## 16. DRAWING INTERPRETATION

- TYPICAL DETAILS
  - DETAILS LABELED AS TYPICAL DETAILS ON THESE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE PLANS CAN BE DETERMINED BY THE TITLE OF THE DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE KEYS AT EACH LOCATION. DECISIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE ENGINEER.


## 17. STRUCTURAL ABBREVIATIONS

ADD'L. ADD.....	ADDITIONAL	LLV.....	LONG LEG VERTICAL
AB, ABOLT.....	ANCHOR BOLT	LOC.N.....	LOCATION
ARCH.....	ARCHITECTURAL	M.....	MOMENT
@.....	AT	MFG, MFG'R.....	MANUFACTURER
BM.....	BEAM	MECH.....	MECHANICAL
BRG.....	BEARING	MPH.....	MILES PER HOUR
BLK.....	BLOCK	MATL.....	MATERIAL
BOT, B.....	BOTTOM, BOTTOM BAR	MAX.....	MAXIMUM
BLDG.....	BUILDING	MFR.....	MANUFACTURER
CL.....	CENTER LINE	MTL.....	METAL
CLR.....	CLEAR	MIN.....	MINIMUM
COL.....	COLUMN	MISC.....	MISCELLANEOUS
CONC.....	CONCRETE	NIC.....	NOT IN CONTRACT
CONN.....	CONNECTION	NO.....	NUMBER
CMU.....	CONCRETE MASONRY UNIT	NTS.....	NOT TO SCALE
CONSTR.....	CONSTRUCTION	O.C.....	ON CENTER
CONT.....	CONTINUOUS	OPNG.....	OPENING
CONTR.....	CONTRACTOR	O.H.....	OPPOSITE HAND
D.....	DEEP	P.....	PAN
DSN.....	DESIGN	PAF.....	POWDER ACTUATED
FTS.....	FOOTING	FASTENER	
DET, DTL.....	DETAIL	P/C.....	PRECAST
DIA, Ø.....	DIAMETER	PL.....	PLATE
DIAG.....	DIAGRAM	PT.....	POST TENSIONED
DIM.....	DIMENSION	PTS.....	POINTS
DWG.....	DRAWING	PHSE.....	PENTHOUSE
DWL.....	DOWEL	PSF.....	POUNDS PER SQ. FOOT
EE.....	EACH END	PSI.....	POUNDS PER SQ. INCH
EF.....	EACH FACE	REF.....	REFERENCE
EW.....	EACH WAY	REV.....	REVISION
ELEV, EL.....	ELEVATION	REINF.....	REINFORCING
ETF.....	ELEVATION TOP OF FOOTING	REQ'D.....	REQUIRED
ETC.....	ETCETERA	REBAR.....	REINFORCING BAR
EQ.....	EQUAL	SCHD, SCHED.....	SCHEDULE
EXIST, EXT'G.....	EXISTING	SECT.....	SECTION
EXP.....	EXPANSION	SLH.....	SHORT LEG HORIZONTAL
EXP JT, E.J.....	EXPANSION JOINT	SLV.....	SHORT LEG VERTICAL
EXT.....	EXTERIOR	SLO.....	SHORT LEG OUT
FI.....	FACE OF	SIM.....	SIMILAR
FIN FL, FFE.....	FINISHED FLOOR	SOG.....	SLAB ON GRADE
FFE.....	FINISHED FLOOR ELEVATION	SPEC.....	SPECIFICATIONS
FL, FLR.....	FLOOR	SO.....	SQUARE
FRT.....	FIRE RETARDANT	STD.....	STANDARD
	TREATED WOOD	STIRR.....	STIRRUPS
FTG.....	FOOTING	STL, STL.....	STEEL
FT.....	FEET, FOOT	STRUCT.....	STRUCTURAL
FLG.....	FLANGE	T.....	TOP
GA, ga.....	GAUGE	THK.....	THICK
GALV.....	GALVANIZED	THRD.....	THREADED
H.....	HEAD	THRU.....	THROUGH
HK.....	HOOK	T/S, T/STL.....	TOP OF STEEL
HR.....	HOUR	T/B, T/BM.....	TOP OF BEAM
HORIZ.....	HORIZONTAL	T/CONC.....	TOP OF CONCRETE
INFO.....	INFORMATION	T/SLAB.....	TOP OF SLAB
INT.....	INTERIOR	T/FTG.....	TOP OF FOOTING
JST.....	JOIST	TYPICAL	
JT.....	JOINT	UNO.....	UNLESS NOTED OTHERWISE
K-FT.....	KIP-FEET	VERT.....	VERTICAL
K/FT.....	KIPS PER FOOT	W.....	WIDE
K.....	KIPS	W/.....	WITH
L.....	LENGTH	WP.....	WORKING POINT

Master Submittal List						
Submittal Type	Description	For Approval by E.O.R.	For Review by E.O.R.	For E.O.R.'s Record	Shop Drawings Signed/Sealed by Professional Engineer?	Calculations Signed/Sealed by Professional Engineer?
<b>Shallow Foundations</b>						
Concrete Mix Design	Should include the following items, at a minimum, for each mix design being submitted: Indication as to what structural components are to be poured with a given mix design (each mix design shall have its own unique name/number), mix design showing quantities by both weight and volume, 30-break history (or alternative data per ACI 318 §5.3/§5.4), cement mill certificate, material certificate(s) for supplementary cementitious materials (fly ash, slag, silica fume), gradation analysis (for both coarse and fine aggregates), cut sheets for all chemical admixtures, slump, air content, water/cement ratio, source data for aggregate silicious reactivity (ASR)	X				
Mild reinforcing		X				
Soil compaction test results				X		
<b>Post-tensioned Slabs on Grade</b>						
Concrete Mix Design	Should include the following items, at a minimum, for each mix design being submitted: Indication as to what structural components are to be poured with a given mix design (each mix design shall have its own unique name/number), mix design showing quantities by both weight and volume, 30-break history (or alternative data per ACI 318 §5.3/§5.4), cement mill certificate, material certificate(s) for supplementary cementitious materials (fly ash, slag, silica fume), gradation analysis (for both coarse and fine aggregates), cut sheets for all chemical admixtures, slump, air content, water/cement ratio, source data for aggregate silicious reactivity (ASR)	X				
Vapor Barrier		X				
PT friction loss calculations		X				X
PT stressing ram calibration				X		
Post-tensioning tendon mill certificates				X		
Unbonded post-tensioned reinforcing		X				
Bonded post-tensioned reinforcing		X				
Concrete products (form oil, sealants, repair materials, etc.)		X				
Embedded items (embed plates, chubby smack, etc.)		X				
Reinforcing steel mill certificates				X		
Horizontal construction joint locations		X				
Mild reinforcing		X				
<b>Steel Stairs</b>						
Stair shop drawings		X			X	
Stair calculations		X				X
<b>Wood Framing</b>						
Wood product information (sawn lumber)		X				
Wood product information (engineered lumber)		X				
Load-bearing stud wall layout		X				
Engineered wood trusses and girder trusses (design by specialty engineer)		X				X
Wood framing hardware		X				
Uplift anchors (as required)		X				
Nailing submittal		X				
<b>Concrete Masonry (CMU)</b>						
CMU product information		X				
Mild reinforcing		X				
Grout mix design		X				
Mortar mix design		X				
Embedded items (embed plates, bearing plates, etc.)		X				
<b>Miscellaneous</b>						
Storefront shop drawings		X			X	X
Window Wall shop drawings		X			X	X
Curtainwall shop drawings		X			X	X
Barrier cable / cable guard rail		X			X	X
Handrail shop drawings (steel, aluminum, other)		X			X	X
<b>Substitutions</b>						
Materials		X				
Member sizes		X				
Methods of construction			X			



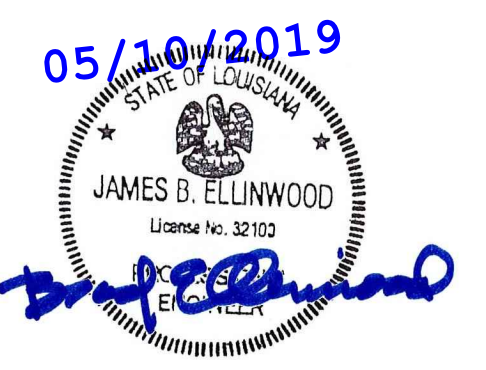
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ISSUE & REVISION RECORD


#	DATE	DESCRIPTION
	08/08/2018	DESIGN DEVELOPMENT
	09/24/2018	PERMIT SET
D	12/05/2018	COORDINATION REVIEW
F	02/07/2019	CONSTRUCTION REVIEW SET
G	03/22/2019	CONSTRUCTION RELEASE
H	05/10/2019	ISSUED FOR CONSTRUCTION

PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER: 1867709  
ISSUE DATE: 05/10/2019  
DRAWN BY/CHECKED BY: DKJ / RAFAEL, RIM  
APPROVED BY: BRAD, JBE  
DRAWING TITLE: MASTER SUBMITTAL LIST  
SHEET NUMBER: S0-05

**S0-05**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328  
PHILLIPSPART.COM 770-394-1616

FOR CONSTRUCTION

## Statement of Special Inspections

Project: *Fremaux Park Apartments*

Location: *Slidell, LA*

Owner: *Waypoint Residential*

Owner's Address: *3475 Piedmont Road NE, Suite 1640, Atlanta, GA 30305*

Architect of Record: *Phillips Partnership*

Structural Engineer of Record: *Ellinwood Machado, LLC*

This **Statement of Special Inspections** is submitted as a condition for permit issuance in accordance with the: *2015 International Building Code*. It includes a Schedule of Special Inspection Services applicable to this project.

The Special Inspections Engineer of Record shall keep records of specified special inspections and testing and shall furnish copies of inspection and testing reports to the building official and to the appropriate registered design professionals of record. Discrepancies from the approved plans and specification and code violations observed during the conduct of special inspections services shall be brought to the immediate attention of the contractor for correction and to the attention of the appropriate registered design professionals of record. A Final Report of Special Inspections documenting completion of specified special inspections and correction of any discrepancies and observed code violations noted in the inspection and testing reports shall be submitted periodically at a frequency agreed upon by the permit applicant and the building official prior to the start of work.

### • Statement of Special Inspections •

## Schedule of Special Inspection Services

Project: *Fremaux Park Apartments*

The following sheets comprise the required schedule of special inspections for this project. The construction divisions which require special inspections for this project are as follows:

<input checked="" type="checkbox"/> Soils and Foundations	<input type="checkbox"/> Cold-Formed Steel
<input checked="" type="checkbox"/> Framing	<input checked="" type="checkbox"/> Spray Fire Resistant
<input checked="" type="checkbox"/> Cast-in-Place Concrete	<input checked="" type="checkbox"/> Special Inspections for Wind Resistance
<input type="checkbox"/> Material	<input checked="" type="checkbox"/> Special Inspections for Seismic Resistance
<input type="checkbox"/> Precast Concrete	<input type="checkbox"/> Wood Construction
<input checked="" type="checkbox"/> Masonry	<input type="checkbox"/> Exterior Insulation and Finish System
<input checked="" type="checkbox"/> Structural Steel	<input type="checkbox"/> Special Cases

### KEY FOR MINIMUM QUALIFICATIONS OF INSPECTION AGENTS (WHERE INDICATED ON SCHEDULE)

PE	PROFESSIONAL ENGINEER
EIT	ENGINEER IN TRAINING
ACI	AMERICAN CONCRETE INSTITUTE CERTIFIED CONCRETE FIELD TESTING TECHNICIAN
AWS	AMERICAN WELDING SOCIETY CERTIFIED WELDING INSPECTOR
ASNT	AMERICAN SOCIETY OF NON-DESTRUCTIVE TESTING - LEVEL II OR III
PCI	PRECAST/PRESTRESSED CONCRETE INSTITUTE INSPECTOR CERTIFICATION - LEVEL II OR III
PTI	POST-TENSIONING INSTITUTE INSPECTOR CERTIFICATION - LEVEL II

### SCHEDULE OF SPECIAL INSPECTION SERVICES

PROJECT		APPLICABILITY TO THIS PROJECT			
MATERIAL/ACTIVITY	SERVICE	Y/N			AGENT*
		Y	EXTENT	AGENT*	
<b>1704.2.5 Inspection of Fabricators</b>					
Verify fabrication/quality control procedures	In-plant review (3)	Y	Periodic	PE	
<b>1705.1.1 Special Cases</b>					
(work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements)	Submittal review, shop (3) and/or field inspection	N		PE	
<b>1705.2 Steel Construction</b>					
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	Y	Each submittal	EIT	
2. Material verification of structural steel	Shop (3) and field inspection	Y	Periodic	PE & AWS	
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Y	Continuous	EIT	
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Y	Periodic	EIT	
5. Structural steel welding:					
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	AWS	
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Y	Observe (4)	AWS	
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Y	Observe or Perform as noted (4)	AWS	
d. Nondestructive testing (NDT) of welded joints: see Commentary					
1. Complete penetration groove welds 5/16" or greater in risk category III or IV	Shop (3) or field ultrasonic testing - 100%	N	Periodic	AWS	
2. Complete penetration groove welds 5/16" or greater in risk category II	Shop (3) or field ultrasonic testing - 10% of welds min.	Y	Periodic	AWS	
3. Thermally cut surfaces of access holes when material t > 2"	Shop (3) or field magnetic Partical or Penetrant testing	Y	Periodic	AWS	
4. Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Shop (3) or field radiographic or Ultrasonic testing	Y	Periodic	AWS	
5. Fabricator's NDT reports when fabricator performs NDT	Verify reports	Y	Each submittal (5)	AWS	

6.	Structural steel bolting:	Shop (3) and field inspection			
a.	Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Y	Observe or Perform as noted (4)	PE or EIT
b.	Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Y	Observe (4)	PE or EIT
1.	Pre-tensioned and slip-critical joints		Y		
a.	Turn-of-nut with matching markings		Y	Periodic	PE or EIT
b.	Direct tension indicator		Y	Periodic	PE or EIT
c.	Twist-off type tension control bolt		Y	Periodic	PE or EIT
d.	Turn-of-nut without matching markings		Y	Continuous	PE or EIT
e.	Calibrated wrench		Y	Continuous	PE or EIT
2.	Snug-tight joints		Y	Periodic	PE or EIT
c.	Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Y	Perform (4)	PE or EIT
7.	Inspection of steel elements of composite construction prior to concrete placement in accordance with QA tasks listed in AISC 360, Table N6.1	Shop (3) and field inspection and testing	Y	Observe or Perform as noted (4)	PE or EIT

### 1705.2.2 Steel Construction Other Than Structural Steel

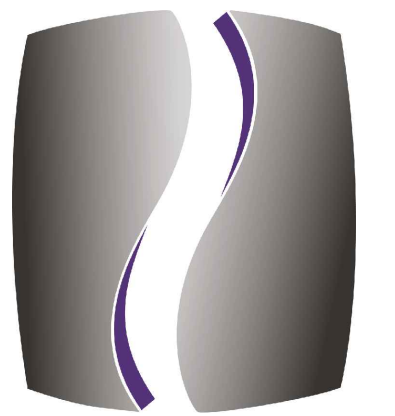
1.	Material verification of cold-formed steel deck:				
a.	Identification markings	Field inspection	N	Periodic	EIT
b.	Manufacturer's certified test reports	Submittal Review	N	Each submittal	EIT
2.	Connection of cold-formed steel deck to supporting structure:	Shop (3) and field inspection			
a.	Welding		N	Periodic	AWS
b.	Other fasteners (in accordance with AISC 360, Section N6)				
1.	Verify fasteners are in conformance with approved submittal		N	Periodic	EIT
2.	Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations		N	Periodic	EIT
3.	Reinforcing steel	Shop (3) and field inspection			
a.	Verification of weldability of steel other than ASTM A706		Y	Periodic	EIT
b.	Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special concrete structural walls and shear reinforcement		Y	Continuous	EIT
c.	Shear reinforcement		Y	Continuous	EIT
d.	Other reinforcing steel		Y	Periodic	EIT
4.	Cold-formed steel trusses spanning 60 feet or greater				
a.	Verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package		N	Periodic	PE or EIT

### 1705.3 Concrete Construction

1.	Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection		Periodic.	EIT
2.	Inspection of prestressing steel installation	Shop (3) and field inspection		Periodic.	EIT or PTI
3.	Inspection of anchors cast in concrete where allowable loads have been increased per section 1908.5 or where strength design is used	Shop (3) and field inspection		Continuous	EIT
4.	Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection		Periodic or as required by the research report issued by an approved source	PE or EIT
5.	Verify use of approved design mix	Shop (3) and field inspection		Periodic	EIT
6.	Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection		Continuous	ACI
7.	Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection		Continuous	ACI
8.	Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection		Periodic	ACI
9.	Inspection of prestressed concrete:	Shop (3) and field inspection			
a.	Application of prestressing force			Continuous	PE or PCI
b.	Grouting of bonded prestressing tendons in the seismic-force-resisting system			Continuous	PE or PCI
10.	Erection of precast concrete members				
a.	Inspect in accordance with construction documents	Field inspection		In accordance with construction documents	EIT or PCI
b.	Perform inspections of welding and bolting in accordance with Section 1705.2	Field inspection		In accordance with Section 1705.2	AWS
11.	Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports		Periodic	ACI
12.	Inspection of formwork for shape, lines, location and dimensions	Field inspection		Periodic	EIT
13.	Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports		Periodic	ACI

### 1705.4 Masonry Construction

<b>A. Level A, B and C Quality Assurance:</b>					
1.	Verify compliance with approved submittals	Field Inspection		Periodic	EIT
<b>B. Level B Quality Assurance:</b>					
1.	Verification of fm and FAAC prior to construction	Testing by unit strength method or prism test method		Periodic	ACI
<b>C. Level C Quality Assurance:</b>					
1.	Verification of fm and FAAC prior to construction and for every 5,000 SF during construction	Testing by unit strength method or prism test method		Periodic	ACI
2.	Verification of proportions of materials in premixed or preblended mortar, prestressing (post-tensioning) grout, and grout other than self-consolidating grout, as delivered to the project site	Field inspection		Continuous	ACI
3.	Verify placement of masonry units	Field inspection		Periodic	EIT
<b>D. Levels B and C Quality Assurance:</b>					
1.	Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered to the project	Field testing		Continuous	ACI
2.	Verify compliance with approved submittals	Field inspection		Periodic	EIT
3.	Verify proportions of site-mixed mortar, grout and prestressing (post-tensioning) grout for bonded tendons	Field inspection		Periodic	ACI
4.	Verify grade, type, and size of reinforcement and anchor bolts, and prestressing (post-tensioning) tendons and anchorages	Field inspection		Periodic	EIT
5.	Verify construction of mortar joints	Field inspection		Periodic	EIT
6.	Verify placement of reinforcement, connectors, and prestressing (post-tensioning) tendons and anchorages	Field inspection		Level B - Periodic Level C - Continuous	EIT EIT
7.	Verify grout space prior to grouting	Field inspection		Level B - Periodic Level C - Continuous	EIT EIT
8.	Verify placement of grout and prestressing (post-tensioning) grout for bonded tendons	Field inspection		Continuous	EIT
9.	Verify size and location of structural masonry elements	Field inspection		Periodic	EIT
10.	Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection		Level B - Periodic Level C - Continuous	EIT EIT



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SEAL



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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER: 1867709

ISSUE DATE: 05/10/2019

DRAWN BY/CHECKED BY: DKJ / RAFAEL, RIM

APPROVED BY: BRAD, JBE

DRAWING TITLE

**SCHEDULE OF SPECIAL INSPECTION**

SHEET NUMBER

**S0-10**

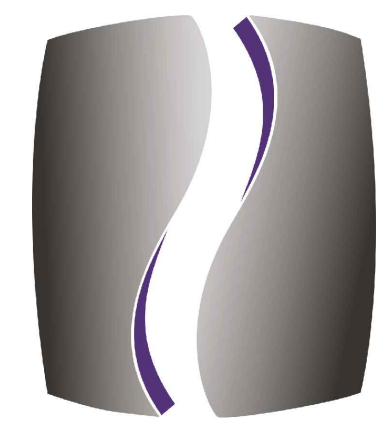
5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

PHILLIPSPART.COM 770-394-1616

**FOR CONSTRUCTION**

11.	Verify welding of reinforcement (see 1705.2.2)	Field inspection	Continuous	AWS
12.	Verify preparation, construction, and protection of masonry during cold weather (temperature below 40° F) or hot weather (temperature above 90° F)	Field inspection	Periodic	EIT
13.	Verify application and measurement of prestressing force	Field inspection	Continuous	EIT or PTI
14.	Verify placement of AAC masonry units and construction of thin-bed mortar joints (first 5000 SF of AAC masonry)	Field inspection	Continuous	EIT
15.	Verify placement of AAC masonry units and construction of thin-bed mortar joints (after the first 5000 SF of AAC masonry)	Field inspection	Level B - Periodic	EIT
16.	Verify properties of thin-bed mortar for AAC masonry (first 5000 SF of AAC masonry)	Field inspection	Level C - Continuous	EIT
17.	Verify properties of thin-bed mortar for AAC masonry (after the first 5000 SF of AAC masonry)	Field inspection	Continuous	ACI
18.	Prepare grout and mortar specimens	Field testing	Level B - Periodic	ACI
19.	Observe preparation of prisms	Field inspection	Level C - Continuous	ACI
<b>1705.5 Wood Construction</b>				
1.	Inspection of the fabrication process of wood structural elements and assemblies in accordance with Section 1704.2.5	In-plant review (3)	Periodic	EIT
2.	For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans	Field inspection	Periodic	EIT
3.	For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection	Periodic	PE or EIT
4.	Metal-plate-connected wood trusses spanning 60 feet or greater; verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	Periodic	EIT
<b>1705.6 Soils</b>				
1.	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Periodic	PE or EIT
2.	Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Periodic	PE or EIT
3.	Perform classification and testing of controlled fill materials.	Field inspection	Periodic	PE or EIT
4.	Verify use of proper materials, densities, and lift thicknesses during placement and compaction of controlled fill	Field inspection	Continuous	PE or EIT
5.	Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly	Field inspection	Periodic	PE or EIT
<b>1705.7 Driven Deep Foundations</b>				
1.	Verify element materials, sizes and lengths comply with requirements	Field inspection	Continuous	
2.	Determine capacities of test elements and conduct additional load tests, as required	Field inspection	Continuous	
3.	Observe driving operations and maintain complete and accurate records for each element	Field inspection	Continuous	
4.	Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element	Field inspection	Continuous	
5.	For steel elements, perform additional inspections per Section 1705.2	See Section 1705.2	See Section 1705.2	
6.	For concrete elements and concrete-filled elements, perform additional inspections per Section 1705.3	See Section 1705.3	See Section 1705.3	
7.	For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge	Field inspection	In accordance with construction documents	
8.	Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	In accordance with construction documents	
<b>1705.8 Cast-in-Place Deep Foundations</b>				
1.	Observe drilling operations and maintain complete and accurate records for each element	Field inspection	Continuous	
2.	Verify placement locations and plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Record concrete or grout volumes	Field inspection	Continuous	
3.	For concrete elements, perform additional inspections in accordance with Section 1705.3	See Section 1705.3	See Section 1705.3	
4.	Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	In accordance with construction documents	
<b>1705.9 Helical Pile Foundations</b>				
1.	Verify installation equipment, pile dimensions, tip elevations, final depth, final installation torque and other data as required.	Field inspection	Continuous	
2.	Perform additional inspections and tests in accordance with the construction documents	Field Inspection and testing	In accordance with construction documents	
<b>1705.10.1 Structural Wood Special Inspections For Wind Resistance</b>				
1.	Inspection of field gluing operations of elements of the main windforce-resisting system	Field inspection	Continuous	PE
2.	Inspection of nailing, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Periodic	PE
<b>1705.10.2 Cold-formed Steel Special Inspections For Wind Resistance</b>				
1.	Inspection during welding operations of elements of the main windforce-resisting system	Shop (3) and field inspection	Periodic	PE
2.	Inspections for screw attachment, bolting, anchoring and other fastening of components within the main windforce-resisting system	Shop (3) and field inspection	Periodic	PE
<b>1705.10.3 Wind-resisting Components</b>				
1.	Roof cladding	Shop (3) and field inspection	Periodic	PE
2.	Wall cladding	Shop (3) and field inspection	Periodic	PE
<b>1705.11.1 Structural Steel Special Inspections for Seismic Resistance</b>				
Inspection of structural steel in accordance with AISC 341		Shop (3) and field inspection	In accordance with AISC 341	PE
<b>1705.11.2 Structural Wood Special Inspections for Seismic Resistance</b>				
1.	Inspection of field gluing operations of elements of the seismic-force resisting system	Field inspection	Continuous	PE
2.	Inspection of nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	Periodic	PE
<b>1705.11.3 Cold-formed Steel Light-Frame Construction Special Inspections for Seismic Resistance</b>				
1.	Inspection during welding operations of elements of the seismic-force-resisting system	Shop (3) and field inspection	Periodic	PE
2.	Inspections for screw attachment, bolting, anchoring and other fastening of components within the seismic-force-resisting system	Shop (3) and field inspection	Periodic	PE
<b>1705.11.4 Designated Seismic Systems Verification</b>				
Inspect and verify that the component label, anchorage or mounting conforms to the certificate of compliance in accordance with Section 1705.12.3		Field inspection	Periodic	PE
<b>1705.11.5 Architectural Components Special Inspections for Seismic Resistance</b>				
1.	Inspection during the erection and fastening of exterior cladding and interior and exterior veneer	Field inspection	Periodic	PE
2.	Inspection during the erection and fastening of interior and exterior nonbearing walls	Field inspection	Periodic	PE
3.	Inspection during anchorage of access floors	Field inspection	Periodic	PE
<b>1705.11.6 Mechanical and Electrical Components Special Inspections for Seismic Resistance</b>				
1.	Inspection during the anchorage of electrical equipment for emergency or standby power systems	Field inspection	Periodic	PE
2.	Inspection during the anchorage of other electrical equipment	Field inspection	Periodic	PE
3.	Inspection during installation and anchorage of piping systems designed to carry hazardous materials, and their associated mechanical units	Field inspection	Periodic	PE
4.	Inspection during the installation and anchorage of HVAC ductwork that will contain hazardous materials	Field inspection	Periodic	PE
5.	Inspection during the installation and anchorage of vibration isolation systems	Field inspection	Periodic	PE
<b>1705.11.7 Storage Racks Special Inspections for Seismic Resistance</b>				
Inspection during the anchorage of storage racks 8 feet or greater in height		Field inspection	Periodic	PE

<b>1705.11.8 Seismic Isolation Systems</b>				
Inspection during the fabrication and installation of isolator units and energy dissipation devices used as part of the seismic isolation system		Shop and field inspection	Periodic	PE
<b>1705.12.1 Concrete Reinforcement Testing and Qualification for Seismic Resistance</b>				
1.	Review certified mill test reports for each shipment of reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review certified mill test reports	Each shipment	PE
2.	Verify reinforcement weldability of ASTM A615 reinforcement used to resist earthquake-induced flexural and axial forces in reinforced concrete special moment frames, special structural walls, and coupling beams connecting special structural walls	Review test reports	Each shipment	PE
<b>1705.12.2 Structural Steel Testing and Qualification for Seismic Resistance</b>				
Test in accordance with the quality assurance requirements of AISC 341		Shop (3) and field testing	Per AISC 341	PE
<b>1705.12.3 Seismic Certification of Nonstructural Components</b>				
Review certificate of compliance for designated seismic system components.		Certificate of compliance review	Each submittal	PE
<b>1705.12.4 Seismic Isolation Systems</b>				
Test seismic isolation system in accordance with ASCE 7 Section 17.8		Prototype testing	Per ASCE 7	
<b>1705.13 Sprayed Fire-resistant Materials</b>				
1.	Verify surface condition preparation of structural members	Field inspection	Periodic	PE or EIT
2.	Verify application of sprayed fire-resistant materials	Field inspection	Periodic	PE or EIT
3.	Verify average thickness of sprayed fire-resistant materials applied to structural members	Field inspection	Periodic	PE or EIT
4.	Verify density of the sprayed fire-resistant material complies with approved fire-resistant design	Field inspection and testing	Per IBC Section 1705.13.5	PE or EIT
5.	Verify the cohesive/adhesive bond strength of the cured sprayed fire-resistant material	Field inspection and testing	Per IBC Section 1705.13.6	PE or EIT
<b>1705.14 Mastic and Intumescent Fire-Resistant Coatings</b>				
Inspect mastic and intumescent fire-resistant coatings applied to structural elements and decks		Field inspection	Periodic	PE or EIT
<b>1705.15 Exterior Insulation and Finish Systems (EIFS)</b>				
1.	Verify materials, details and installations are per the approved construction documents	Field inspection	Periodic	
2.	Inspection of water-resistive barrier over sheathing substrate	Field inspection	Periodic	
<b>1705.16 Fire-Resistant Penetrations and Joints</b>				
1.	Inspect penetration firestop systems	Field testing	Per ASTM E2174	
2.	Inspect fire-resistant joint systems	Field testing	Per ASTM E2393	
<b>1705.17 Smoke Control Systems</b>				
1.	Leakage testing and recording of device locations prior to concealment	Field testing	Periodic	
2.	Prior to occupancy and after sufficient completion, pressure difference testing, flow measurements, and detection and control verification	Field testing	Periodic	
<b>INSPECTION AGENTS</b>				
<b>FIRM</b>		<b>ADDRESS</b>		
1.				
2.				
3.				
4.				
<b>Notes:</b>				
1.	<i>The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.</i>			
2.	<i>The list of Special Inspectors may be submitted as a separate document, if noted so above.</i>			
3.	<i>Special Inspections as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2</i>			
4.	<i>Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.</i>			
5.	<i>NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, NT.</i>			
Are Requirements for Seismic Resistance included in the Statement of Special Inspections?		Yes	No	
Are Requirements for Wind Resistance included in the Statement of Special Inspections?		Yes	No	



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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER: 1867709

ISSUE DATE: 05/10/2019

DRAWN BY/CHECKED BY: DKJ / RAFAEL, RIM

APPROVED BY: BRAD, JBE

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**SCHEDULE OF SPECIAL INSPECTION**

SHEET NUMBER

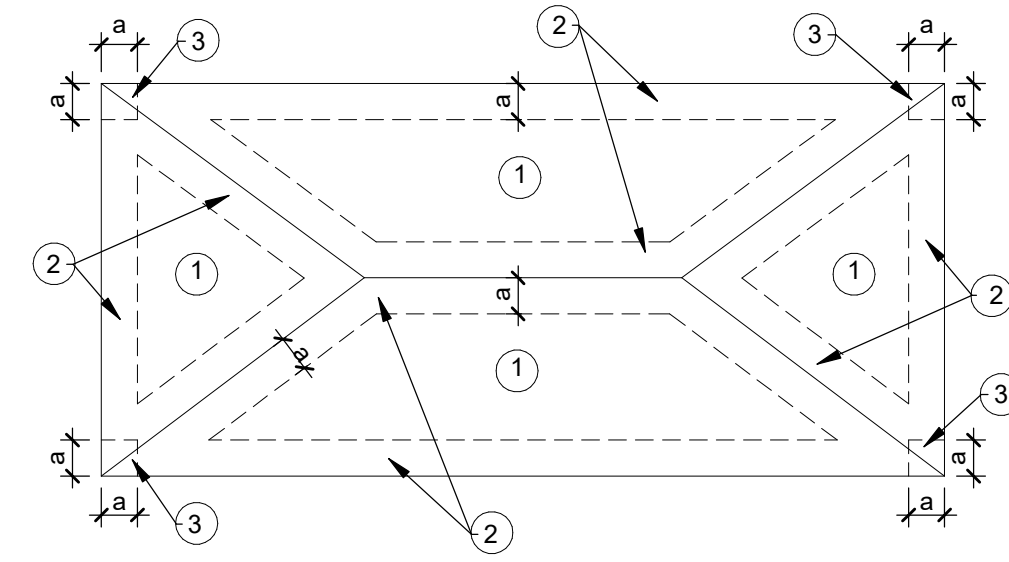
**S0-11**

5901 PEACHTREE DUNWOODY RD.  
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ATLANTA, GEORGIA 30328

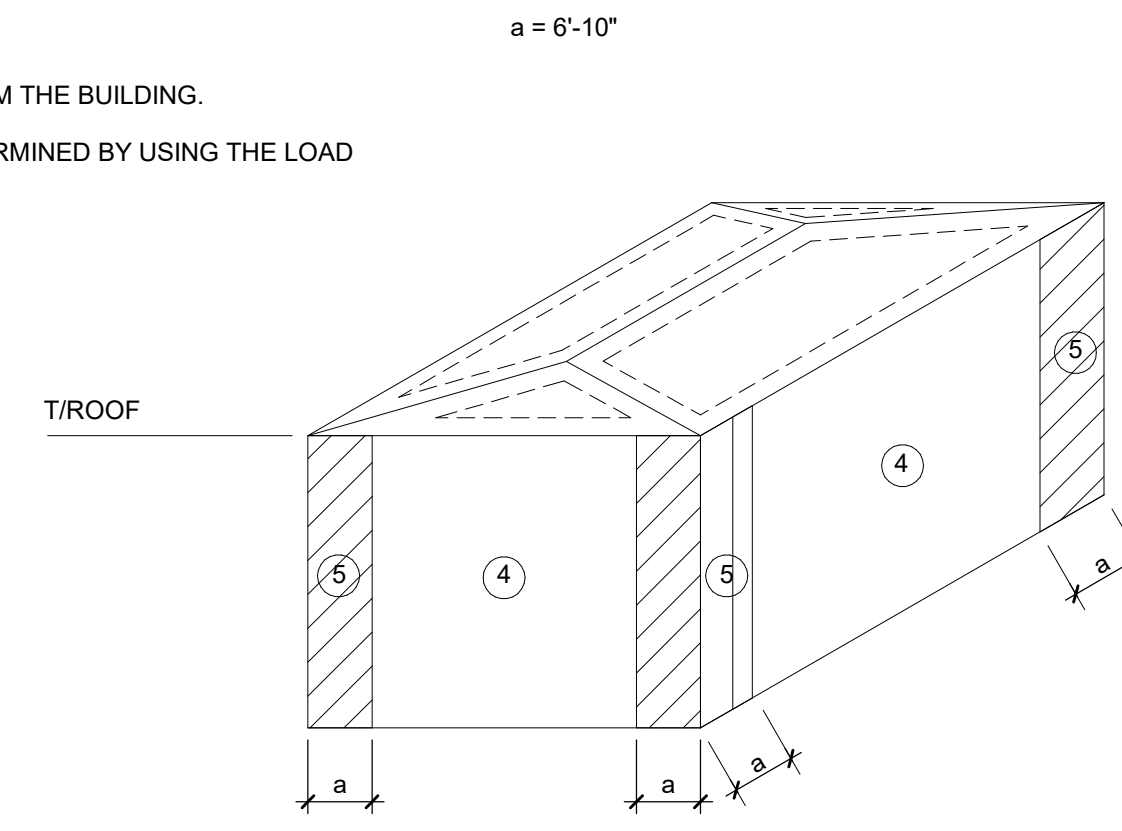
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**FOR CONSTRUCTION**

- NOTES:
1. WIND PRESSURES SHOWN IN THIS DIAGRAM ARE ULTIMATE LEVEL (FACTORED) LOADS PER ASCE 7-10 AND THE 2015 INTERNATIONAL BUILDING CODE. WHERE SERVICE LEVEL (UNFACTORED) WIND PRESSURES ARE NEEDED FOR DESIGN, THE VALUES IN THIS DIAGRAM SHALL BE MULTIPLIED BY A FACTOR OF 0.60.
  2. TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN.
    - FOR TRIBUTARY AREAS BETWEEN THE VALUES SHOWN, LINEARLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE.
    - FOR TRIBUTARY AREAS EXCEEDING 500 SQUARE FEET, USE THE VALUES FOR 500 SQUARE FEET.
  3. POSITIVE PRESSURES ACT TOWARD THE BUILDING, NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
  4. SEE DIAGRAMS FOR LOCATIONS OF ZONES.
  5. SERVICE LEVEL (UNFACTORED) NET UPLIFT PRESSURE ON ROOF FRAMING SHALL BE DETERMINED BY USING THE LOAD COMBINATION  $0.6D \pm 0.6W$ .
    - D = DEAD LOAD (20 PSF TYPICAL)
    - W = WIND LOAD (SEE ADJACENT TABLE)



HIP ROOF PLAN (GENERIC BUILDING SHOWN)



WALLS (GENERIC BUILDING SHOWN)

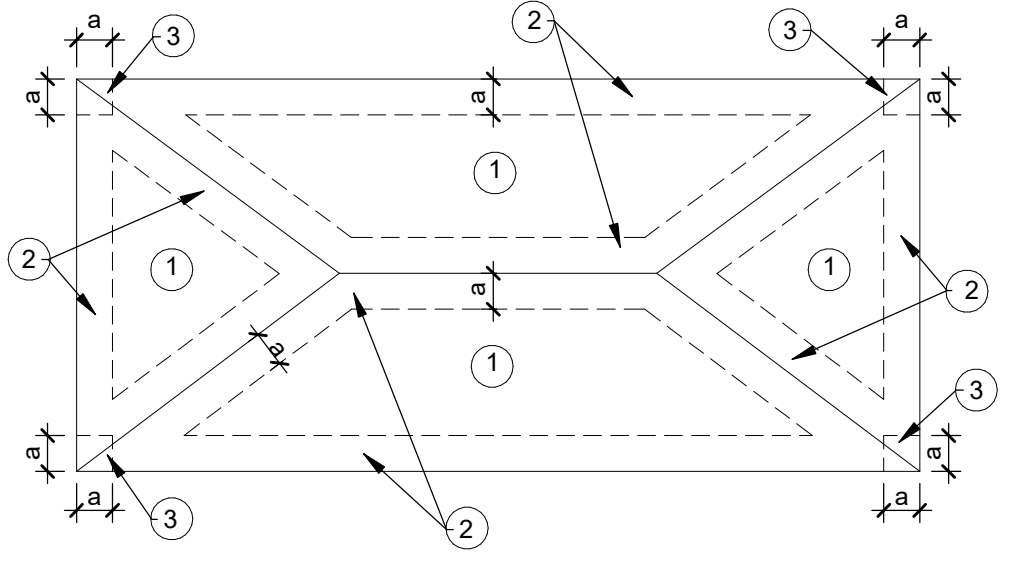
COMPONENT & CLADDING DESIGN WALL PRESSURES				
ROOF (PSF)				
SLOPED				
ZONE	10 SF	50 SF	100 SF	500 SF
1	+30.5 -48.4	+24.2 -45.2	+21.5 -43.9	+21.5 -43.9
2	+30.5 -84.2	+24.2 -68.5	+21.5 -61.8	+21.5 -61.8
3	+30.5 -84.2	+24.2 -68.5	+21.5 -61.8	+21.5 -61.8
OVERHANGS (PSF)				
ZONE	10 SF	50 SF	100 SF	500 SF
2 AND 3	-98.5	-98.5	-98.5	-98.5
WALLS (PSF)				
ZONE	20 SF	50 SF	100 SF	500 SF
4	+50.5 -54.9	+47.3 -51.8	+44.9 -49.4	+39.4 -43.9
5	+50.5 -66.0	+47.3 -59.7	+44.9 -54.9	+39.4 -43.9

**1** COMPONENT & CLADDING WIND LOADS - BUILDINGS A & B

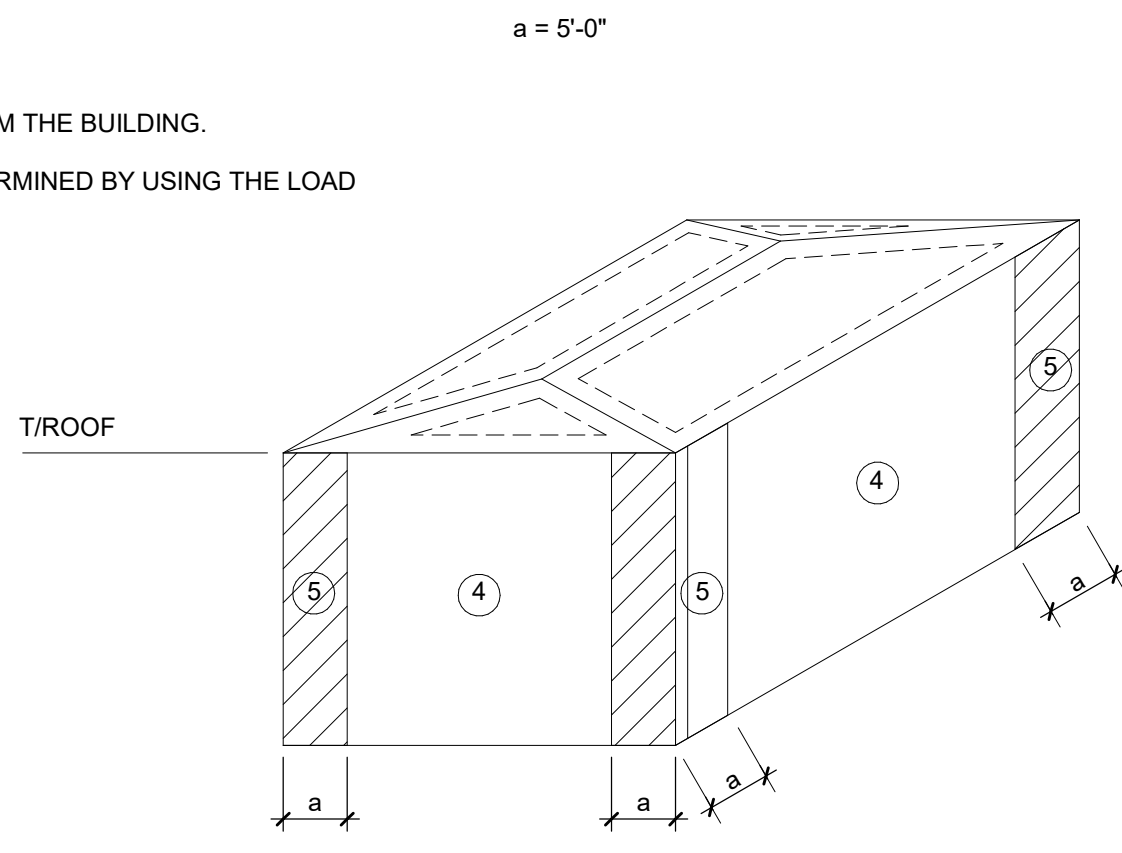
S0-20

SCALE: 3/4" = 1'-0"

- NOTES:
1. WIND PRESSURES SHOWN IN THIS DIAGRAM ARE ULTIMATE LEVEL (FACTORED) LOADS PER ASCE 7-10 AND THE 2015 INTERNATIONAL BUILDING CODE. WHERE SERVICE LEVEL (UNFACTORED) WIND PRESSURES ARE NEEDED FOR DESIGN, THE VALUES IN THIS DIAGRAM SHALL BE MULTIPLIED BY A FACTOR OF 0.60.
  2. TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN.
    - FOR TRIBUTARY AREAS BETWEEN THE VALUES SHOWN, LINEARLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE.
    - FOR TRIBUTARY AREAS EXCEEDING 500 SQUARE FEET, USE THE VALUES FOR 500 SQUARE FEET.
  3. POSITIVE PRESSURES ACT TOWARD THE BUILDING, NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
  4. SEE DIAGRAMS FOR LOCATIONS OF ZONES.
  5. SERVICE LEVEL (UNFACTORED) NET UPLIFT PRESSURE ON ROOF FRAMING SHALL BE DETERMINED BY USING THE LOAD COMBINATION  $0.6D \pm 0.6W$ .
    - D = DEAD LOAD (20 PSF TYPICAL)
    - W = WIND LOAD (SEE ADJACENT TABLE)



HIP ROOF PLAN (GENERIC BUILDING SHOWN)



WALLS (GENERIC BUILDING SHOWN)

COMPONENT & CLADDING DESIGN WALL PRESSURES				
ROOF (PSF)				
HIP ROOF				
ZONE	10 SF	50 SF	100 SF	500 SF
ALL ZONES	+43.6	-40.8	+39.6	+39.6
1	-47.6	-42.0	-39.6	-39.6
2	-55.7	-50.1	-47.6	-47.6
3	-55.7	-50.1	-47.6	-47.6
WALLS (PSF)				
ZONE	20 SF	50 SF	100 SF	500 SF
4	+45.5 -49.5	+42.7 -46.7	+40.5 -44.5	+35.5 -39.6
5	+45.5 -59.5	+42.7 -53.8	+40.5 -49.5	+35.5 -39.6

**2** COMPONENT & CLADDING WIND LOADS - CLUBHOUSE

S0-20

SCALE: 3/4" = 1'-0"



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PROJECT

FREMAUX PARK  
APARTMENTS

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

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APPROVED BY BRAD, JBE

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STRUCTURAL LOADING  
INFORMATION

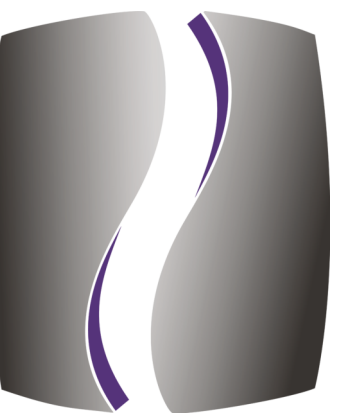
SHEET NUMBER

**S0-20**

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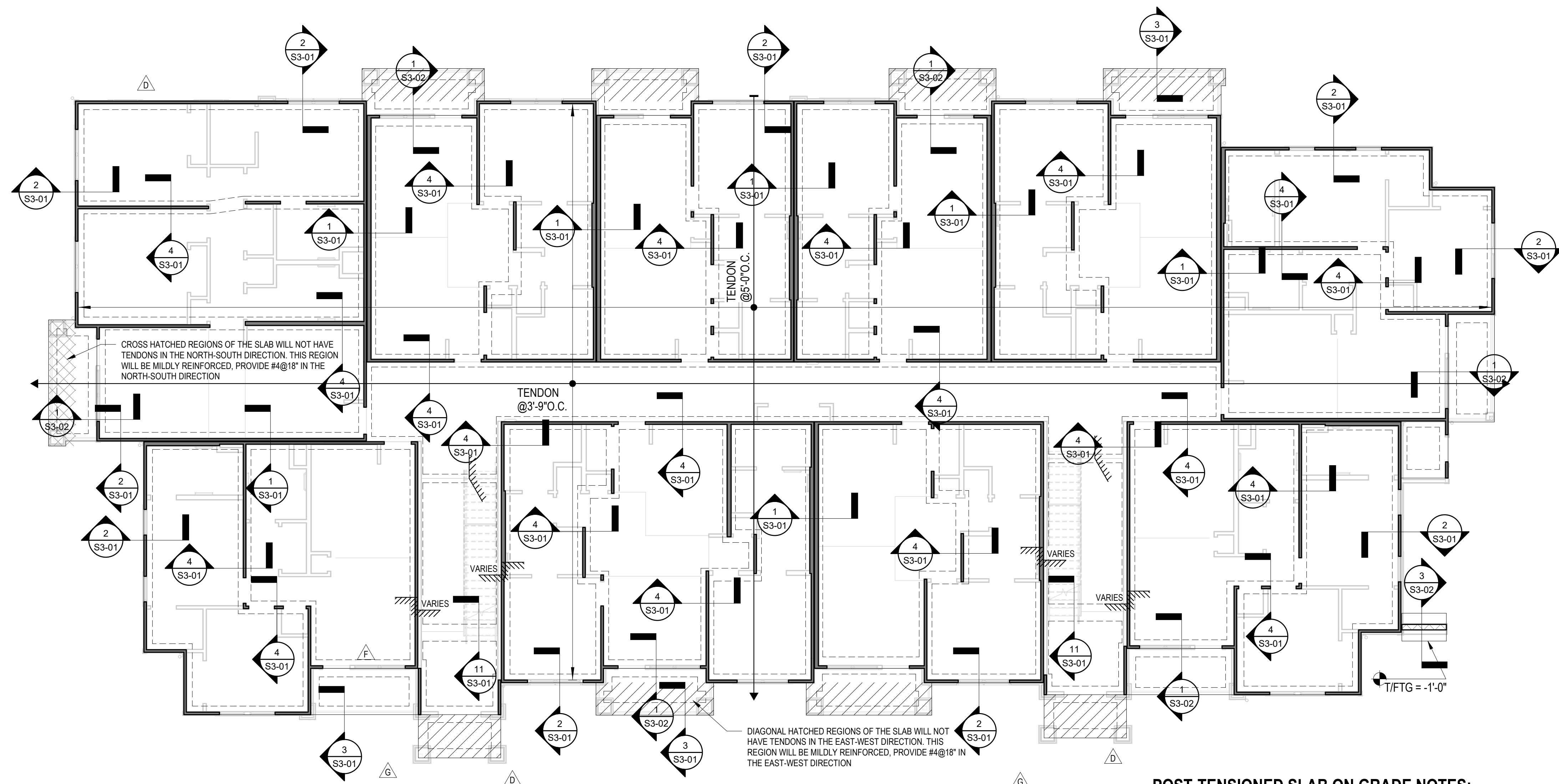
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SPREAD FOOTING SCHEDULE				
MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F3.0	3'-0"	3'-0"	12"	4#4 EA. WAY
F4.0	4'-0"	4'-0"	14"	4#5 EA. WAY

FOUNDATION NOTES:

- FOUNDATIONS ARE DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE, INTEGRAL THICKENED SLAB ELEMENTS AND INTEGRAL SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR NEWLY PLACED COMPACTED, ENGINEERED FILL.
- SEE SHEETS S3-0# FOR FOUNDATION DETAILS.
- XXXXX DENOTES CMU WALL. SEE SHEETS S4-20 FOR CMU WALL DETAILS.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.
- TYPICAL SLAB ON GRADE SHALL BE A POST-TENSIONED, 4" THICK (MINIMUM) CONCRETE SLAB.
- SEE ARCH. DRAWINGS FOR ALL SLAB SLOPES AND CRICKETS FOR DRAINAGE.
- WHERE SLAB ON GRADE MUST SLOPE TO DRAIN, CONTRACTOR TO THICKEN SLAB AS REQUIRED TO MAINTAIN 4" MINIMUM THICKNESS, TYP. SEE SHEETS S3-0# AND GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
- PROVIDE ONE (1) LAYER OF 10 MIL VAPOR BARRIER UNDER THE SLAB ON GRADE, TYP.
- ALL FOOTINGS TO BE CENTERED UNDER WALLS, COLUMNS, BEAM BEARING, PIERS AND PILASTERS, UNLESS NOTED OTHERWISE.
- ALL SPREAD FOOTINGS SUPPORTING WOOD COLUMNS SHALL BE POURED MONOLITHICALLY WITH THE SLAB ON GRADE. SEE DETAILS S6-01 FOR ADDITIONAL INFORMATION.

- F5.0 INDICATES FOOTING MARK. SEE SCHEDULE.
  - T/FTG= INDICATES TOP OF FOOTING ELEVATION.
- SEE ARCH. FOR TRASH ENCLOSURE, COMPACTOR, AND TRANSFORMER/EQUIPMENT PADS. UNLESS NOTED OTHERWISE ON PLANS, EQUIPMENT PADS SHALL BE 6" THICK REINFORCED CONCRETE SLAB ON GRADE W/ #4@12" E.W. CENTERED IN SLAB.
- CONCRETE SIDEWALKS AND STOOPS NOT WITHIN SCOPE OF STRUCTURAL DRAWINGS. SEE CIVIL/LANDSCAPE DRAWINGS, TYP.
- DENOTES SLAB STEP.
- PROVIDE SIMPSON "CB, CBS, OR CBSQ" TYPE COLUMN BASES AT ALL FREE-STANDING ENGINEERED LUMBER COLUMNS. "FREE-STANDING" COLUMNS AND/OR STUD PACKS SHALL BE DEFINED AS THOSE COLUMNS/STUD PACKS THAT ARE NOT WITHIN THE PLAN OF A STUD WALL.
- SEE TYPICAL DETAILS ON S6-00C FOR THICKENED AREAS AT SHEAR WALL ENDS FOR ANCHOR EMBEDMENT.
- IT WILL BE NECESSARY TO INSTALL EMBED PLATES IN THE CONCRETE SLAB ON GRADE AT SHEAR WALL ANCHORAGE LOCATIONS. IF THE CONTRACTOR INTENDS TO USE A THREADED ROD SYSTEM FOR THE SHEAR WALL ANCHORAGE, SPECIFIC COORDINATION WILL BE REQUIRED TO DETERMINE LOCATIONS WHERE EMBED PLATES ARE REQUIRED PER THE STRUCTURAL ANCHORAGE DETAILS ON THE S6-00# SHEETS. IT IS RECOMMENDED THAT THE THREADED ROD SYSTEM SHOP DRAWINGS BE SUBMITTED AT A TIME WHERE THESE EMBED PLATES CAN BE COORDINATED WITH THE SLAB ON GRADE CONSTRUCTION.

POST-TENSIONED SLAB ON GRADE NOTES:

- FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE WITH INTEGRAL/MONOLITHIC THICKENED SLAB ELEMENTS AND/OR INTEGRAL SPREAD FOOTINGS.
- SLAB ON GRADE SHALL BE POST-TENSIONED, 4" THICK,  $f_c = 3,000$  PSI, ON PREPARED SUBGRADE, AND SHALL BE REINFORCED AS SHOWN ON THE POST-TENSIONED REINFORCING PLANS AND PER THE STRUCTURAL SECTIONS AND DETAILS. POST-TENSIONED SLAB ON GRADE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION JOINTS, TYP.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. SEE ARCHITECT'S DIMENSION CONTROL PLANS FOR ALL DIMENSIONS, DEPRESSIONS, SLOPES, ETC. NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DIMENSIONS SHOWN ON STRUCTURAL FOUNDATION PLANS ARE TO EDGE OF SLAB, CENTERLINE OF COLUMN, OR CENTERLINE OF THICKENED SLAB CONDITIONS, UNLESS NOTED OTHERWISE ON PLAN. COORDINATE DIMENSIONS W/ARCHITECTURAL DRAWINGS AND NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DENOTES DEAD OR ANCHORED END OF POST-TENSIONING TENDON.
- DENOTES LIVE OR STRESSING END OF POST-TENSIONING TENDON.
- ALL TENDONS SHALL BE 1/2" DIAMETER, 270K 7-STRAND WIRE LOW RELAXATION CABLE WITH PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING CONFORMING TO ASTM A-416.
- ALL TENDONS SHALL BE ANCHORED AT 28.9 K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33.0 K PER TENDON.
- FINAL TENDON FORCE SHALL BE 27.0 KIPS (AFTER LOSSES).
- ALL TENDONS SPANNING EAST/WEST SHALL HAVE A UNIFORM A DRAPE HEIGHT OF 1.75". ALL TENDONS SPANNING NORTH SOUTH SHALL HAVE A TENDON DRAPE HEIGHT OF 2.25"
- TENDON SPACING SHALL BE AS SHOWN ON THE POST-TENSION PLAN.
- THE FIRST TENDON (AT THE EDGE OF SLAB SHALL) BE LOCATED NO CLOSER THAN 0'-6" FROM EDGE OF SLAB AND NO FURTHER THAN 0'-8" FROM THE EDGE OF SLAB, TYP.
- SEE GENERAL NOTES (SECTION 5) FOR ADDITIONAL POST-TENSIONED SLAB ON GRADE REQUIREMENTS AND SPECIFICATIONS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL EXTERIOR SLAB AND CURB INFORMATION. SEE CIVIL PLANS FOR GRADE ELEVATION AT EXTERIOR PERIMETER.
- FINISH GRADE SHOULD BE 8" TO 12" BELOW FFE, SEE DETAILS ON S3-01.
- CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED BY THE GC AND THE PT SUPPLIER. CONSTRUCTION LOCATIONS SHALL BE APPROVED BY THE DESIGN TEAM.
- SEE FOUNDATION PLANS FOR SPREAD FOOTING LOCATIONS AND FOOTING SCHEDULE.
- SEE TYPICAL DETAILS FOR THICKENED SLAB AT STAIR LANDINGS.
- PROVIDE (2) #4 BAR AT MID DEPTH OF FOUNDATION SLAB AT ALL RE-ENTRANT CORNERS.
- IN SOME LOCATIONS, IT WILL BE NECESSARY TO INSTALL AN EMBED PLATE IN THE POST-TENSIONED CONCRETE SLAB ON GRADE. THE LOCATIONS WHERE THIS WILL BE NECESSARY MUST BE COORDINATED WITH THE SHEAR WALL BRACING PLANS (S6-0#), SHEAR WALL ANCHORAGE SCHEDULE (DETAIL S6-00A), AND EMBED PLATE DETAILS (S6-00E).

FOR CONSTRUCTION

FREMAUX PARK APARTMENTS

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

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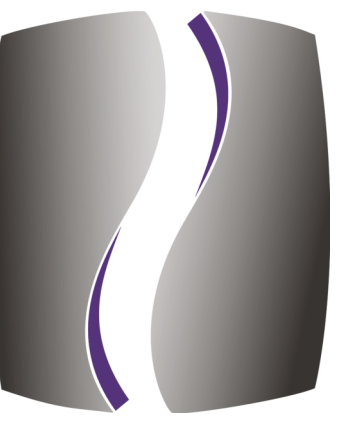
FOUNDATION PLAN - BUILDING A

SHEET NUMBER

S1-1.1

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

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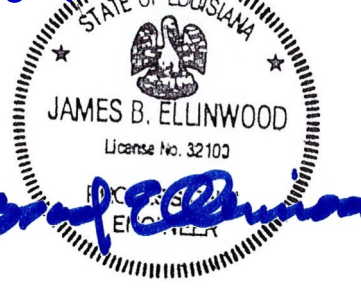


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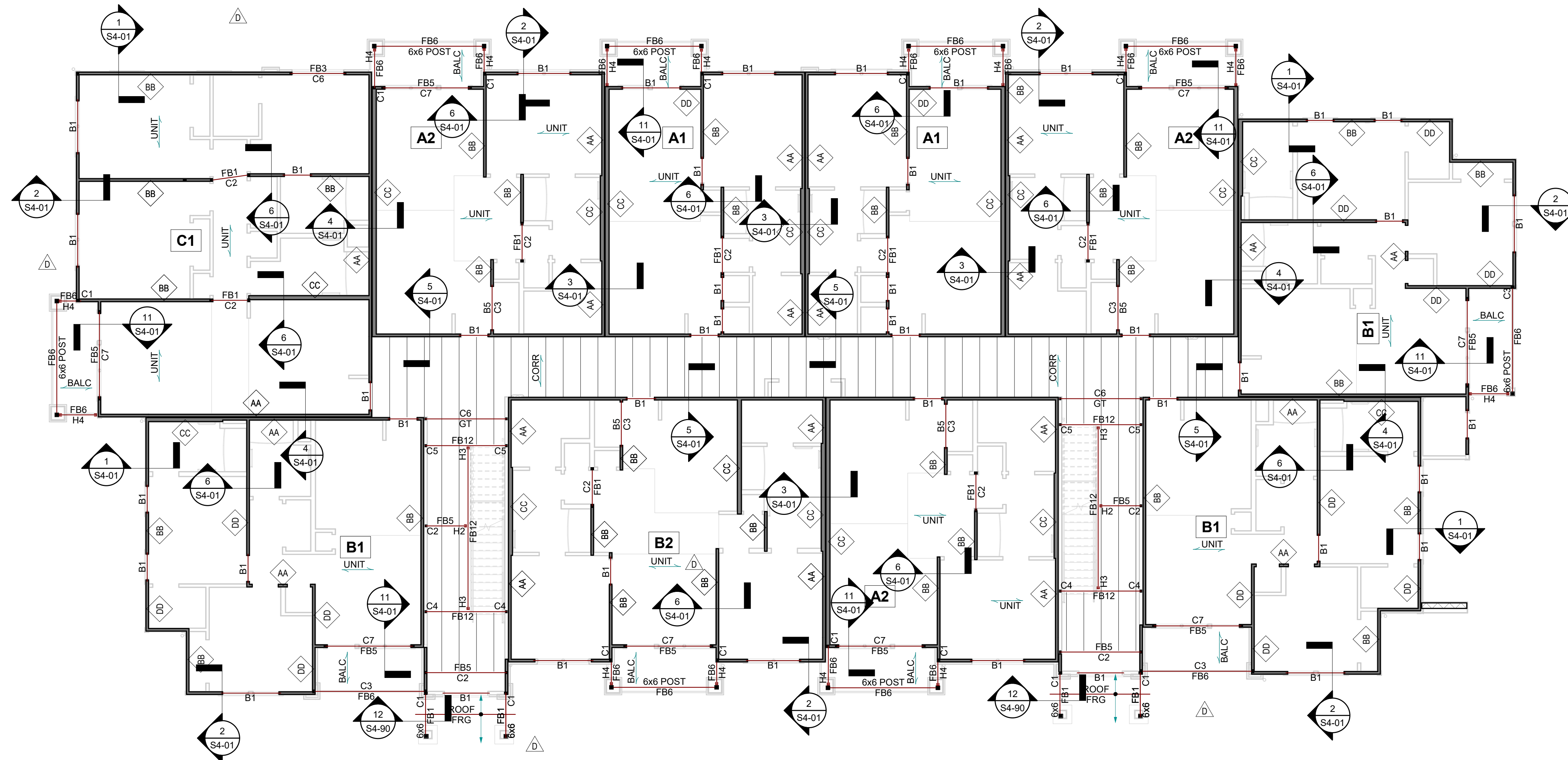
LEVEL 2 FRAMING PLAN  
- BUILDING A

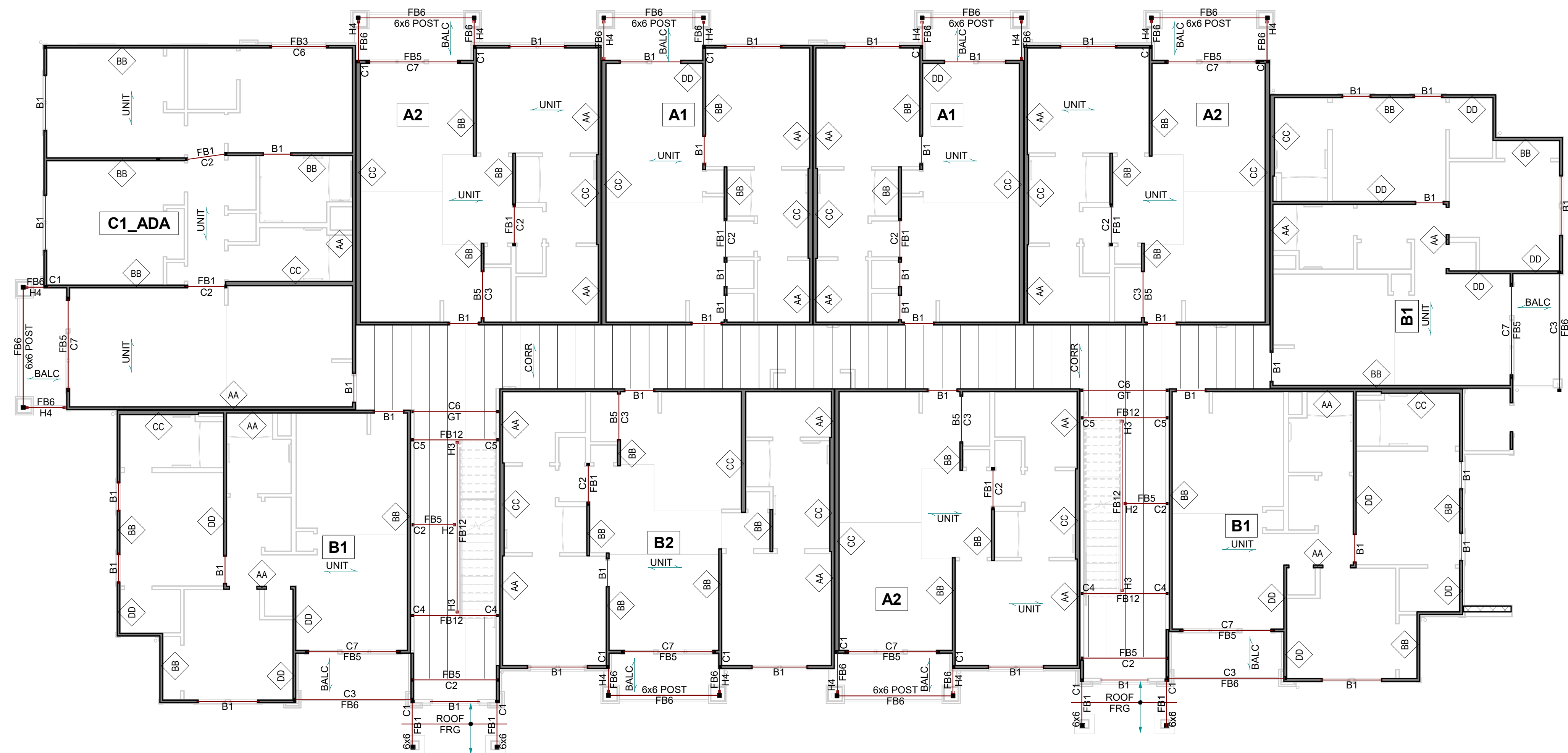
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S1-1.2

5901 PEACHTREE DUNWOODY RD.  
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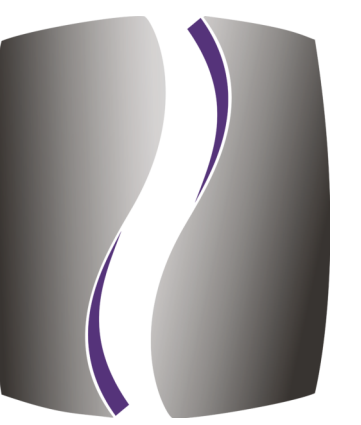
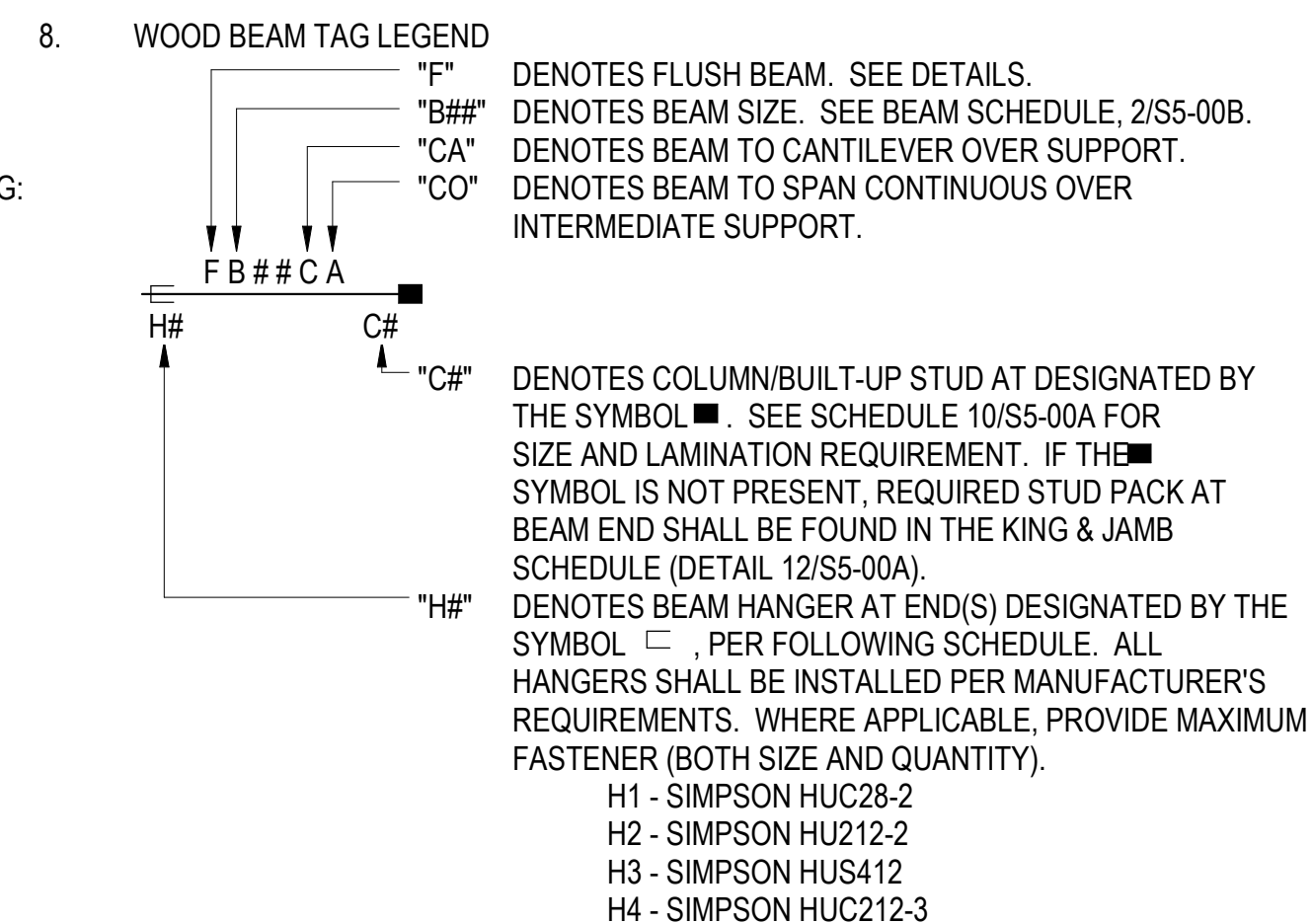
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**FLOOR FRAMING PLAN NOTES:**

- FLOOR FRAMING SHALL BE AS FOLLOWS:
  - UNIT DENOTES UNIT FRAMING, 18" OPEN-WEB TRUSSES @ 24" O.C. UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS
  - CORR DENOTES CORRIDOR FRAMING, 16" FOR CORRIDOR OPEN-WEB TRUSSES @ 24" O.C., UNLESS INDICATED OTHERWISE PER THE FOLLOWING HATCHING:
  - BALC DENOTES BALCONY FRAMING, 2x8 @ 16" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS.
  - STOR DENOTES STORAGE FRAMING, 18" OPEN-WEB TRUSSES @ 24" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS
- SHADED WALLS INDICATE LOAD BEARING WALLS BELOW.
- XX DENOTES WALL STUDS TO BE CONSTRUCTED OF WALL TYPE "XX". REFERENCE S5-00A FOR STUD SCHEDULE.
- USE PRESSURE TREATED SAWN LUMBER AND WOLMANIZED ENGINEERED LUMBER AT ALL WOOD FRAMING EXPOSED TO WEATHER.
- XX DENOTES C.M.U. WALL - SEE SHEET S4-20 FOR ADD'L INFO.
- SEE KING & JAMB SCHEDULE (DETAIL 8/S5-00B) FOR TYPICAL NUMBER OF STUDS REQUIRED AT HEADER BEAMS.
- SEE S4-0# SERIES DRAWINGS FOR TYPICAL FLOOR FRAMING DETAILS AND INFORMATION.



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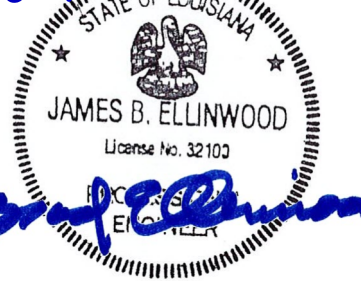


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**LEVEL 2 FRAMING PLAN - BUILDING A**

SHEET NUMBER

**S1-1.2B**

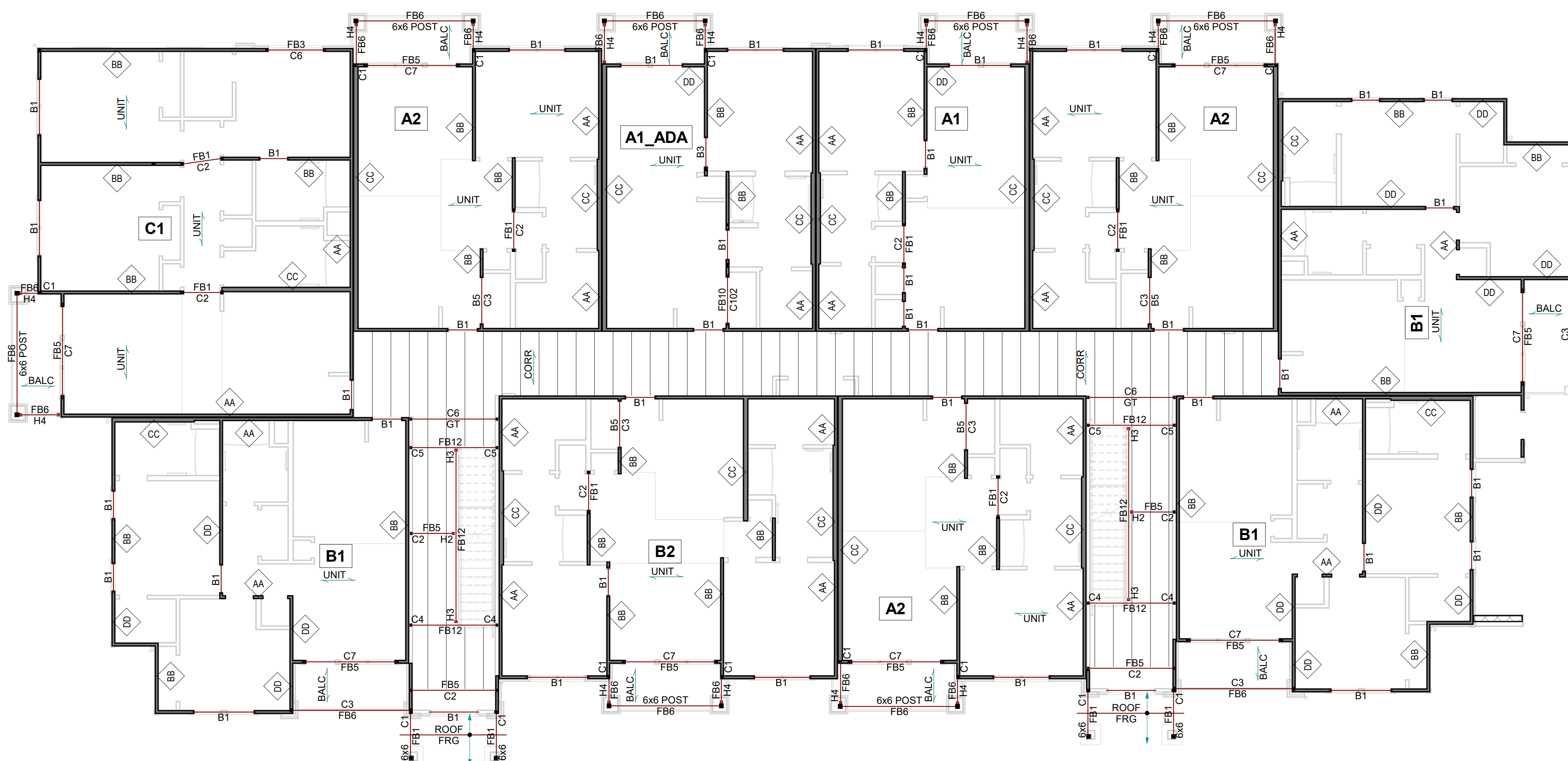
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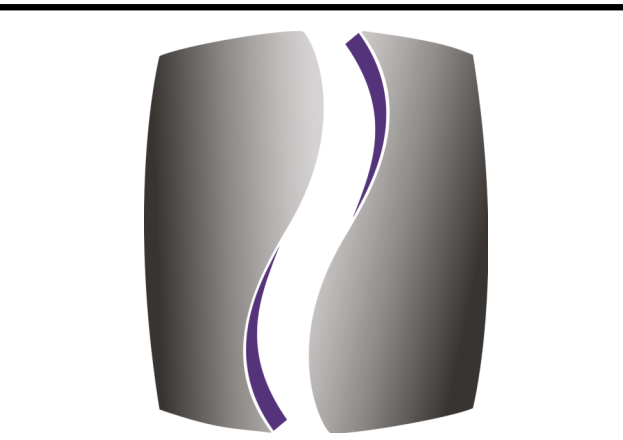
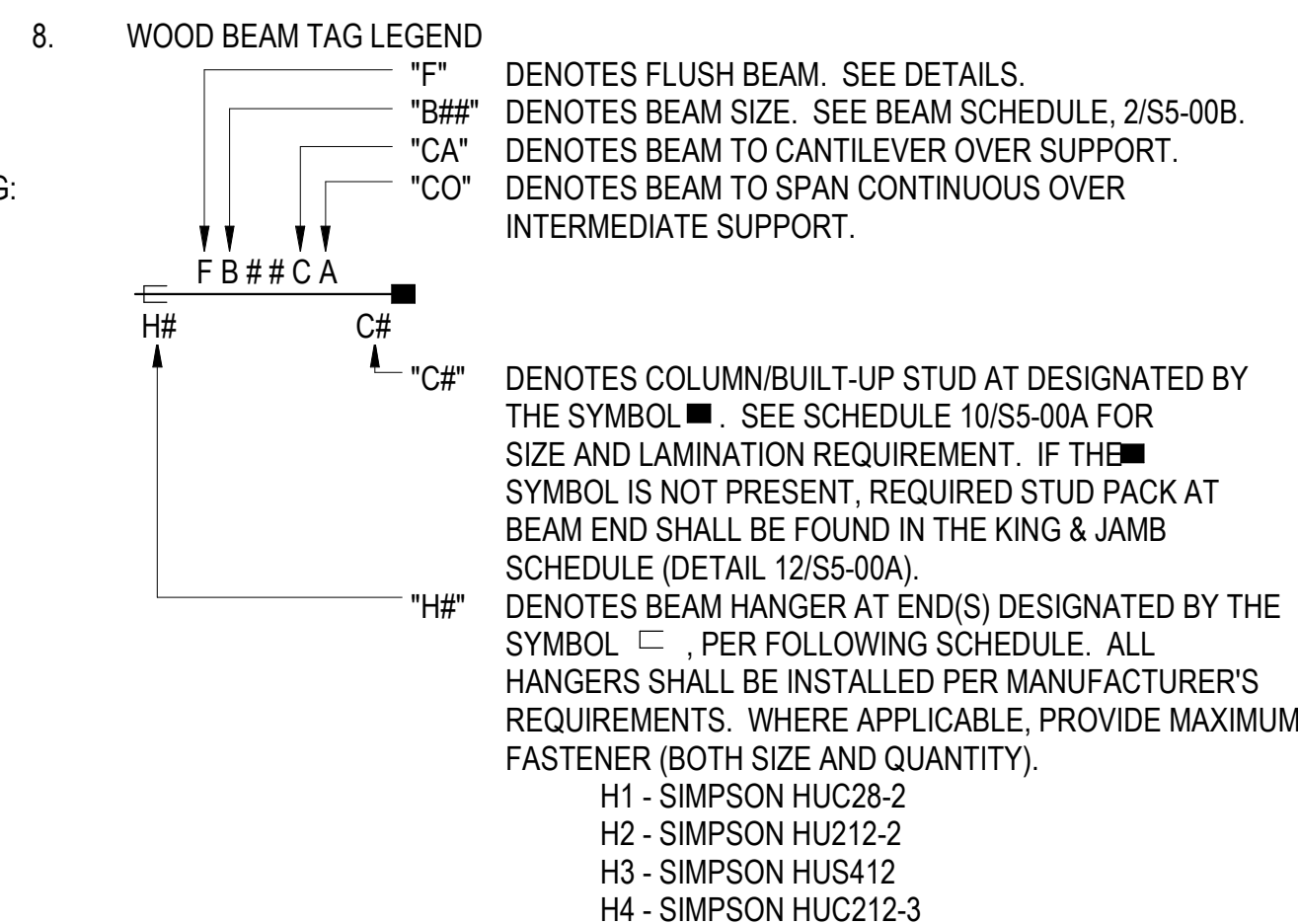
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**FLOOR FRAMING PLAN NOTES:**

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  - CORR DENOTES CORRIDOR FRAMING, 16" FOR CORRIDOR OPEN-WEB TRUSSES @ 24" O.C., UNLESS INDICATED OTHERWISE PER THE FOLLOWING HATCHING:
  - BALC DENOTES BALCONY FRAMING, 2x8 @ 16" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS.
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- SHADED WALLS INDICATE LOAD BEARING WALLS BELOW.
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- XX DENOTES C.M.U. WALL - SEE SHEET S4-20 FOR ADD'L INFO.
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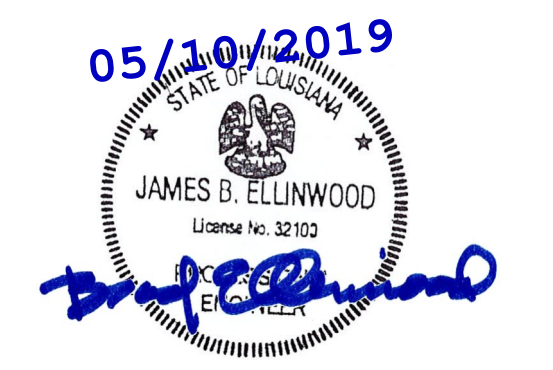
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**ISSUE & REVISION RECORD**

#	DATE	DESCRIPTION
G	03/22/2019	CONSTRUCTION RELEASE
H	05/10/2019	ISSUED FOR CONSTRUCTION

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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER \_\_\_\_\_  
ISSUE DATE \_\_\_\_\_ 1867709  
DRAWN BY/CHECKED BY \_\_\_\_\_ 05/10/2019  
APPROVED BY \_\_\_\_\_ DKJ / RAFAEL, RIM  
BRAD, JBE

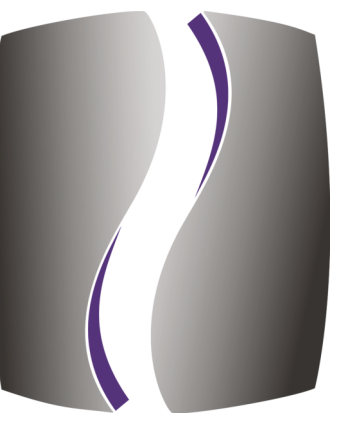
**LEVEL 2 FRAMING PLAN - BUILDING A**

SHEET NUMBER \_\_\_\_\_

**S1-1.2C**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328  
PHILLIPSPART.COM 770-394-1616

**FOR CONSTRUCTION**



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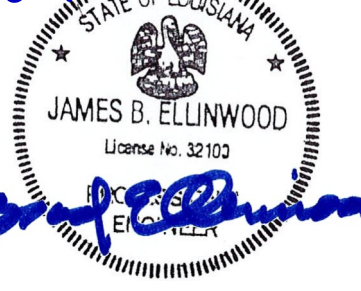


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PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

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DRAWING TITLE

LEVEL 3 FRAMING PLAN  
- BUILDING A

SHEET NUMBER

S1-1.3

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

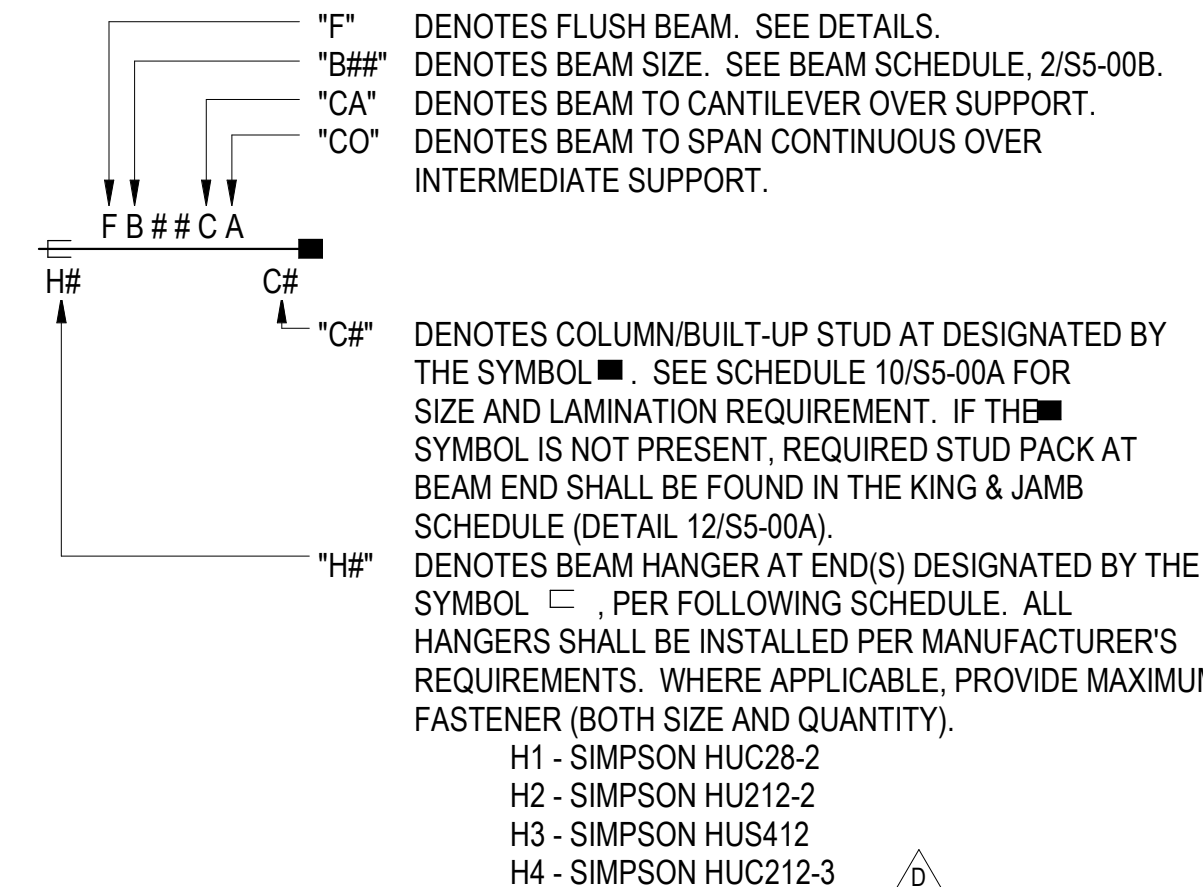
PHILLIPSPART.COM 770-394-1616



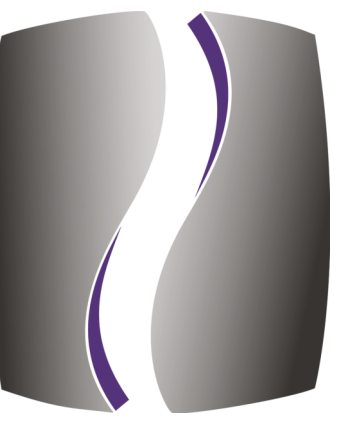
FLOOR FRAMING PLAN NOTES:

- FLOOR FRAMING SHALL BE AS FOLLOWS:
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  - CORR DENOTES CORRIDOR FRAMING, 16" FOR CORRIDOR OPEN-WEB TRUSSES @ 24" O.C., UNLESS INDICATED OTHERWISE PER THE FOLLOWING HATCHING:
    - BALC DENOTES BALCONY FRAMING, 2x8 @ 16" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS.
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- SHADED WALLS INDICATE LOAD BEARING WALLS BELOW.
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- XX DENOTES C.M.U. WALL - SEE SHEET S4-20 FOR ADD'L INFO.
- SEE KING & JAMB SCHEDULE (DETAIL 8/S5-00B) FOR TYPICAL NUMBER OF STUDS REQUIRED AT HEADER BEAMS.
- SEE S4-0# SERIES DRAWINGS FOR TYPICAL FLOOR FRAMING DETAILS AND INFORMATION.

WOOD BEAM TAG LEGEND



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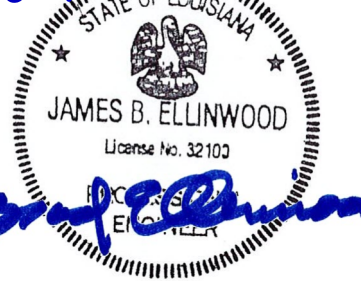


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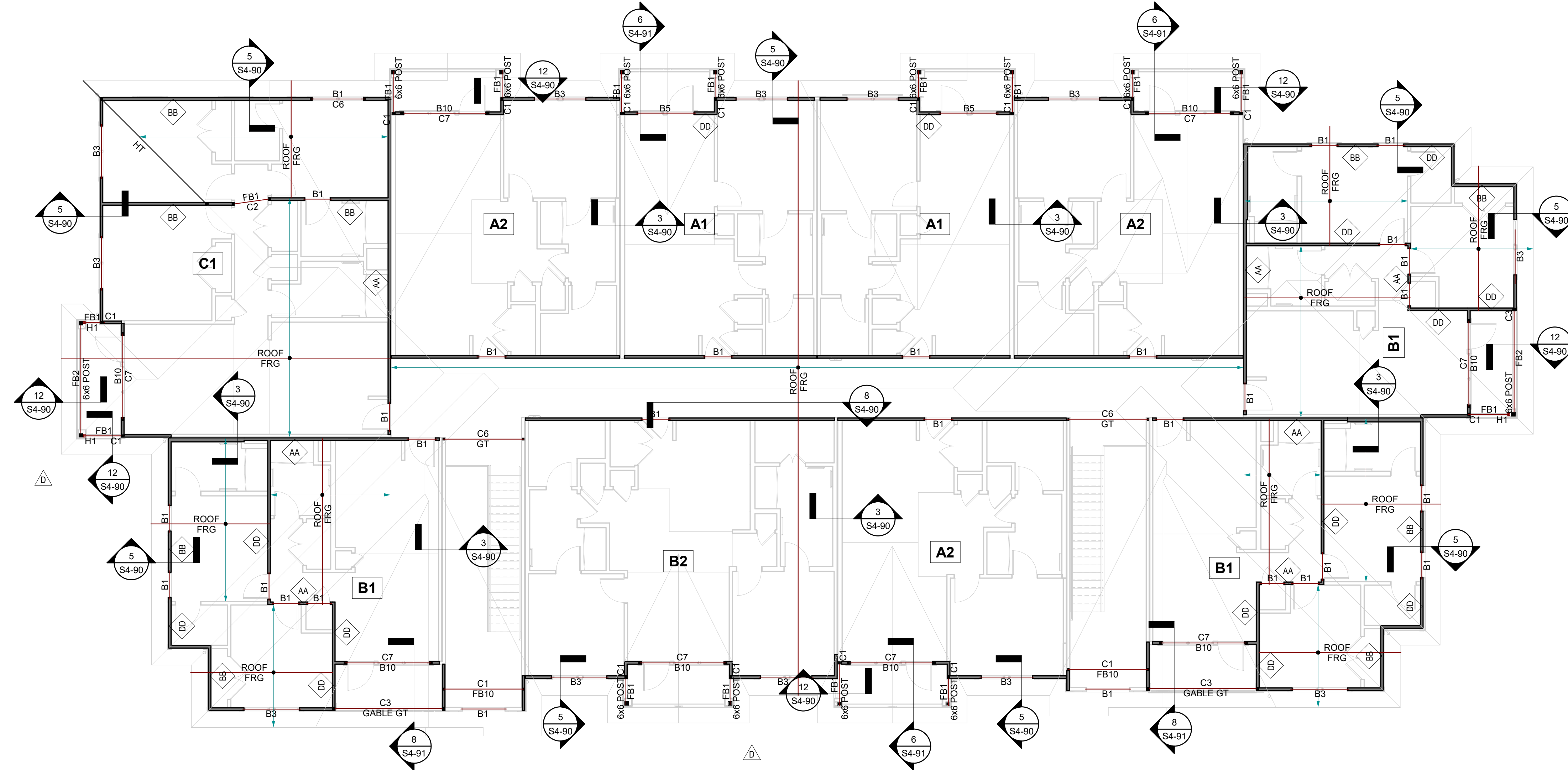
ROOF FRAMING PLAN - BUILDING A

SHEET NUMBER

S1-1.4

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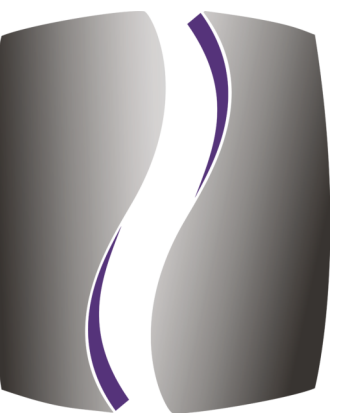
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ROOF FRAMING PLAN NOTES:

- SEE S4-9# SERIES FOR ROOF FRAMING SECTIONS AND DETAILS.
- ROOF FRG DENOTES PRE-ENGINEERED OPEN WEB ROOF TRUSSES @ 24" O.C. TRUSS DEPTH VARIES WITH SLOPE OF ROOF (TOP CHORD TO SLOPE AS REQD TO ACHIEVE ROOF SLOPES). SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- "GT" DENOTES A GIRDER TRUSS, TYP.
- "HT" DENOTES A HIP TRUSS, TYP.
- "DBL" DENOTES A DOUBLE TRUSS, TYP.
- "FB" DENOTES A FLUSH BEAM, TYP.
- ALL ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER, WHICH SHALL INCLUDE ALL TRUSS-TO-TRUSS CONNECTIONS. SEE GENERAL NOTES ON SHEET S0-01 AND S0-03 FOR ADDITIONAL INFORMATION.
- TRUSS MANUFACTURER SHALL DESIGN ALL ROOF TRUSS COMPONENTS FOR THE NET UPLIFT WIND PRESSURE RESULTING FROM THE WIND PRESSURES SHOWN IN DETAILS ON S0-020 TYP. SEE ROOF FRAMING PLANS AND DETAILS FOR ADD'L INFORMATION.
- ALIGN ROOF TRUSSES WITH OR PROVIDE ADDITIONAL ROOF TRUSSES ABOVE ALL TOP FLOOR SHEAR WALLS. SEE S6-## SERIES (BRACING PLANS) FOR SHEAR WALL LOCATIONS, TYP.
- ALL ROOF TRUSSES TO BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE TIE-DOWN ANCHORS FOR ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE POSTS/STUDPACKS BELOW ENDS OF ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) CONTINUOUS TO FOUNDATION, ACCORDING TO THE FOLLOWING CRITERIA:
  - FOR GT/DBL SPANS  $\leq 20'-0"$ , PROVIDE 2-2x4
  - FOR GT/DBL SPANS  $\leq 35'-0"$ , PROVIDE 4-2x4
  - FOR GT/DBL SPANS  $\leq 50'-0"$ , PROVIDE 5-2x4

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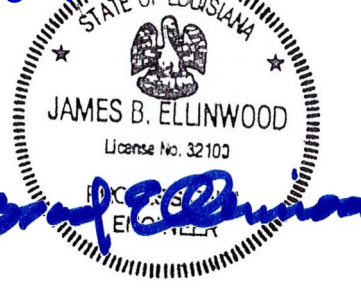


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FOUNDATION PLAN - BUILDING B

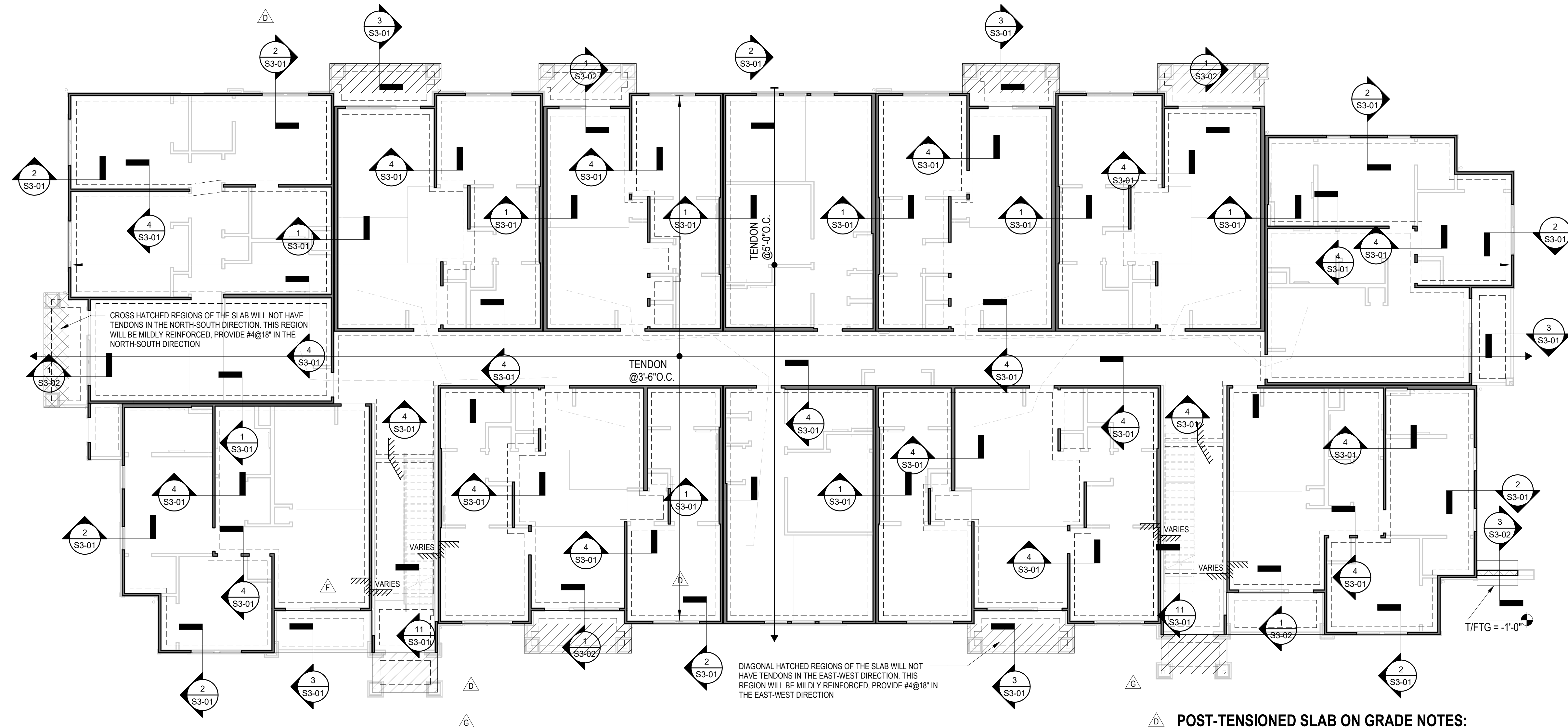
SHEET NUMBER

S1-2.1

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

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SPREAD FOOTING SCHEDULE				
MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F3.0	3'-0"	3'-0"	12"	4#4 EA. WAY
F4.0	4'-0"	4'-0"	14"	4#5 EA. WAY

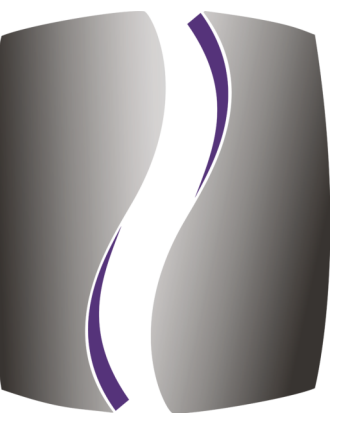
FOUNDATION NOTES:

- FOUNDATIONS ARE DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE, INTEGRAL THICKENED SLAB ELEMENTS AND INTEGRAL SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR NEWLY PLACED COMPACTED, ENGINEERED FILL.
- SEE SHEETS S3-0# FOR FOUNDATION DETAILS.
- XXXXX DENOTES CMU WALL. SEE SHEETS S4-20 FOR CMU WALL DETAILS.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.
- TYPICAL SLAB ON GRADE SHALL BE A POST-TENSIONED, 4" THICK (MINIMUM) CONCRETE SLAB.
- SEE ARCH. DRAWINGS FOR ALL SLAB SLOPES AND CRICKETS FOR DRAINAGE.
- WHERE SLAB ON GRADE MUST SLOPE TO DRAIN, CONTRACTOR TO THICKEN SLAB AS REQUIRED TO MAINTAIN 4" MINIMUM THICKNESS, TYP. SEE SHEETS S3-0# AND GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
- PROVIDE ONE (1) LAYER OF 10 MIL VAPOR BARRIER UNDER THE SLAB ON GRADE, TYP.
- ALL FOOTINGS TO BE CENTERED UNDER WALLS, COLUMNS, BEAM BEARING, PIERS AND PILASTERS, UNLESS NOTED OTHERWISE.
- ALL SPREAD FOOTINGS SUPPORTING WOOD COLUMNS SHALL BE POURED MONOLITHICALLY WITH THE SLAB ON GRADE. SEE DETAILS S/S3.01 FOR ADDITIONAL INFORMATION.

- F3.0  
 T/FTG= INDICATES TOP OF FOOTING ELEVATION.
- INDICATES FOOTING MARK. SEE SCHEDULE.
- SEE ARCH. FOR TRASH ENCLOSURE, COMPACTOR, AND TRANSFORMER/EQUIPMENT PADS. UNLESS NOTED OTHERWISE ON PLANS, EQUIPMENT PADS SHALL BE 6" THICK REINFORCED CONCRETE SLAB ON GRADE W/ #4@12" E.W. CENTERED IN SLAB.
- CONCRETE SIDEWALKS AND STOOPS NOT WITHIN SCOPE OF STRUCTURAL DRAWINGS. SEE CIVIL/LANDSCAPE DRAWINGS, TYP.
- DENOTES SLAB STEP.
- PROVIDE SIMPSON "CB, CBS, OR CBSQ" TYPE COLUMN BASES AT ALL FREE-STANDING ENGINEERED LUMBER COLUMNS. "FREE-STANDING" COLUMNS AND/OR STUD PACKS SHALL BE DEFINED AS THOSE COLUMNS/STUD PACKS THAT ARE NOT WITHIN THE PLAN OF A STUD WALL.
- SEE TYPICAL DETAILS ON S6-00C FOR THICKENED AREAS AT SHEAR WALL ENDS FOR ANCHOR EMBEDMENT.
- IT WILL BE NECESSARY TO INSTALL EMBED PLATES IN THE CONCRETE SLAB ON GRADE AT SHEAR WALL ANCHORAGE LOCATIONS. IF THE CONTRACTOR INTENDS TO USE A THREADED ROD SYSTEM FOR THE SHEAR WALL ANCHORAGE, SPECIFIC COORDINATION WILL BE REQUIRED TO DETERMINE LOCATIONS WHERE EMBED PLATES ARE REQUIRED PER THE STRUCTURAL ANCHORAGE DETAILS ON THE S6-00# SHEETS. IT IS RECOMMENDED THAT THE THREADED ROD SYSTEM SHOP DRAWINGS BE SUBMITTED AT A TIME WHERE THESE EMBED PLATES CAN BE COORDINATED WITH THE SLAB ON GRADE CONSTRUCTION.

POST-TENSIONED SLAB ON GRADE NOTES:

- FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE WITH INTEGRAL/MONOLITHIC THICKENED SLAB ELEMENTS AND/OR INTEGRAL SPREAD FOOTINGS.
- SLAB ON GRADE SHALL BE POST-TENSIONED, 4" THICK,  $f_c = 3,000$  PSI, ON PREPARED SUBGRADE, AND SHALL BE REINFORCED AS SHOWN ON THE POST-TENSIONED REINFORCING PLANS AND PER THE STRUCTURAL SECTIONS AND DETAILS. POST-TENSIONED SLAB ON GRADE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION JOINTS, TYP.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. SEE ARCHITECT'S DIMENSION CONTROL PLANS FOR ALL DIMENSIONS, DEPRESSIONS, SLOPES, ETC. NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DIMENSIONS SHOWN ON STRUCTURAL FOUNDATION PLANS ARE TO EDGE OF SLAB, CENTERLINE OF COLUMN, OR CENTERLINE OF THICKENED SLAB CONDITIONS, UNLESS NOTED OTHERWISE ON PLAN. COORDINATE DIMENSIONS W/ARCHITECTURAL DRAWINGS AND NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DENOTES DEAD OR ANCHORED END OF POST-TENSIONING TENDON.
- DENOTES LIVE OR STRESSING END OF POST-TENSIONING TENDON.
- ALL TENDONS SHALL BE 1/2" DIAMETER, 270K 7-STRAND WIRE LOW RELAXATION CABLE WITH PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING CONFORMING TO ASTM A-416.
- ALL TENDONS SHALL BE ANCHORED AT 28.9 K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33.0 K PER TENDON.
- FINAL TENDON FORCE SHALL BE 27.0 KIPS (AFTER LOSSES).
- ALL TENDONS SPANNING EAST/WEST SHALL HAVE A UNIFORM A DRAPE HEIGHT OF 1.75". ALL TENDONS SPANNING NORTH SOUTH SHALL HAVE A TENDON DRAPE HEIGHT OF 2.25"
- TENDON SPACING SHALL BE AS SHOWN ON THE POST-TENSION PLAN.
- THE FIRST TENDON (AT THE EDGE OF SLAB SHALL) BE LOCATED NO CLOSER THAN 0'-6" FROM EDGE OF SLAB AND NO FURTHER THAN 0'-8" FROM THE EDGE OF SLAB, TYP.
- SEE GENERAL NOTES (SECTION 5) FOR ADDITIONAL POST-TENSIONED SLAB ON GRADE REQUIREMENTS AND SPECIFICATIONS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL EXTERIOR SLAB AND CURB INFORMATION. SEE CIVIL PLANS FOR GRADE ELEVATION AT EXTERIOR PERIMETER.
- FINISH GRADE SHOULD BE 8" TO 12" BELOW FFE. SEE DETAILS ON S3-01.
- CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED BY THE GC AND THE PT SUPPLIER. CONSTRUCTION LOCATIONS SHALL BE APPROVED BY THE DESIGN TEAM.
- SEE FOUNDATION PLANS FOR SPREAD FOOTING LOCATIONS AND FOOTING SCHEDULE.
- SEE TYPICAL DETAILS FOR THICKENED SLAB AT STAIR LANDINGS.
- PROVIDE (2) #4 BAR AT MID DEPTH OF FOUNDATION SLAB AT ALL RE-ENTRANT CORNERS.
- IN SOME LOCATIONS, IT WILL BE NECESSARY TO INSTALL AN EMBED PLATE IN THE POST-TENSIONED CONCRETE SLAB ON GRADE. THE LOCATIONS WHERE THIS WILL BE NECESSARY MUST BE COORDINATED WITH THE SHEAR WALL BRACING PLANS (S6-0#), SHEAR WALL ANCHORAGE SCHEDULE (DETAIL 8/S6-00A), AND EMBED PLATE DETAILS (S6-00E).



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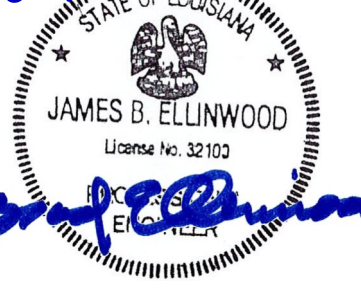


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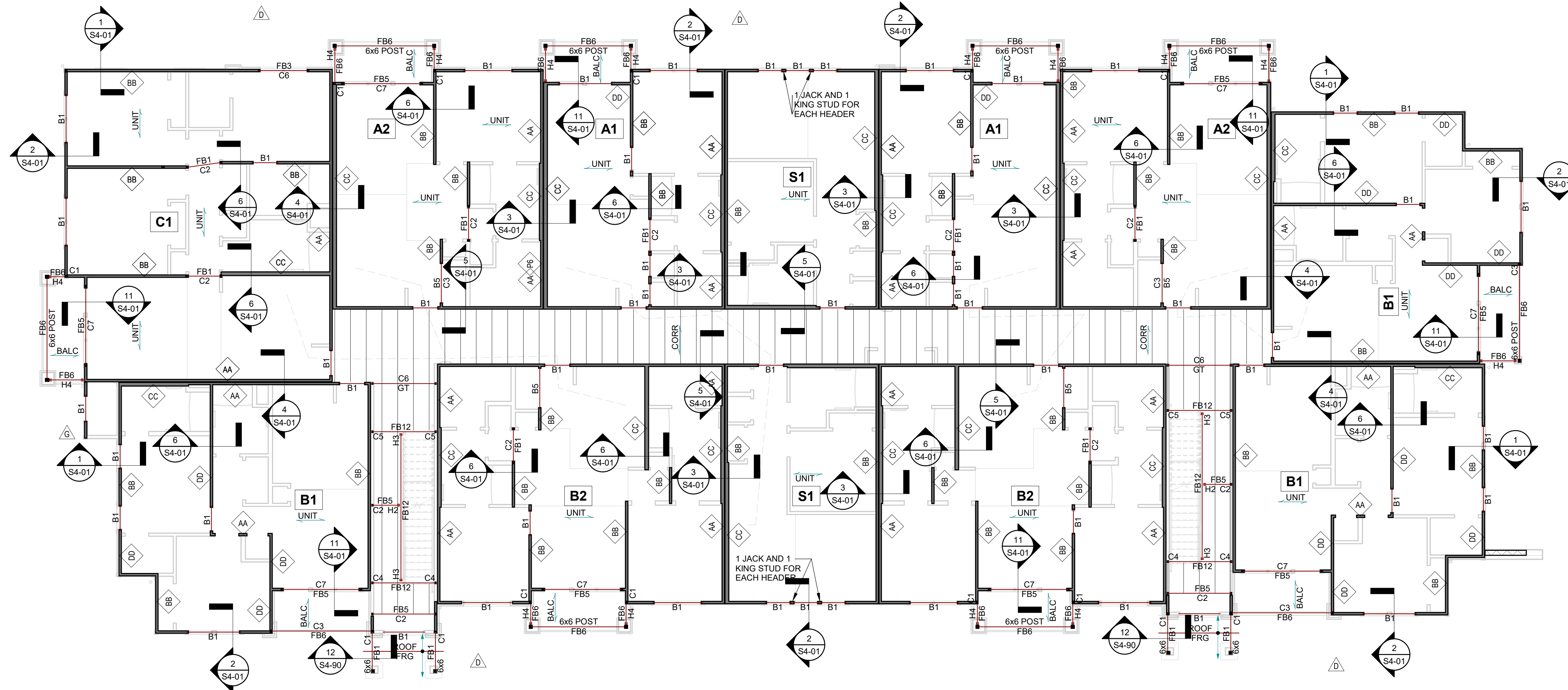
LEVEL 2 FRAMING PLAN  
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SHEET NUMBER

S1-2.2

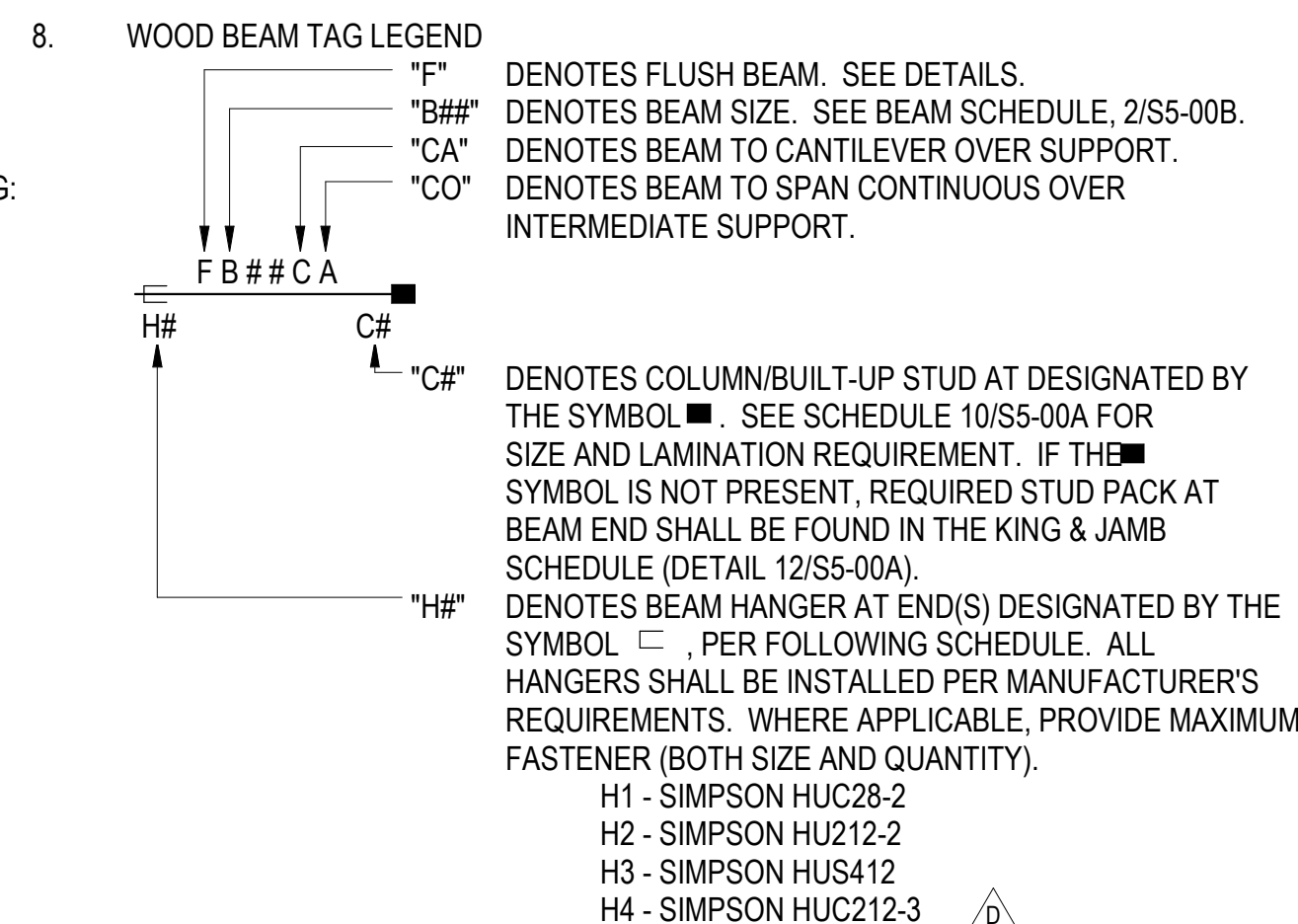
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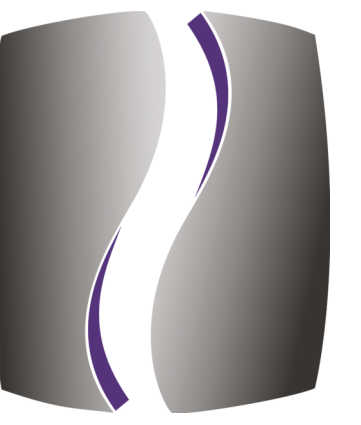


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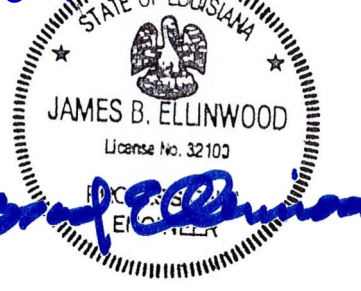


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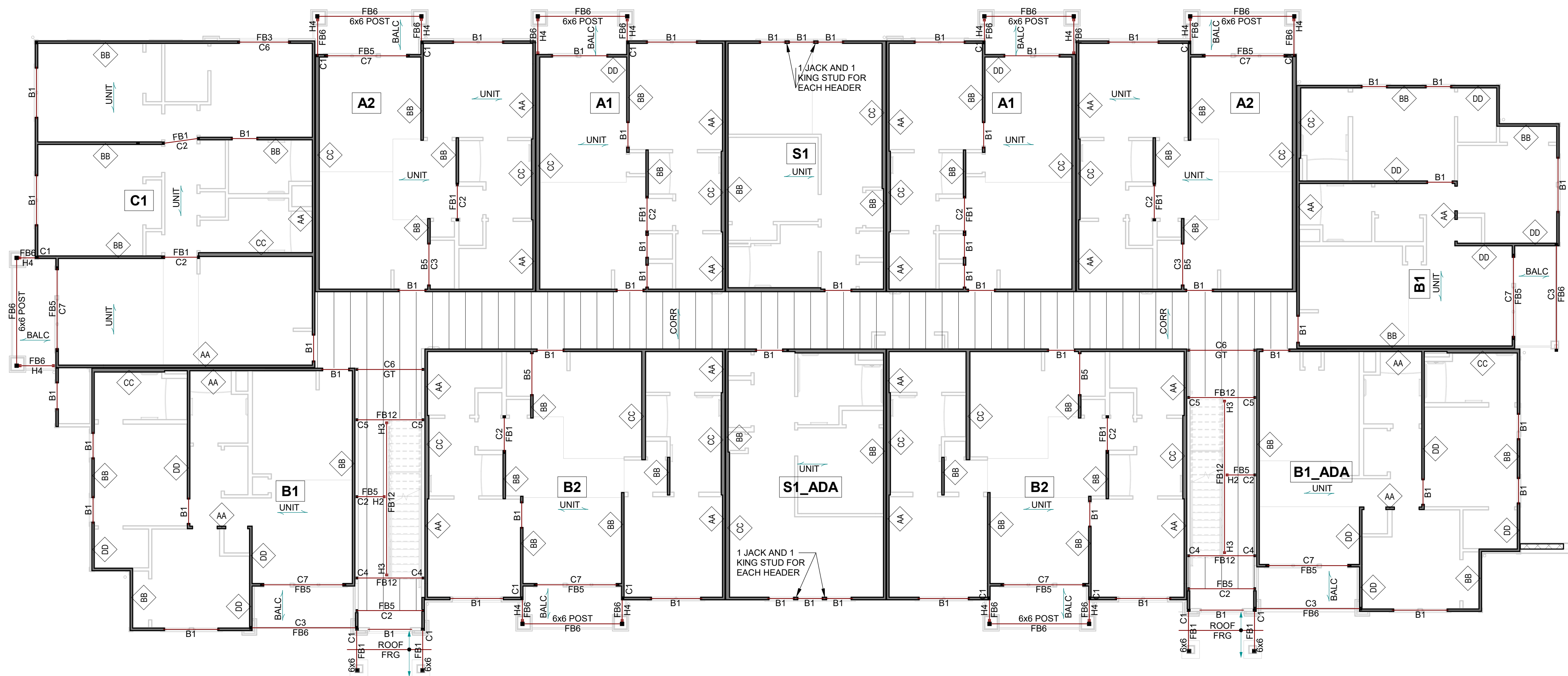
LEVEL 2 FRAMING PLAN  
- BUILDING B

SHEET NUMBER

S1-2.2B

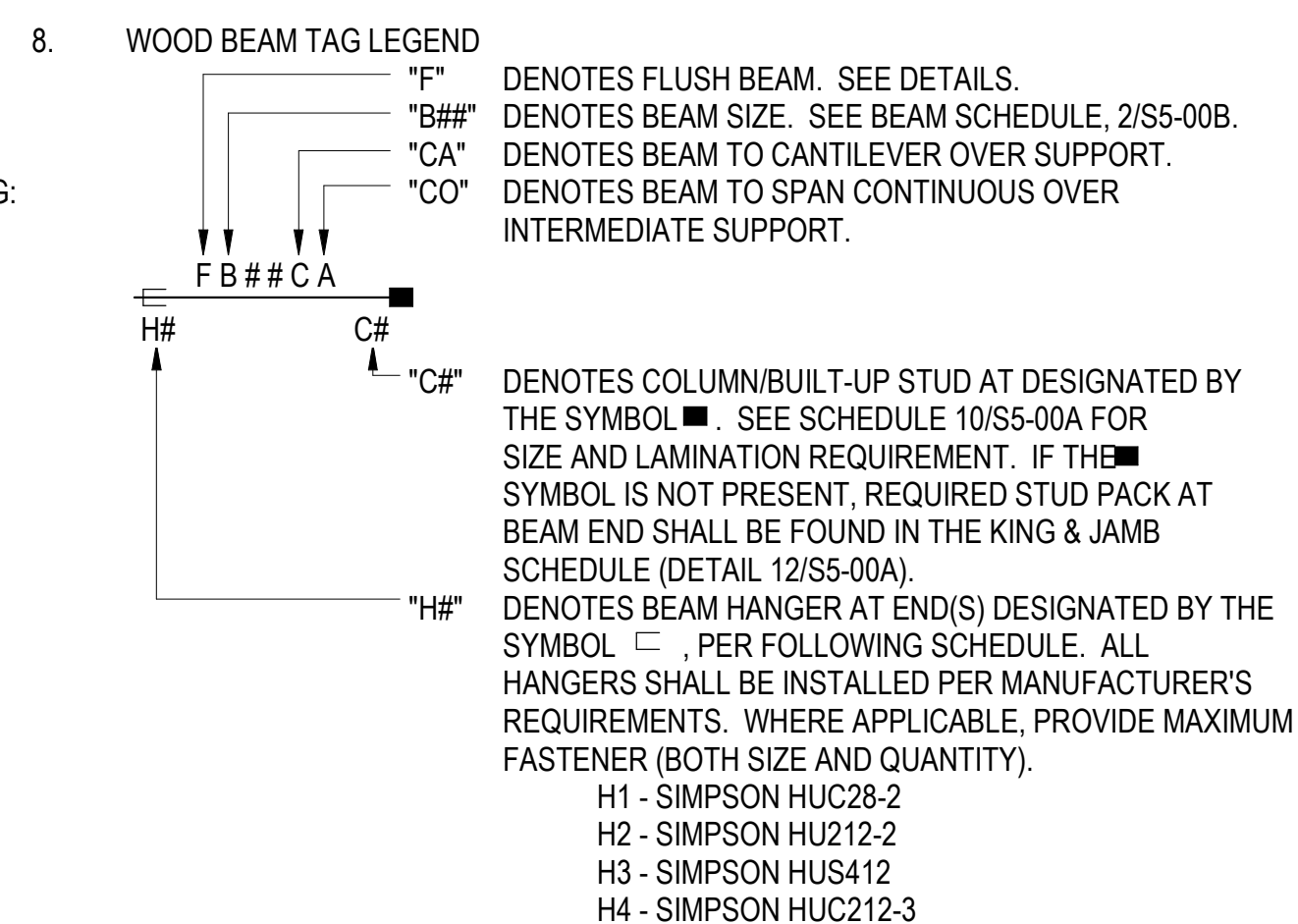
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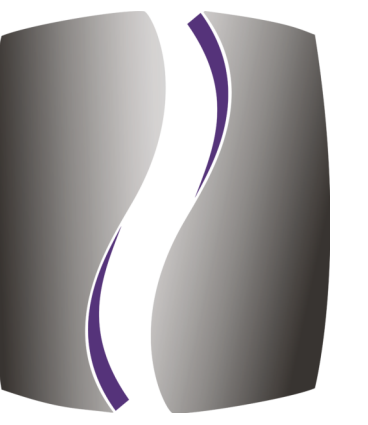


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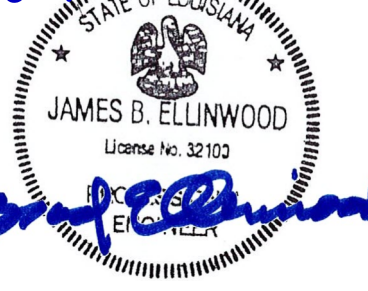


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PROJECT

FREMAUX PARK  
APARTMENTS

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

DRAWING TITLE

LEVEL 2 FRAMING PLAN  
- BUILDING B

SHEET NUMBER

S1-2.2C

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

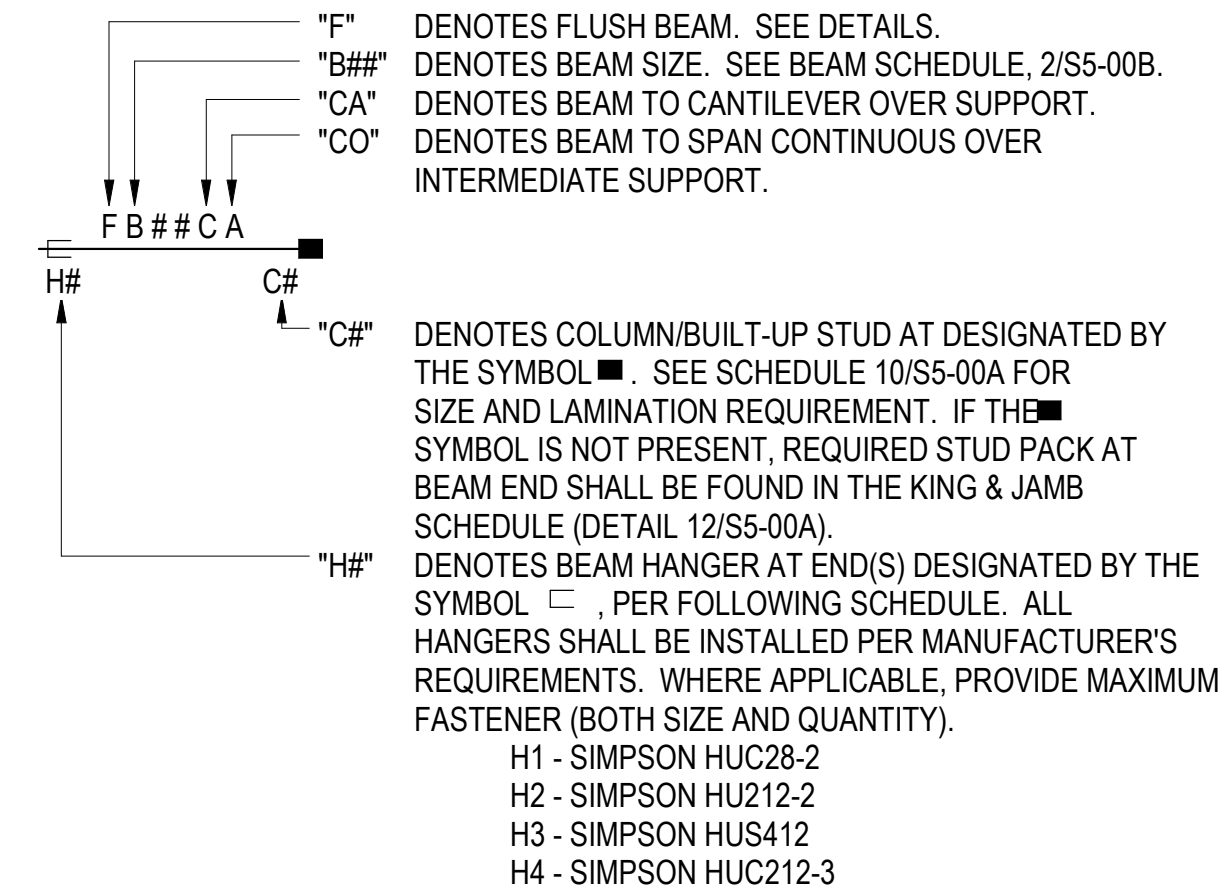
PHILLIPSPART.COM 770-394-1616



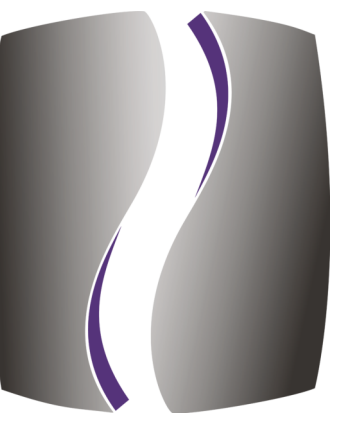
FLOOR FRAMING PLAN NOTES:

- FLOOR FRAMING SHALL BE AS FOLLOWS:
  - UNIT DENOTES UNIT FRAMING, 18" OPEN-WEB TRUSSES @ 24" O.C. UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS
  - CORR DENOTES CORRIDOR FRAMING, 16" FOR CORRIDOR OPEN-WEB TRUSSES @ 24" O.C., UNLESS INDICATED OTHERWISE PER THE FOLLOWING HATCHING:
  - BALC DENOTES BALCONY FRAMING, 2x8 @ 16" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS.
  - STOR DENOTES STORAGE FRAMING, 18" OPEN-WEB TRUSSES @ 24" O.C., UNLESS NOTED OTHERWISE ON BUILDING FRAMING PLANS
- SHADED WALLS INDICATE LOAD BEARING WALLS BELOW.
- XX DENOTES WALL STUDS TO BE CONSTRUCTED OF WALL TYPE "XX". REFERENCE S5-00A FOR STUD SCHEDULE.
- USE PRESSURE TREATED SAWN LUMBER AND WOLMANIZED ENGINEERED LUMBER AT ALL WOOD FRAMING EXPOSED TO WEATHER.
- XX DENOTES C.M.U. WALL - SEE SHEET S4-20 FOR ADD'L INFO.
- SEE KING & JAMB SCHEDULE (DETAIL 8/S5-00B) FOR TYPICAL NUMBER OF STUDS REQUIRED AT HEADER BEAMS.
- SEE S4-0# SERIES DRAWINGS FOR TYPICAL FLOOR FRAMING DETAILS AND INFORMATION.

WOOD BEAM TAG LEGEND



FOR CONSTRUCTION



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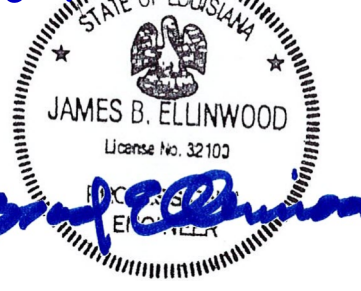


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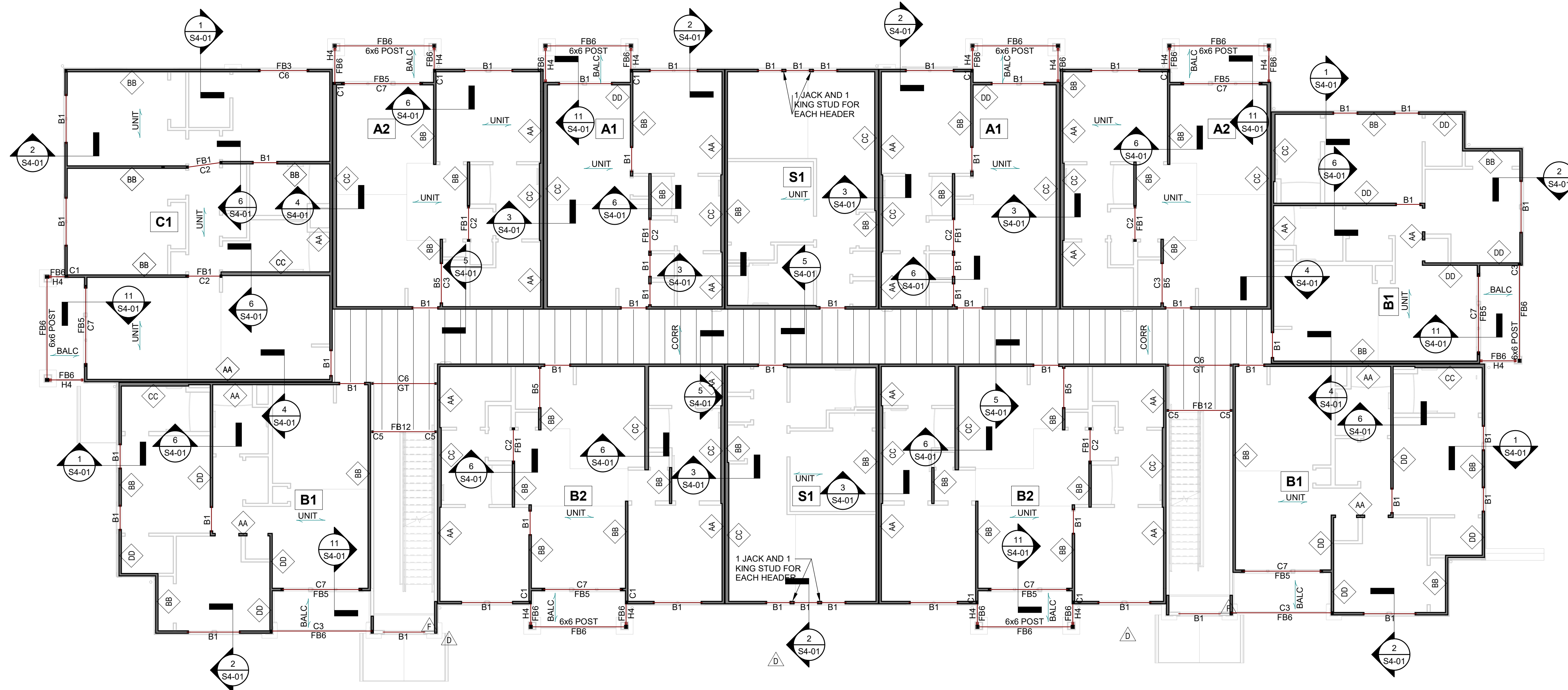
LEVEL 3 FRAMING PLAN  
- BUILDING B

SHEET NUMBER

S1-2.3

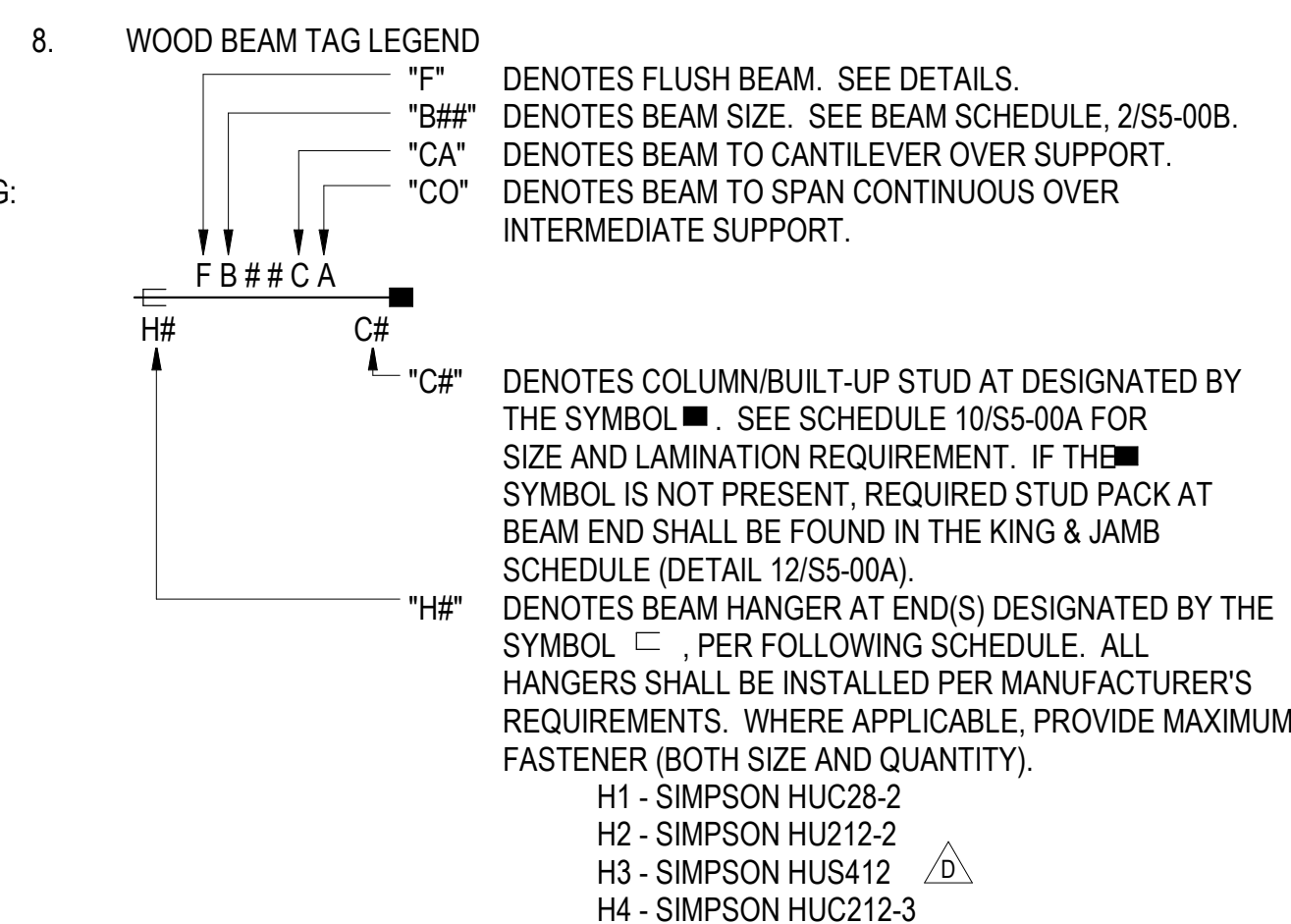
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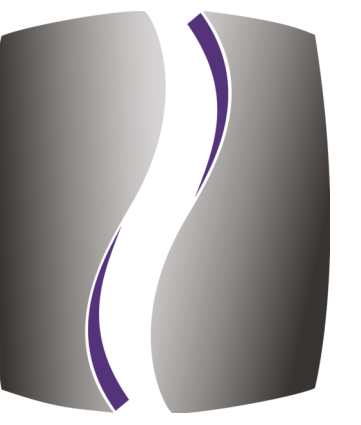


FLOOR FRAMING PLAN NOTES:

- FLOOR FRAMING SHALL BE AS FOLLOWS:
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- SEE KING & JAMB SCHEDULE (DETAIL 8/S5-00B) FOR TYPICAL NUMBER OF STUDS REQUIRED AT HEADER BEAMS.
- SEE S4-0# SERIES DRAWINGS FOR TYPICAL FLOOR FRAMING DETAILS AND INFORMATION.



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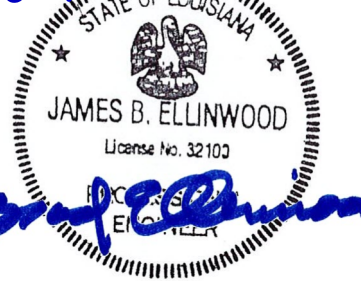


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BRAD, JBE

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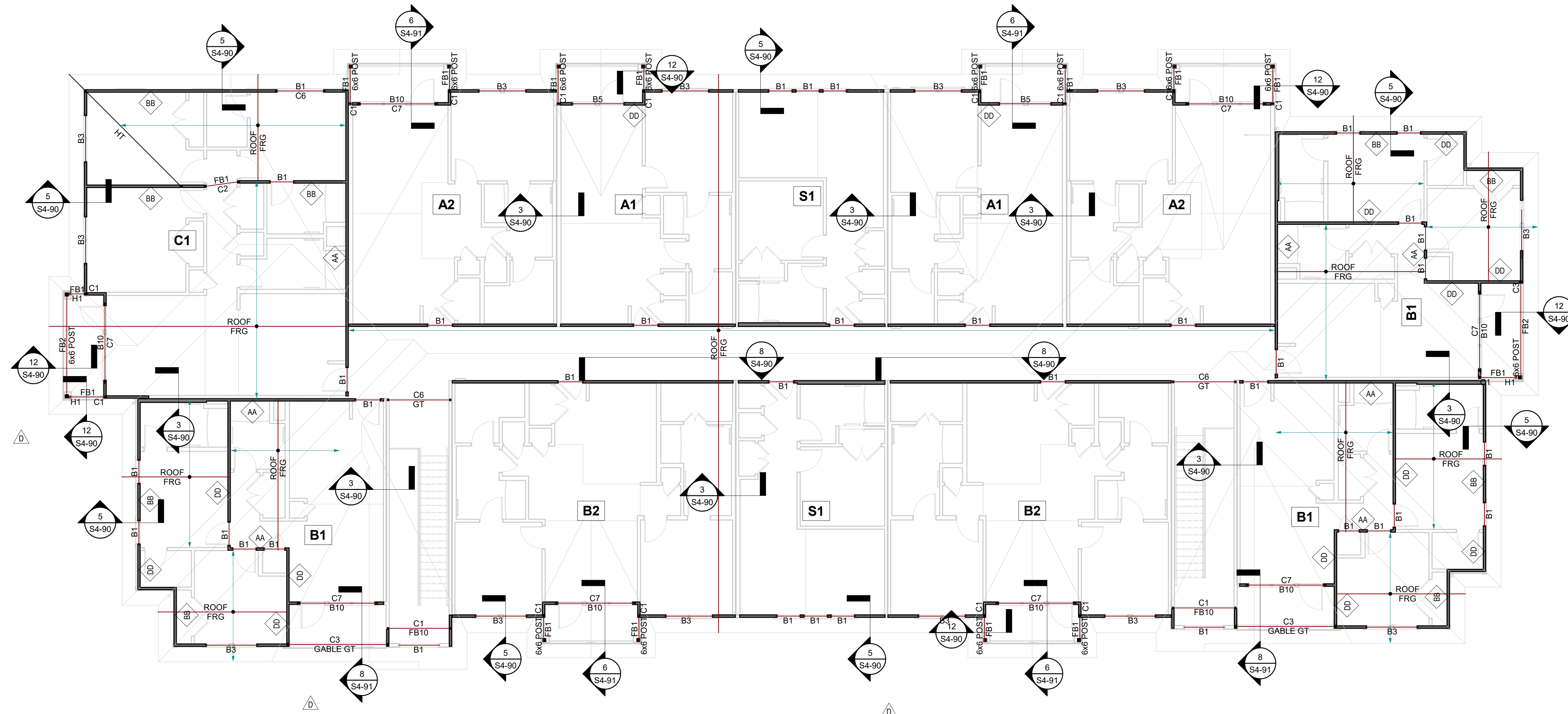
ROOF FRAMING PLAN -  
BUILDING B

SHEET NUMBER

S1-2.4

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

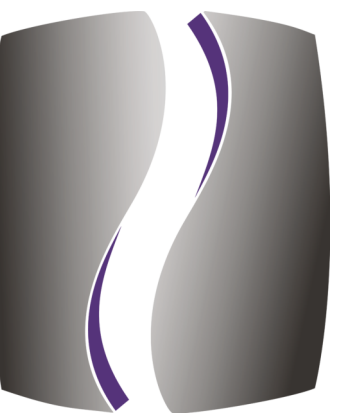
PHILLIPSPART.COM 770-394-1616



ROOF FRAMING PLAN NOTES:

- SEE S4-9# SERIES FOR ROOF FRAMING SECTIONS AND DETAILS.
- ROOF FRG DENOTES PRE-ENGINEERED OPEN WEB ROOF TRUSSES @ 24" O.C. TRUSS DEPTH VARIES WITH SLOPE OF ROOF (TOP CHORD TO SLOPE AS REQ'D TO ACHIEVE ROOF SLOPES). SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- "GT" DENOTES A GIRDER TRUSS, TYP.
- "HT" DENOTES A HIP TRUSS, TYP.
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- "FB" DENOTES A FLUSH BEAM, TYP.
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- TRUSS MANUFACTURER SHALL DESIGN ALL ROOF TRUSS COMPONENTS FOR THE NET UPLIFT WIND PRESSURE RESULTING FROM THE WIND PRESSURES SHOWN IN DETAILS ON S0-02 TYP. SEE ROOF FRAMING PLANS AND DETAILS FOR ADD'L INFORMATION.
- ALIGN ROOF TRUSSES WITH OR PROVIDE ADDITIONAL ROOF TRUSSES ABOVE ALL TOP FLOOR SHEAR WALLS. SEE S6-# SERIES (BRACING PLANS) FOR SHEAR WALL LOCATIONS, TYP.
- ALL ROOF TRUSSES TO BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE TIE-DOWN ANCHORS FOR ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE POSTS/STUDPACKS BELOW ENDS OF ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) CONTINUOUS TO FOUNDATION, ACCORDING TO THE FOLLOWING CRITERIA:
  - FOR GT/DBL SPANS  $\leq 20'-0"$ , PROVIDE 2-2x4
  - FOR GT/DBL SPANS  $\leq 35'-0"$ , PROVIDE 4-2x4
  - FOR GT/DBL SPANS  $\leq 50'-0"$ , PROVIDE 5-2x4

FOR CONSTRUCTION



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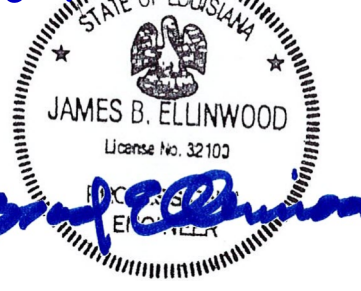


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FOUNDATION PLAN - CLUBHOUSE

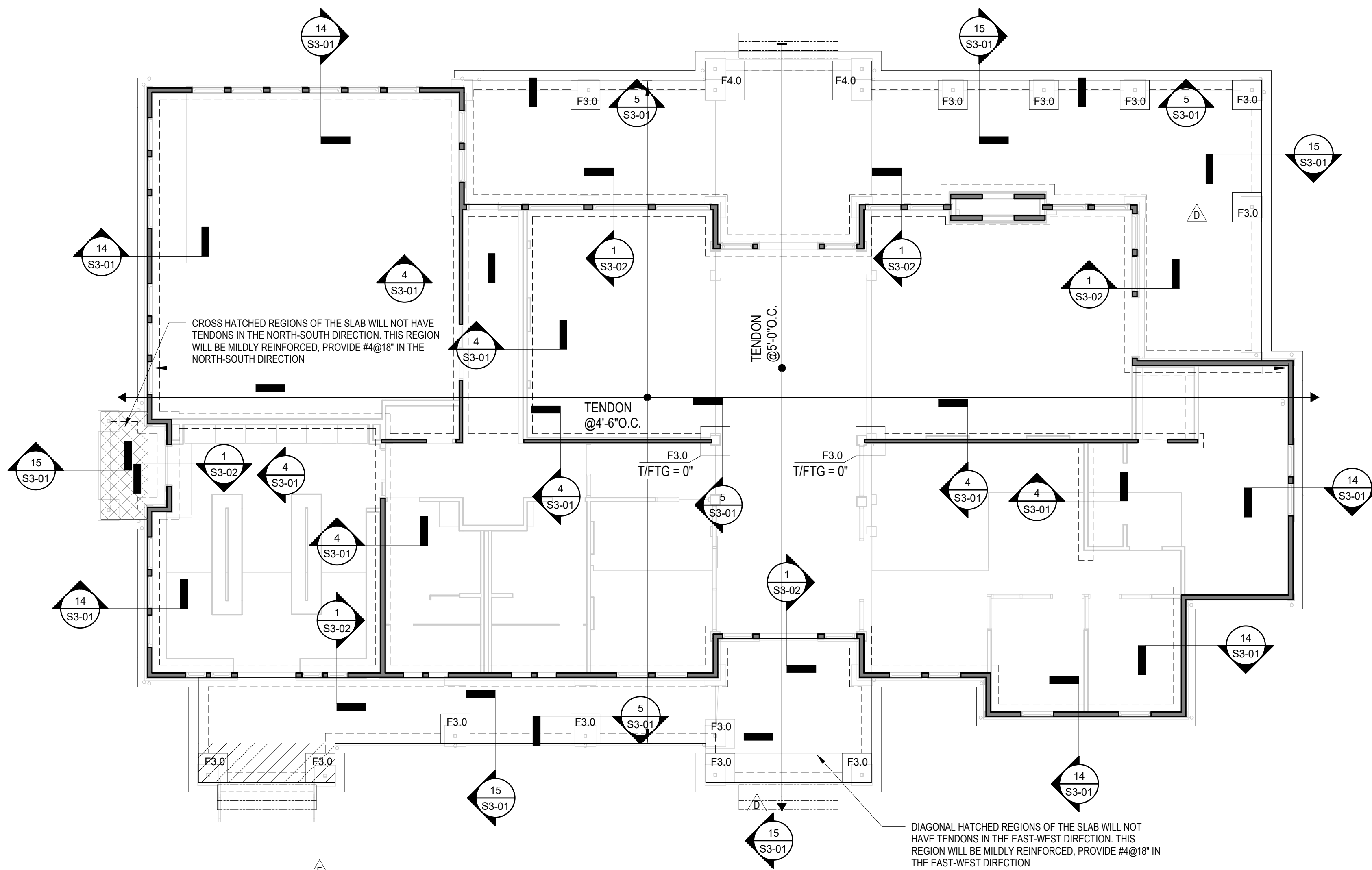
SHEET NUMBER

S1-3.1

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POST-TENSIONED SLAB ON GRADE NOTES:

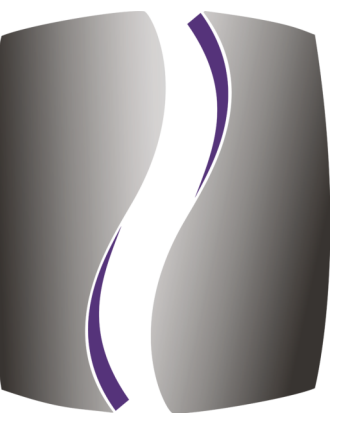
- FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE WITH INTEGRAL/MONOLITHIC THICKENED SLAB ELEMENTS AND/OR INTEGRAL SPREAD FOOTINGS.
- SLAB ON GRADE SHALL BE POST-TENSIONED, 4" THICK,  $f_c = 3,000$  PSI, ON PREPARED SUBGRADE, AND SHALL BE REINFORCED AS SHOWN ON THE POST-TENSIONED REINFORCING PLANS AND PER THE STRUCTURAL SECTIONS AND DETAILS. POST-TENSIONED SLAB ON GRADE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION JOINTS, TYP.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. SEE ARCHITECT'S DIMENSION CONTROL PLANS FOR ALL DIMENSIONS, DEPRESSIONS, SLOPES, ETC. NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DIMENSIONS SHOWN ON STRUCTURAL FOUNDATION PLANS ARE TO EDGE OF SLAB, CENTERLINE OF COLUMN, OR CENTERLINE OF THICKENED SLAB CONDITIONS, UNLESS NOTED OTHERWISE ON PLAN. COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DENOTES DEAD OR ANCHORED END OF POST-TENSIONING TENDON.
- DENOTES LIVE OR STRESSING END OF POST-TENSIONING TENDON.
- ALL TENDONS SHALL BE 1/2" DIAMETER, 270K 7-STRAND WIRE LOW RELAXATION CABLE WITH PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING CONFORMING TO ASTM A-416.
- ALL TENDONS SHALL BE ANCHORED AT 28.9 K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33.0 K PER TENDON.
- FINAL TENDON FORCE SHALL BE 27.0 KIPS (AFTER LOSSES).
- ALL TENDONS SPANNING EAST/WEST SHALL HAVE A UNIFORM A DRAPE HEIGHT OF 1.75". ALL TENDONS SPANNING NORTH SOUTH SHALL HAVE A TENDON DRAPE HEIGHT OF 2.25".
- TENDON SPACING SHALL BE AS SHOWN ON THE POST-TENSION PLAN.
- THE FIRST TENDON (AT THE EDGE OF SLAB SHALL) BE LOCATED NO CLOSER THAN 0'-6" FROM EDGE OF SLAB AND NO FURTHER THAN 0'-8" FROM THE EDGE OF SLAB, TYP.
- SEE GENERAL NOTES (SECTION 5) FOR ADDITIONAL POST-TENSIONED SLAB ON GRADE REQUIREMENTS AND SPECIFICATIONS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL EXTERIOR SLAB AND CURB INFORMATION. SEE CIVIL PLANS FOR GRADE ELEVATION AT EXTERIOR PERIMETER.
- FINISH GRADE SHOULD BE 8" TO 12" BELOW FFE, SEE DETAILS ON S3-01.
- CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED BY THE GC AND THE PT SUPPLIER. CONSTRUCTION LOCATIONS SHALL BE APPROVED BY THE DESIGN TEAM.
- SEE FOUNDATION PLANS FOR SPREAD FOOTING LOCATIONS AND FOOTING SCHEDULE.
- SEE TYPICAL DETAILS FOR THICKENED SLAB AT STAIR LANDINGS.
- PROVIDE (2) #4 BAR AT MID DEPTH OF FOUNDATION SLAB AT ALL RE-ENTRANT CORNERS.
- IN SOME LOCATIONS, IT WILL BE NECESSARY TO INSTALL AN EMBED PLATE IN THE POST-TENSIONED CONCRETE SLAB ON GRADE. THE LOCATIONS WHERE THIS WILL BE NECESSARY MUST BE COORDINATED WITH THE SHEAR WALL BRACING PLANS (S6-0#), SHEAR WALL ANCHORAGE SCHEDULE (DETAIL 8/S6-00A), AND EMBED PLATE DETAILS (S6-00E).

FOUNDATION NOTES:

- FOUNDATIONS ARE DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE, INTEGRAL THICKENED SLAB ELEMENTS AND INTEGRAL SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR NEWLY PLACED COMPACTED, ENGINEERED FILL.
- SEE SHEETS S3-0# FOR FOUNDATION DETAILS.
- XXXXX DENOTES CMU WALL. SEE SHEETS S4-20 FOR CMU WALL DETAILS.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.
- TYPICAL SLAB ON GRADE SHALL BE A POST-TENSIONED, 4" THICK (MINIMUM) CONCRETE SLAB.
- SEE ARCH. DRAWINGS FOR ALL SLAB SLOPES AND CRICKETS FOR DRAINAGE.
- WHERE SLAB ON GRADE MUST SLOPE TO DRAIN, CONTRACTOR TO THICKEN SLAB AS REQUIRED TO MAINTAIN 4" MINIMUM THICKNESS, TYP. SEE SHEETS S3-0# AND GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
- PROVIDE ONE (1) LAYER OF 10 MIL VAPOR BARRIER UNDER THE SLAB ON GRADE, TYP.
- ALL FOOTINGS TO BE CENTERED UNDER WALLS, COLUMNS, BEAM BEARING, PIERS AND PILASTERS, UNLESS NOTED OTHERWISE.
- ALL SPREAD FOOTINGS SUPPORTING WOOD COLUMNS SHALL BE POURED MONOLITHICALLY WITH THE SLAB ON GRADE. SEE DETAILS S5/S3.01 FOR ADDITIONAL INFORMATION.
- INDICATES FOOTING MARK. SEE SCHEDULE.  
INDICATES TOP OF FOOTING ELEVATION.
- SEE ARCH. FOR TRASH ENCLOSURE, COMPACTOR, AND TRANSFORMER/EQUIPMENT PADS. UNLESS NOTED OTHERWISE ON PLANS, EQUIPMENT PADS SHALL BE 6" THICK REINFORCED CONCRETE SLAB ON GRADE W/ #4@12" E.W. CENTERED IN SLAB.
- CONCRETE SIDEWALKS AND STOOPS NOT WITHIN SCOPE OF STRUCTURAL DRAWINGS. SEE CIVIL/LANDSCAPE. DRAWINGS, TYP.
- DENOTES SLAB STEP.
- PROVIDE SIMPSON "CB, CBS, OR CBSQ" TYPE COLUMN BASES AT ALL FREE-STANDING ENGINEERED LUMBER COLUMNS. "FREE-STANDING" COLUMNS AND/OR STUD PACKS SHALL BE DEFINED AS THOSE COLUMNS/STUD PACKS THAT ARE NOT WITHIN THE PLAN OF A STUD WALL.
- SEE TYPICAL DETAILS ON S6-00C FOR THICKENED AREAS AT SHEAR WALL ENDS FOR ANCHOR EMBEDMENT.
- IT WILL BE NECESSARY TO INSTALL EMBED PLATES IN THE CONCRETE SLAB ON GRADE AT SHEAR WALL ANCHORAGE LOCATIONS. IF THE CONTRACTOR INTENDS TO USE A THREADED ROD SYSTEM FOR THE SHEAR WALL ANCHORAGE, SPECIFIC COORDINATION WILL BE REQUIRED TO DETERMINE LOCATIONS WHERE EMBED PLATES ARE REQUIRED PER THE STRUCTURAL ANCHORAGE DETAILS ON THE S6-00# SHEETS. IT IS RECOMMENDED THAT THE THREADED ROD SYSTEM SHOP DRAWINGS BE SUBMITTED AT A TIME WHERE THESE EMBED PLATES CAN BE COORDINATED WITH THE SLAB ON GRADE CONSTRUCTION.

- POWER ACTUATED FASTENERS ARE NOT TO BE USED AT BEARING WALLS IN THE CLUBHOUSE. INTERIOR BEARING, NON-SHEAR WALL SILL PLATE ANCHORS MUST BE THREADED ROD ANCHORS PER NOTE 1 OF GENERAL NOTE SECTION 8 ON SHEET S0-03.
- TSP TIES SHALL BE INSTALLED AT 48" O.C. AT THE BASE OF ALL INTERIOR BEARING WALL STUDS TO ANCHOR THE WALL TO THE SILL PLATE. SIMILAR TO NOTE 4 OF DETAIL 14/S4-91 FOR TOP PLATE ANCHORAGE.
- ALL COLUMNS SUPPORTING BEAMS, TIMBER TRUSSES, OR GIRDER TRUSSES MORE THAN 6'-0" LONG SHALL BE ANCHORED TO THE FOUNDATION WITH SIMPSON HTT4 TENSION TIES.

SPREAD FOOTING SCHEDULE				
MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F3.0	3'-0"	3'-0"	12"	#4 EA. WAY
F4.0	4'-0"	4'-0"	14"	#5 EA. WAY



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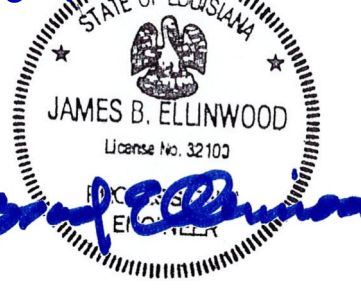


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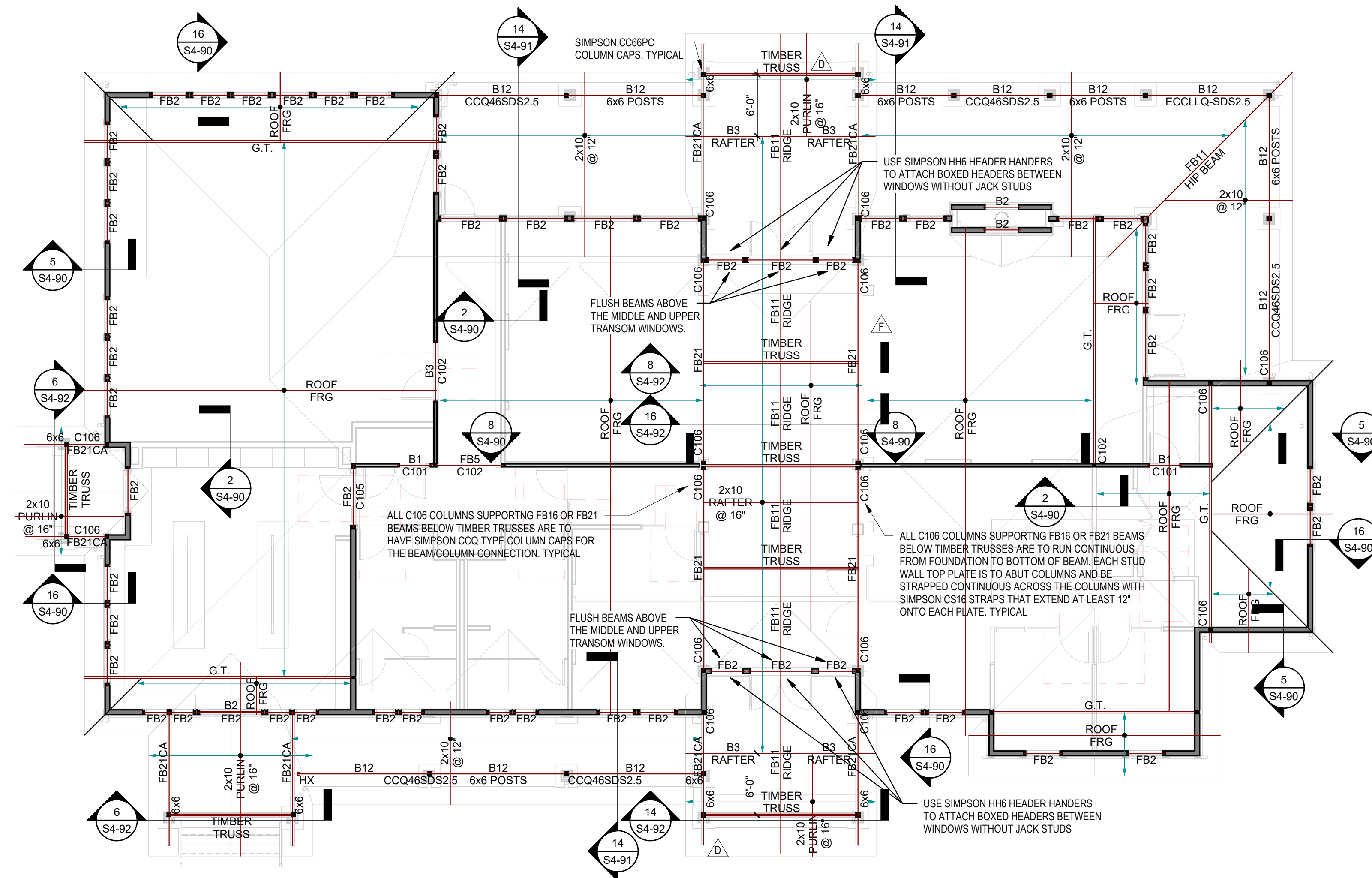
ROOF FRAMING PLAN - CLUBHOUSE

SHEET NUMBER

S1-3.2

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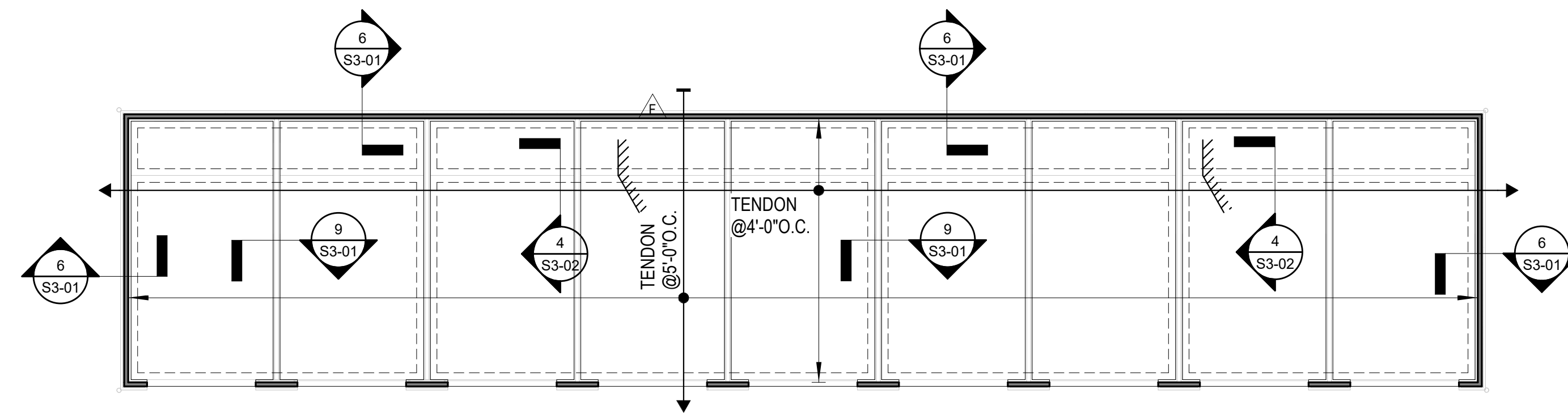
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ROOF FRAMING PLAN NOTES:

- SEE S4-9# SERIES FOR ROOF FRAMING SECTIONS AND DETAILS.
- ROOF DENOTES PRE-ENGINEERED OPEN WEB ROOF TRUSSES @ 24" O.C.  
FRG , TYP. TRUSS DEPTH VARIES WITH SLOPE OF ROOF (TOP CHORD TO SLOPE AS REQ'D TO ACHIEVE ROOF SLOPES). SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
- "GT" DENOTES A GIRDER TRUSS, TYP.
- "HT" DENOTES A HIP TRUSS, TYP.
- "DBL" DENOTES A DOUBLE TRUSS, TYP.
- "FB" DENOTES A FLUSH BEAM, TYP.
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- TRUSS MANUFACTURER SHALL DESIGN ALL ROOF TRUSS COMPONENTS FOR THE NET UPLIFT WIND PRESSURE RESULTING FROM THE WIND PRESSURES SHOWN IN DETAILS ON S0-02, TYP.
- ALIGN ROOF TRUSSES WITH OR PROVIDE ADDITIONAL ROOF TRUSSES ABOVE ALL TOP FLOOR SHEAR WALLS. SEE S6-## SERIES (BRACING PLANS) FOR SHEAR WALL LOCATIONS, TYP.
- ALL ROOF TRUSSES TO BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE TIE-DOWN ANCHORS FOR ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
- PROVIDE POSTS/STUDPACKS BELOW ENDS OF ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) CONTINUOUS TO FOUNDATION, ACCORDING TO THE FOLLOWING CRITERIA:
  - FOR GT/DBL SPANS ≤ 20'-0", PROVIDE 2-2x4
  - FOR GT/DBL SPANS ≤ 35'-0", PROVIDE 4-2x4
  - FOR GT/DBL SPANS ≤ 50'-0", PROVIDE 5-2x4
- ALL STUD WALLS, BEARING AND NON-BEARING, TO BE FINGER JOINTED No. 2 SYP @ 16" O.C.
  - EXTERIOR TO BE 2x6 @ 16" O.C.
  - INTERIOR TO BE 2x4 @ 16" O.C. UNLESS ARCHITECTURAL SPECIFIES 2x6
- ALL OPENINGS TO RECEIVE 1 JAMB STUD AND 1 KING STUD U.N.O. ON PLAN.
- EACH STUD THAT RECEIVES A SIMPSON TSP TIE AT THE TOP PLATES PER NOTE 4 OF DETAIL 14/S4-90 SHALL ALSO HAVE A MATCHING TSP TIE INSTALLED AT THE FLOOR FOR THE SILL PLATE TO STUD CONNECTION.

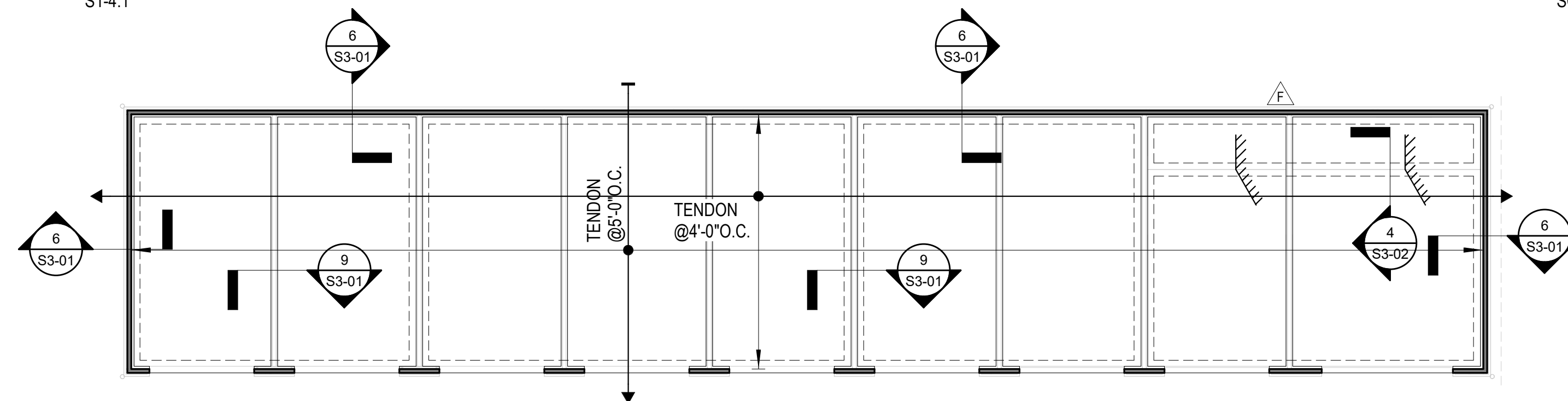
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**1 FOUNDATION PLAN - GARAGE BUILDING 1**

S1-4.1

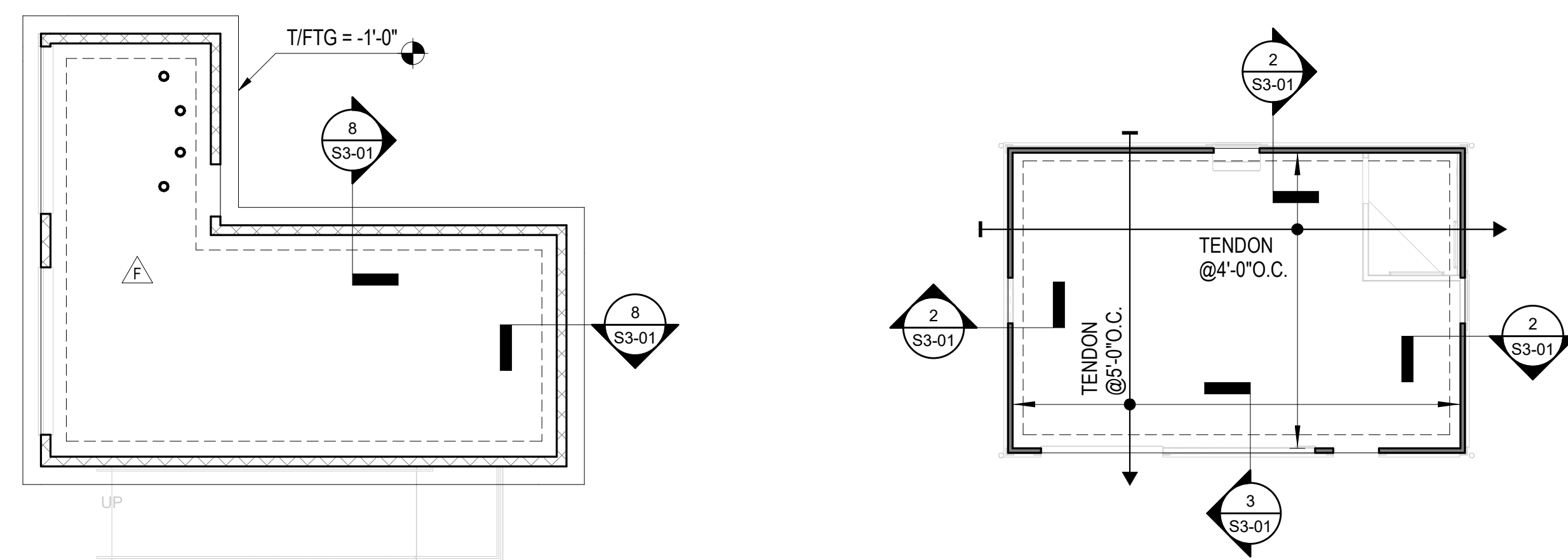
SCALE: 1/8" = 1'-0"



**2 FOUNDATION PLAN - GARAGE BUILDING 2**

S1-4.1

SCALE: 1/8" = 1'-0"



**4 FOUNDATION PLAN - TRASH COMPACTOR BUILDING**

S1-4.1

SCALE: 1/8" = 1'-0"

**3 FOUNDATION PLAN - MAINTENANCE BUILDING**

S1-4.1

SCALE: 1/8" = 1'-0"

**FOUNDATION NOTES:**

- FOUNDATIONS ARE DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF CONCRETE SLAB ON GRADE, INTEGRAL THICKENED SLAB ELEMENTS AND INTEGRAL SPREAD FOOTINGS BEARING ON RESIDUAL SOILS OR NEWLY PLACED COMPACTED, ENGINEERED FILL.
- SEE SHEETS S3-0# FOR FOUNDATION DETAILS.
- XXXXX DENOTES CMU WALL. SEE SHEETS S4-20 FOR CMU WALL DETAILS.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN.
- TYPICAL SLAB ON GRADE SHALL BE A POST-TENSIONED, 4" THICK (MINIMUM) CONCRETE SLAB, DESIGNED BY OTHERS.
- SEE ARCH. DRAWINGS FOR ALL SLAB SLOPES AND CRICKETS FOR DRAINAGE.
- WHERE SLAB ON GRADE MUST SLOPE TO DRAIN, CONTRACTOR TO THICKEN SLAB AS REQUIRED TO MAINTAIN 4" MINIMUM THICKNESS, TYP. SEE SHEETS S3-0# AND GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.
- PROVIDE ONE (1) LAYER OF 10 MIL VAPOR BARRIER UNDER THE SLAB ON GRADE, TYP. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- ALL FOOTINGS TO BE CENTERED UNDER WALLS, COLUMNS, BEAM BEARING, PIERS AND PILASTERS, UNLESS NOTED OTHERWISE.
- ALL SPREAD FOOTINGS SUPPORTING WOOD COLUMNS SHALL BE POURED MONOLITHICALLY WITH THE SLAB ON GRADE. SEE DETAIL S/S3.01 FOR ADDITIONAL INFORMATION.
- F5.0 INDICATES FOOTING MARK. SEE SCHEDULE.  
T/FTG= INDICATES TOP OF FOOTING ELEVATION.
- SEE ARCH. FOR TRASH ENCLOSURE, COMPACTOR, AND TRANSFORMER/EQUIPMENT PADS. UNLESS NOTED OTHERWISE ON PLANS, EQUIPMENT PADS SHALL BE 6" THICK REINFORCED CONCRETE SLAB ON GRADE W/ #4@12" E.W. CENTERED IN SLAB.
- CONCRETE SIDEWALKS AND STOOPS NOT WITHIN SCOPE OF STRUCTURAL DRAWINGS. SEE CIVIL/ARCH. DRAWINGS, TYP.
- /// DENOTES SLAB STEP.
- PROVIDE SIMPSON "PB" TYPE COLUMN BASES AT ALL FREE-STANDING ENGINEERED LUMBER COLUMNS. "FREE-STANDING" COLUMNS AND/OR STUD PACKS SHALL BE DEFINED AS THOSE COLUMNS/STUD PACKS THAT ARE NOT WITHIN THE PLAN OF A STUD WALL.
- SEE TYPICAL DETAILS ON S6-00C FOR THICKENED AREAS AT SHEAR WALL ENDS FOR ANCHOR EMBEDMENT.
- IT WILL BE NECESSARY TO INSTALL EMBED PLATES IN THE CONCRETE SLAB ON GRADE AT SHEAR WALL ANCHORAGE LOCATIONS. IF THE CONTRACTOR INTENDS TO USE A THREADED ROD SYSTEM FOR THE SHEAR WALL ANCHORAGE, SPECIFIC COORDINATION WILL BE REQUIRED TO DETERMINE LOCATIONS WHERE EMBED PLATES ARE REQUIRED PER THE STRUCTURAL ANCHORAGE DETAILS ON THE S6-00# SHEETS. IT IS RECOMMENDED THAT THE THREADED ROD SYSTEM SHOP DRAWINGS BE SUBMITTED AT A TIME WHERE THESE EMBED PLATES CAN BE COORDINATED WITH THE SLAB ON GRADE CONSTRUCTION.

SPREAD FOOTING SCHEDULE				
MARK	LENGTH	WIDTH	THICKNESS	REINFORCEMENT
F3.0	3'-0"	3'-0"	12"	4# EA. WAY
F4.0	4'-0"	4'-0"	14"	4#5 EA. WAY

**POST-TENSIONED SLAB ON GRADE NOTES:**

- FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE BEARING PRESSURES:  
2,000 PSF POST-TENSIONED CONCRETE SLAB ON GRADE WITH INTEGRAL/MONOLITHIC THICKENED SLAB ELEMENTS AND/OR INTEGRAL SPREAD FOOTINGS.
- SLAB ON GRADE SHALL BE POST-TENSIONED, 4" THICK,  $f_c = 3,000$  PSI, ON PREPARED SUBGRADE, AND SHALL BE REINFORCED AS SHOWN ON THE POST-TENSIONED REINFORCING PLANS AND PER THE STRUCTURAL SECTIONS AND DETAILS. POST-TENSIONED SLAB ON GRADE SHALL BE POURED MONOLITHICALLY BETWEEN CONSTRUCTION JOINTS, TYP.
- SEE ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. SEE ARCHITECT'S DIMENSION CONTROL PLANS FOR ALL DIMENSIONS, DEPRESSIONS, SLOPES, ETC. NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DIMENSIONS SHOWN ON STRUCTURAL FOUNDATION PLANS ARE TO EDGE OF SLAB, CENTERLINE OF COLUMN, OR CENTERLINE OF THICKENED SLAB CONDITIONS, UNLESS NOTED OTHERWISE ON PLAN. COORDINATE DIMENSIONS W/ARCHITECTURAL DRAWINGS AND NOTIFY ARCHITECT OF ALL DISCREPANCIES AND/OR INCONSISTENCIES PRIOR TO POURING CONCRETE.
- DENOTES DEAD OR ANCHORED END OF POST-TENSIONING TENDON.
- DENOTES LIVE OR STRESSING END OF POST-TENSIONING TENDON.
- ALL TENDONS SHALL BE 1/2" DIAMETER, 270K 7-STRAND WIRE LOW RELAXATION CABLE WITH PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING CONFORMING TO ASTM A-416.
- ALL TENDONS SHALL BE ANCHORED AT 28.9 K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33.0 K PER TENDON.
- FINAL TENDON FORCE SHALL BE 27.0 KIPS (AFTER LOSSES).
- ALL TENDONS SPANNING EAST/WEST SHALL HAVE A UNIFORM A DRAPE HEIGHT OF 1.75". ALL TENDONS SPANNING NORTH SOUTH SHALL HAVE A TENDON DRAPE HEIGHT OF 2.25".
- TENDON SPACING SHALL BE AS SHOWN ON THE POST-TENSION PLAN.
- THE FIRST TENDON (AT THE EDGE OF SLAB) SHALL BE LOCATED NO CLOSER THAN 0'-6" FROM EDGE OF SLAB AND NO FURTHER THAN 0'-8" FROM THE EDGE OF SLAB, TYP.
- SEE GENERAL NOTES (SECTION 5) FOR ADDITIONAL POST-TENSIONED SLAB ON GRADE REQUIREMENTS AND SPECIFICATIONS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL EXTERIOR SLAB AND CURB INFORMATION. SEE CIVIL PLANS FOR GRADE ELEVATION AT EXTERIOR PERIMETER.
- FINISH GRADE SHOULD BE 8" TO 12" BELOW FFE, SEE DETAILS ON S3-01.
- CONSTRUCTION JOINT LOCATIONS SHALL BE COORDINATED BY THE GC AND THE PT SUPPLIER. CONSTRUCTION LOCATIONS SHALL BE APPROVED BY THE DESIGN TEAM.
- SEE FOUNDATION PLANS FOR SPREAD FOOTING LOCATIONS AND FOOTING SCHEDULE.
- SEE TYPICAL DETAILS FOR THICKENED SLAB AT STAIR LANDINGS.
- PROVIDE (2) #4 BAR AT MID DEPTH OF FOUNDATION SLAB AT ALL RE-ENTRANT CORNERS.
- IN SOME LOCATIONS, IT WILL BE NECESSARY TO INSTALL AN EMBED PLATE IN THE POST-TENSIONED CONCRETE SLAB ON GRADE. THE LOCATIONS WHERE THIS WILL BE NECESSARY MUST BE COORDINATED WITH THE SHEAR WALL BRACING PLANS (S6-0#), SHEAR WALL ANCHORAGE SCHEDULE (DETAIL S/S6-00A), AND EMBED PLATE DETAILS (S6-00E).

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PROJECT: FREMAUX PARK APARTMENTS

CLIENT: SLIDELL, LA

WAYPOINT RESIDENTIAL

PHILLIPS JOB NUMBER: 1867709

ISSUE DATE: 05/10/2019

DRAWN BY/CHECKED BY: DKJ / RAFAEL, RIM

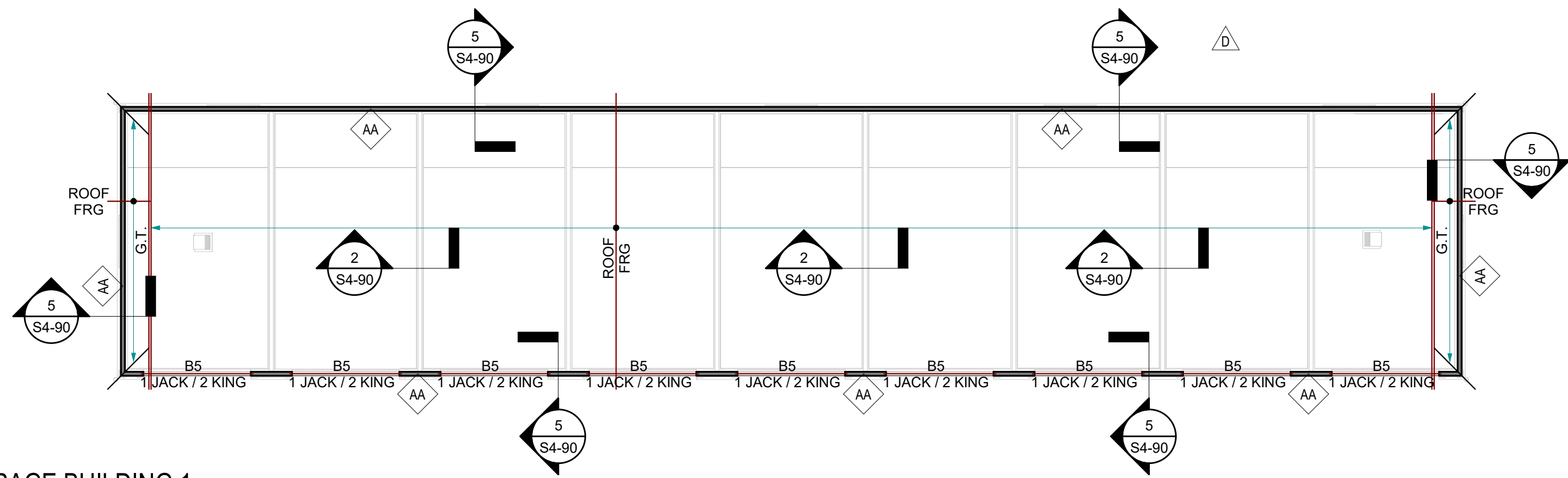
APPROVED BY: BRAD, JBE

DRAWING TITLE: ANCILLARY BUILDINGS FOUNDATION PLANS

SHEET NUMBER: S1-4.1

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328  
PHILLIPSPART.COM 770-394-1616

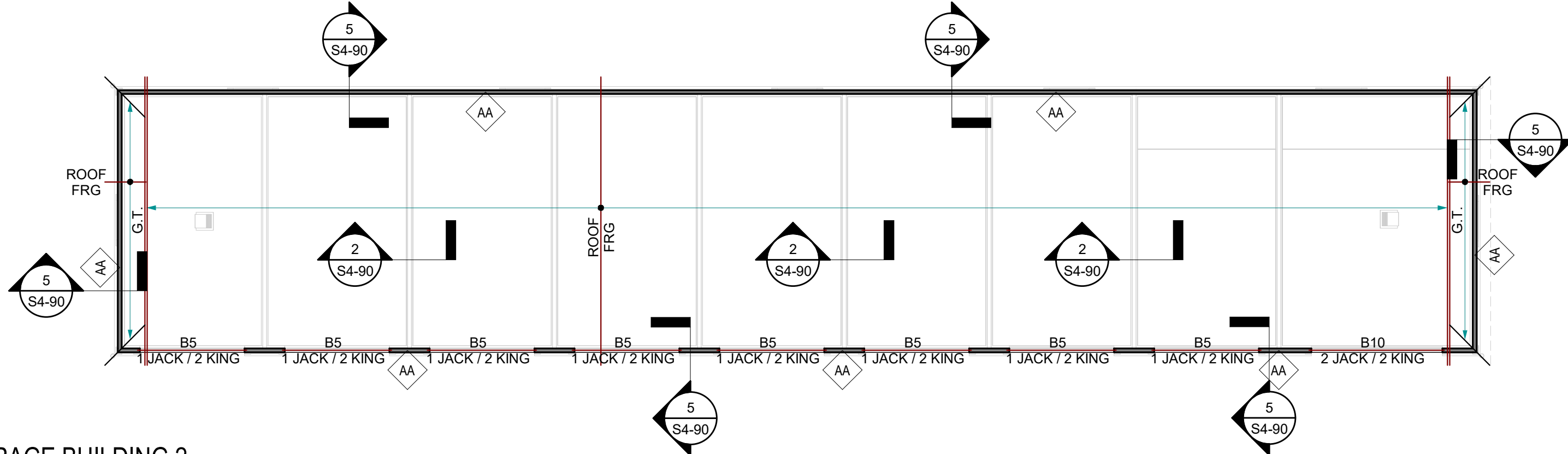
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**1** ROOF FRAMING PLAN - GARAGE BUILDING 1

S1-4.2

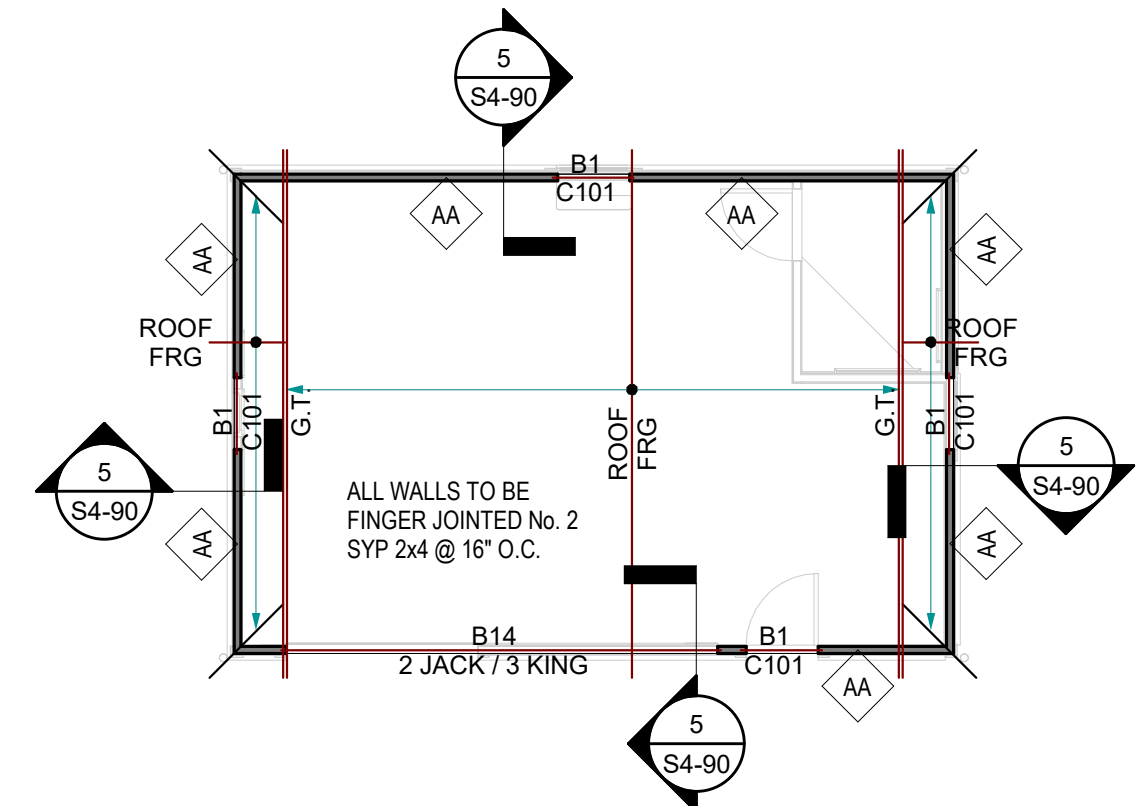
SCALE: 1/8" = 1'-0"



**2** ROOF FRAMING PLAN - GARAGE BUILDING 2

S1-4.2

SCALE: 1/8" = 1'-0"



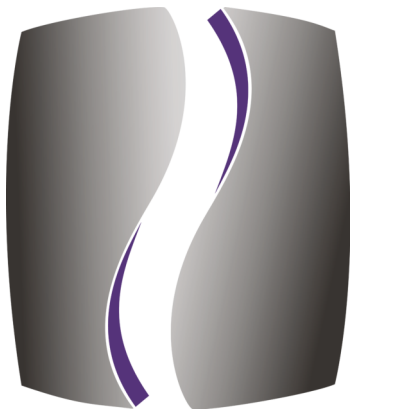
**3** ROOF FRAMING PLAN - MAINTENANCE BUILDING

S1-4.2

SCALE: 1/8" = 1'-0"

**ROOF FRAMING PLAN NOTES:**

1. SEE S4-9# SERIES FOR ROOF FRAMING SECTIONS AND DETAILS.
2. DENOTES PRE-ENGINEERED OPEN WEB ROOF TRUSSES @ 24" O.C. TYP. TRUSS DEPTH VARIES WITH SLOPE OF ROOF (TOP CHORD TO SLOPE AS REQD TO ACHIEVE ROOF SLOPES). SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
3. "GT" DENOTES A GIRDER TRUSS, TYP.
4. "HT" DENOTES A HIP TRUSS, TYP.
5. "DBL" DENOTES A DOUBLE TRUSS, TYP.
6. "FB" DENOTES A FLUSH BEAM, TYP.
7. ALL ROOF TRUSSES SHALL BE DESIGNED BY THE TRUSS MANUFACTURER, WHICH SHALL INCLUDE ALL TRUSS-TO-TRUSS CONNECTIONS. SEE GENERAL NOTES ON SHEET S0-01 AND S0-03 FOR ADDITIONAL INFORMATION.
8. TRUSS MANUFACTURER SHALL DESIGN ALL ROOF TRUSS COMPONENTS FOR THE NET UPLIFT WIND PRESSURE RESULTING FROM THE WIND PRESSURES SHOWN IN DETAILS ON S0-02, TYP.
9. ALIGN ROOF TRUSSES WITH OR PROVIDE ADDITIONAL ROOF TRUSSES ABOVE ALL TOP FLOOR SHEAR WALLS. SEE S6-## SERIES (BRACING PLANS) FOR SHEAR WALL LOCATIONS, TYP.
10. ALL ROOF TRUSSES TO BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
11. PROVIDE TIE-DOWN ANCHORS FOR ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO ROOF ANCHORAGE AND TIE-DOWN NOTES - SEE DETAIL 14/S4-90.
12. PROVIDE POSTS/STUDPACKS BELOW ENDS OF ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) CONTINUOUS TO FOUNDATION, ACCORDING TO THE FOLLOWING CRITERIA:
  - A. FOR GT/DBL SPANS  $\leq 20'-0"$ , PROVIDE 2-2x4
  - B. FOR GT/DBL SPANS  $\leq 35'-0"$ , PROVIDE 4-2x4
  - C. FOR GT/DBL SPANS  $\leq 50'-0"$ , PROVIDE 5-2x4
13. ALL STUD WALLS, BEARING AND NON-BEARING, TO BE FINGER JOINTED No. 2 SYP @ 16" O.C.
  - A. EXTERIOR TO BE 2x6 @ 16" O.C.
  - B. INTERIOR TO BE 2x4 @ 16" O.C. UNLESS ARCHITECTURAL SPECIFIES 2x6
14. ALL OPENINGS TO RECEIVE 1 JAMB STUD AND 1 KING STUD U.N.O. ON PLAN.
15. EACH STUD THAT RECEIVES A SIMPSON TSP TIE AT THE TOP PLATES PER NOTE 4 OF DETAIL 14/S4-90 SHALL ALSO HAVE A MATCHING TSP TIE INSTALLED AT THE FLOOR FOR THE SILL PLATE TO STUD CONNECTION.



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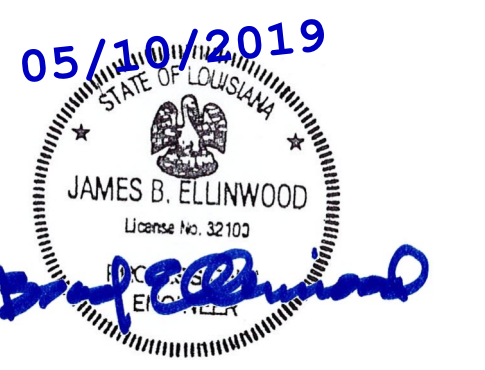
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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

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APPROVED BY BRAD, JBE

DRAWING TITLE

**ANCILLARY BUILDINGS FRAMING PLANS**

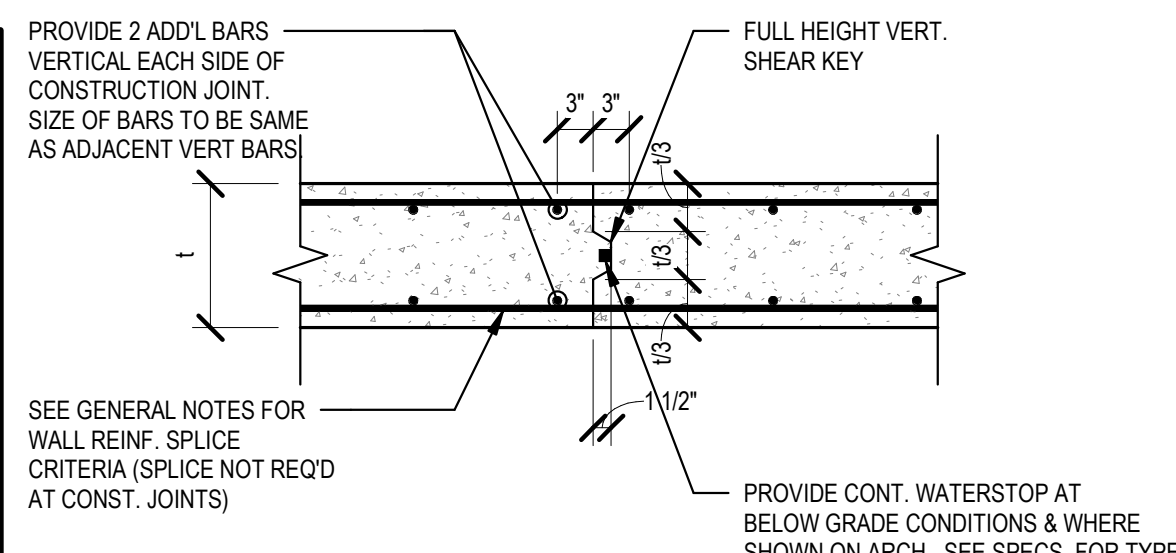
SHEET NUMBER

**S1-4.2**

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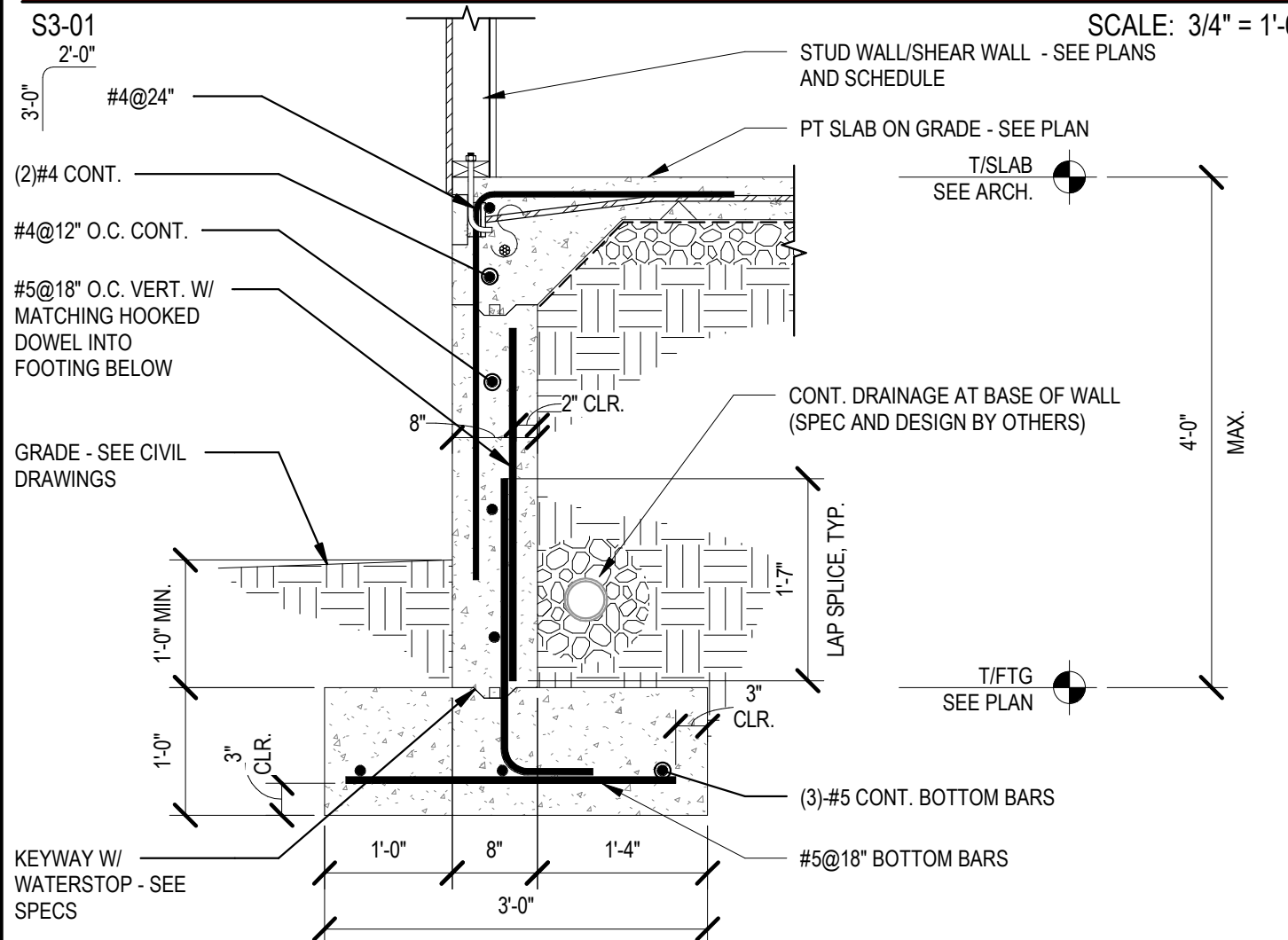
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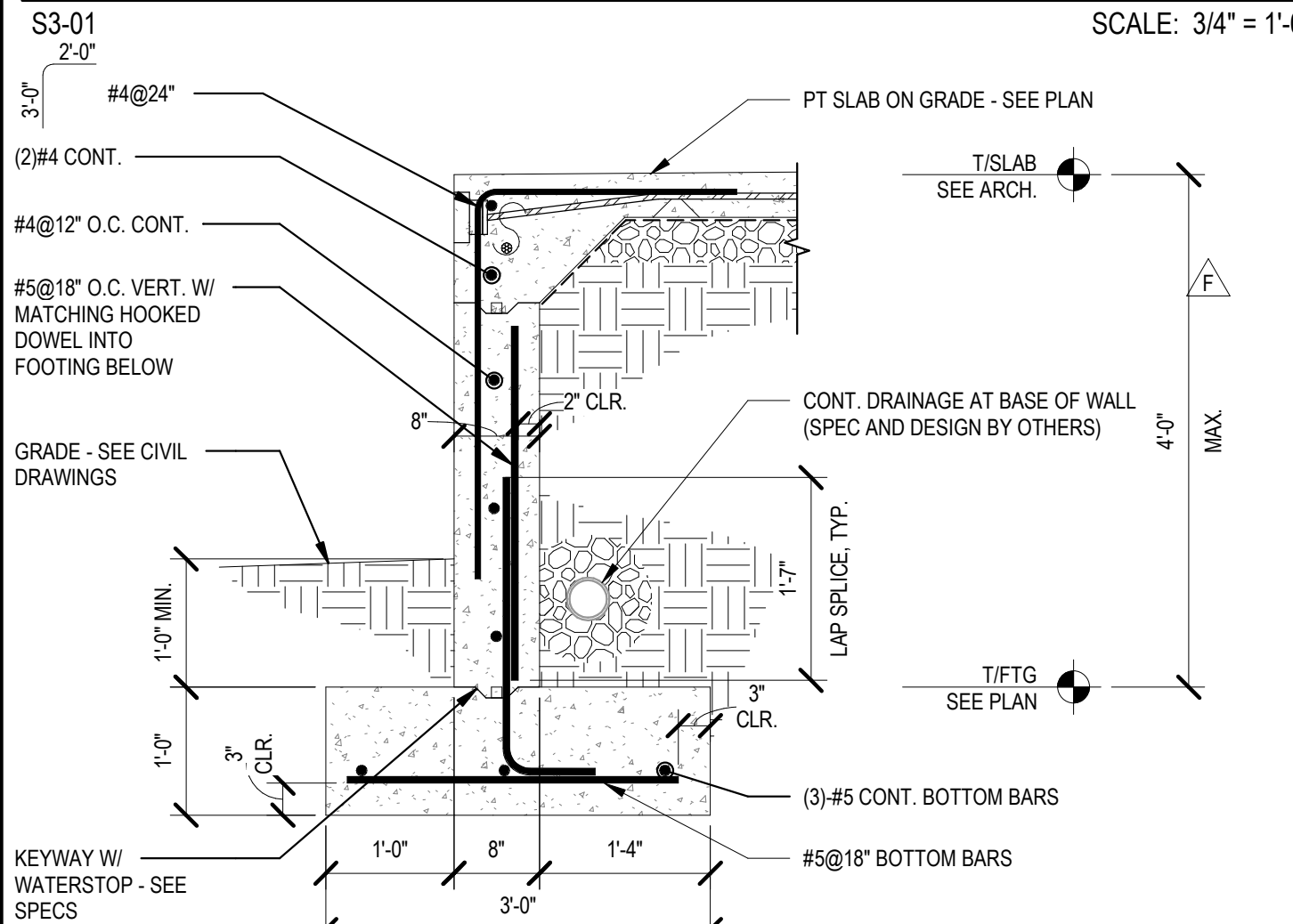


- NOTES:
- SUBMIT PROPOSED WALL JOINT LOCATIONS FOR APPROVAL PRIOR TO POURING WALL. WHERE WALLS SPAN HORIZONTALLY, JOINTS SHALL BE LOCATED IN THE MIDDLE THIRD OF WALL SPANS, U.N.O. SEE PLANS FOR LOCATIONS WHERE WALLS SPAN HORIZONTALLY.
  - SEE ARCHITECTURAL DRAWINGS FOR TREATMENT OF EXPOSED CONSTRUCTION JOINTS (RUB OUT, PROVIDE REVEAL, OR OTHER) AND OTHER JOINT REQUIREMENTS.
  - PROVIDE WALL CONSTRUCTION JOINTS SUCH THAT NO SINGLE WALL POUR EXCEEDS 60'-0" IN LENGTH.
  - THERE SHALL BE NO VERTICAL CONSTRUCTION JOINT IN WALL WITHIN 5'-0" OF CORNERS UNLESS DETAILED ON THE STRUCTURAL DRAWINGS.
  - PROVIDE CONTROL JOINTS AT 20'-0" MAX. WITH 3/4" CHAMFER STRIP EACH FACE. DISCONTINUE EVERY OTHER HORIZONTAL BAR IN EACH FACE AT CONTROL JOINT.

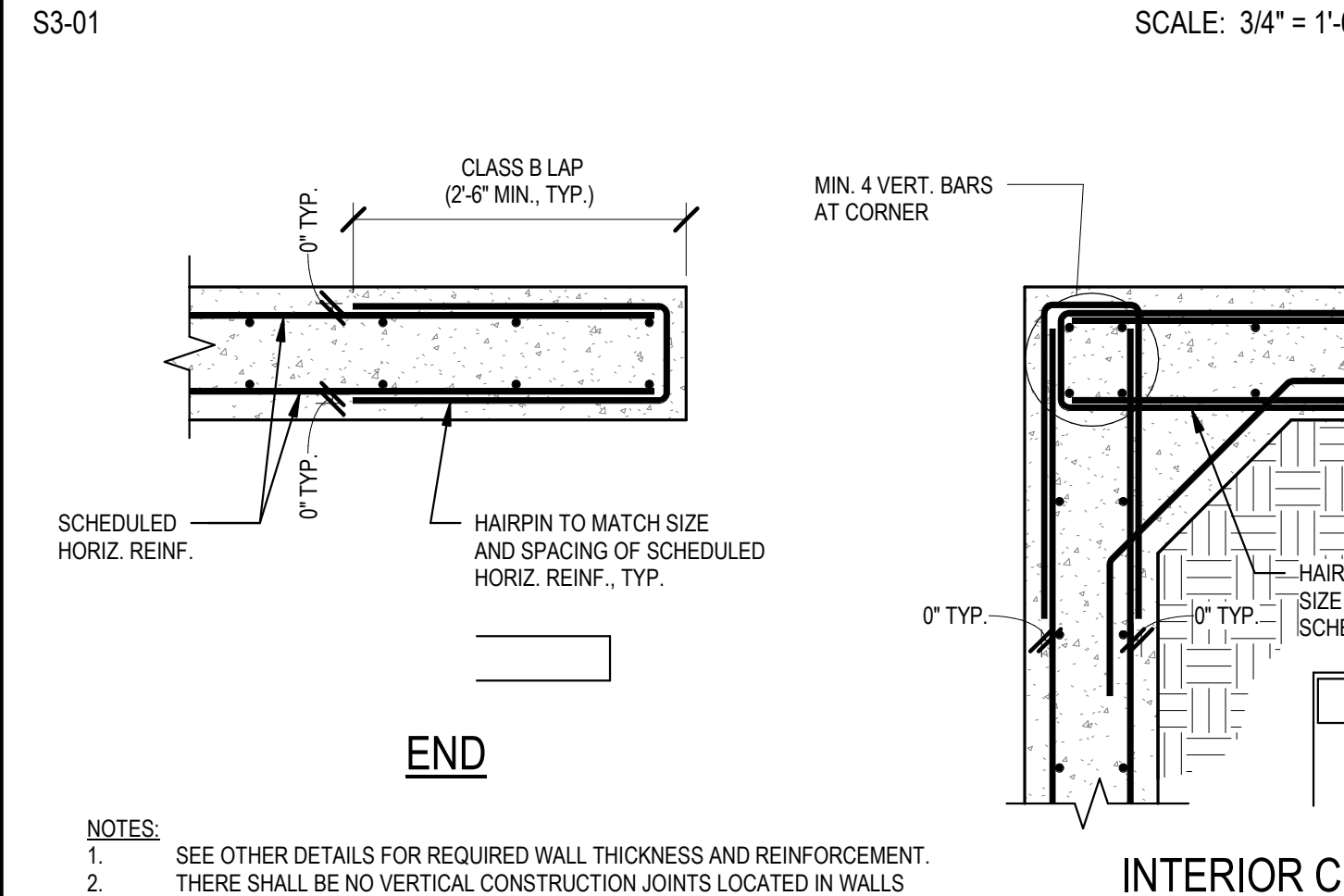
**13 VERTICAL CONSTRUCTION JOINT AT CONCRETE WALL**



**14 SECTION THRU CONCRETE RETAINING WALL**

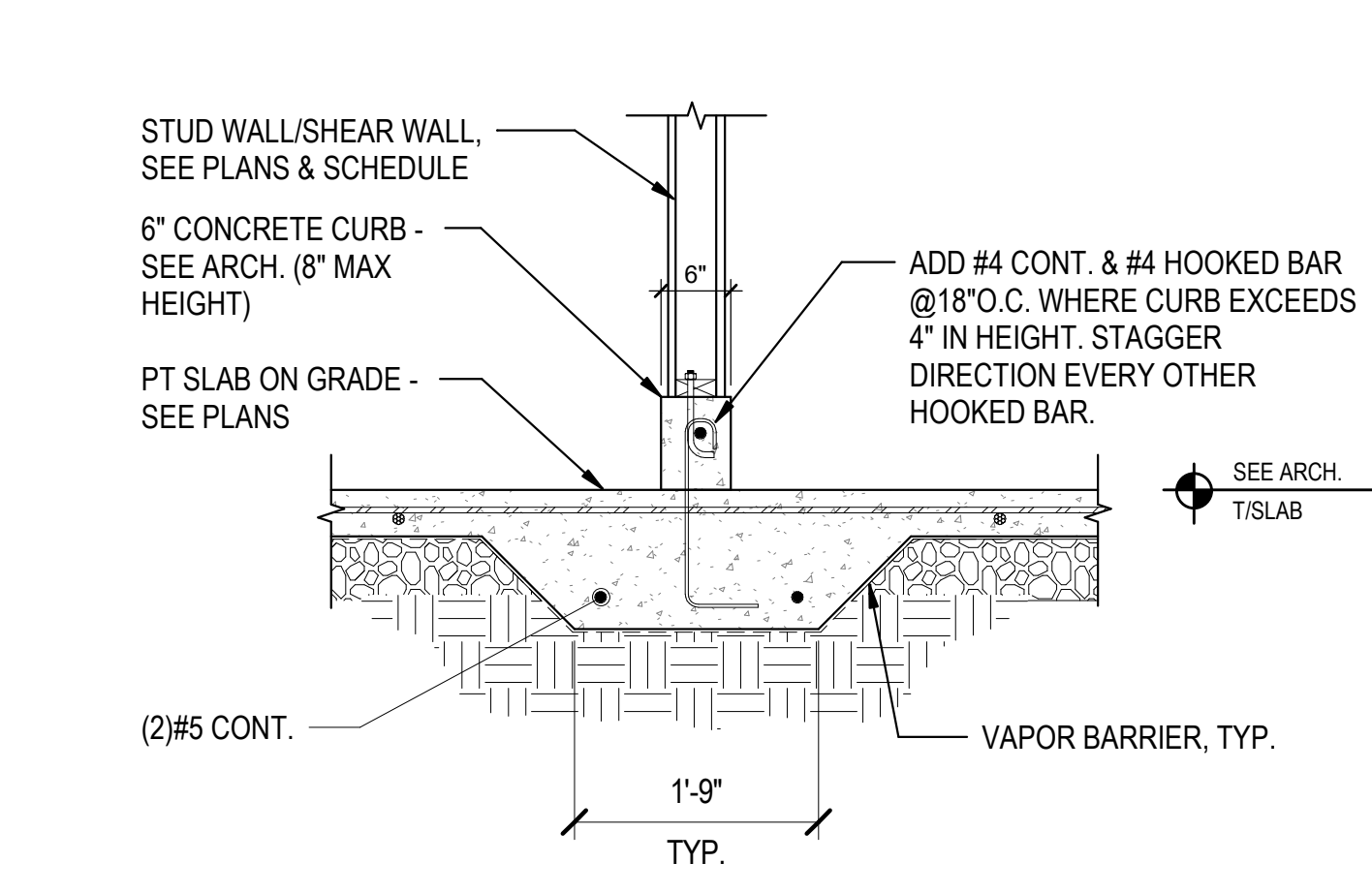


**15 SECTION THRU CONCRETE RETAINING WALL**



**16 REINFORCING STEEL AT CORNERS AND ENDS OF CONCRETE WALLS**

- NOTES:
- SEE OTHER DETAILS FOR REQUIRED WALL THICKNESS AND REINFORCEMENT.
  - THERE SHALL BE NO VERTICAL CONSTRUCTION JOINTS LOCATED IN WALLS WITHIN 5'-0" OF CORNERS UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS.

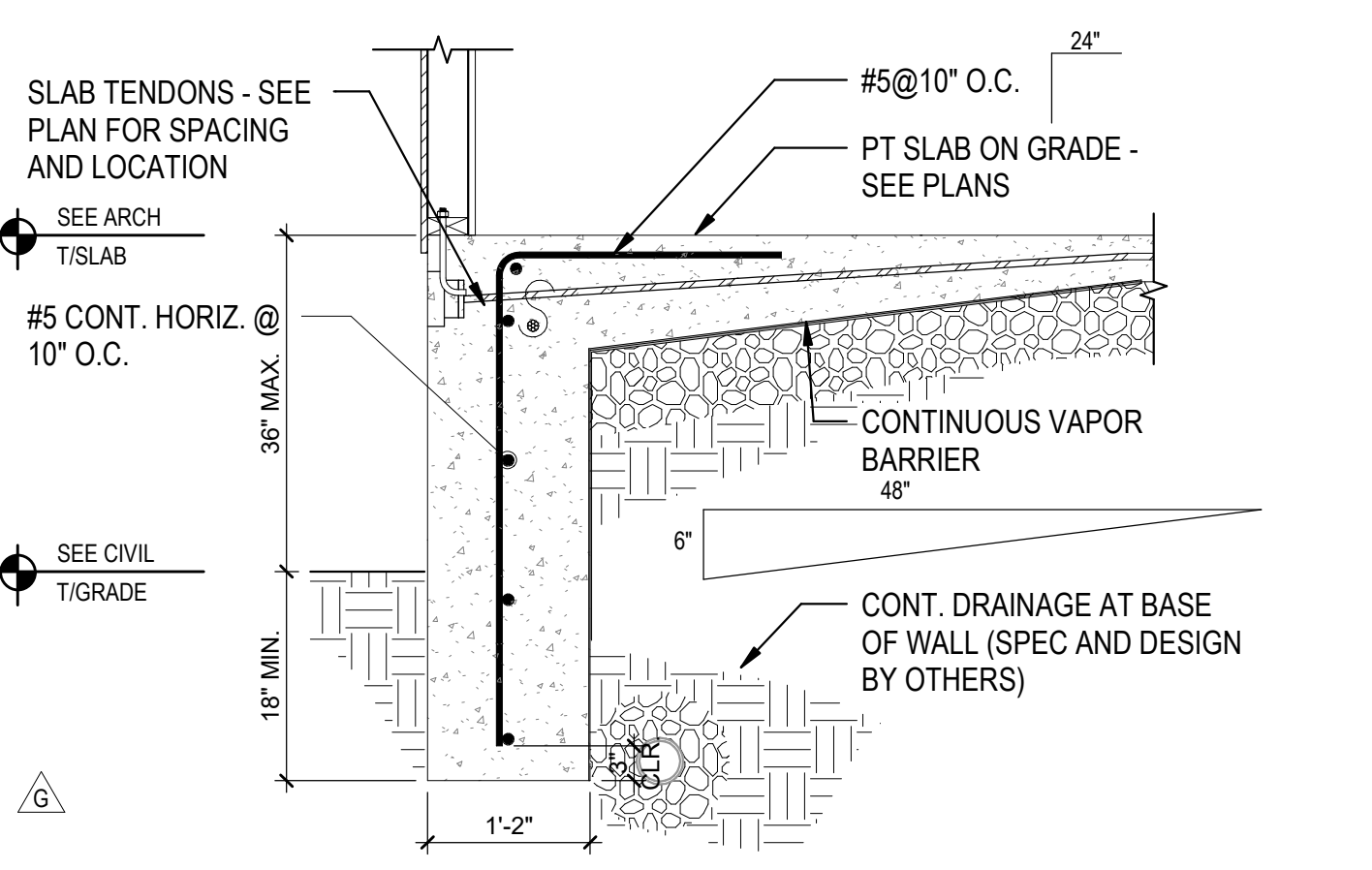


**9 SECTION THRU CURB IN GARAGES**

S3-01

SCALE: 3/4" = 1'-0"

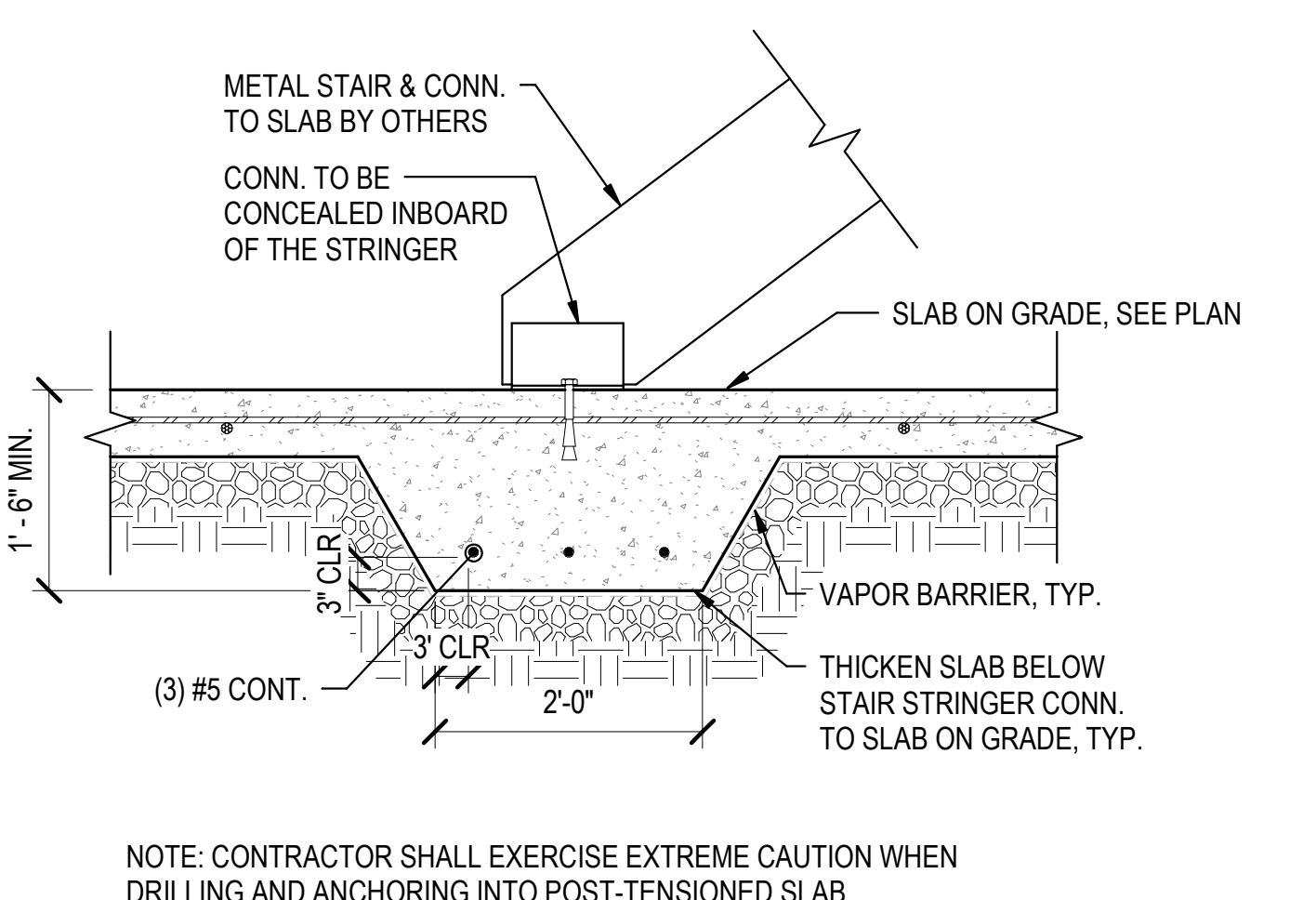
THIS DETAIL MAY BE USED AS AN ALTERNATE TO DETAILS 14 OR 15/S3-01 TO ELIMINATE THE INDEPENDENT FOOTING AND KNEE WALL



**10 SECTION THRU EXTENDED MONOLITHIC KNEE WALL**

S3-01

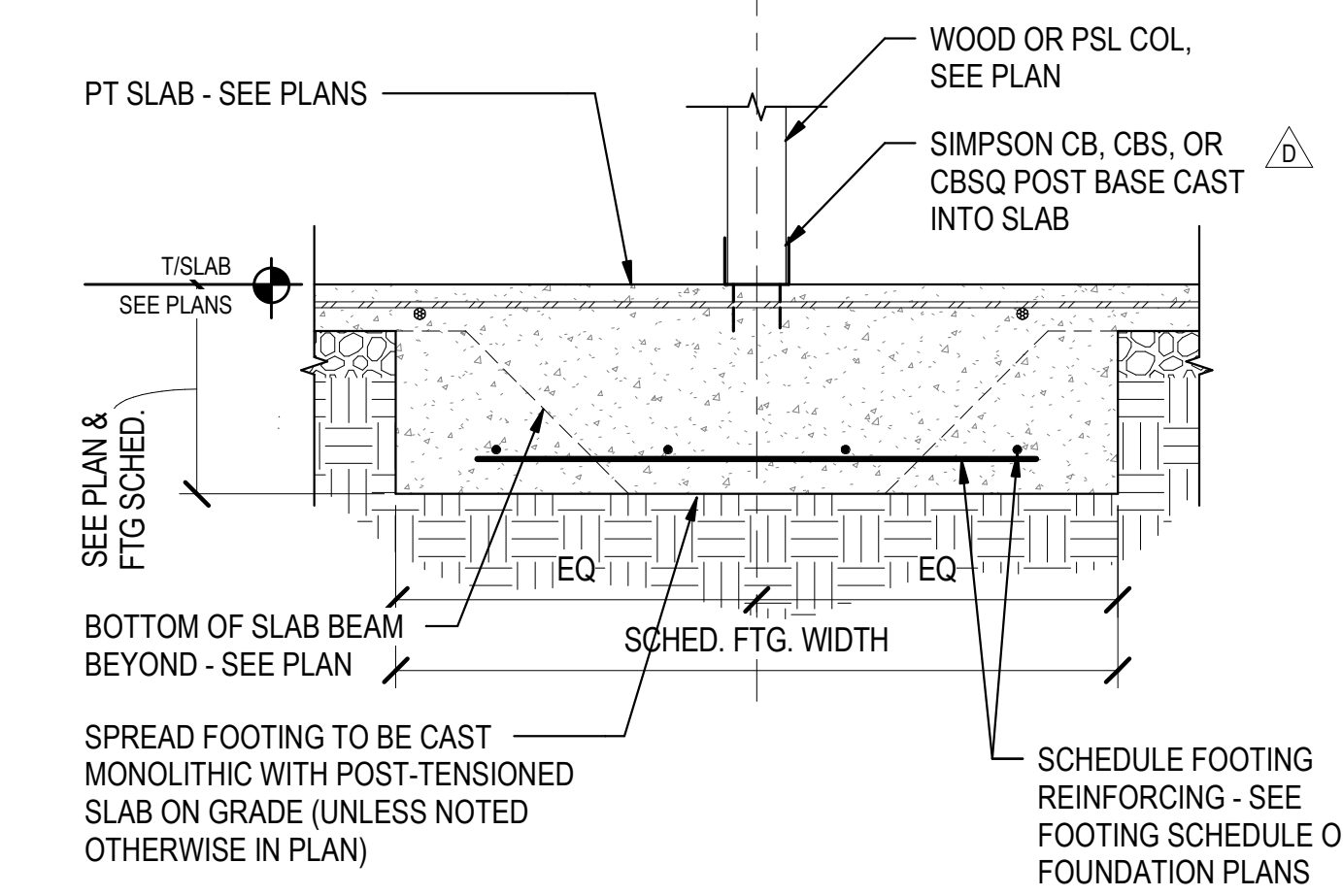
SCALE: 3/4" = 1'-0"



**11 THICKENED SLAB AT STAIR DETAIL**

S3-01

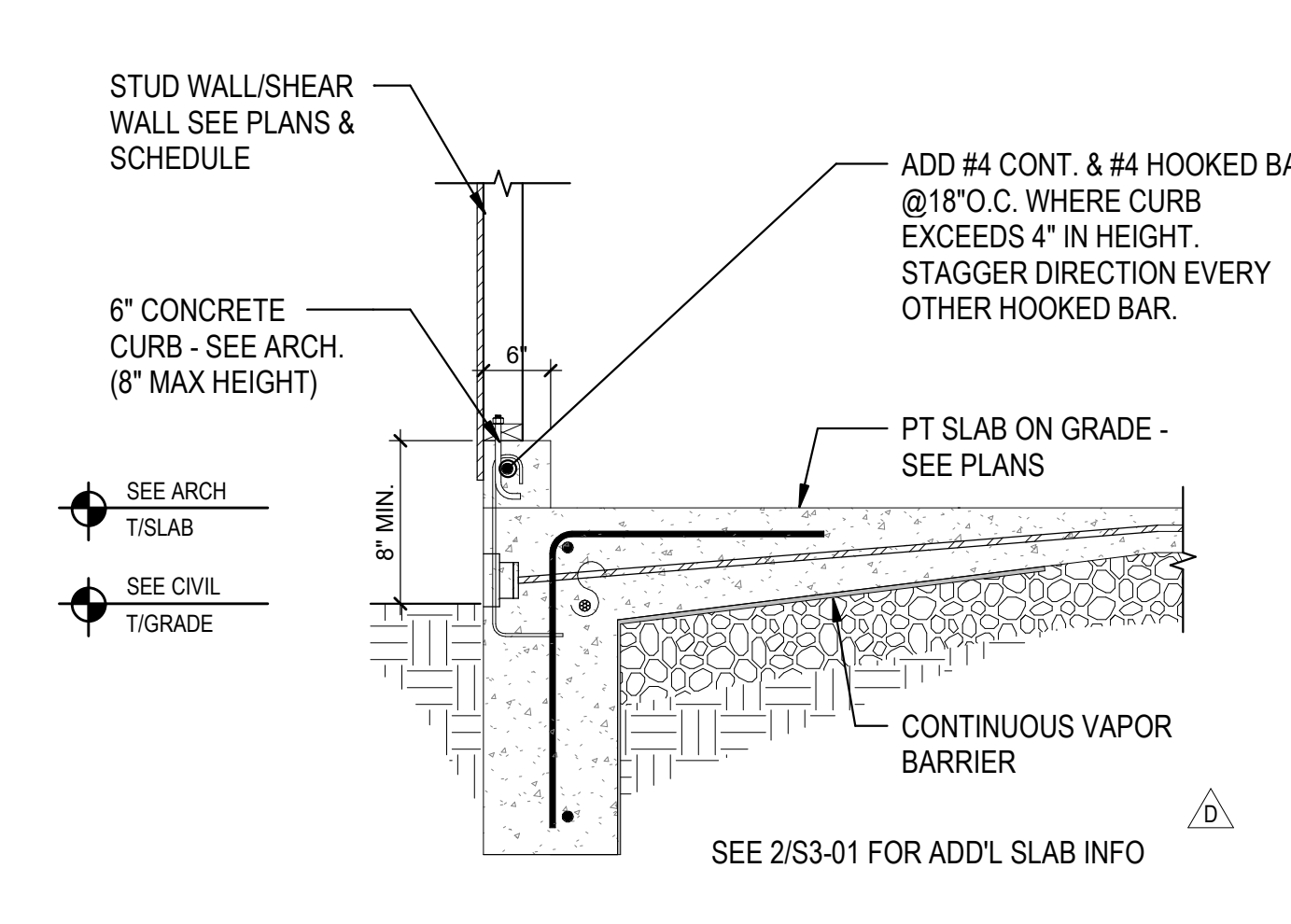
SCALE: 3/4" = 1'-0"



**5 SECTION THROUGH INTERIOR MONOLITHIC SPREAD FOOTING**

S3-01

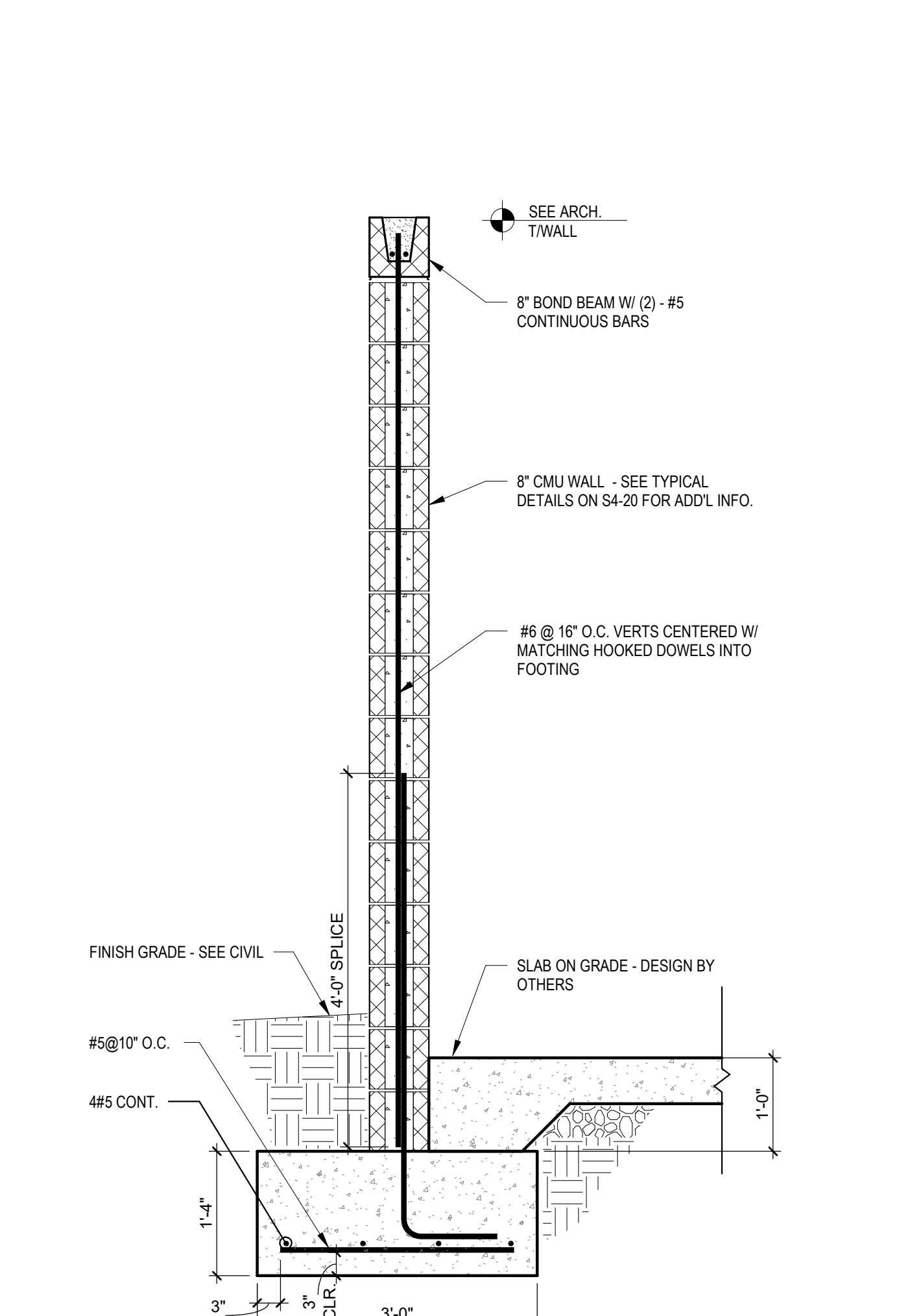
SCALE: 3/4" = 1'-0"



**6 SECTION THRU CURB AT EXTERIOR SLAB EDGE**

S3-01

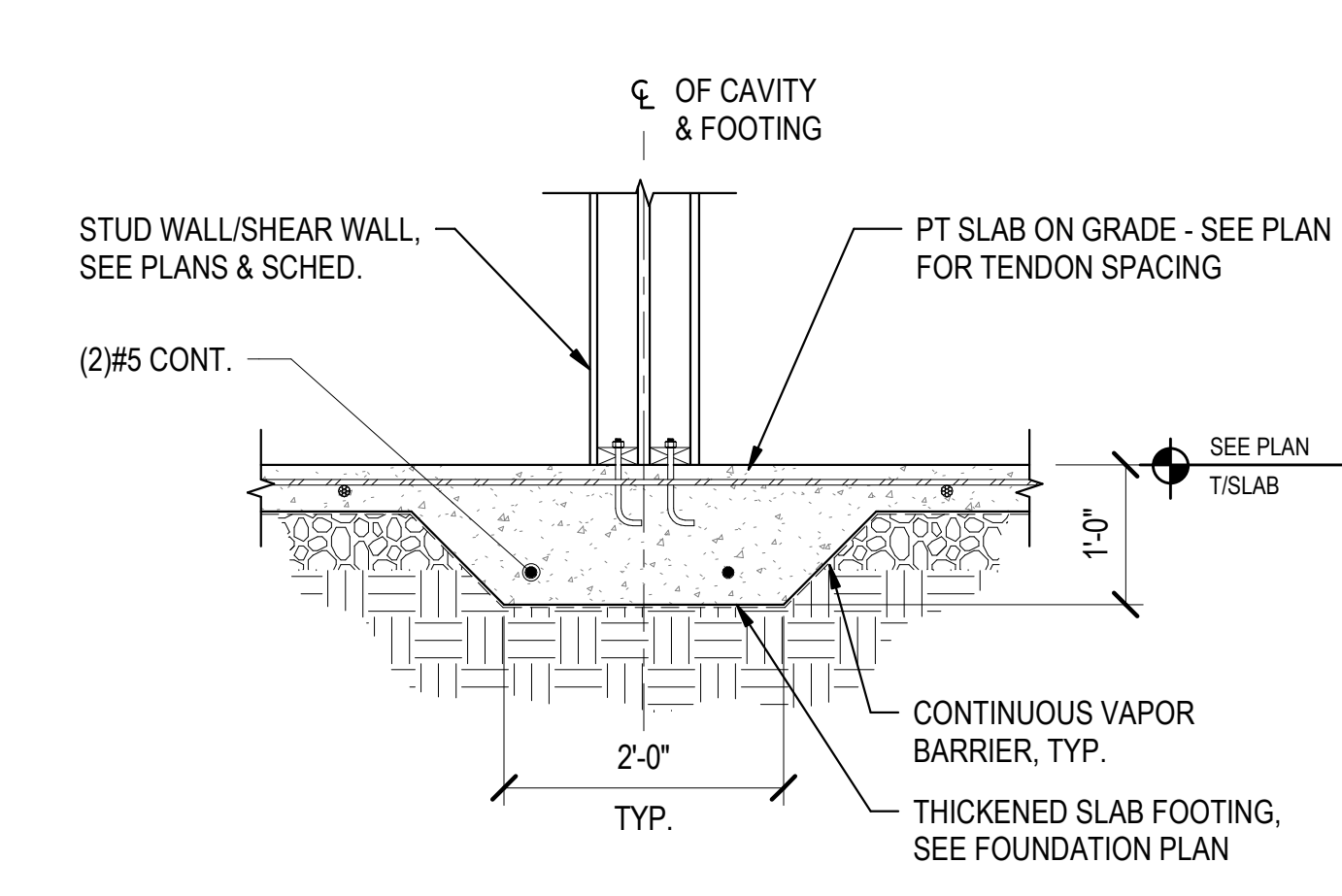
SCALE: 3/4" = 1'-0"



**8 SECTION THRU TRASH ENCLOSURE WALL**

S3-01

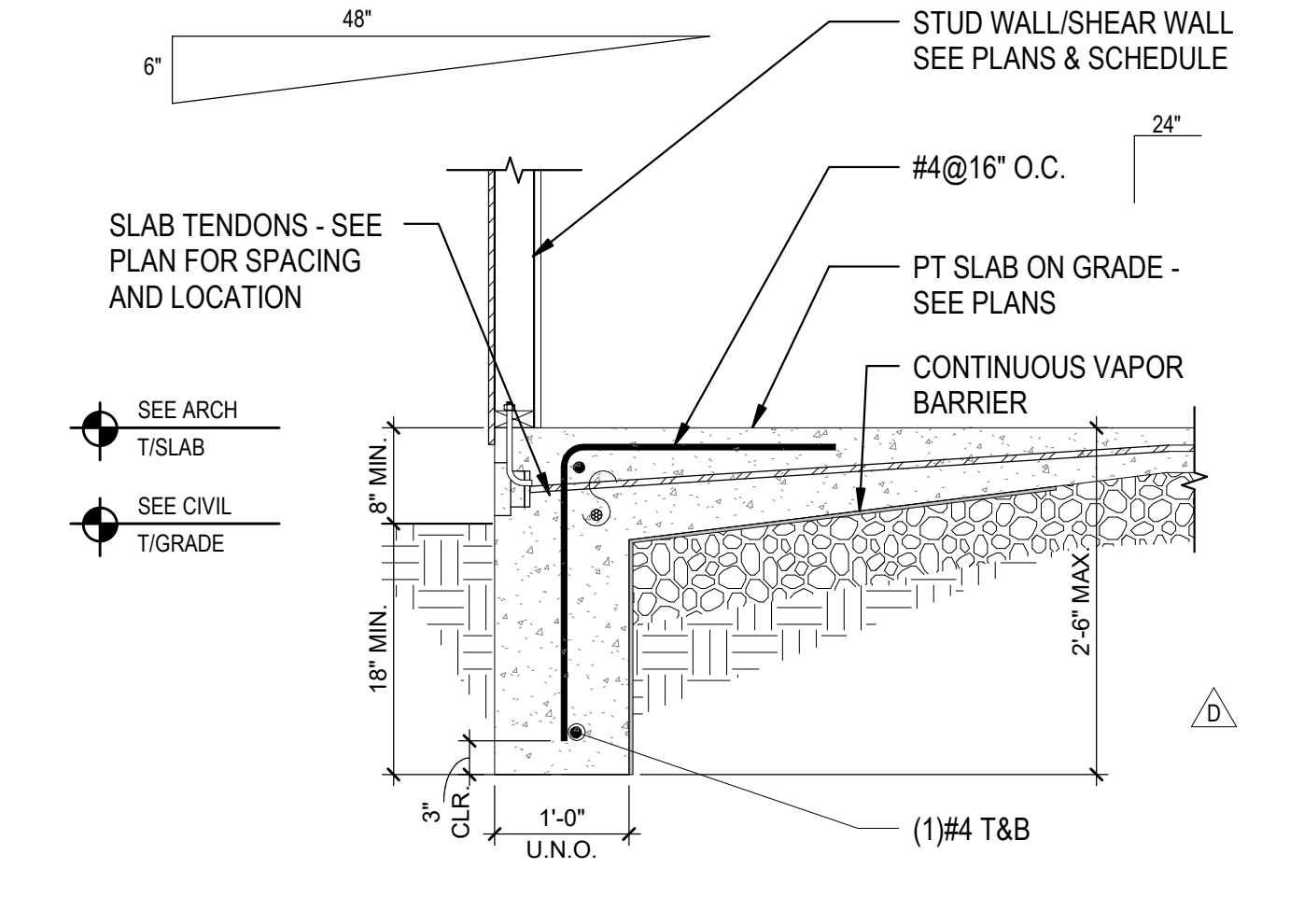
SCALE: 3/4" = 1'-0"



**1 SECTION THRU PARTY WALL FOOTING**

S3-01

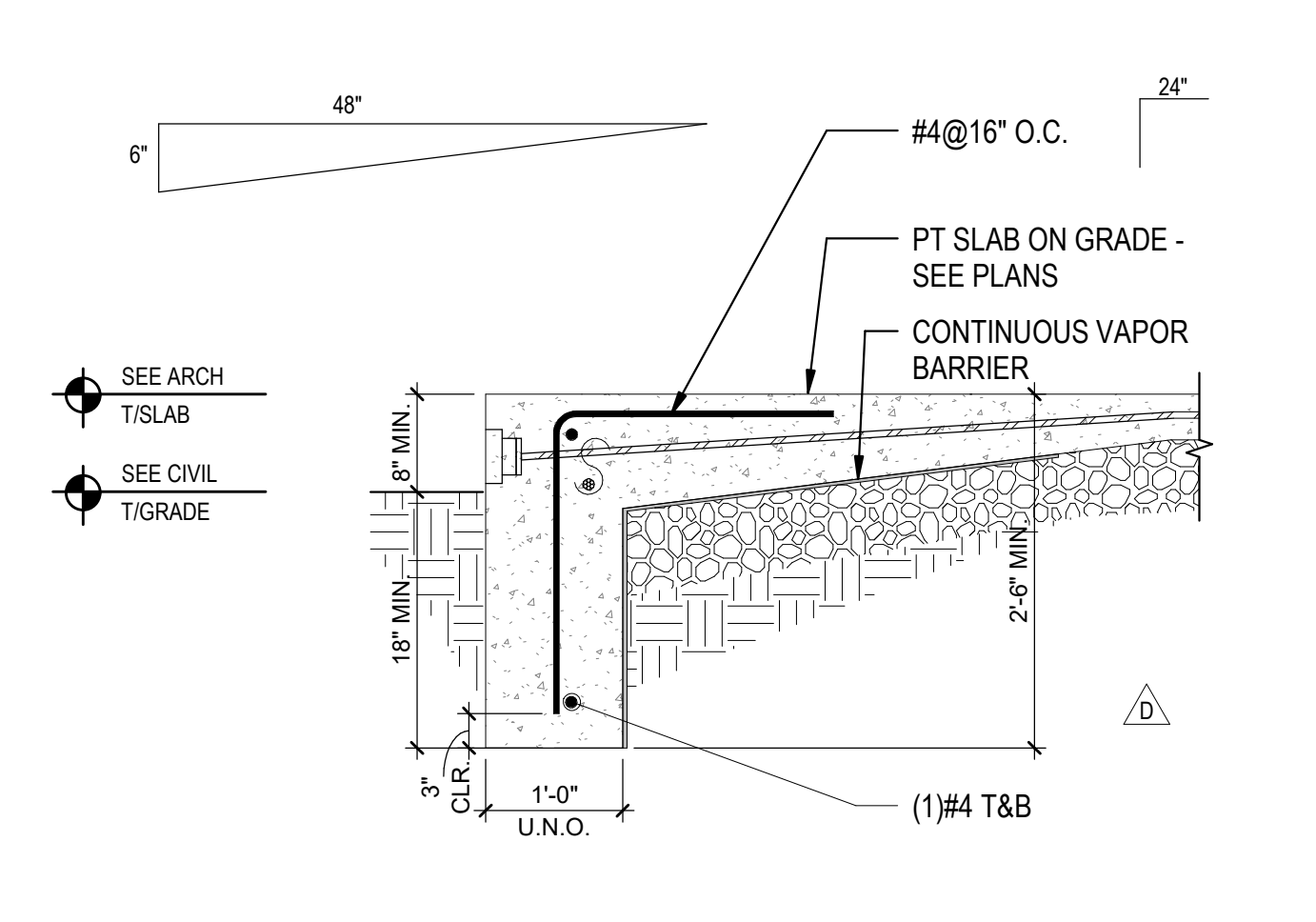
SCALE: 3/4" = 1'-0"



**2 SECTION THRU EXTERIOR SLAB EDGE TRUNDOWN**

S3-01

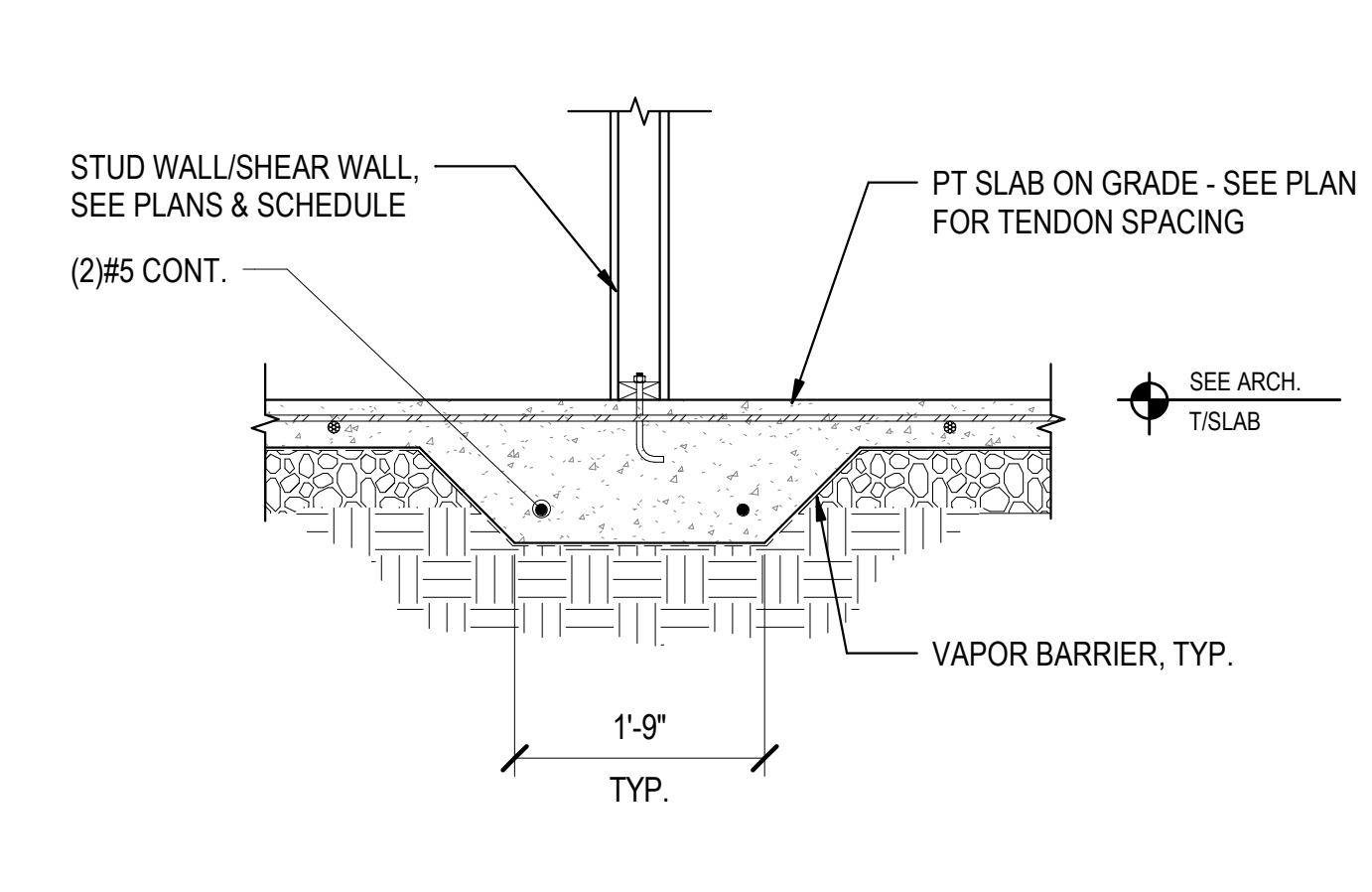
SCALE: 3/4" = 1'-0"



**3 SECTION THRU EXTERIOR SLAB EDGE**

S3-01

SCALE: 3/4" = 1'-0"



**4 SECTION THRU THICKENED SLAB AT BEARING WALL AND/OR SHEAR WALL**

S3-01

SCALE: 3/4" = 1'-0"



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PROJECT

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PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

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DRAWING TITLE FOUNDATION SECTIONS AND DETAILS

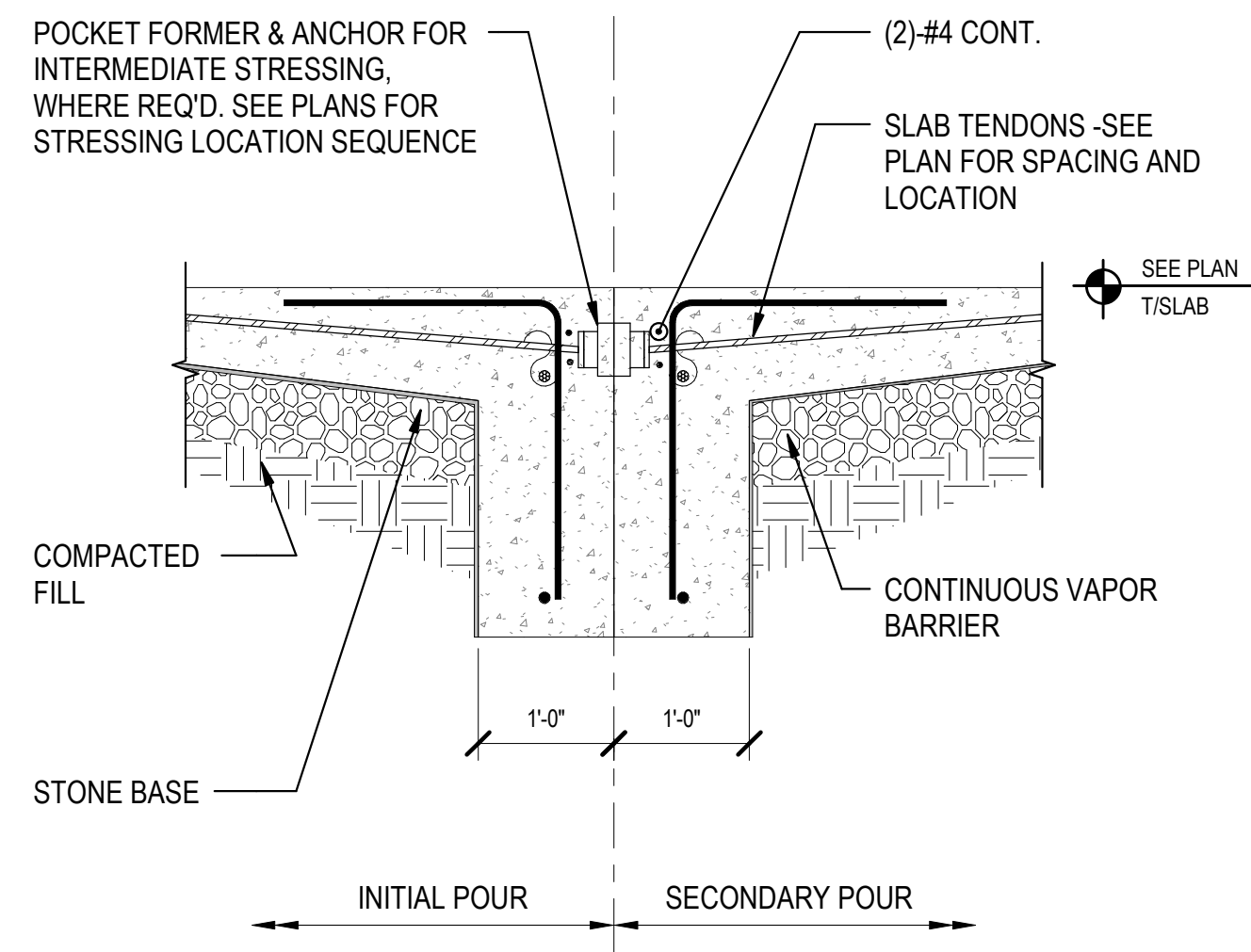
SHEET NUMBER

**S3-01**

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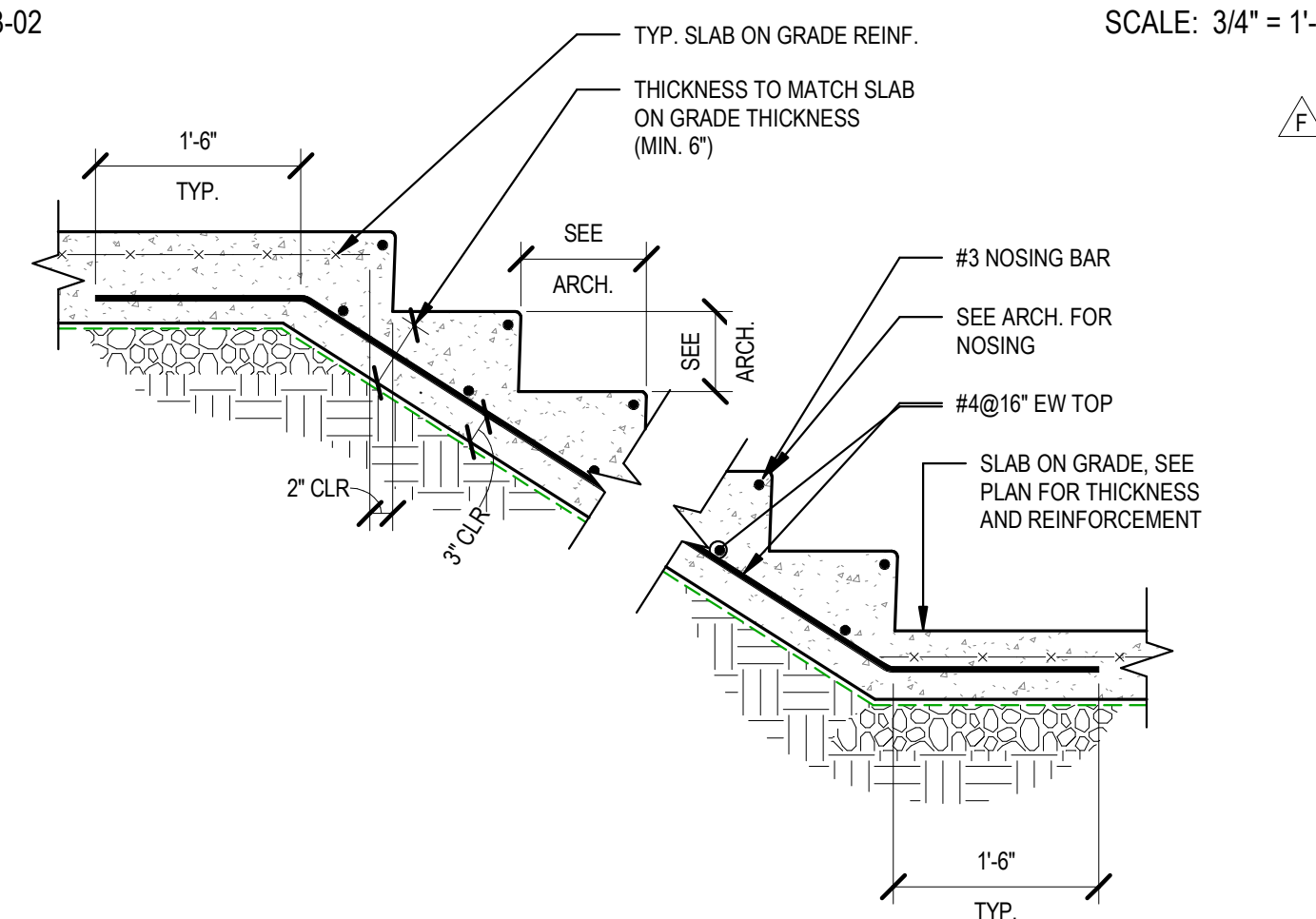
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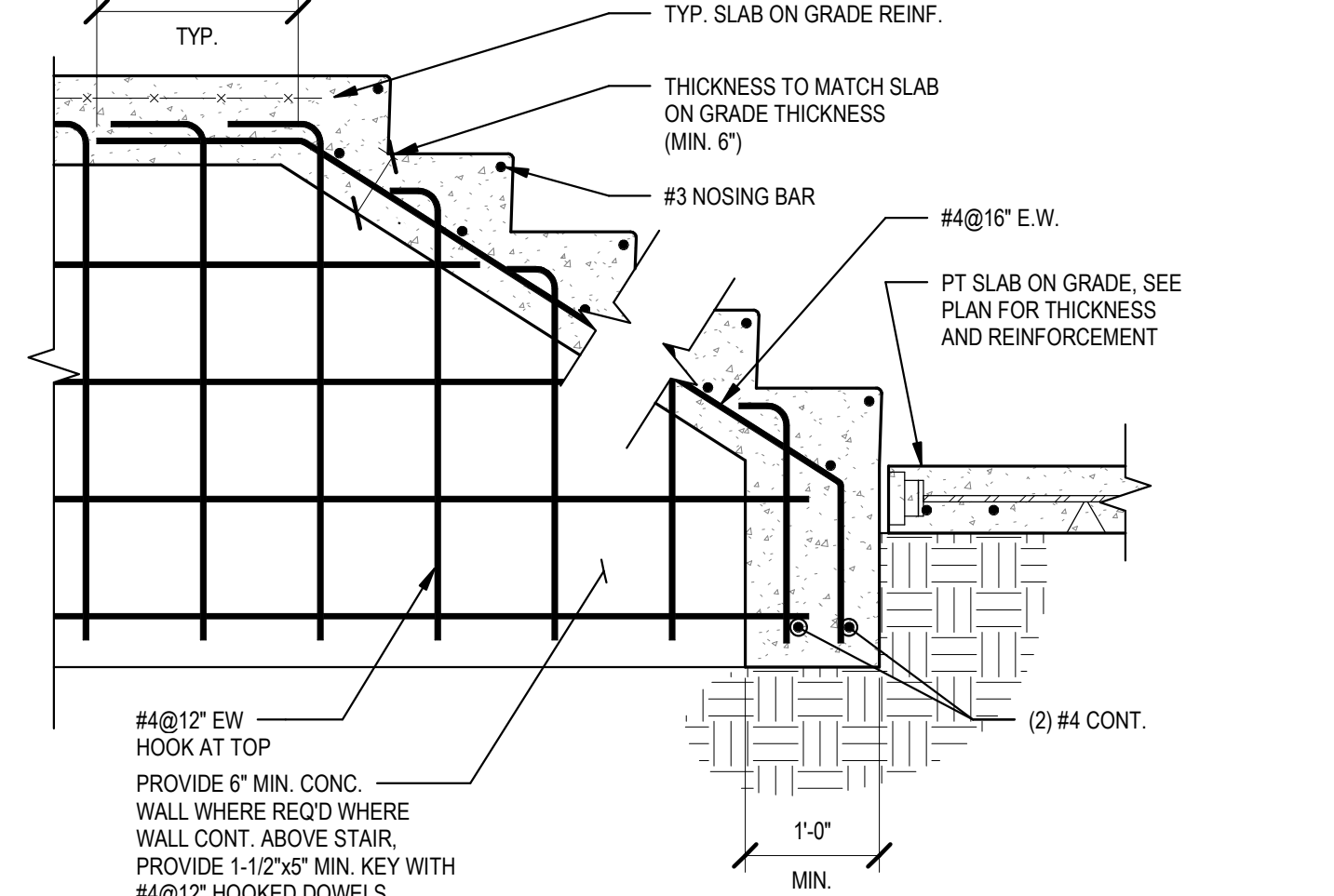
**5 POST-TENSIONED SLAB ON GRADE CONSTRUCTION JOINT**

S3-02 SCALE: 3/4" = 1'-0"



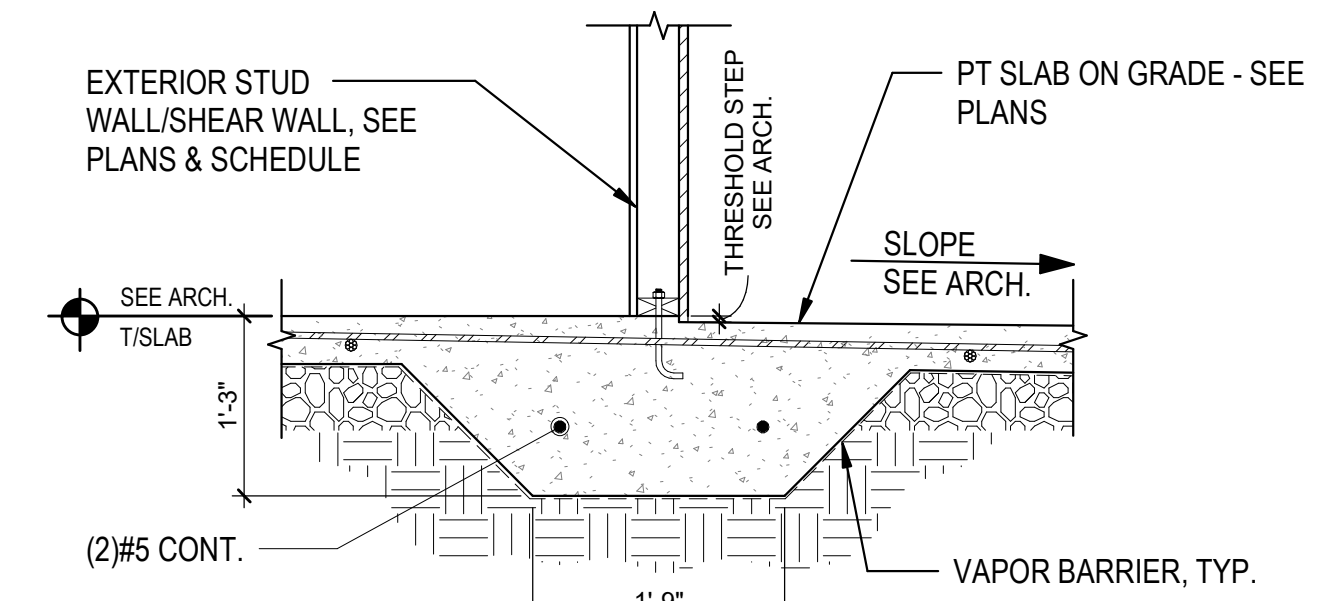
**6 CONCRETE STAIR-ON-GRADE**

S3-02 SCALE: 3/4" = 1'-0"



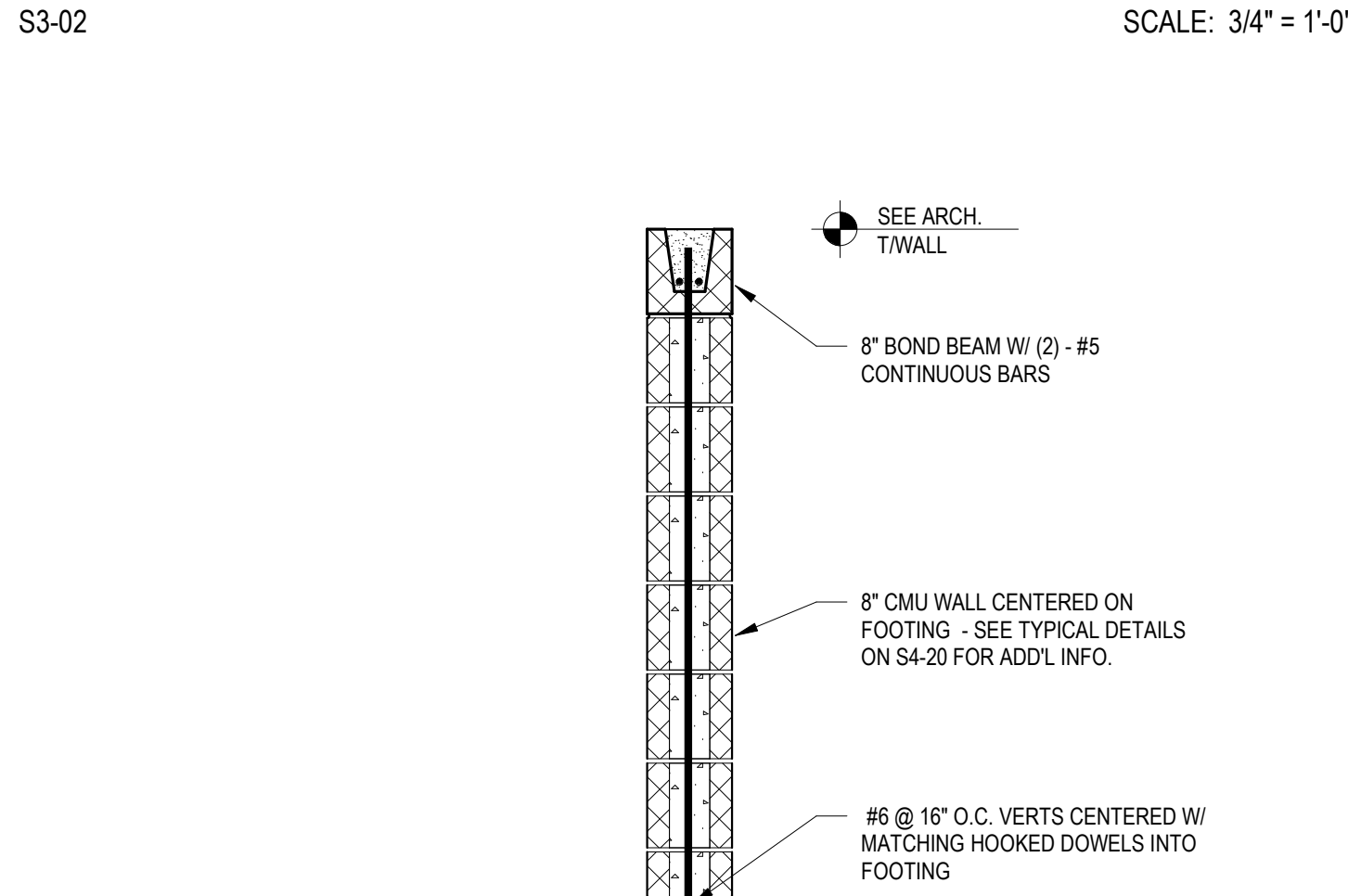
**7 CONCRETE STAIR-ON-GRADE WITH CHEEK WALL**

S3-02 SCALE: 3/4" = 1'-0"



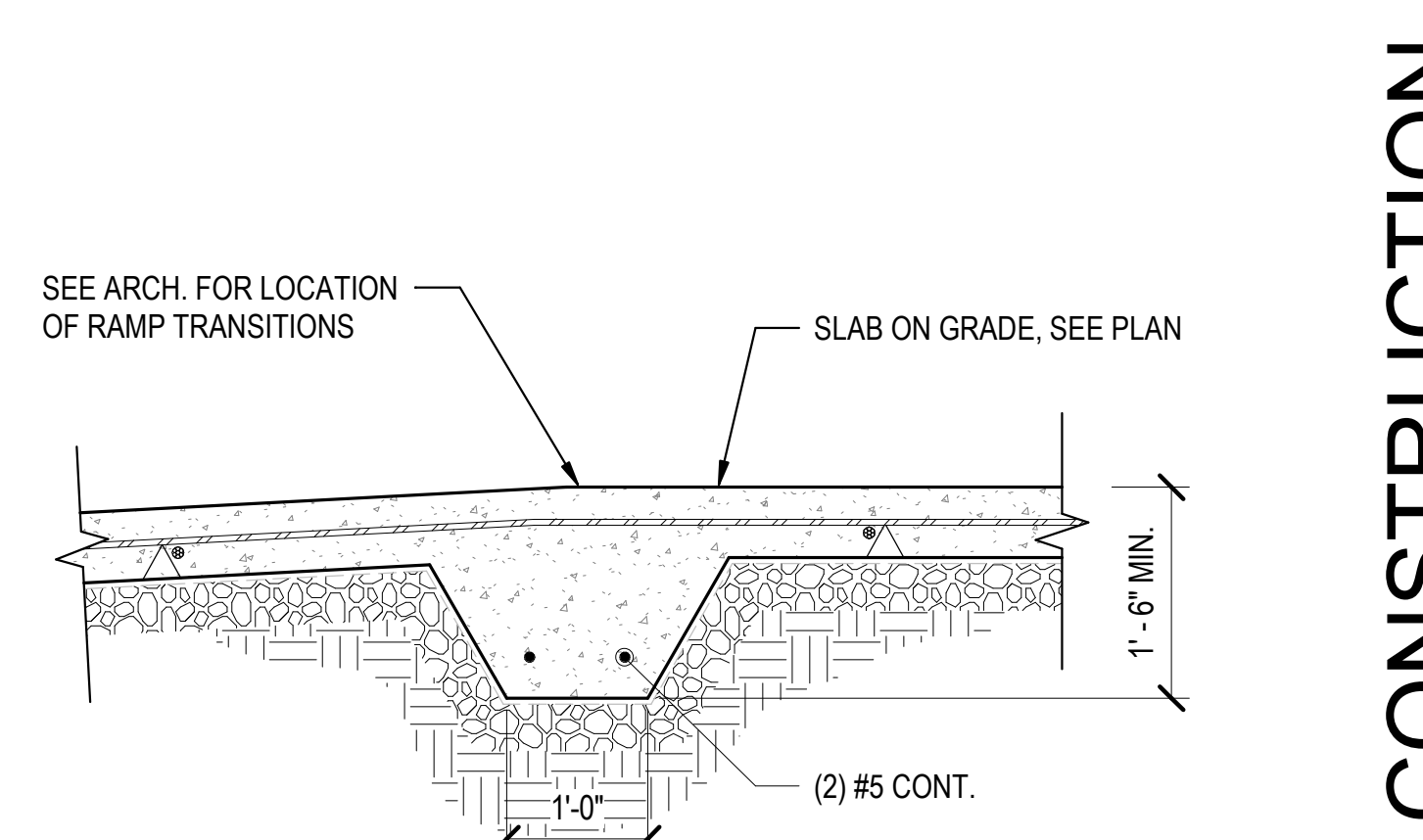
**1 SECTION THRU THICKENED SLAB AT EXTERIOR WALL AND PATIO**

S3-02 SCALE: 3/4" = 1'-0"



**3 SECTION THRU SCREEN WALL**

S3-02 SCALE: 3/4" = 1'-0"



**4 RAMP ON GRADE**

S3-02 SCALE: 3/4" = 1'-0"



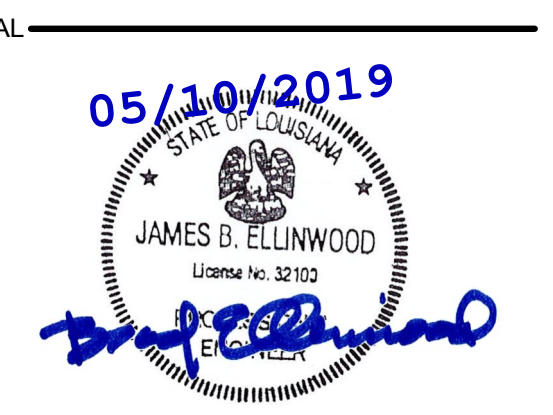
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**FOUNDATION SECTIONS AND DETAILS**

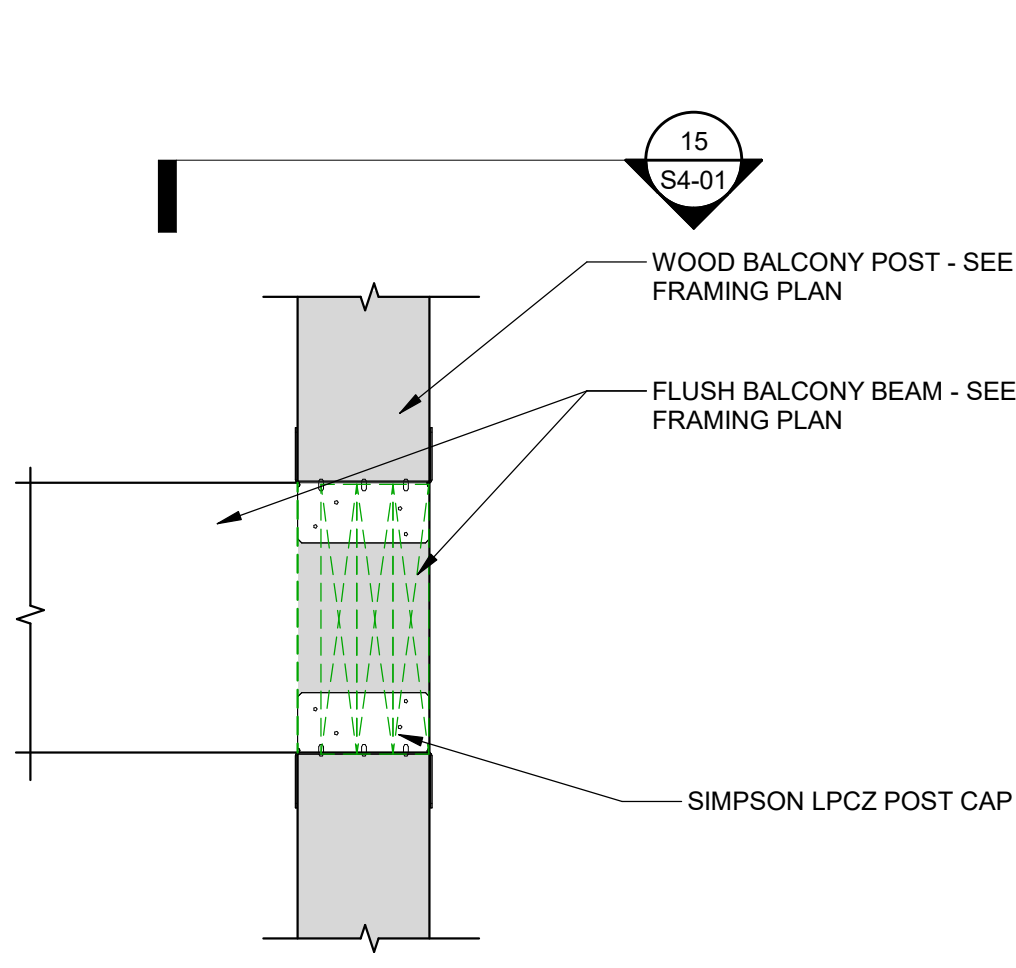
SHEET NUMBER

**S3-02**

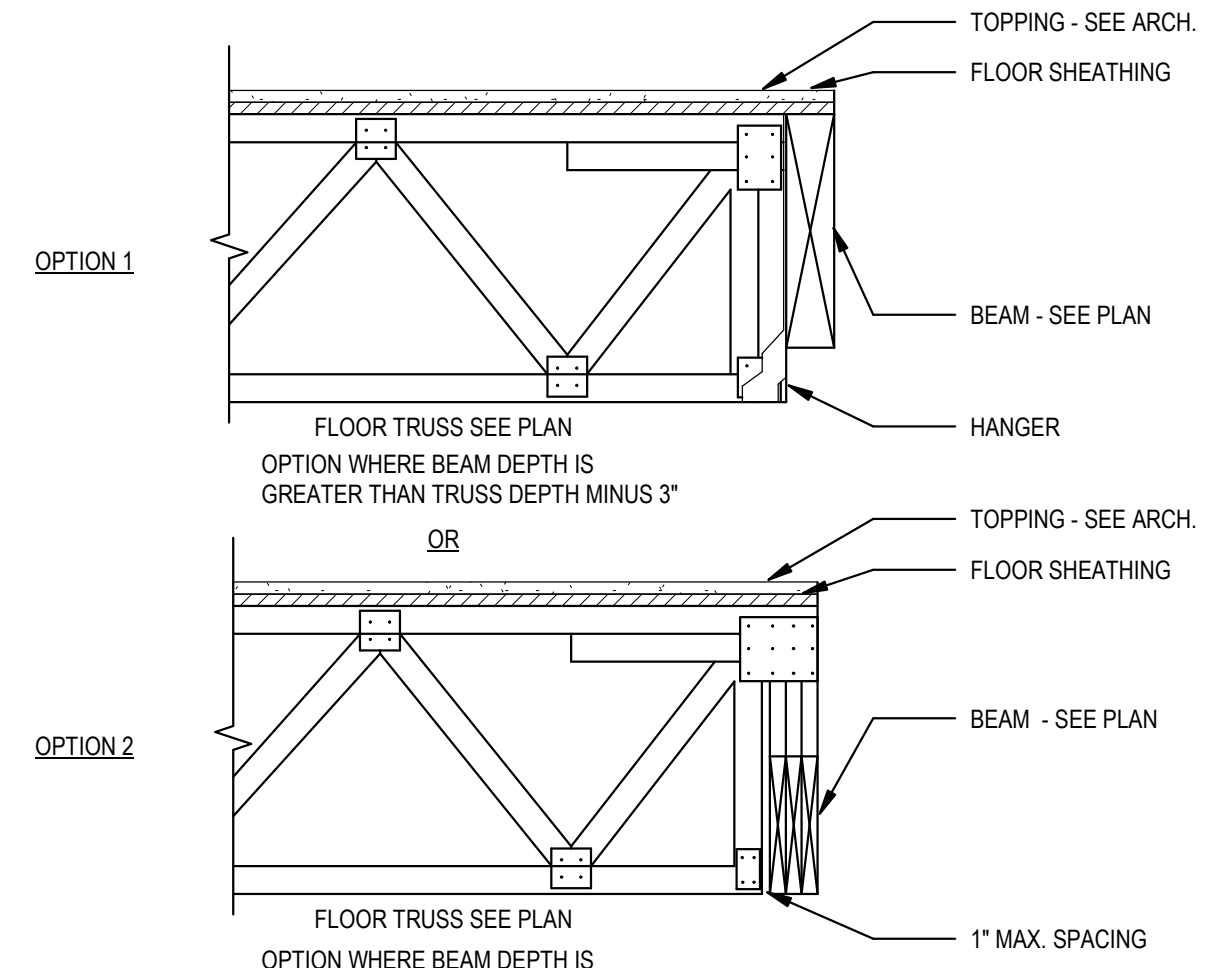
5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

PHILLIPSPART.COM 770-394-1616

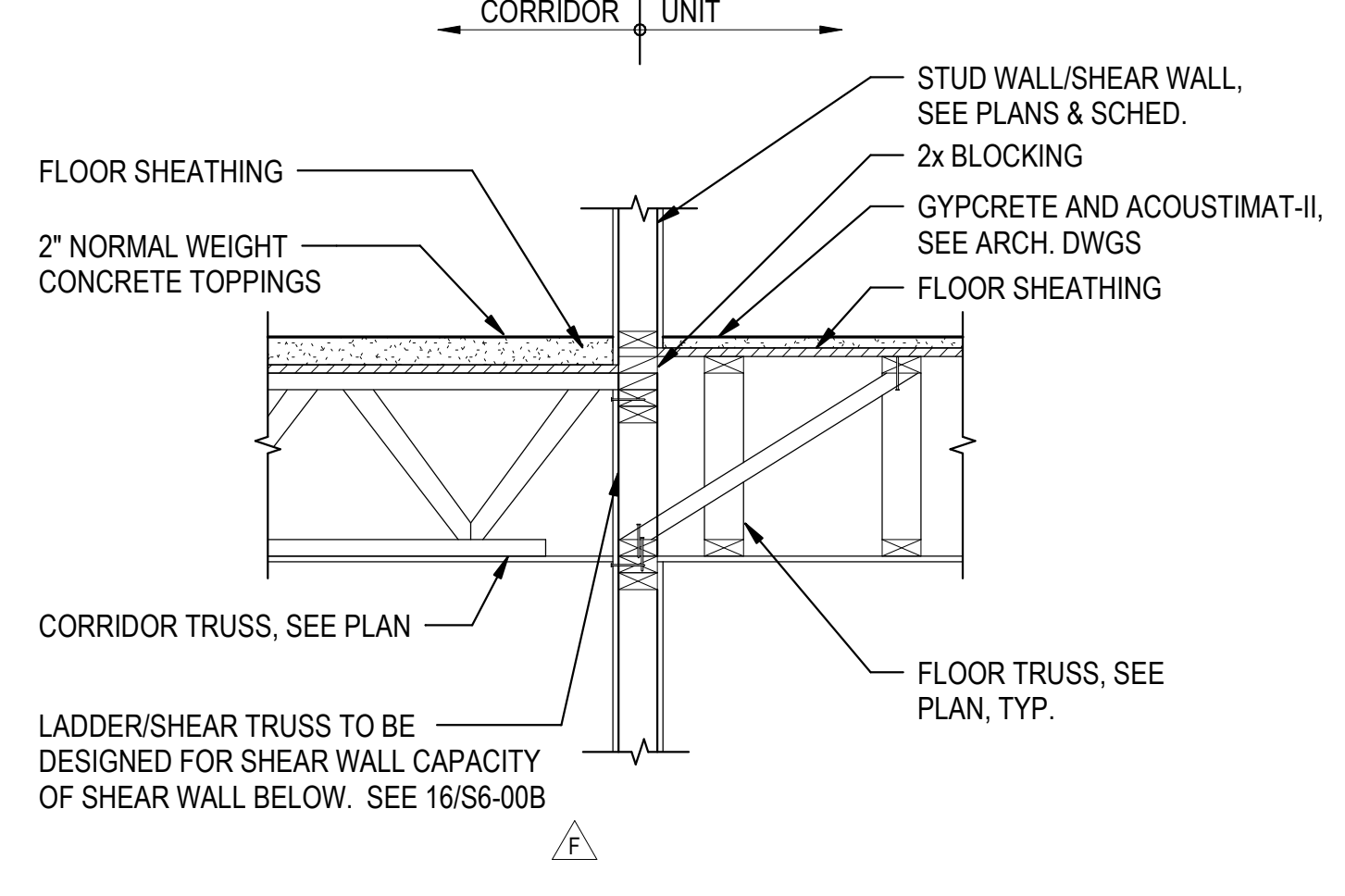
**FOR CONSTRUCTION**



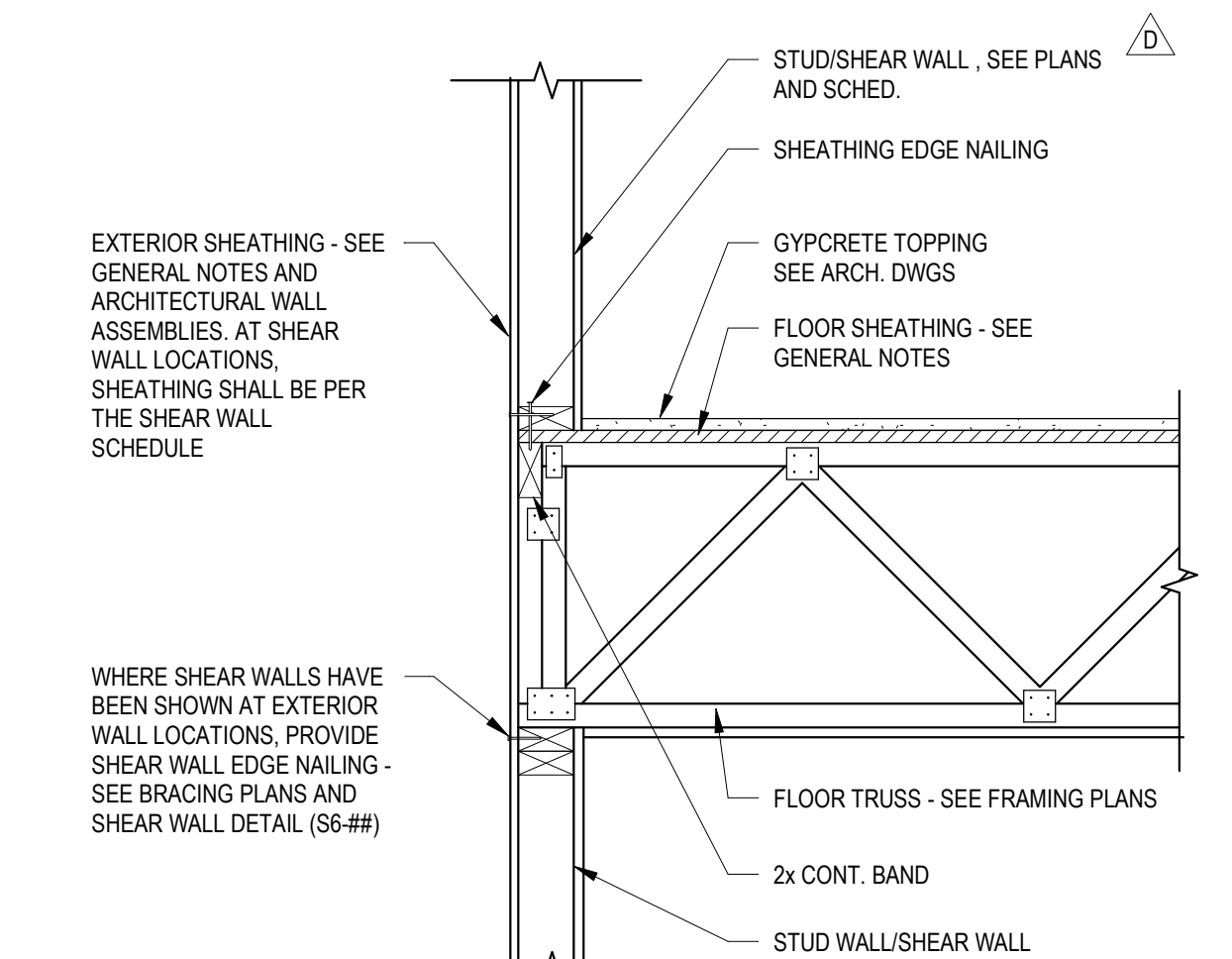
**13 BALCONY BEAM TO POST CONNECTION DETAIL**  
S4-01 E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"



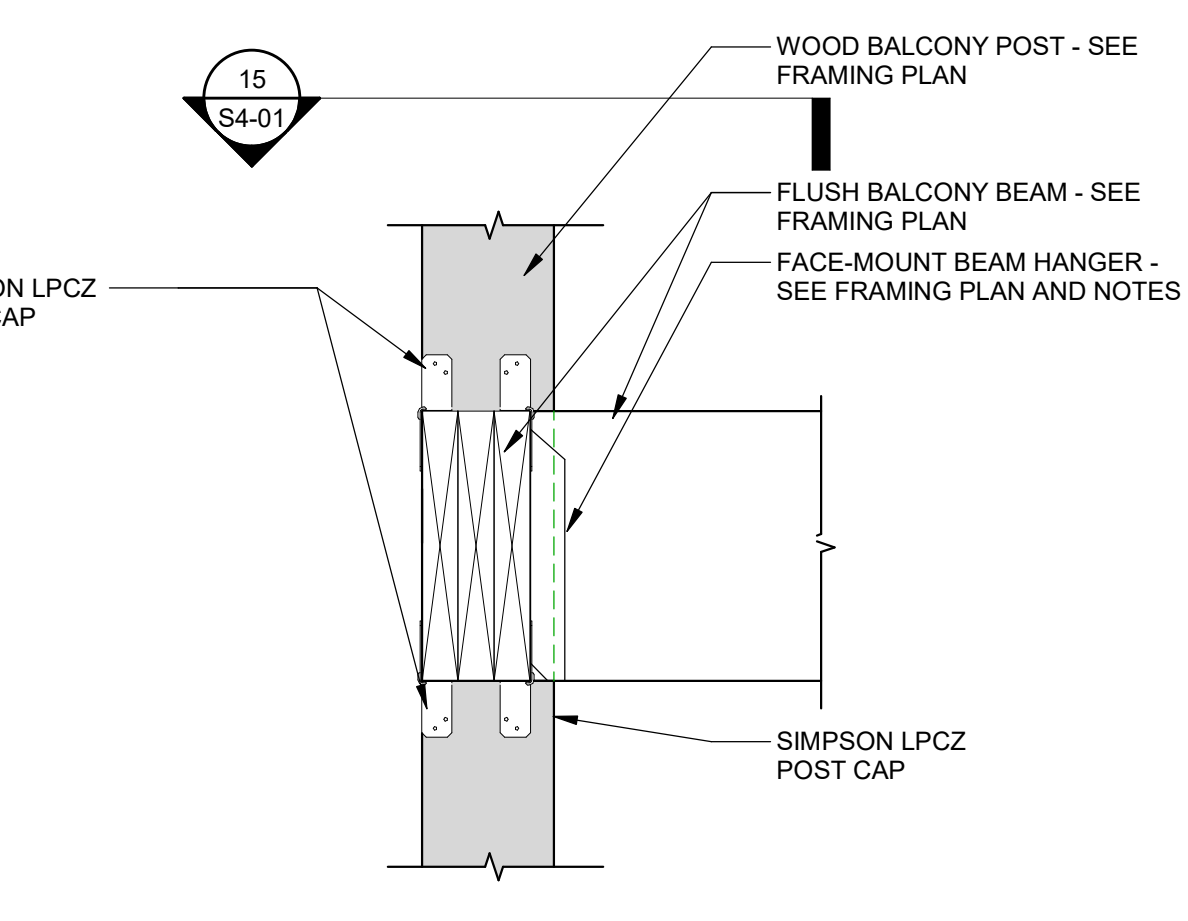
**9 SECTION AT FLUSH BEAM**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



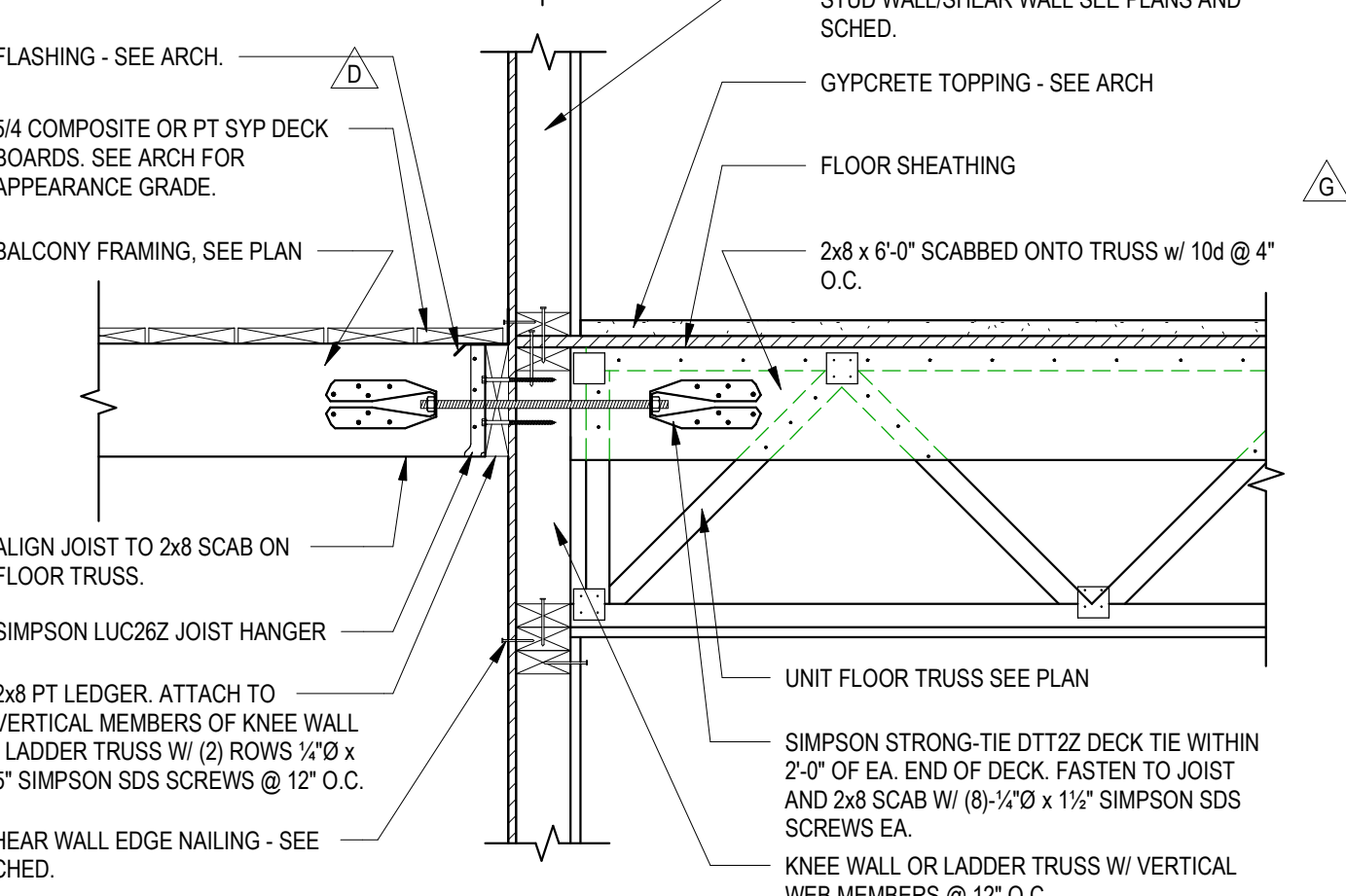
**5 SECTION ALONG FLOOR FRAMING AT CORRIDOR**  
S4-01 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



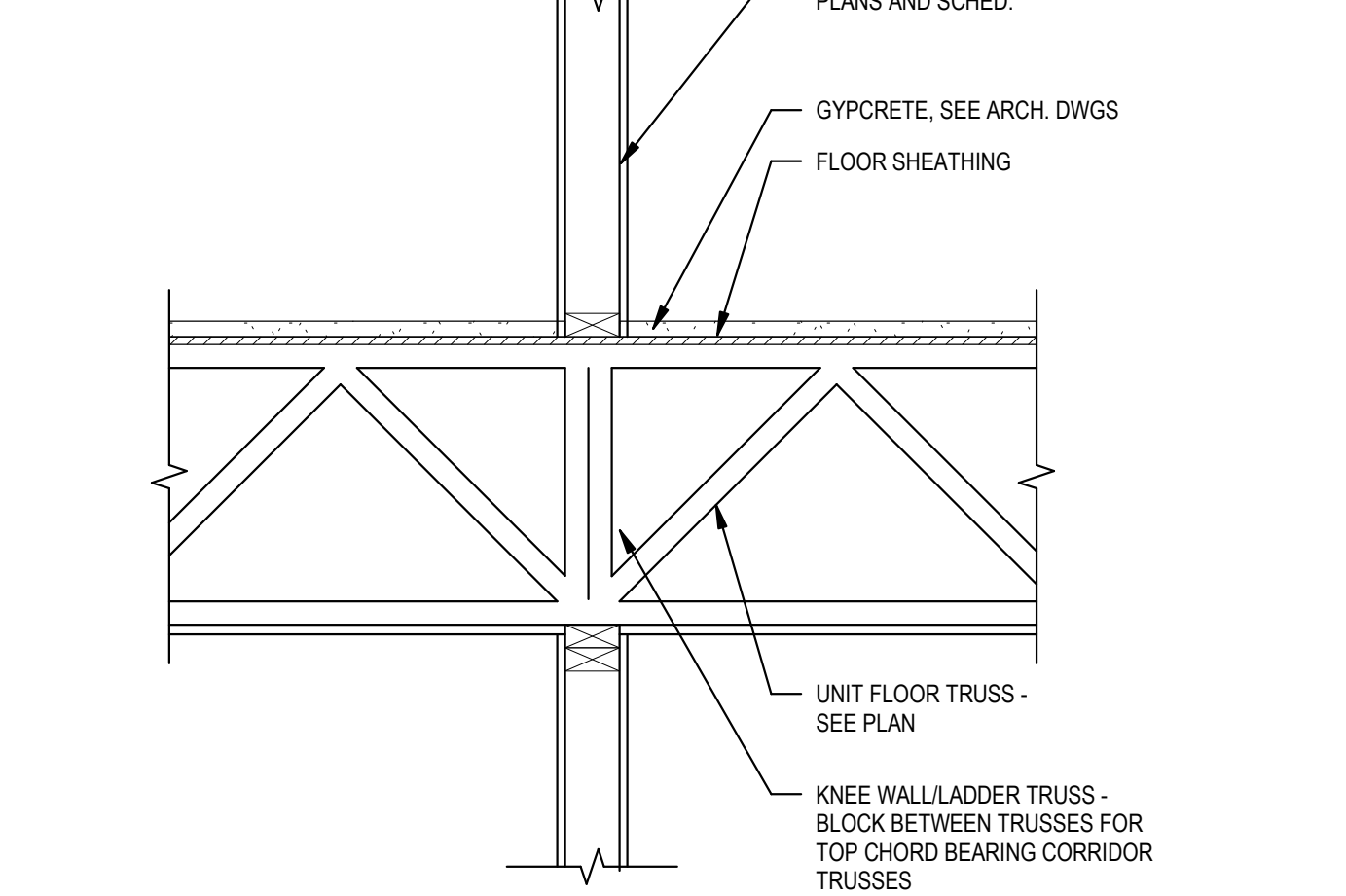
**1 EXTERIOR LOAD-BEARING WALL AT TYPE V CONSTRUCTION**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



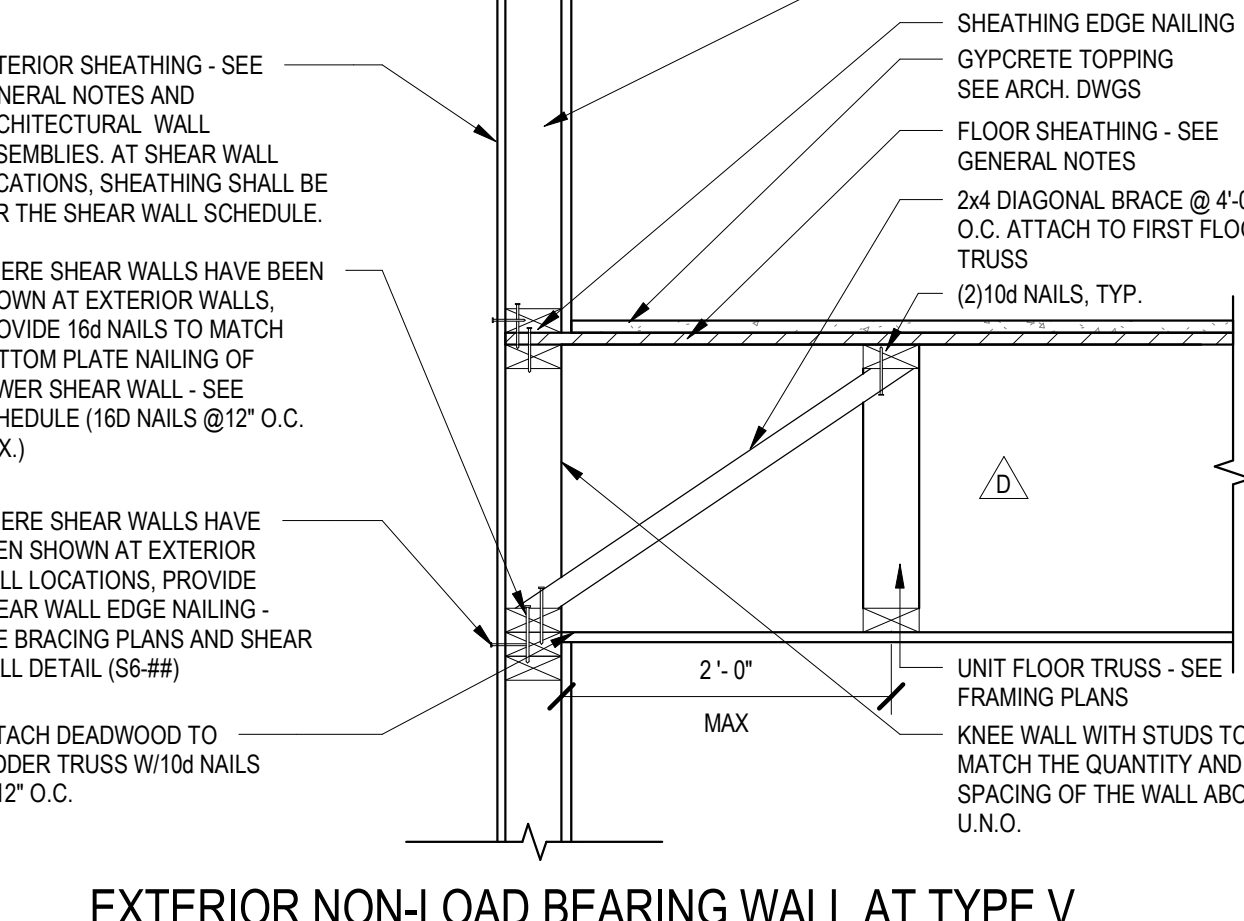
**14 BALCONY BEAM TO POST CONNECTION DETAIL**  
S4-01 E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"



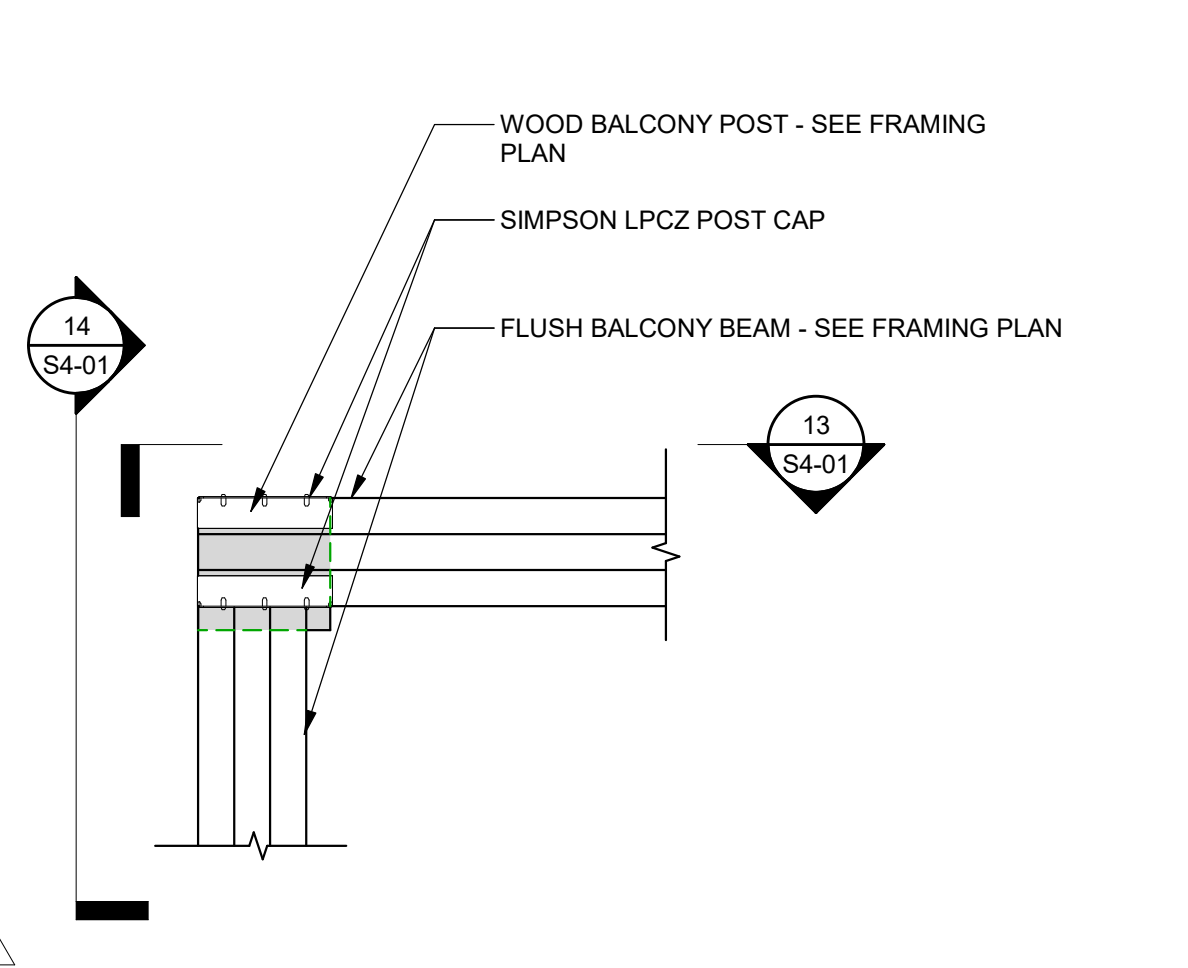
**10 FLOOR FRAMING AT BALCONY**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



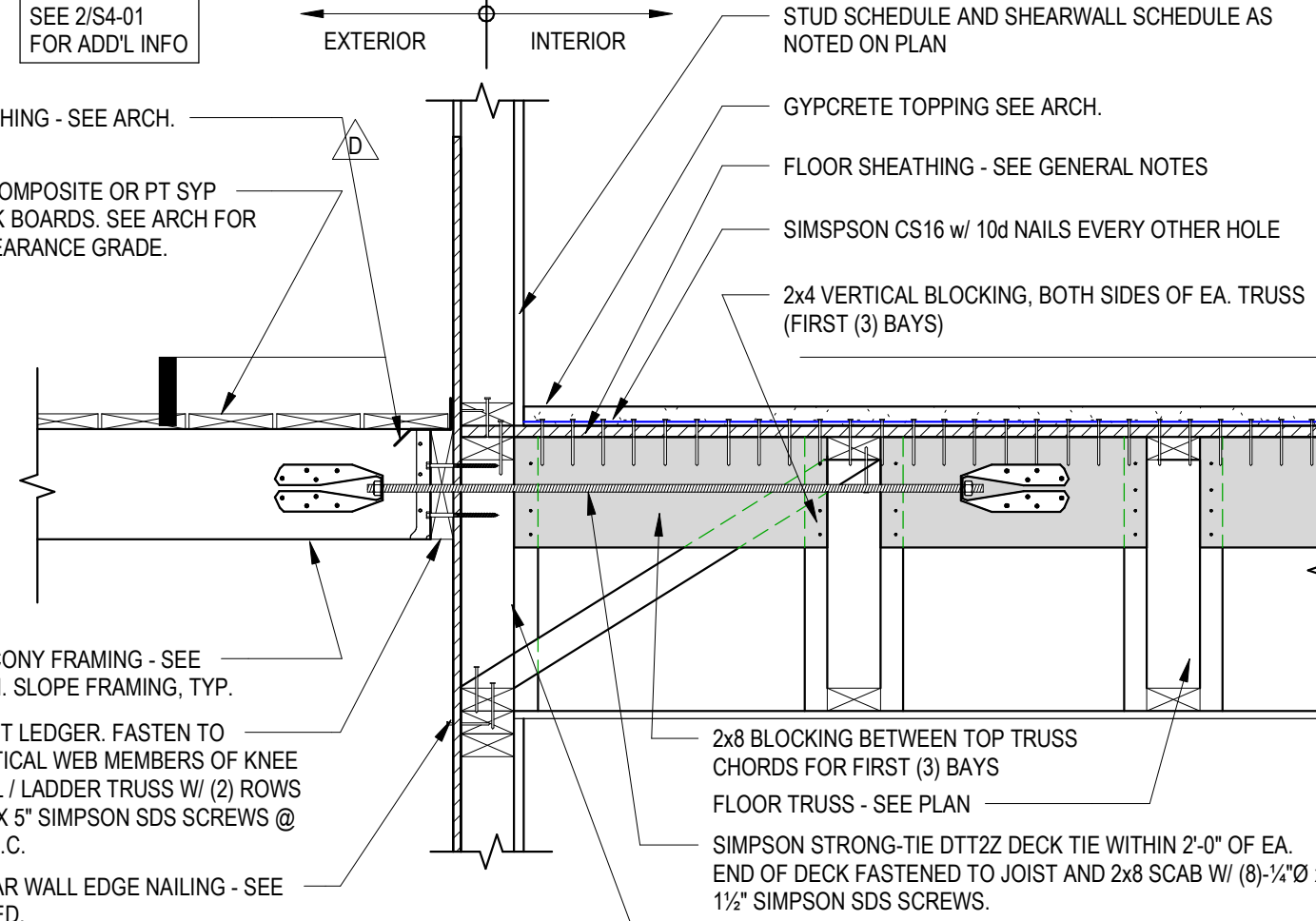
**6 SECTION ALONG FLOOR FRAMING AT BEARING WALL**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



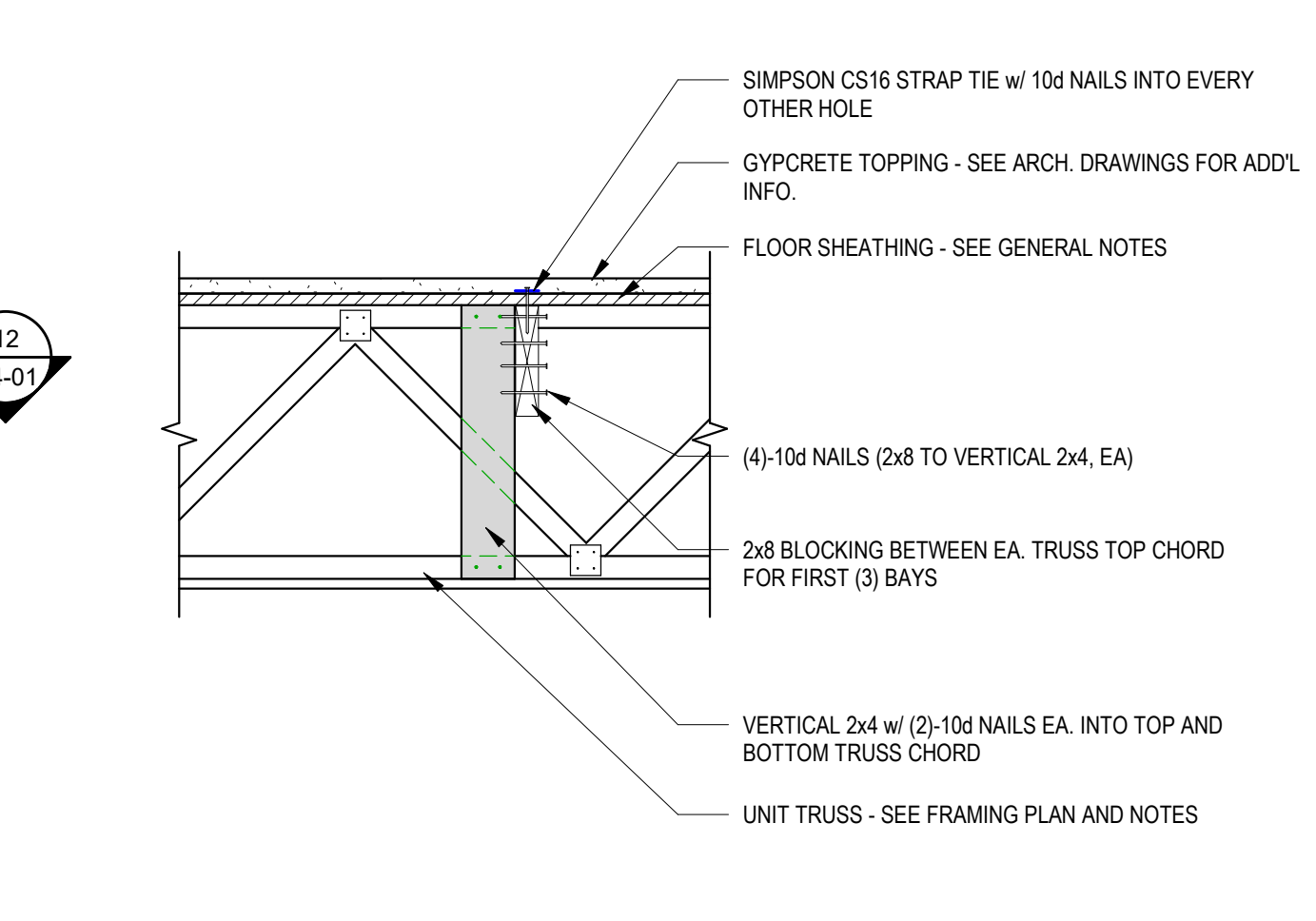
**2 EXTERIOR NON-LOAD BEARING WALL AT TYPE V CONSTRUCTION**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



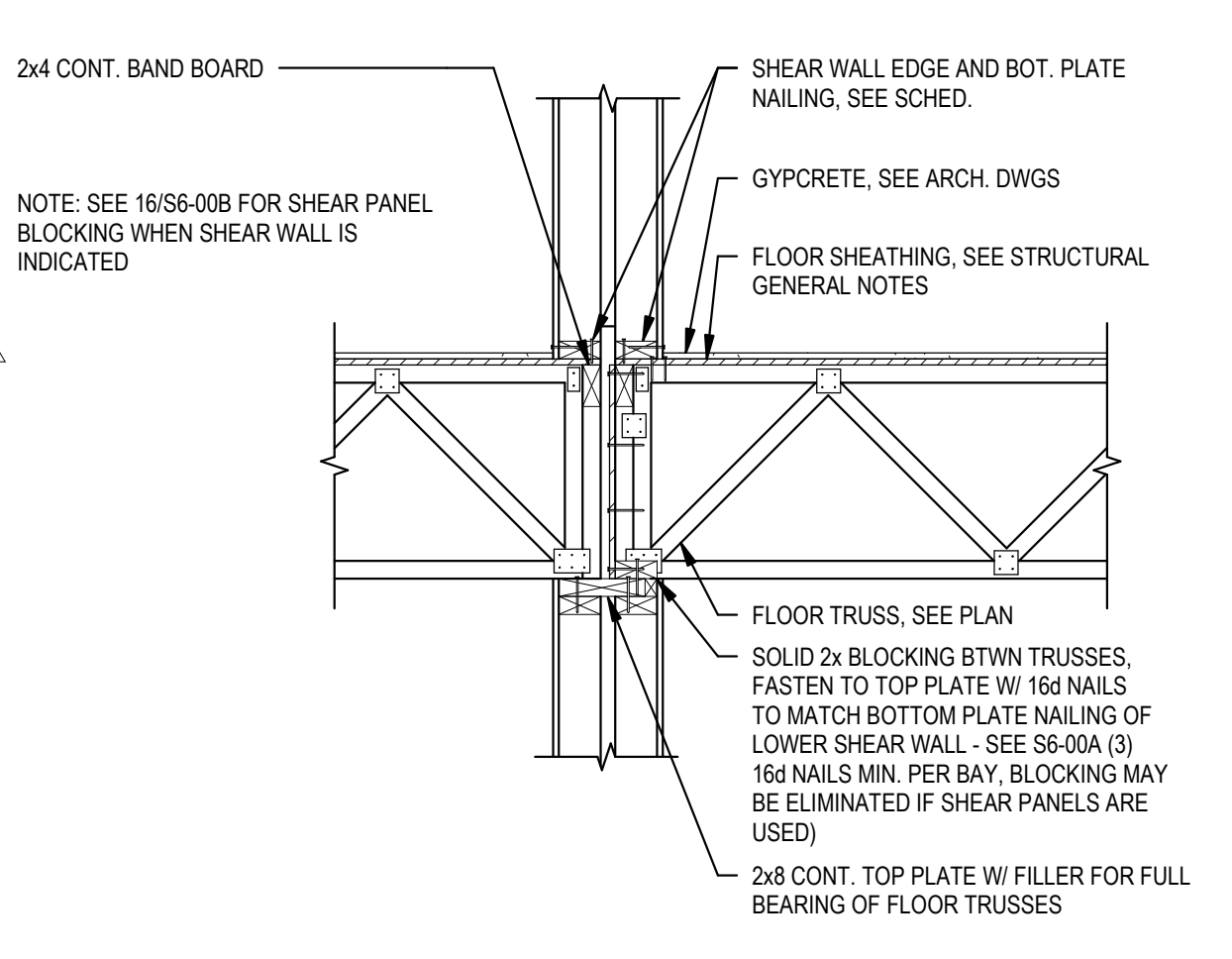
**15 BALCONY BEAM TO POST CONNECTION DETAIL**  
S4-01 E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"



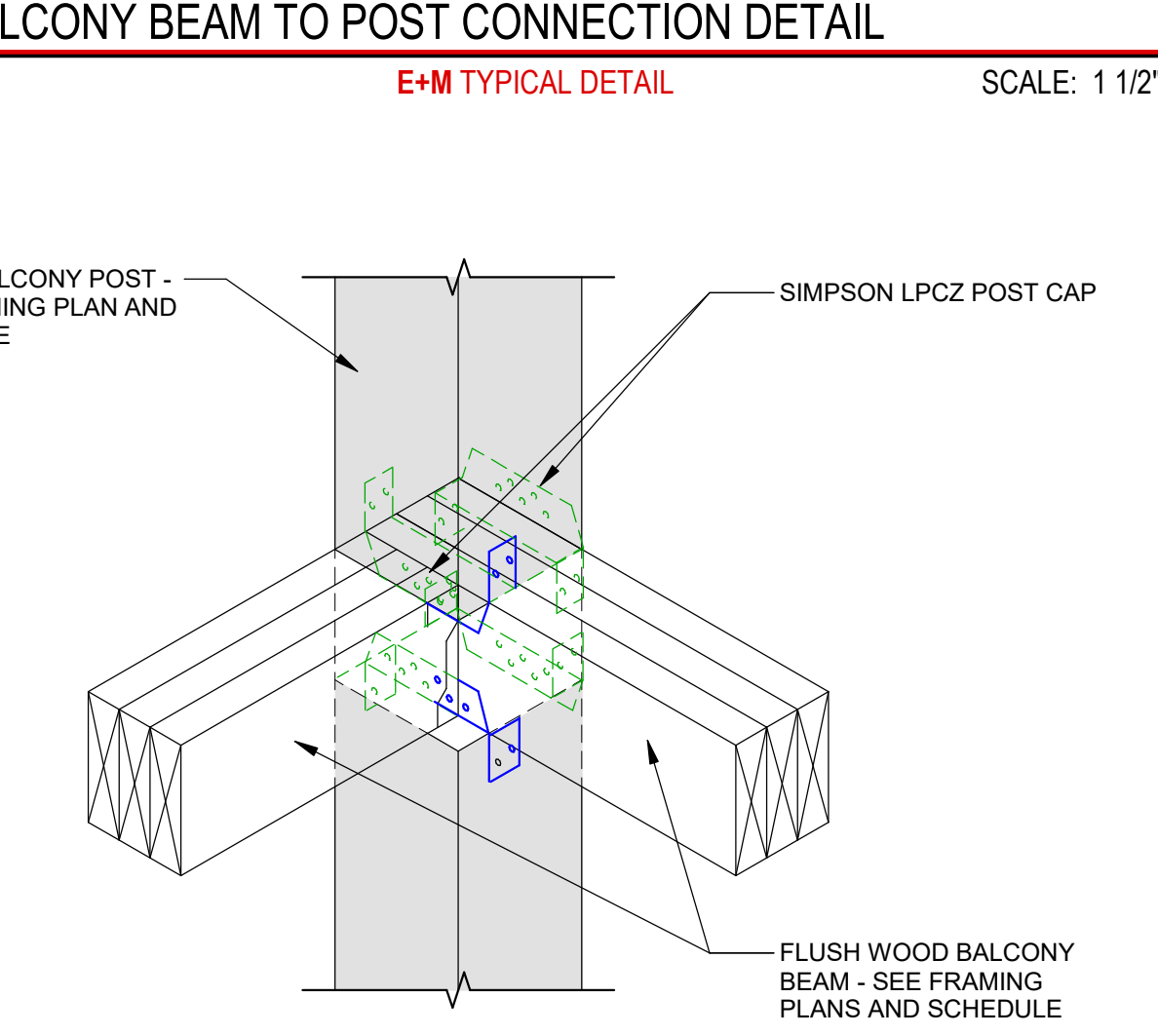
**11 FLOOR FRAMING AT BALCONY**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



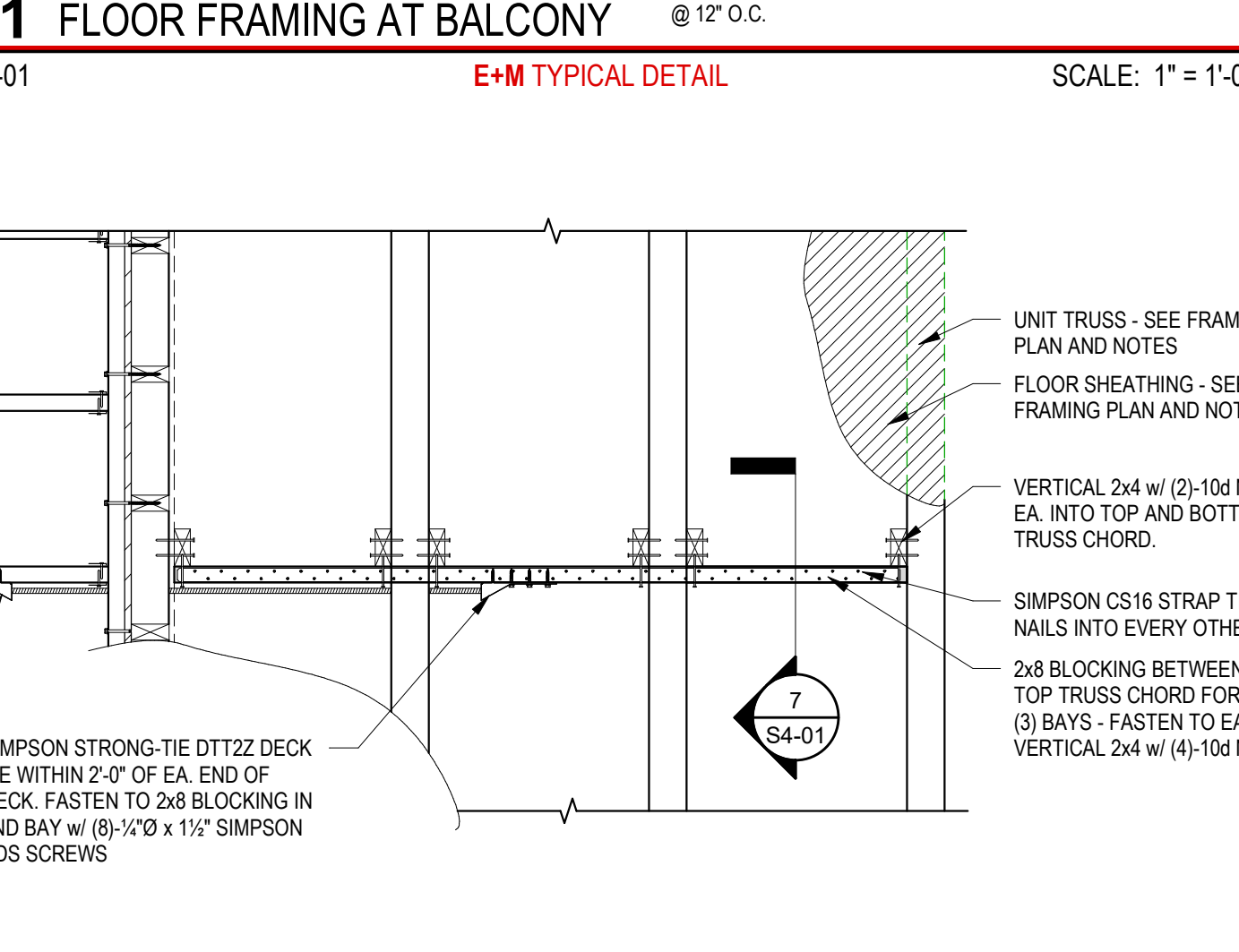
**7 SECTION THRU BLOCKING AT DECK TIE BACK**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



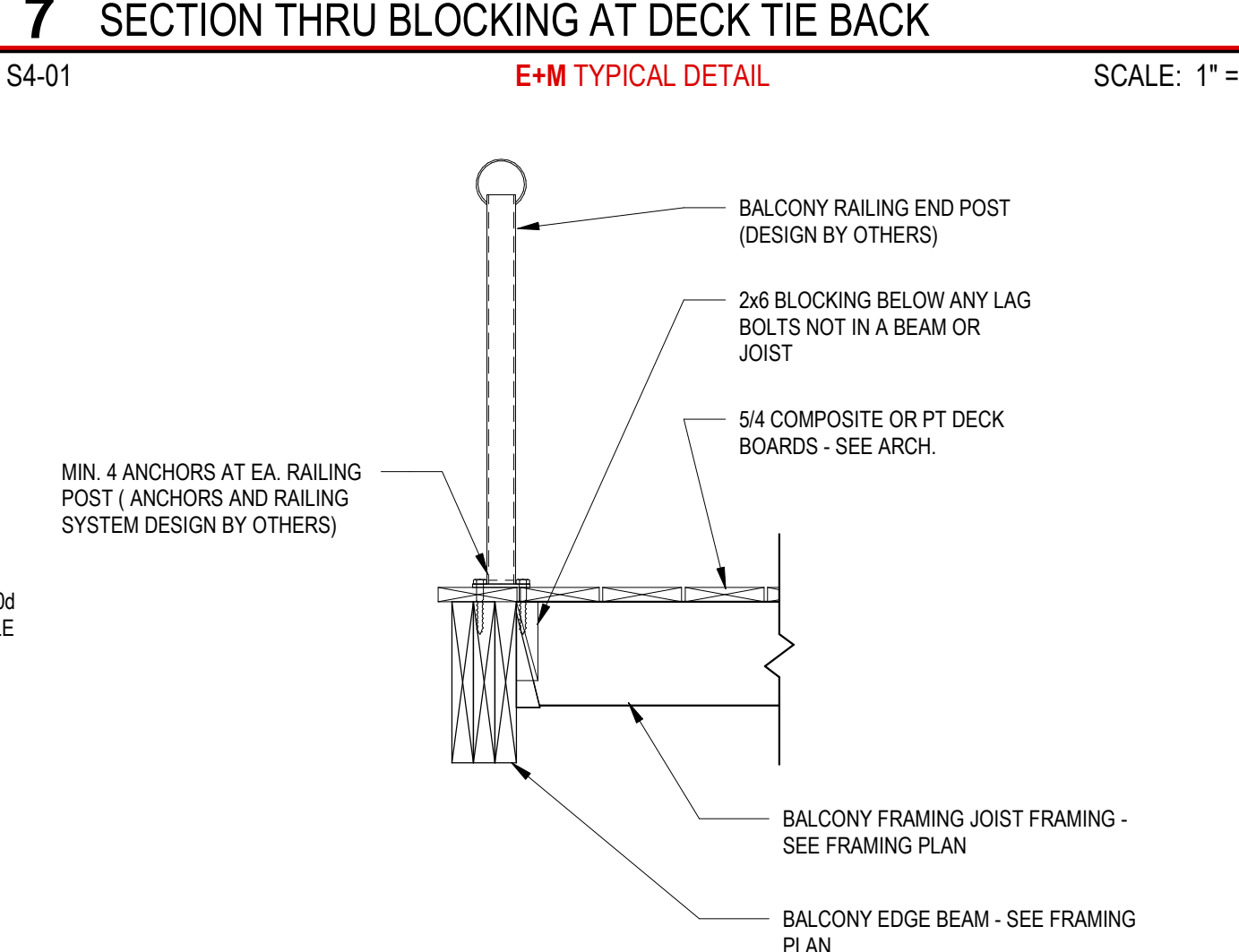
**3 SECTION AT TENANT SEPARATION WALL**  
S4-01 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



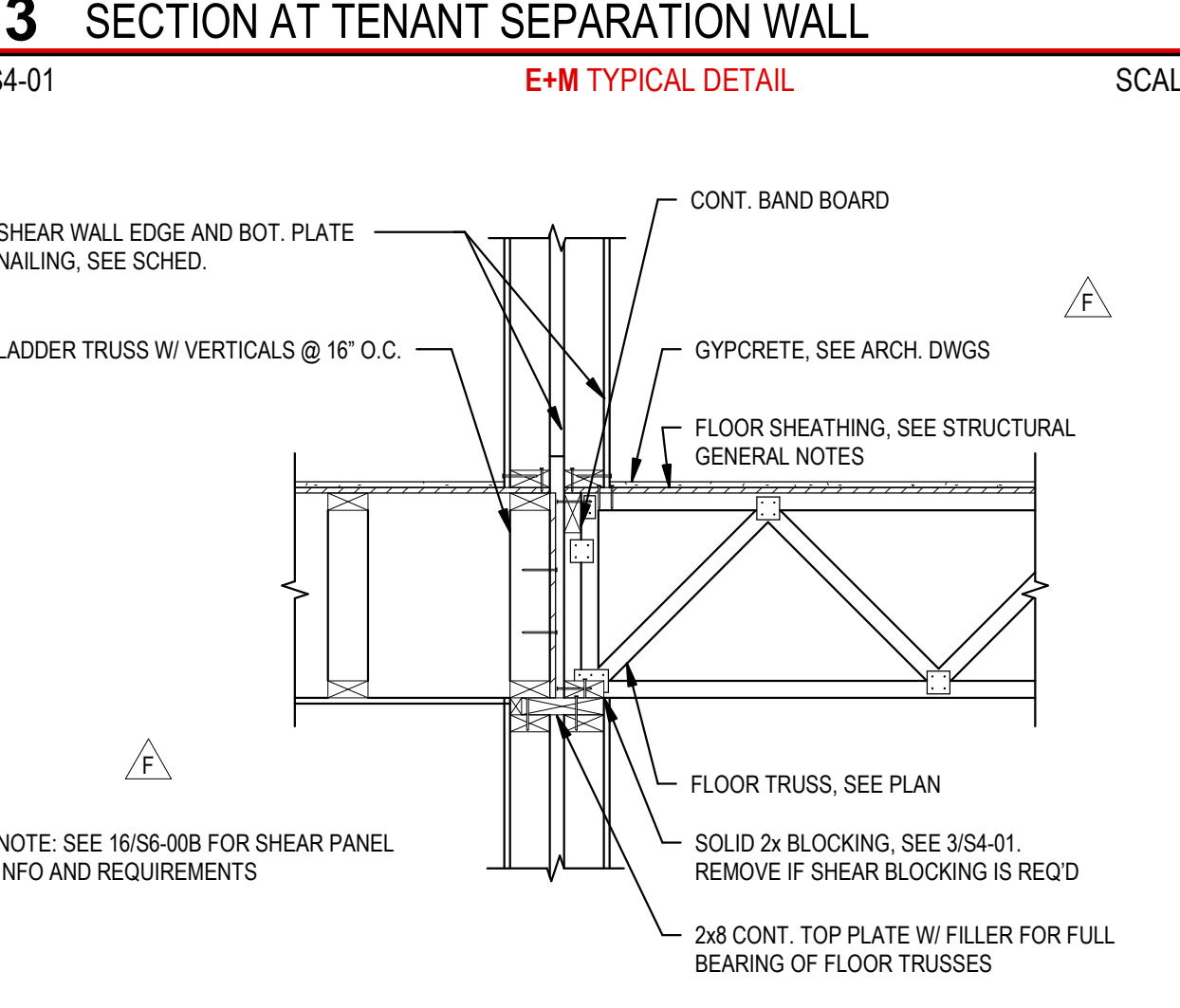
**16 BEAM TO POST CONNECTION DETAIL**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



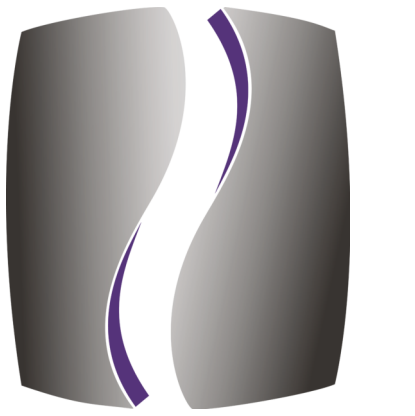
**12 SECTION THRU BLOCKING AT BALCONY TIE BACK**  
S4-01 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**8 RAILING POST CONNECTION TO WOOD-FRAMED BALCONY**  
S4-01 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



**4 SECTION AT TENANT SEPARATION WALL**  
S4-01 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



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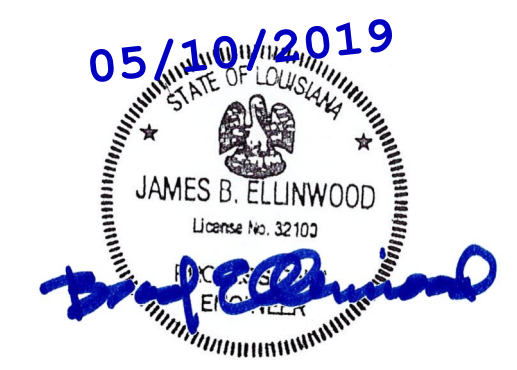
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ISSUE & REVISION RECORD

#	DATE	DESCRIPTION
	09/24/2018	PERMIT SET
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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

DRAWING TITLE WOOD FRAMING SECTIONS AND DETAILS

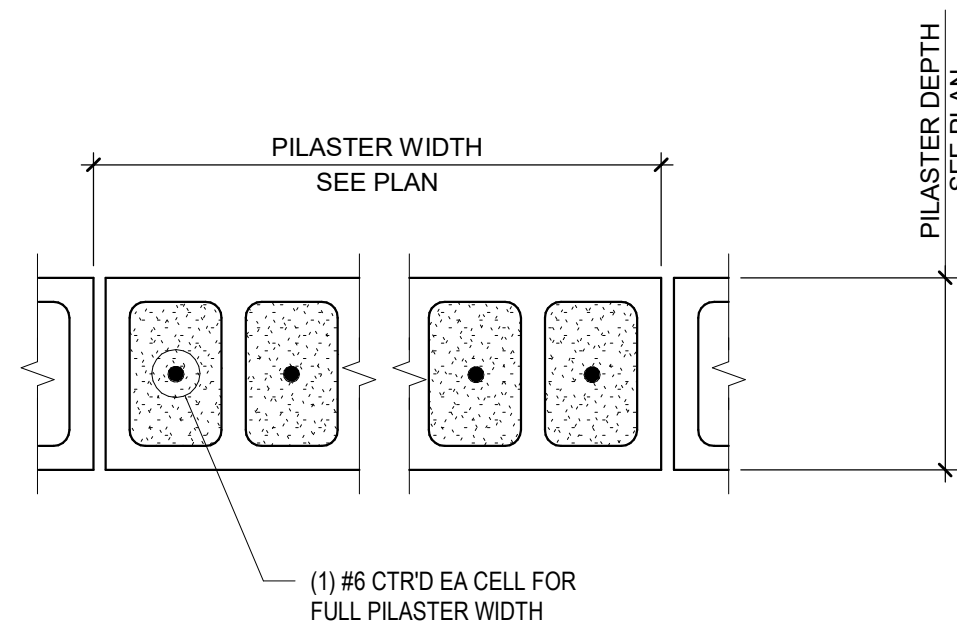
SHEET NUMBER

**S4-01**

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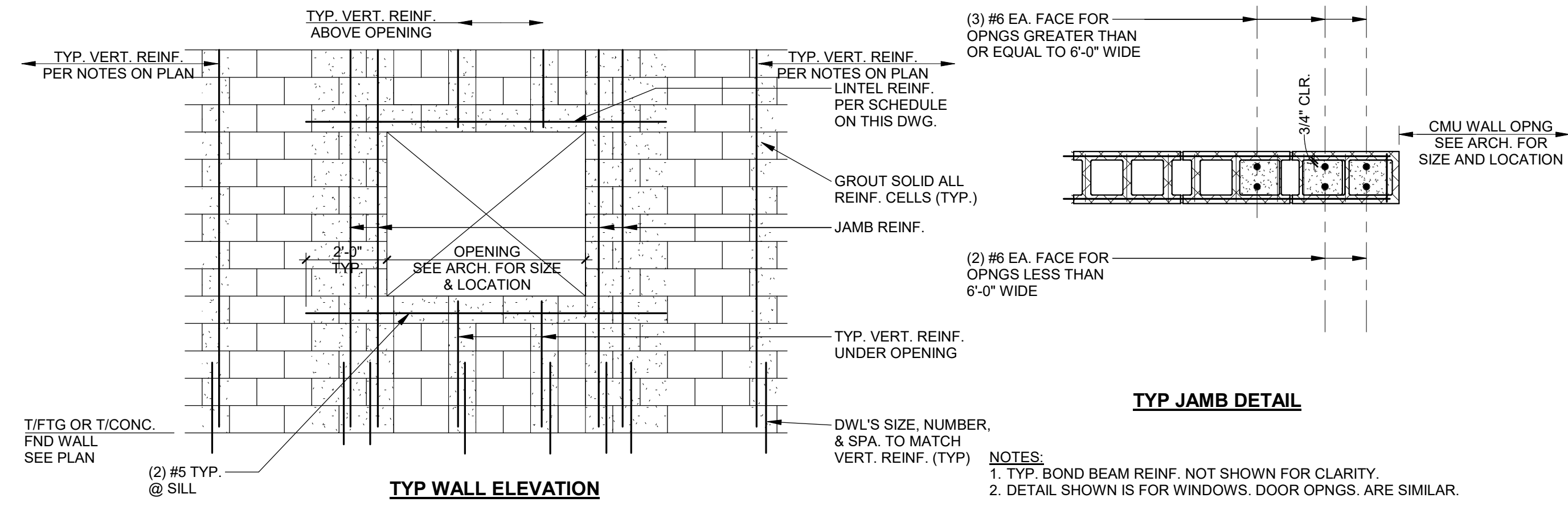
PHILLIPSPART.COM 770-394-1616

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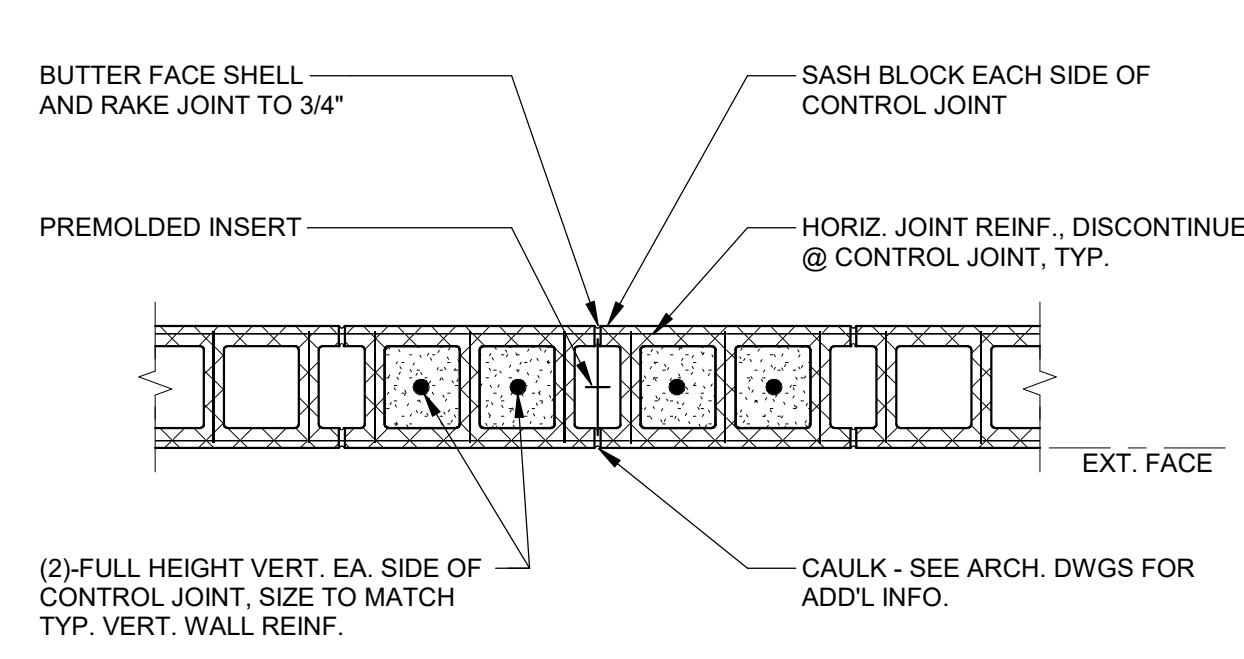
**13** CMU PILASTER DETAIL

S4-20 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



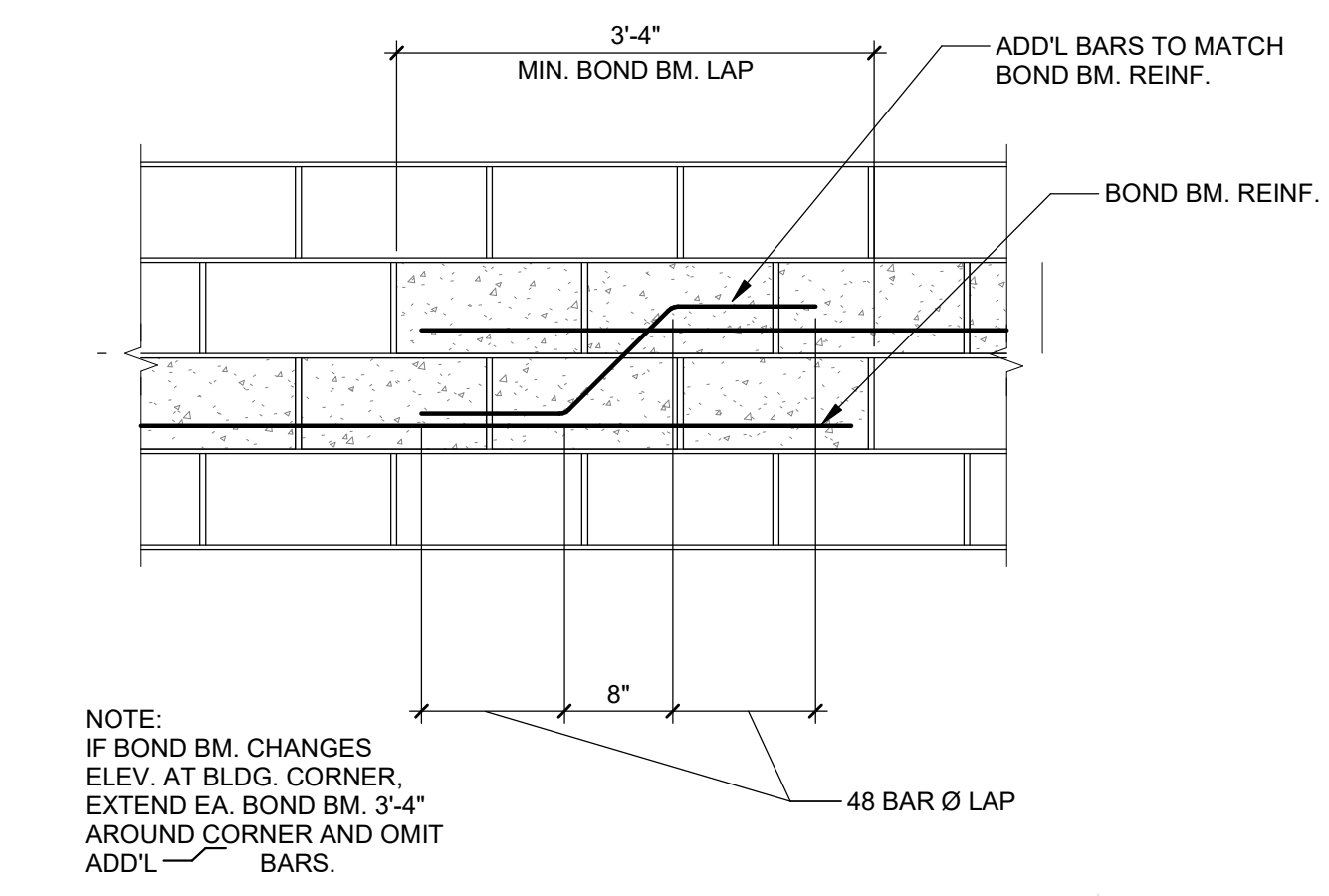
**9** CMU WALL OPENINGS DETAIL

S4-20 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



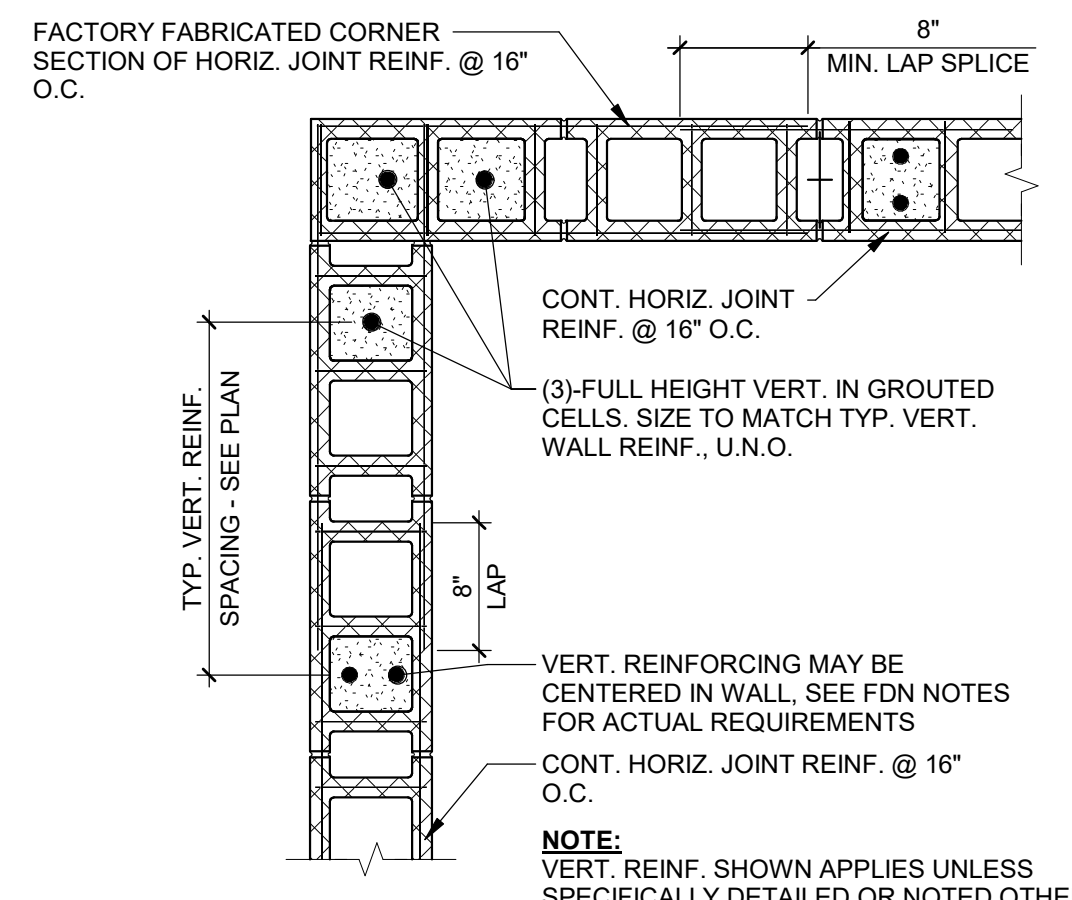
**1** CMU WALL CONTROL JOINT DETAIL

S4-20 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



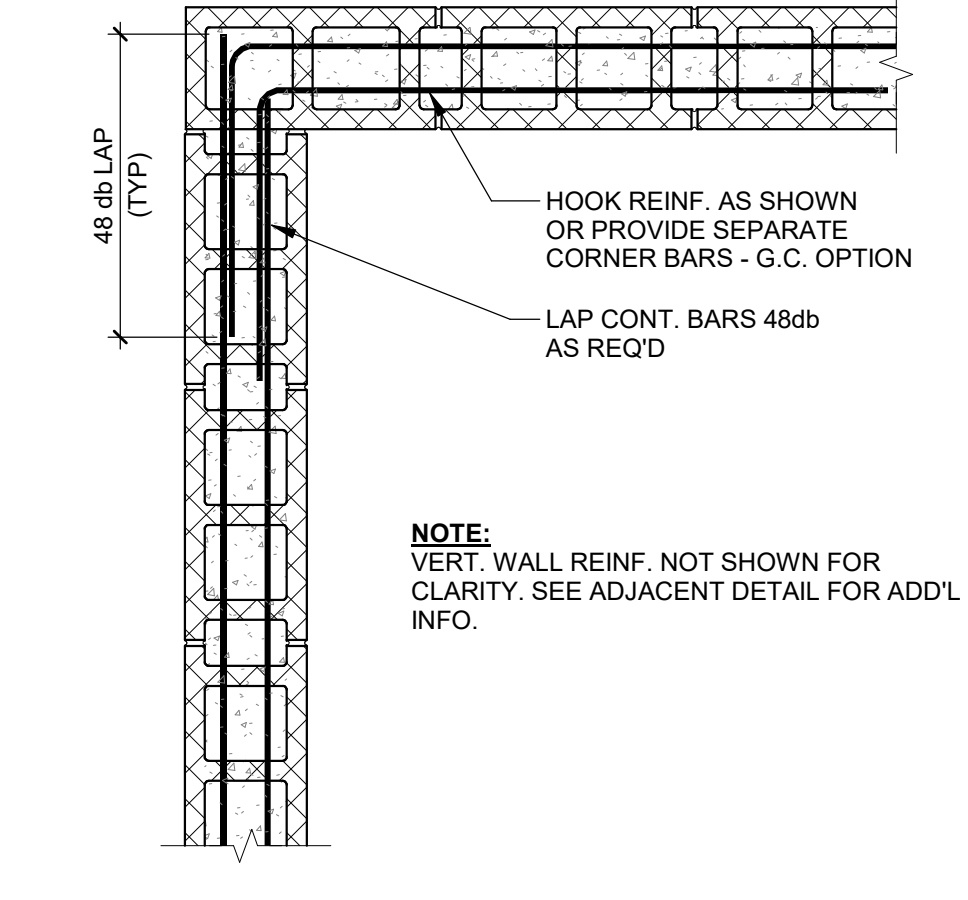
**10** CMU BOND BEAM STEP

S4-20 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**6** CMU WALL VERTICAL REINFORCING DETAIL

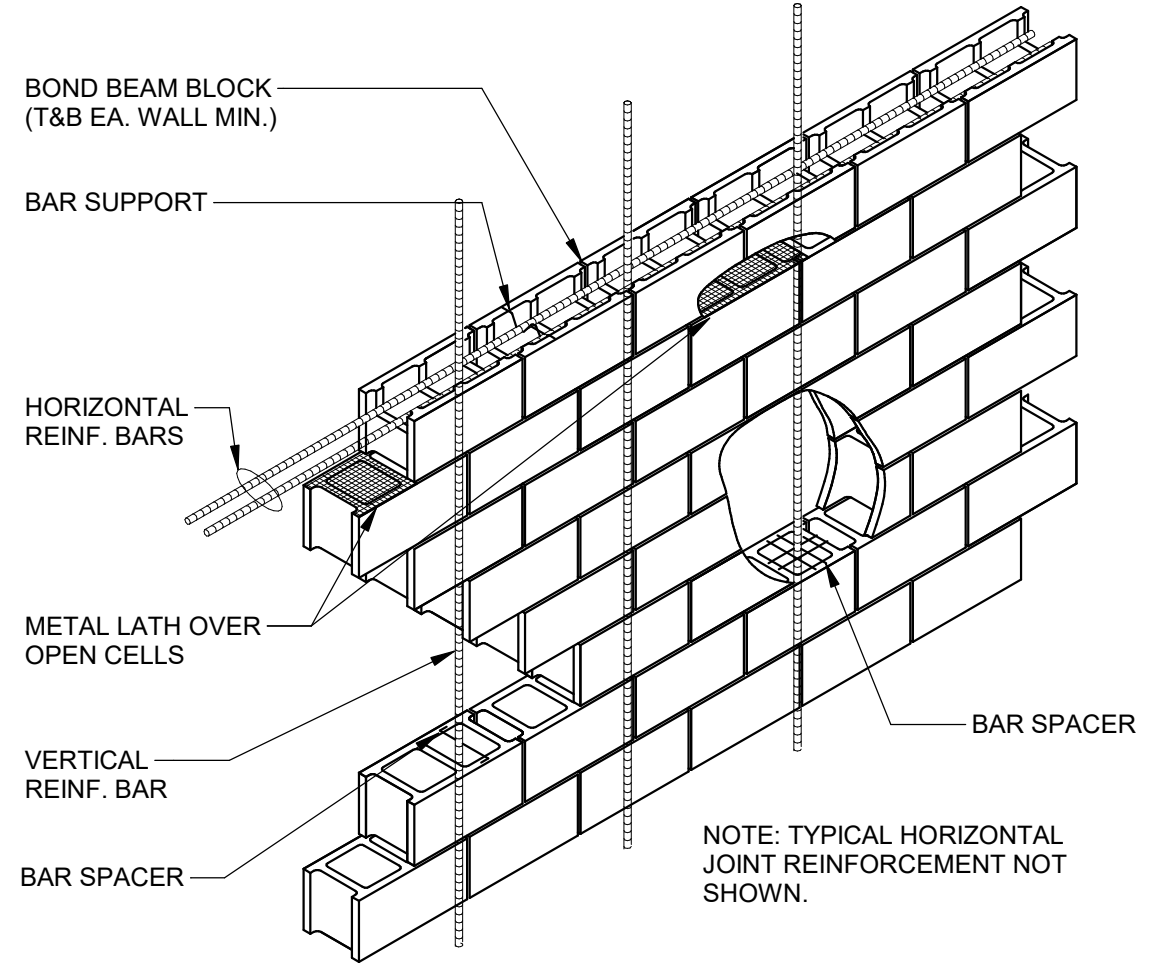
S4-20 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



**2** CMU WALL BOND BEAM CORNER DETAIL

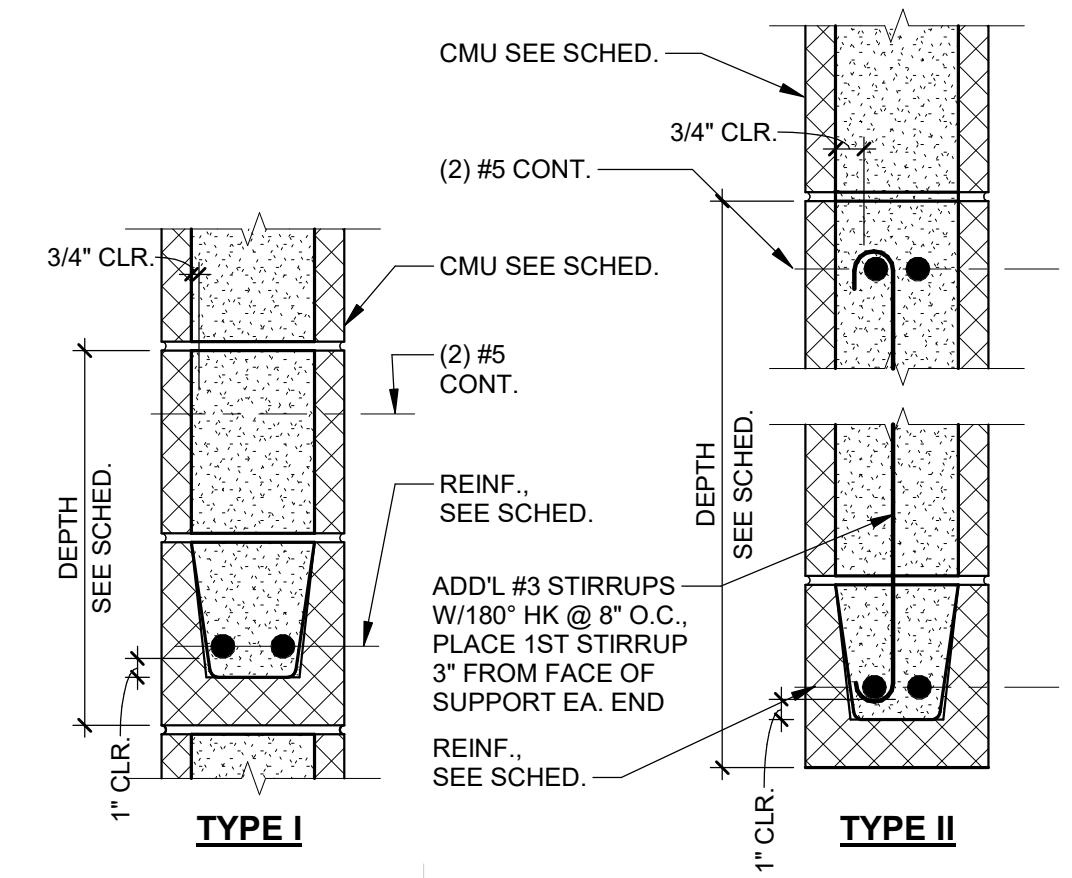
S4-20 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"

- NOTES:
1. GROUT CMU CELLS AT VERTICAL REINFORCING ONLY.
  2. ANCHOR ALL VERTICAL REINF. TO FOUNDATION USING MATCHING DOWELS.
  3. PROVIDE ADDITIONAL (2)-#5 VERTICAL REINF. EA. SIDE OF OPENING OR CONTROL JOINT.
  4. PROVIDE CONT. 9 GA TRUSS TYPE HORIZ. JOINT REINF. @ 16" O.C.
  5. REINFORCE CMU CORES AT ALL CORNERS W/ (2)-#6
  6. USE BOND BEAM W/ (2)-#5 MIN. AT TOP AND BOT. OF WALL AND 10'-0" CENTERS MAX.
  7. EXTERIOR LOCATION APPLIES WHERE CMU WALL OR FINISH ON CMU WALL IS DIRECTLY EXPOSED TO WIND. INCLUDING INTERIOR WALLS LOCATED IN WIND PATHS. INTERIOR LOCATION APPLIES WHERE CMU WALL IS NOT DIRECTLY EXPOSED TO WIND.



**15** RUNNING BOND NON-LOAD BEARING CMU WALL REINFORCEMENT

S4-20 E+M TYPICAL DETAIL SCALE: 1/2" = 1'-0"

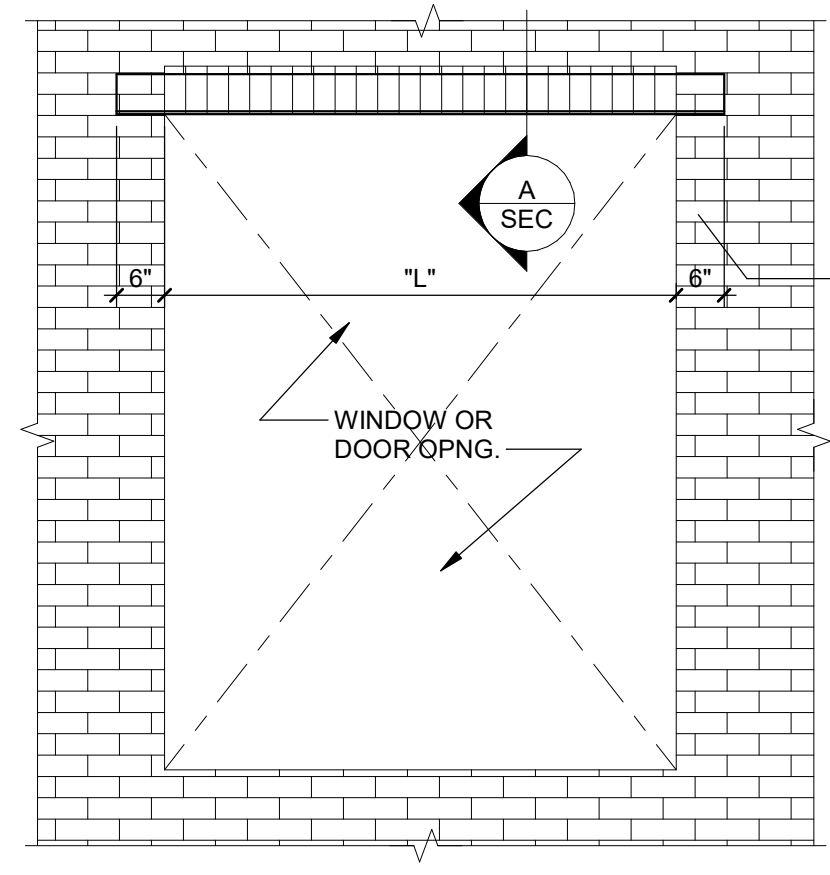


**7** CMU WALL LINTEL DETAIL AND SCHEDULE

S4-20 E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"

CLEAR SPAN	12" BLOCK			8" BLOCK		
	LINTEL DEPTH	REINF. (BOT.)	COMMENTS	LINTEL DEPTH	REINF. (BOT.)	COMMENTS
UP TO 4'-0"	16"	(2) #5	TYPE I	16"	(2) #5	TYPE I
4'-1" TO 8'-0"	24"	(2) #6	TYPE I	24"	(2) #6	TYPE I
8'-1" TO 10'-0"	32"	(2) #8	TYPE II	32"	(2) #8	TYPE II
10'-1" TO 14'-8"	40"	(2) #8	TYPE II	40"	(2) #8	TYPE II
GREATER THAN 14'-8"	NOTE #2	NOTE #2	TYPE III	NOTE #2	NOTE #2	TYPE III

- NOTES:
1. SEE TYPICAL SCHEMATIC CMU WALL OPENING ELEVATION FOR ADDITIONAL INFORMATION.
  2. PRECAST LINTEL DEPTH AND REINFORCING SHALL BE DETERMINED BY THE PRECAST SUPPLIER. SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.



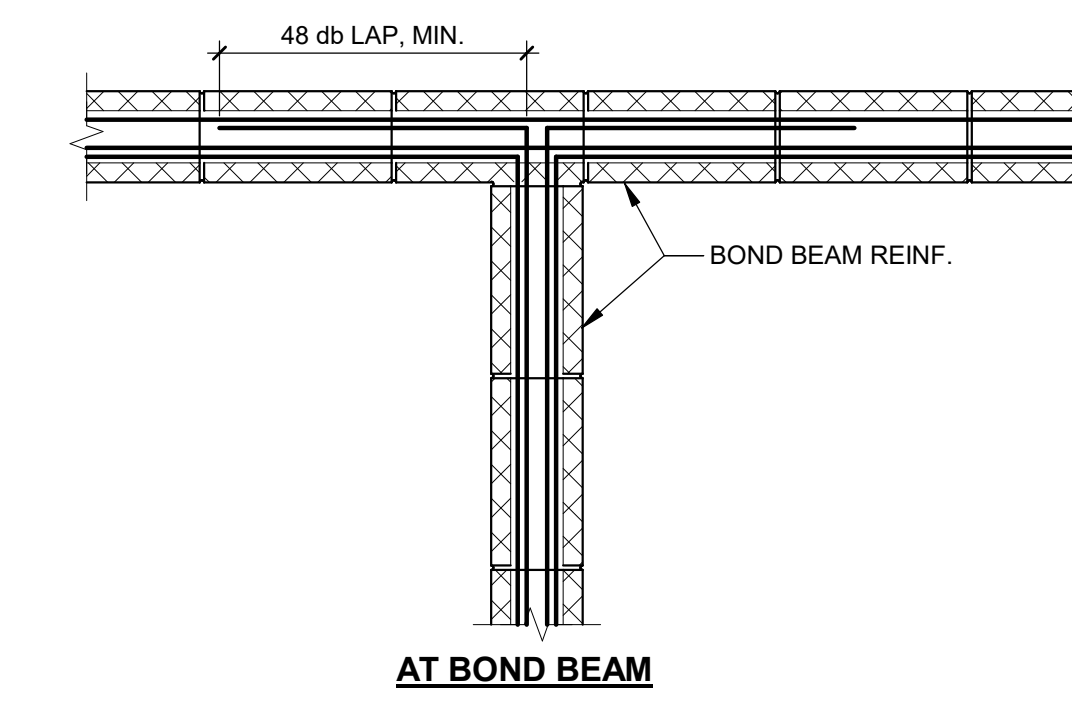
**16** STEEL ANGLE BRICK LOOSE LINTEL

S4-20 E+M TYPICAL DETAIL SCALE: 1/2" = 1'-0"

**BRICK LOOSE STEEL LINTEL SCHEDULE**

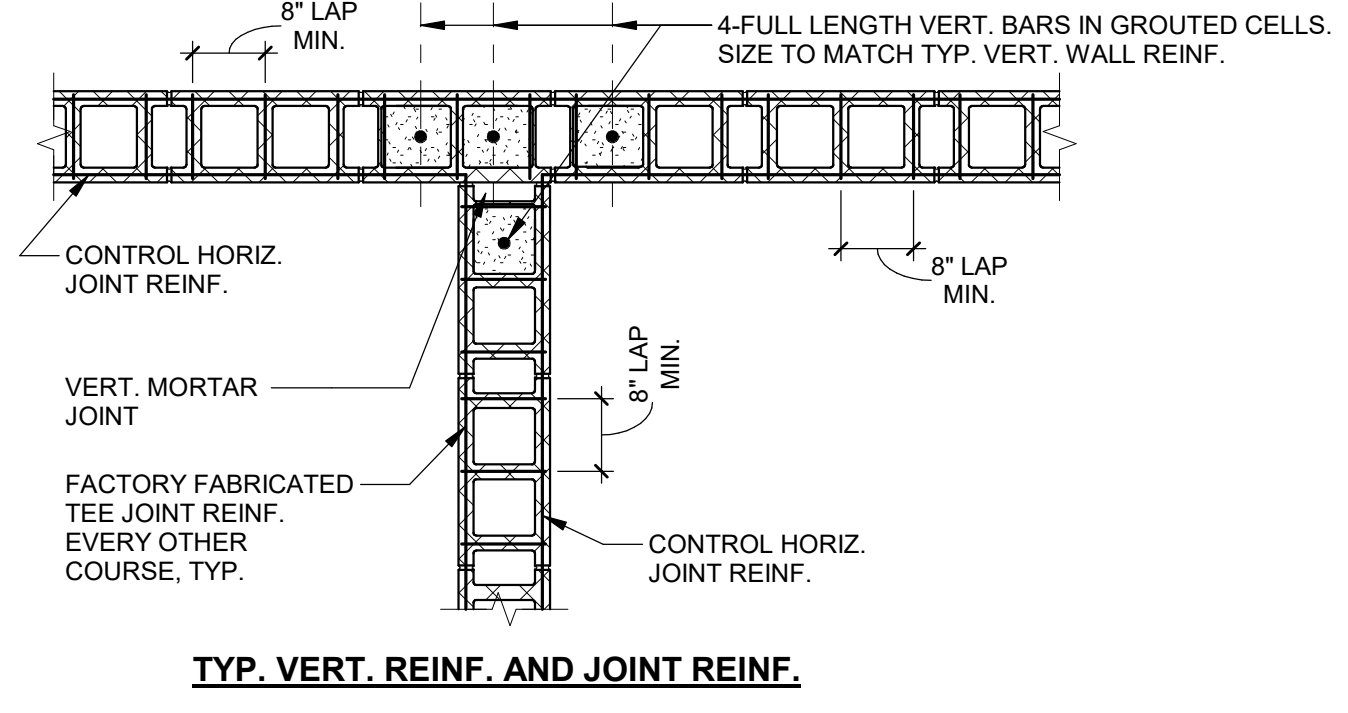
A. STEEL LINTEL SCHEDULE FOR MASONRY OPENING IS AS FOLLOWS:

MAXIMUM SPAN L	STEEL ANGLE SIZE (LLV)
3'-0"	L3 1/2 x 3 1/2 x 1/4 x 4'-0"
4'-0"	L5 x 3 1/2 x 1/4 x 5'-0"
5'-0"	L5 x 3 1/2 x 5/16 x 6'-0"
8'-0"	L5 x 3 1/2 x 3/8 x 9'-0"
9'-0"	L6 x 4 x 3/8 x 10'-0"
12'-0"	L7 x 4 x 1/2 x 13'-0"



**8** CMU WALL INTERSECTION DETAIL

S4-20 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**1** TYP. VERT. REINF. AND JOINT REINF.

S4-20 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



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**FREMAUX PARK APARTMENTS**

SLIDELL, LA

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PHILLIPS JOB NUMBER 1867709  
ISSUE DATE 05/10/2019  
DRAWN BY/CHECKED BY DR / JBE  
APPROVED BY BRAD, JBE  
DRAWING TITLE

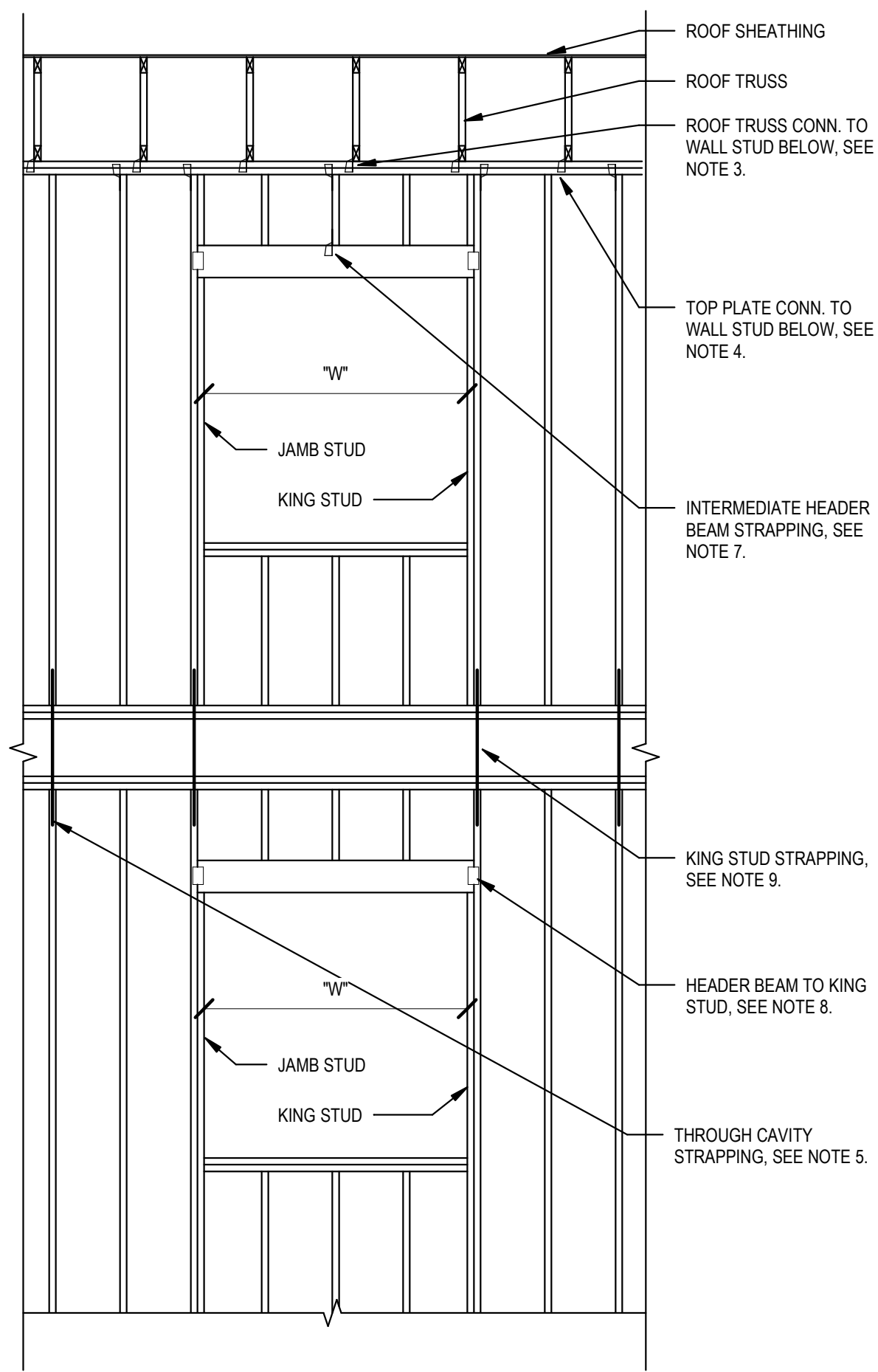
**CMU FRAMING SECTIONS AND DETAILS**

SHEET NUMBER

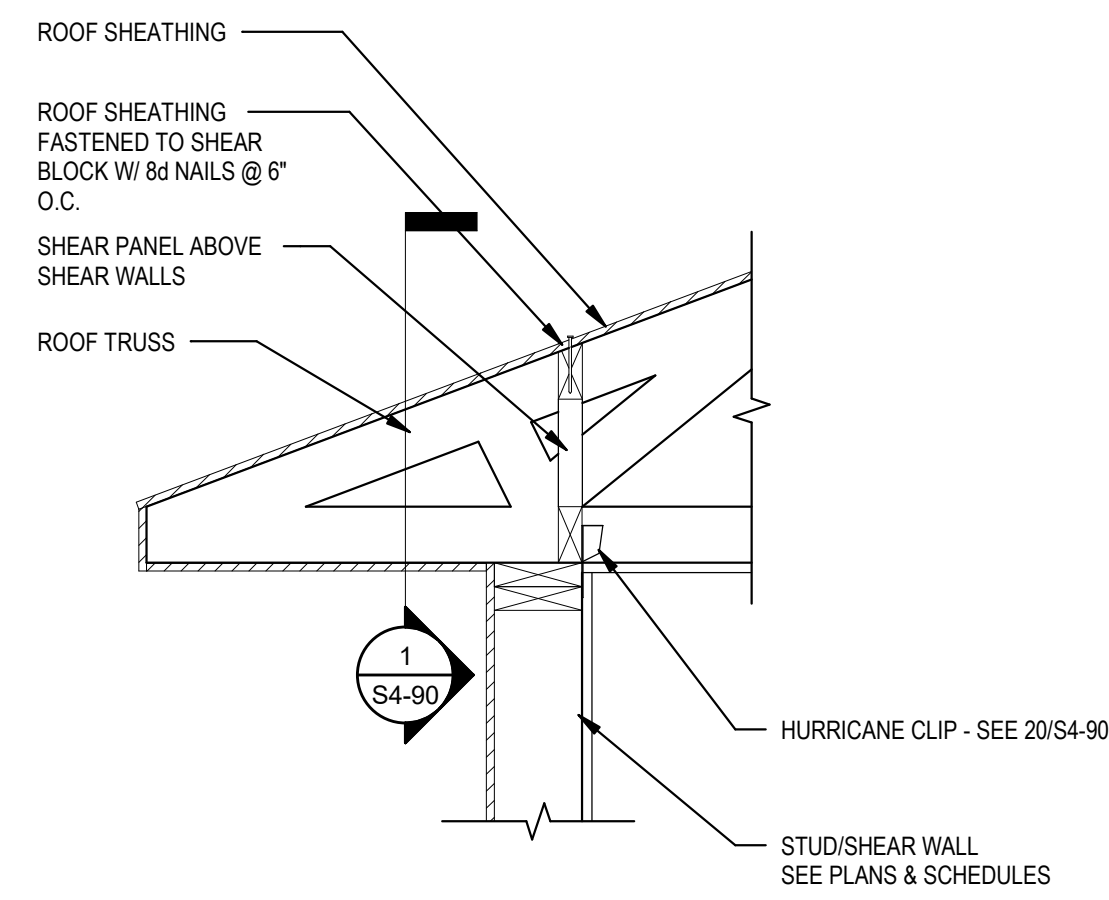
**S4-20**

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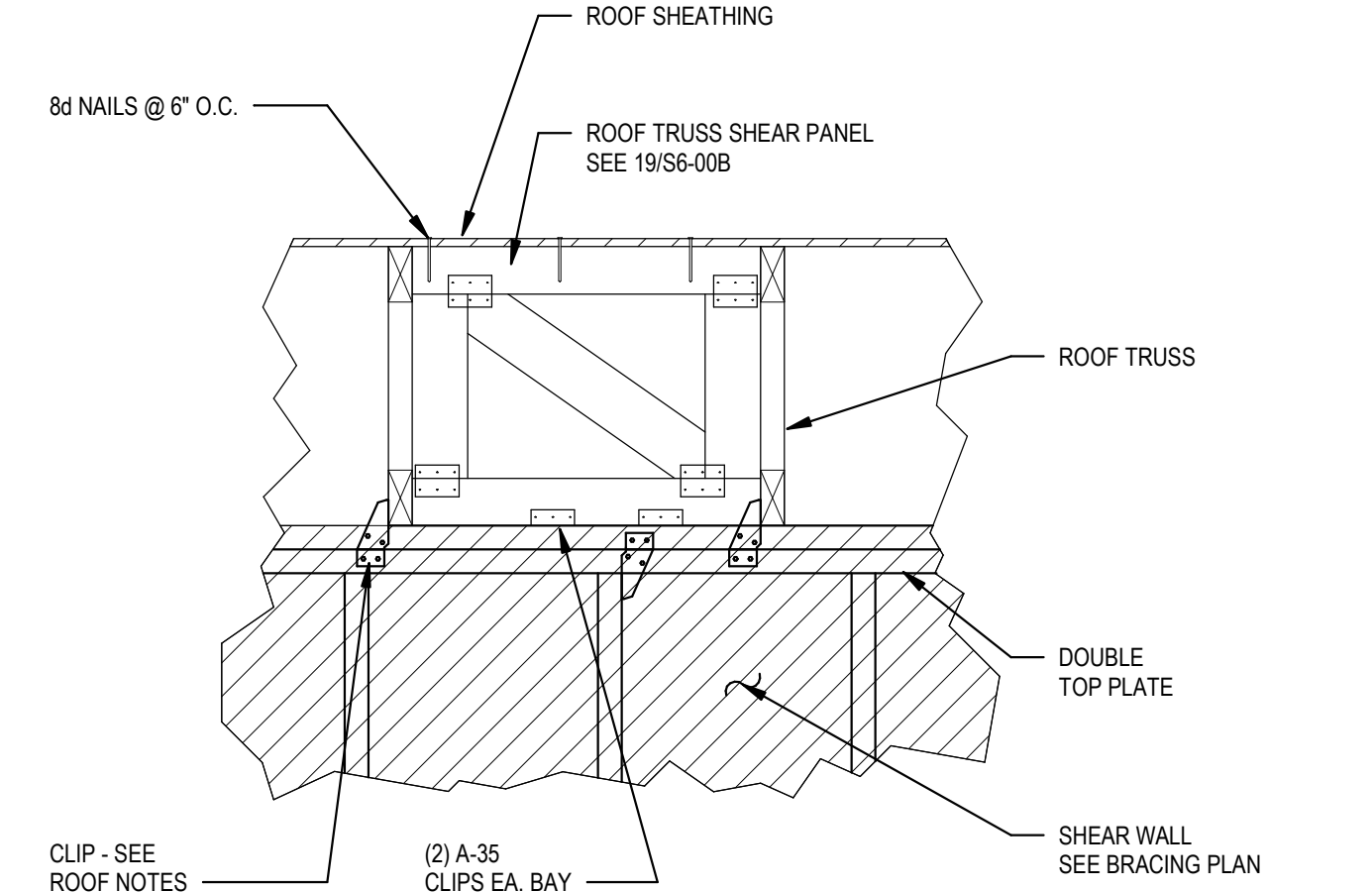
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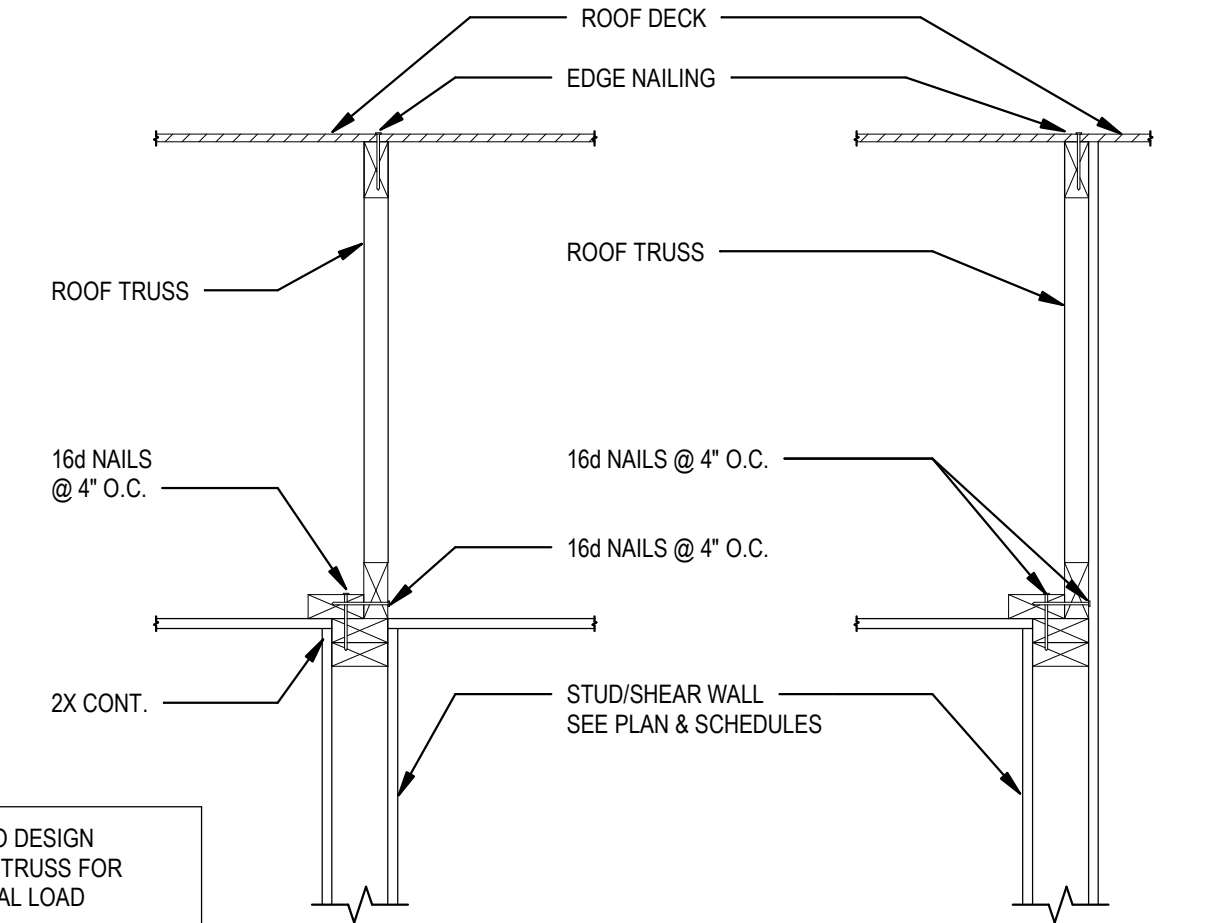
- SEE S1-# SERIES FOR ROOF FRAMING PLANS.
- SEE S4-9# SERIES FOR ROOF FRAMING SECTIONS AND DETAILS.
- ALL ROOF TRUSSES TO BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO THE FOLLOWING CRITERIA:
  - ATTACH ROOF TRUSS TO TOP PLATE WITH ONE (1) SIMPSON H2.5A HURRICANE TIE, TYP.
  - FOR ROOF TRUSSES LOCATED WITHIN "a" OF BUILDING EDGE, ATTACH ROOF TRUSS TO TOP PLATE WITH TWO (2) SIMPSON H2.5A HURRICANE TIES. SEE DETAIL 6/S0-20 FOR DEFINITION OF "a".
- TOP PLATE TO BE MECHANICALLY FASTENED TO A FULL-HEIGHT WALL STUD W/ (1) SIMPSON TSP TIE (STAGGER EA. FACE OF TOP PLATE) @ 48" O.C. MAX. SPACING. THIS CONNECTION SHALL ALSO BE PROVIDED WITHIN 2'-0" (MAX.) OF ALL CORNERS, AT KING STUDS ON EA. SIDE OF ALL OPENINGS, AND AT INTERMEDIATE CRIPPLE STUDS ABOVE HEADER BEAMS AS REQUIRED. PROVIDE CS16 STRAPPING AT 48" O.C. THROUGH UPPER TWO (2) FLOOR CAVITIES. ALIGN W/ TIES PRESCRIBED IN NOTE 4.
- ATTACH ALL HEADER BEAMS TO KING STUDS WITH (3)-10d TOE NAILS AT EA. END OF HEADER (6 NAILS TOTAL). SPACE NAILS @ 3" O.C. MIN.
- PROVIDE INTERMEDIATE STRAPPING OF HEADER BEAM TO CRIPPLE STUD ABOVE (AT ROOF FRAMING LEVEL ONLY) ACCORDING TO THE FOLLOWING CRITERIA:
  - WHERE  $W \leq 4'-0"$ , NO ADDITIONAL STRAPPING REQUIRED.
  - WHERE  $4'-0" < W \leq 6'-0"$ , PROVIDE (1)-H2.5A AT MID-SPAN.
  - WHERE  $4'-0" < W \leq 9'-0"$ , PROVIDE (1)-H2.5A AT THIRD POINTS (2 TIES TOTAL).
- PROVIDE CONNECTION BETWEEN HEADER BEAM AND KING STUDS ACCORDING TO THE FOLLOWING CRITERIA:
  - WHERE  $W \leq 4'-0"$ , NO ADDITIONAL CONNECTION REQUIRED.
  - WHERE  $4'-0" < W \leq 6'-0"$ , NO ADDITIONAL CONNECTION REQUIRED.
  - WHERE  $6'-0" < W \leq 9'-0"$ , PROVIDE (1)-LTP4 AT EA. SUPPORT (TWO PLATES TOTAL).
- PROVIDE STRAPPING FROM KING STUD(S) TO KING STUD(S) THROUGH FLOOR CAVITY ACCORDING TO THE FOLLOWING CRITERIA:
  - WHERE  $W \leq 4'-0"$ , PROVIDE CS16 STRAP AT UPPER TWO (2) FLOOR LEVELS.
  - WHERE  $4'-0" < W \leq 6'-0"$ , PROVIDE CS16 STRAP AT UPPER TWO (2) FLOOR LEVELS.
  - WHERE  $6'-0" < W \leq 9'-0"$ , PROVIDE CS16 STRAP AT UPPER TWO (2) FLOOR LEVELS.
- PROVIDE TIEDOWN ANCHORS FOR ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO THE FOLLOWING CRITERIA:
  - FOR GT/DBL SPAN  $\leq 20'-0"$ , PROVIDE:
    - \* LGT2 GIRDER TIEDOWN W/ (16)-16d SINKERS INTO GT/DBL AND (14)-16d SINKERS INTO POST/STUDPACK BELOW
  - GT/DBL SPAN  $\leq 35'-0"$ , PROVIDE:
    - \* MGT GIRDER TIEDOWN W/ (22)-10d NAILS INTO GT/DBL AND (15) 3/8" A307 THREADED ROD ANCHOR
    - \* HDU4 W/ (10)-1/2" X 2 1/2" SDS SCREWS
  - FOR GT/DBL SPAN  $\leq 50'-0"$ , PROVIDE:
    - \* HGT-2 GIRDER TIEDOWN W/ (16)-10d NAILS INTO GT/DBL AND (25) 3/8" A307 THREADED ROD ANCHOR
    - (2)-HTT4 TENSION TIES W/ (18)-16d NAILS INTO POST/STUDPACK BELOW
- PROVIDE FLOOR-TO-FLOOR STRAPPING OF POSTS/STUDPACKS SUPPORTING GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) ACCORDING TO THE FOLLOWING CRITERIA:
  - FOR GT/DBL SPANS  $\leq 20'-0"$ , PROVIDE (1)-CS16 W/ 12" END LENGTHS AND (11)-10d NAILS ABOVE AND BELOW
  - FOR GT/DBL SPANS  $\leq 35'-0"$ , PROVIDE (2)-CS16 W/ 12" END LENGTHS AND (11)-10d NAILS ABOVE AND BELOW (EA. STRAP)
  - FOR GT/DBL SPANS  $\leq 50'-0"$ , PROVIDE (1)-CAST 14 W/ 30" END LENGTHS AND (33)-16d NAILS ABOVE AND BELOW
- PROVIDE POSTS/STUDPACKS BELOW ENDS OF ALL GIRDER TRUSSES (GT) AND DOUBLE TRUSSES (DBL) CONTINUOUS TO FOUNDATION PER ROOF FRAMING NOTES - SEE ROOF FRAMING PLANS.
- UNLESS NOTED OTHERWISE, INSTALL ALL SPECIFIED SIMPSON HARDWARE AS DIRECTED BY SIMPSON PRODUCT DATA.



**5 SECTION HIP ROOF**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"

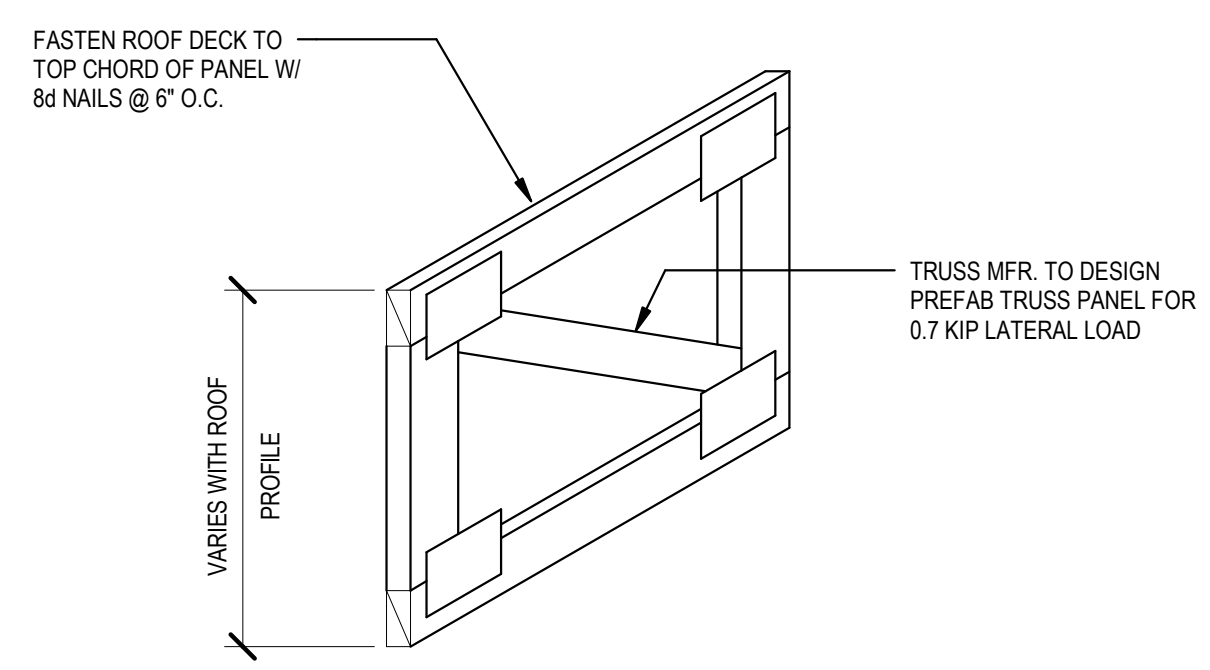


**1 EXTERIOR WALL SHEAR SECTION**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"

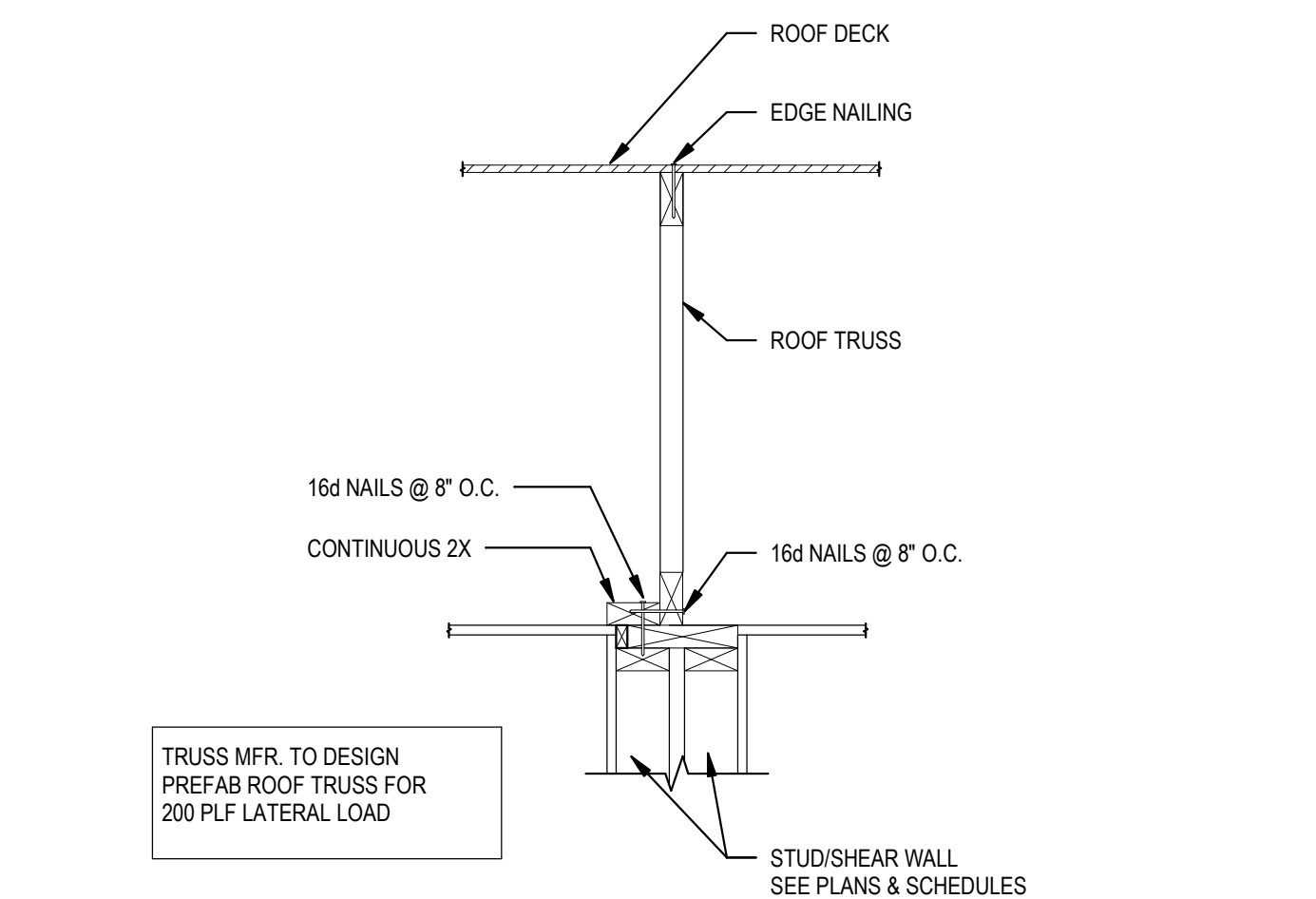


**2 ROOF TRUSS @ SHEAR WALL**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"

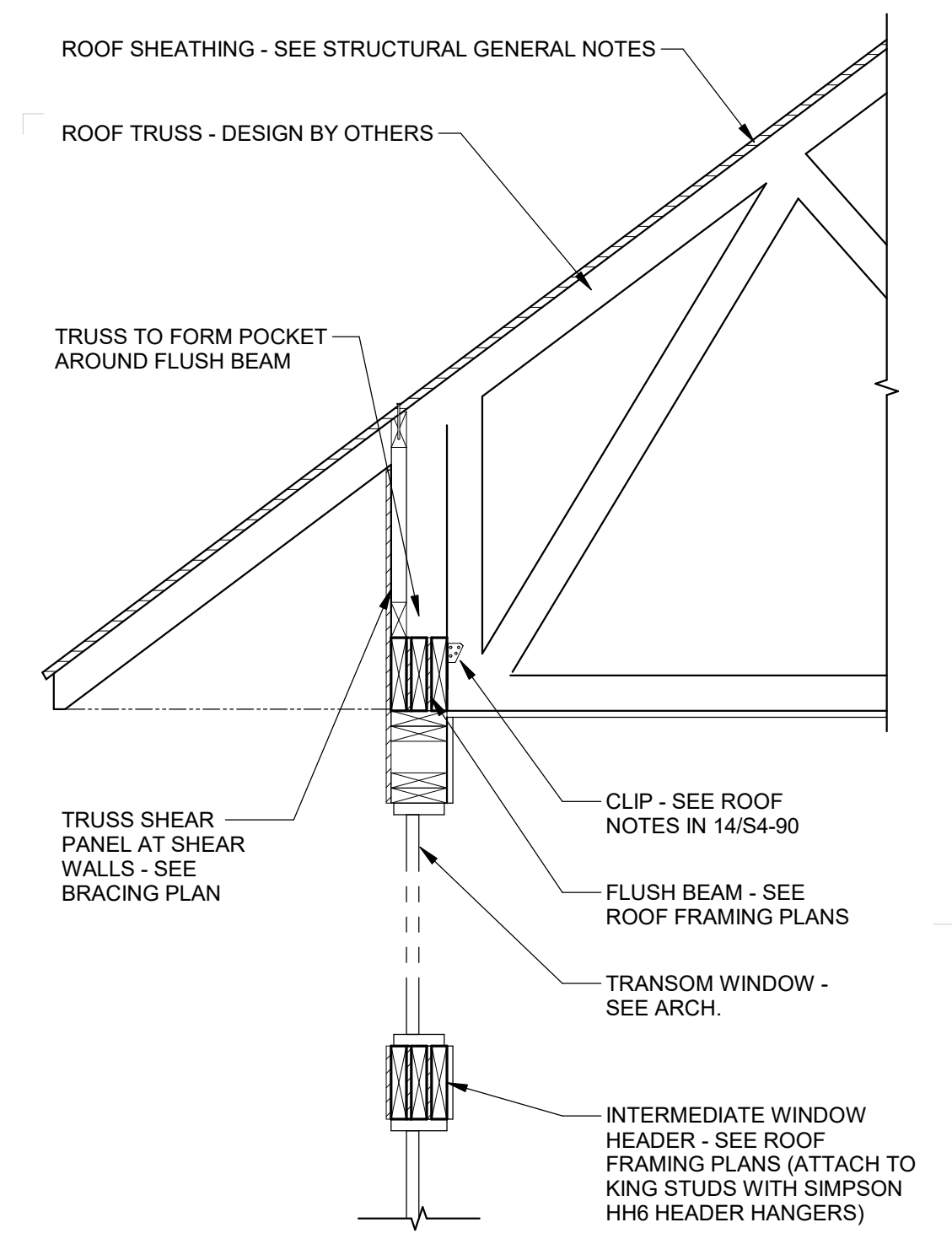
**14 ROOF ANCHORAGE AND TIE-DOWN NOTES**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



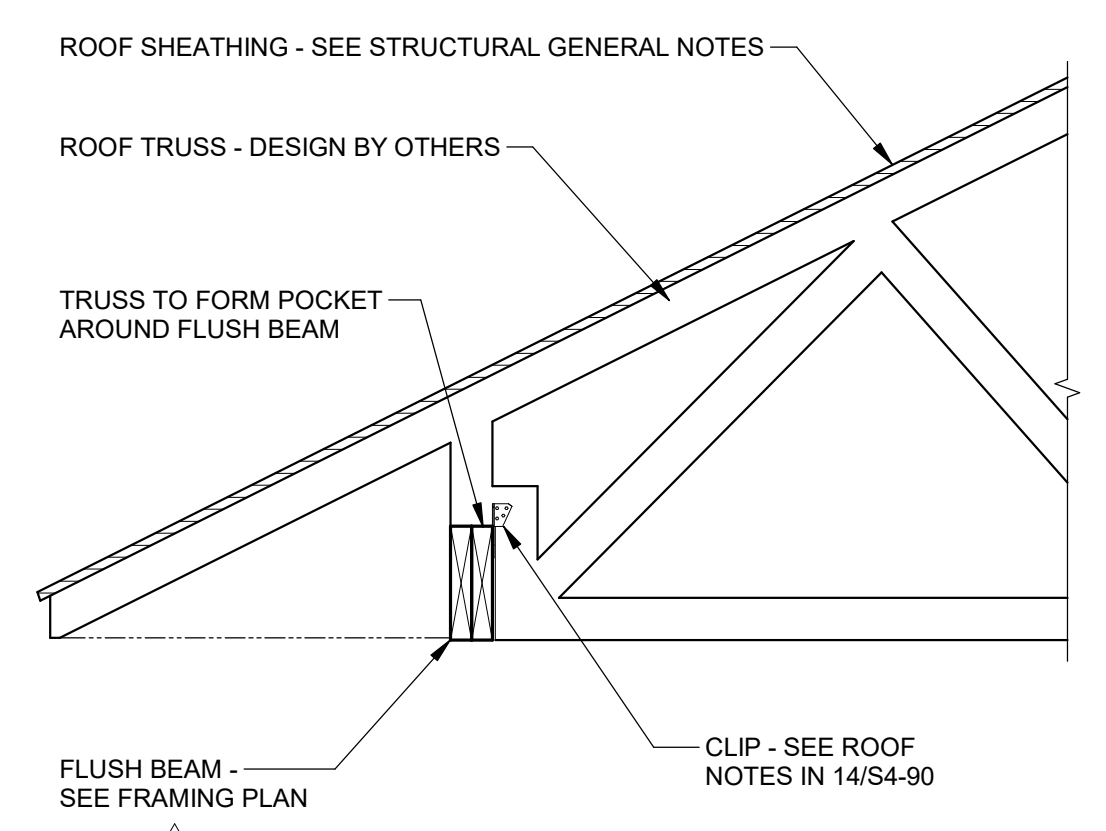
**7 ROOF TRUSS SHEAR PANEL**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



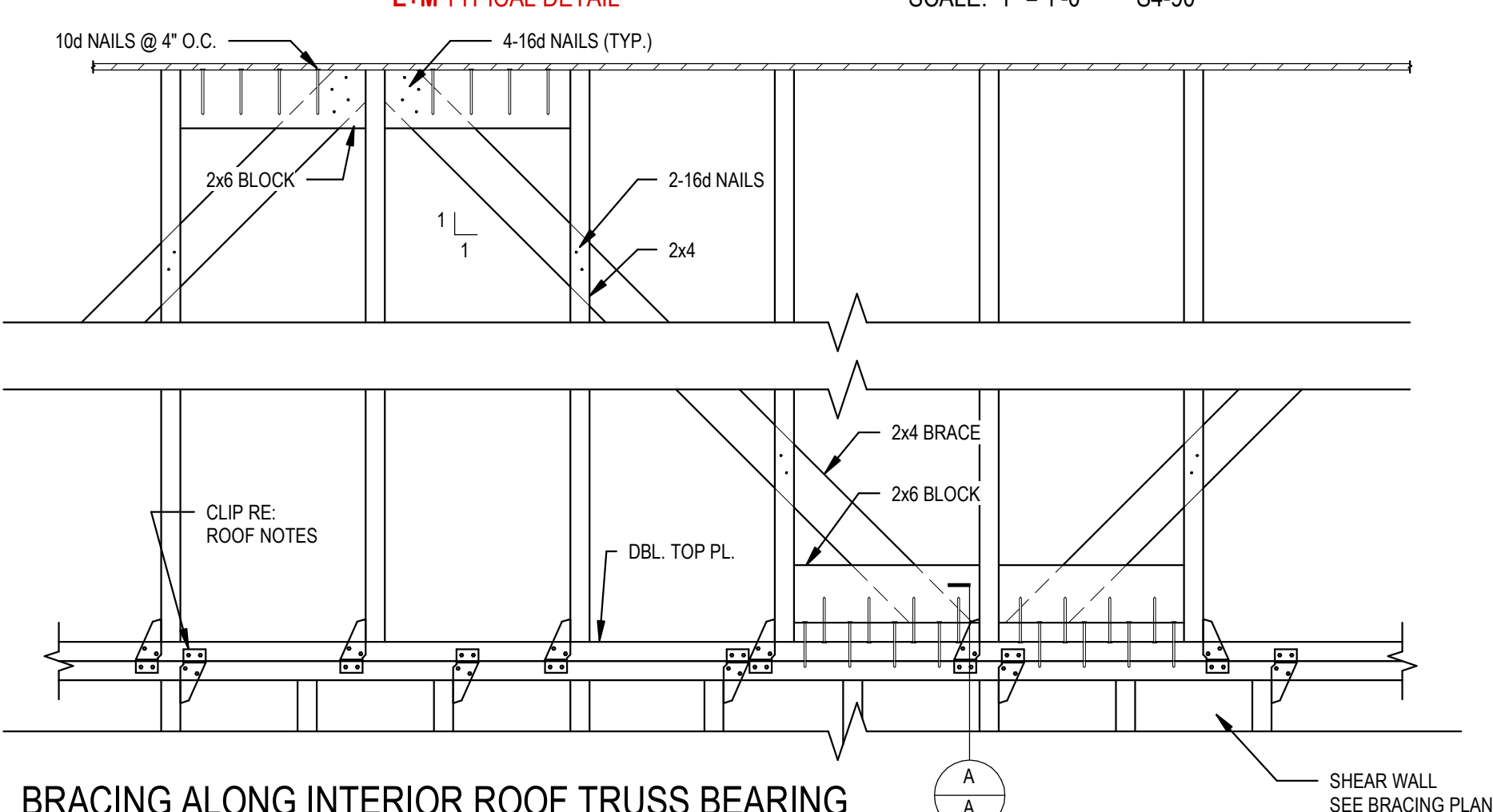
**3 SECTION @ PARTY WALL**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



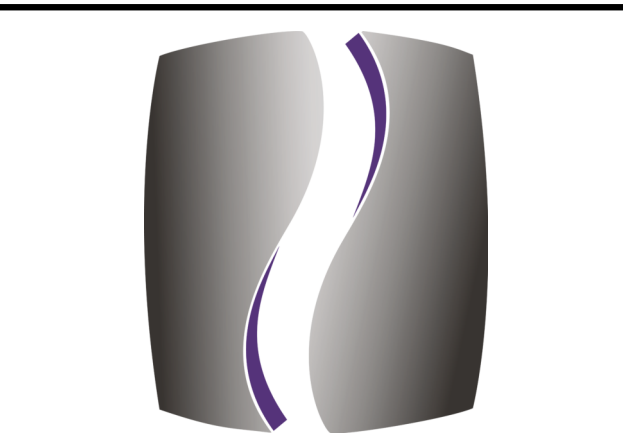
**16 ROOF TRUSS AT TRANSOM WINDOWS**  
S4-90 SCALE: 3/4" = 1'-0"



**12 S4-90/12 - ROOF GABLE TRUSS AT PATIOS**  
S4-90 E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**8 BRACING ALONG INTERIOR ROOF TRUSS BEARING LOCATIONS**  
S4-90 E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



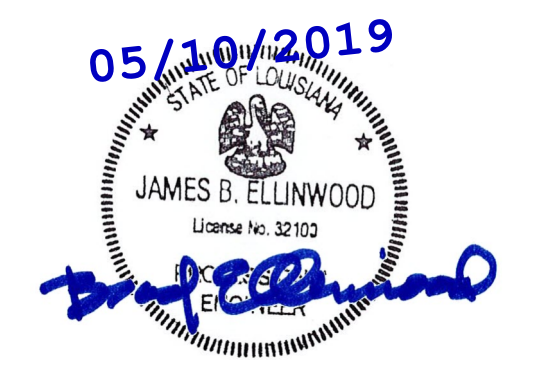
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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA  
CLIENT

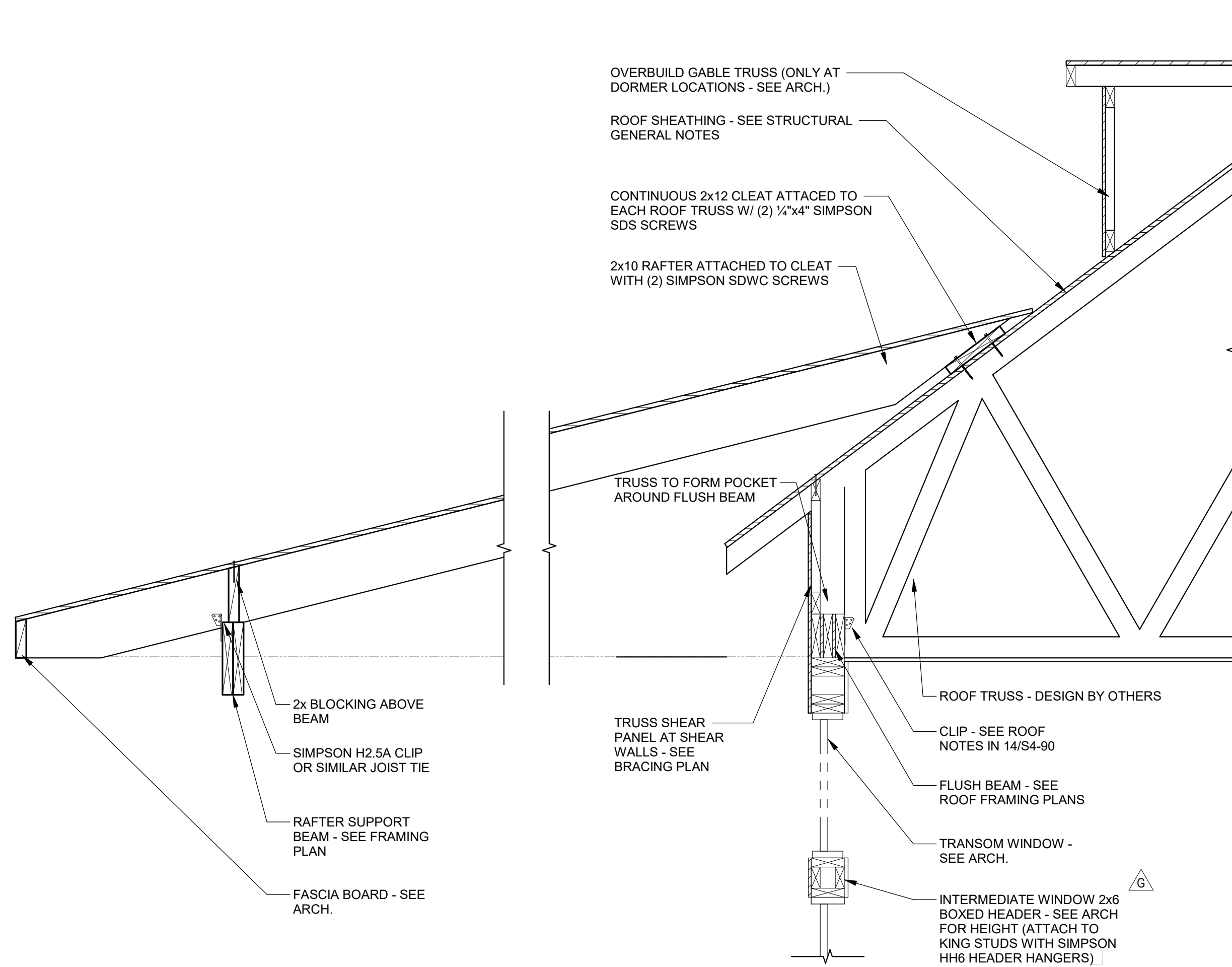


PHILLIPS JOB NUMBER 1867709  
ISSUE DATE 05/10/2019  
DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM  
APPROVED BY BRAD, JBE  
DRAWING TITLE WOOD ROOF FRAMING SECTIONS AND DETAILS  
SHEET NUMBER

**S4-90**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328  
PHILLIPSPART.COM 770-394-1616

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**14 PATIO ROOF FRAMING**

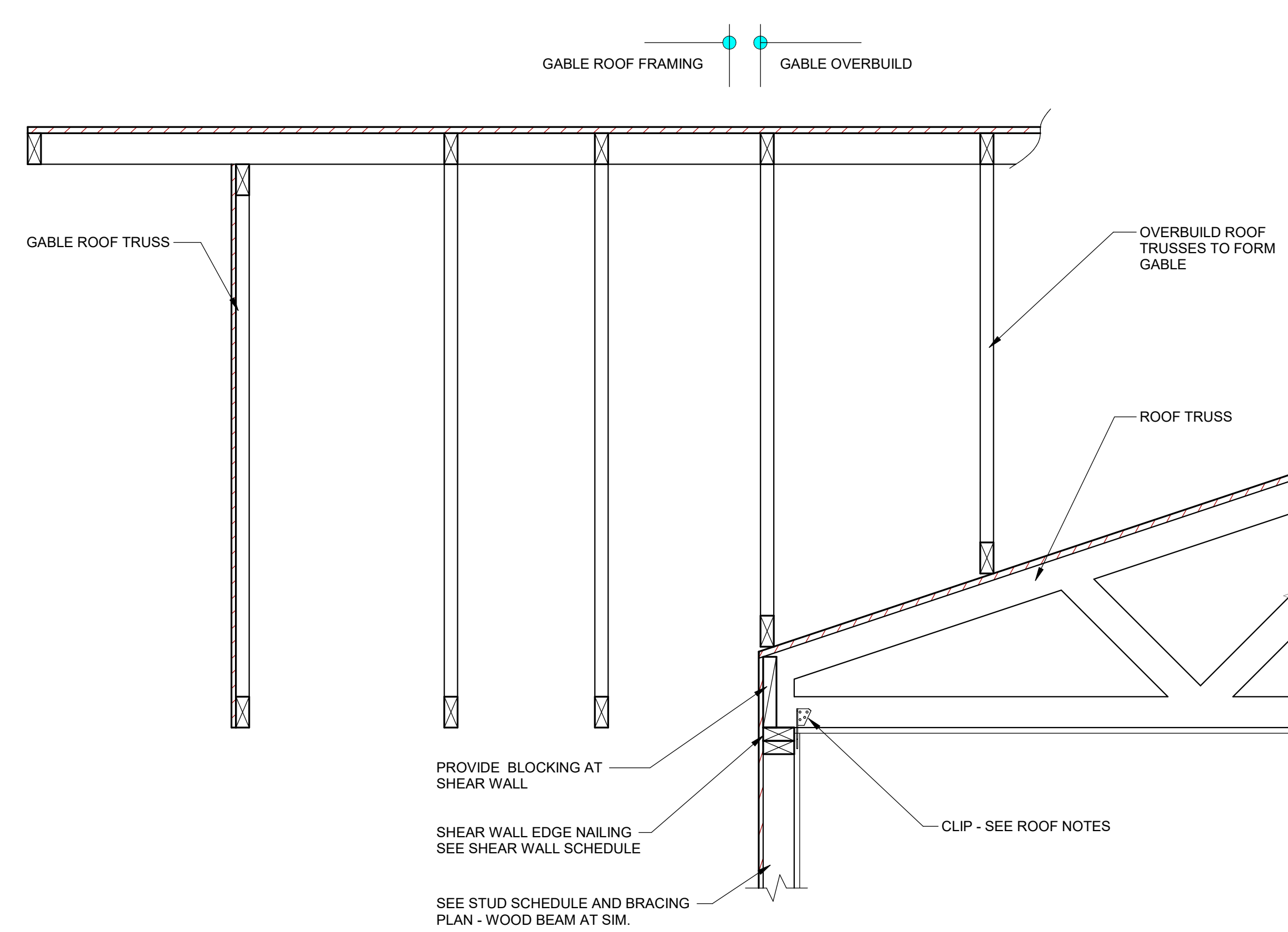
S4-91

SCALE: 3/4" = 1'-0"

**6 ROOF TRUSS BEARING AT EXTERIOR WALL**

S4-91

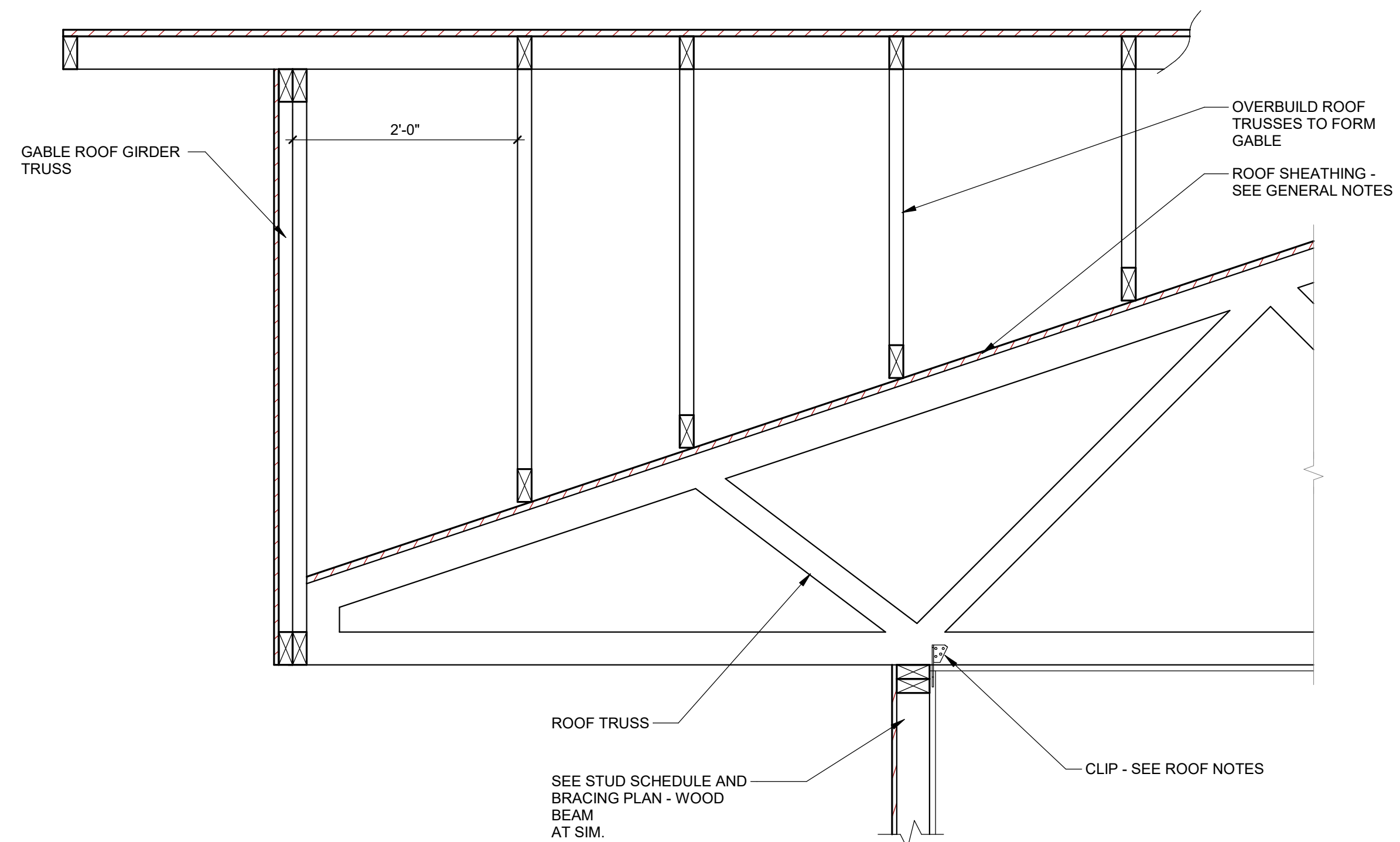
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**6 ROOF TRUSS BEARING AT EXTERIOR WALL**

S4-91

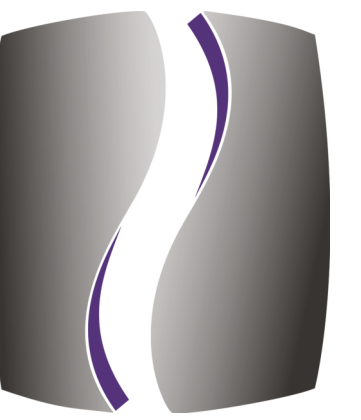
SCALE: 1" = 1'-0"



**8 ROOF TRUSS BEARING AT EXTERIOR WALL**

S4-91

SCALE: 1" = 1'-0"



**PHILLIPS**

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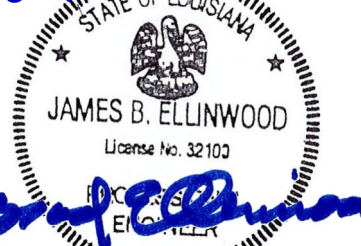


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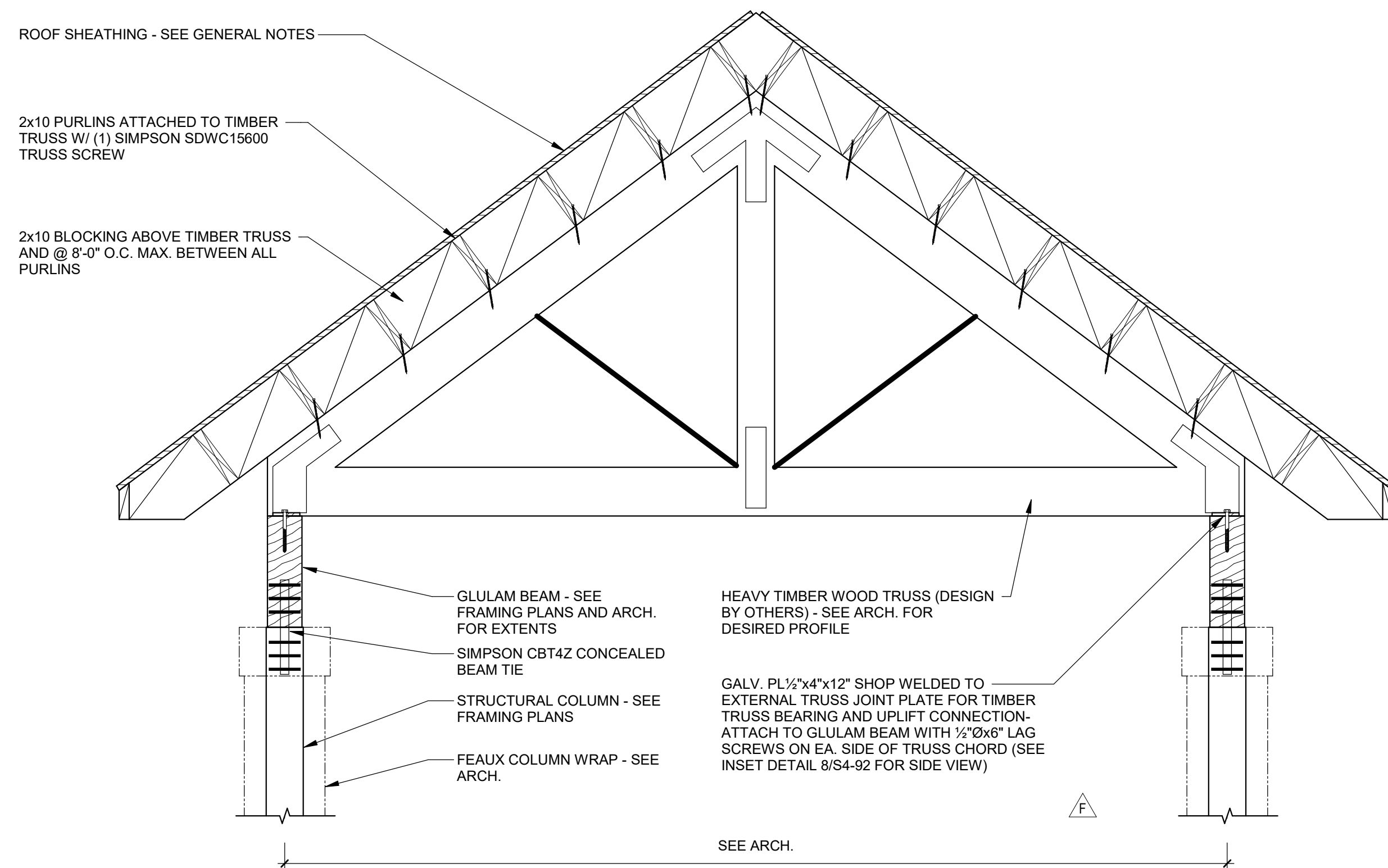
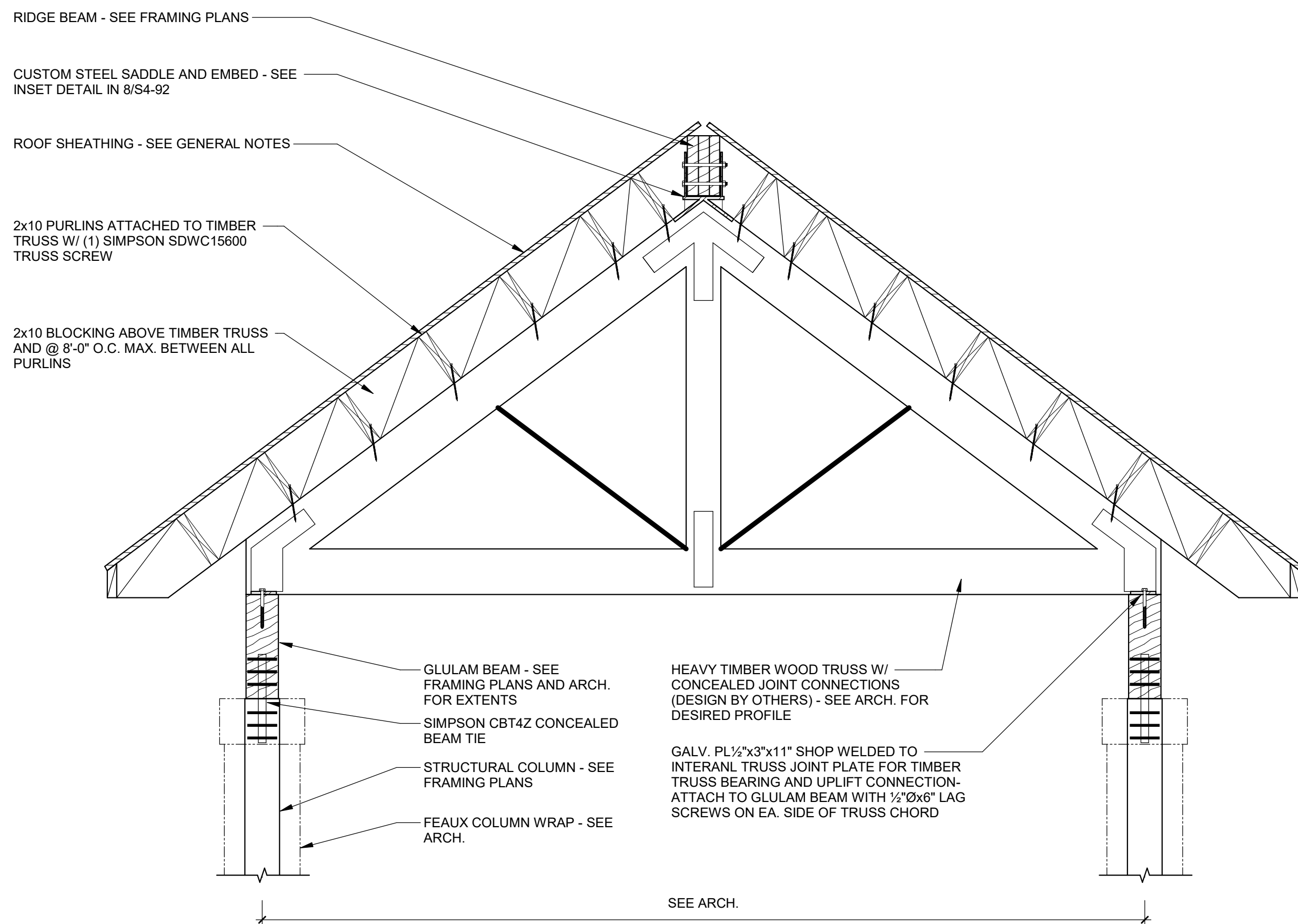
SHEET NUMBER

**S4-91**

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**14 EXTERIOR HEAVY TIMBER TRUSS**

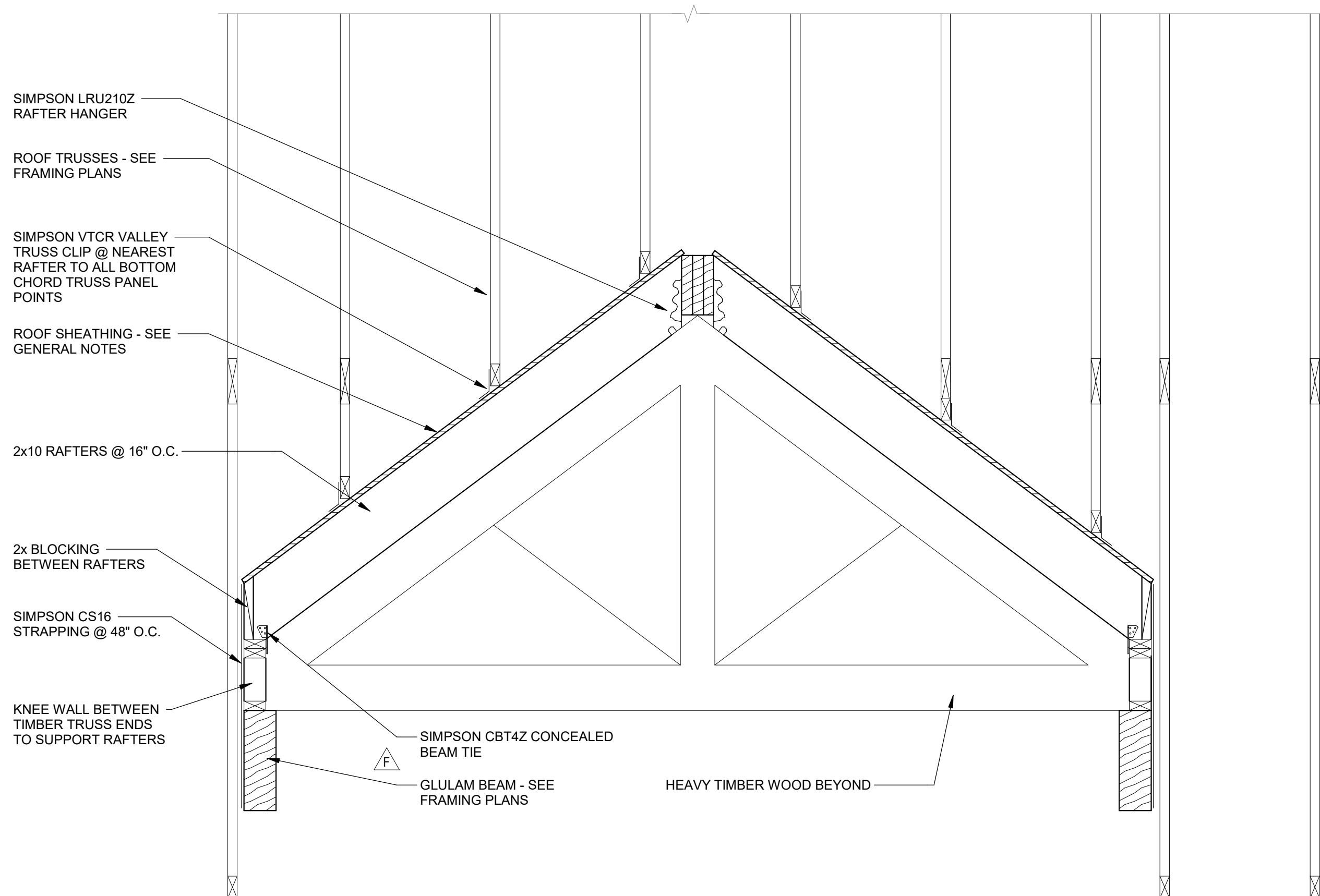
S4-92

SCALE: 3/4" = 1'-0"

**6 EXTERIOR HEAVY TIMBER TRUSS**

S4-92

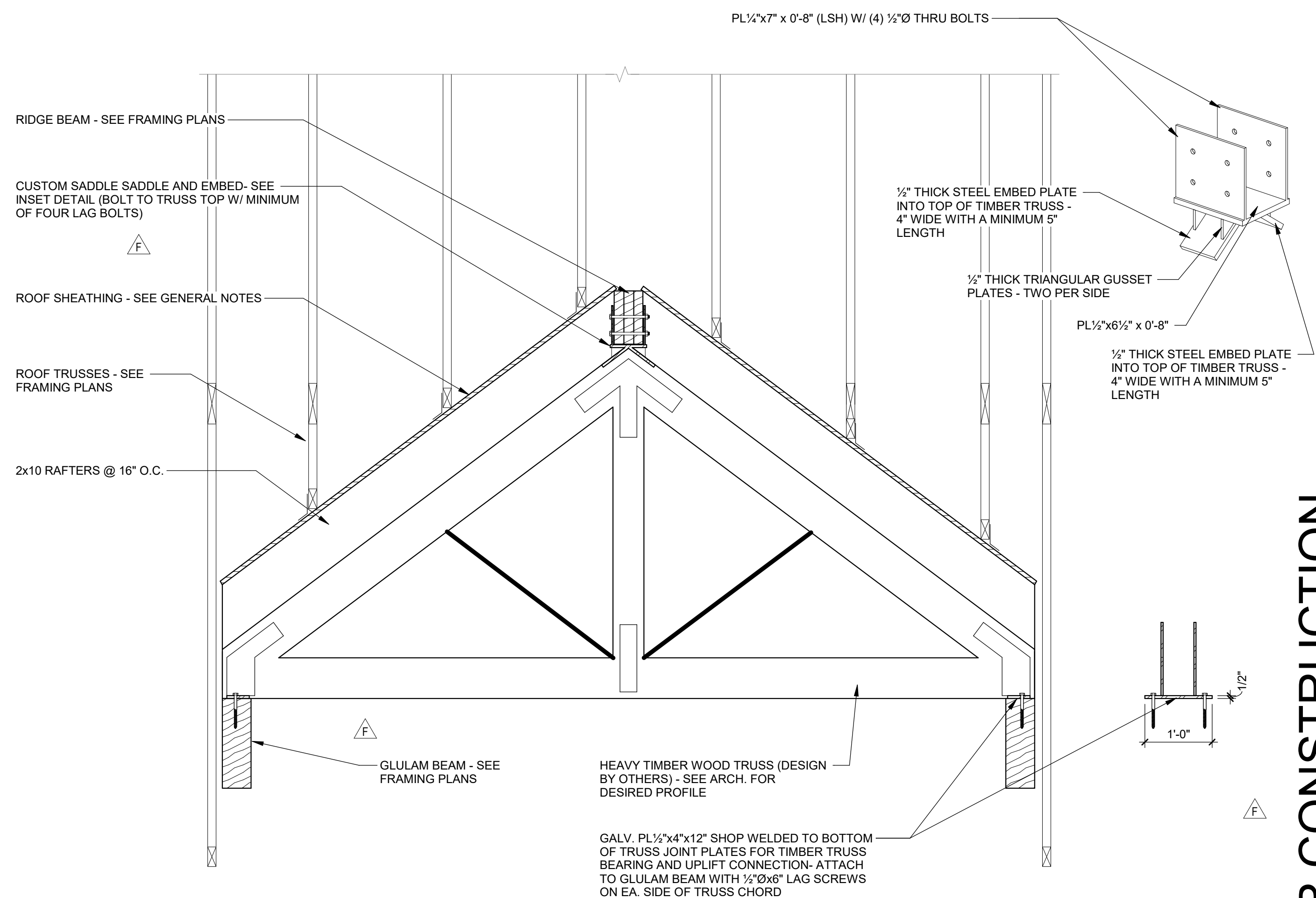
SCALE: 3/4" = 1'-0"



**16 INTERIOR VAULTED CEILING/ROOF**

S4-92

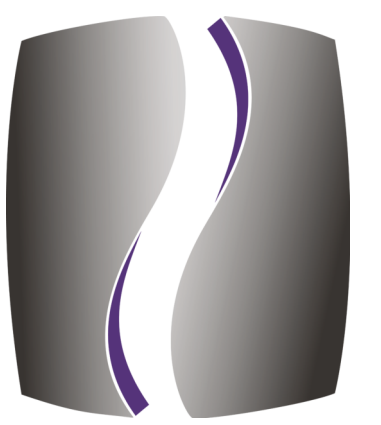
SCALE: 3/4" = 1'-0" S4-92



**8 INTERIOR HEAVY TIMBER TRUSS**

S4-92

SCALE: 3/4" = 1'-0"



**PHILLIPS**

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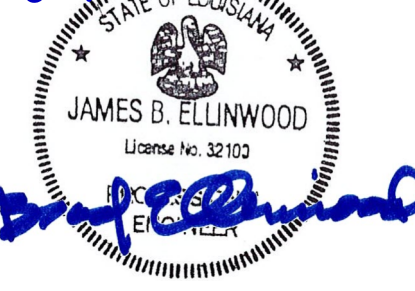


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SLIDELL, LA

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PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

DRAWING TITLE

**WOOD ROOF FRAMING SECTIONS AND DETAILS**

SHEET NUMBER

**S4-92**

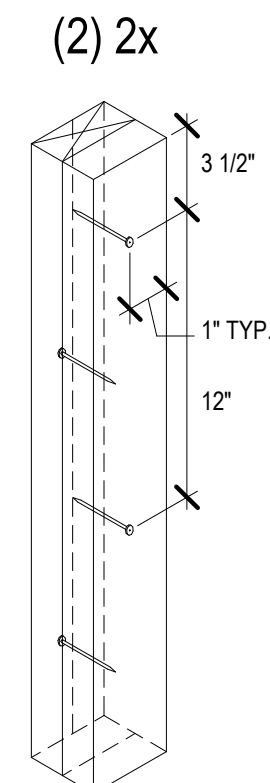
5901 PEACHTREE DUNWOODY RD.  
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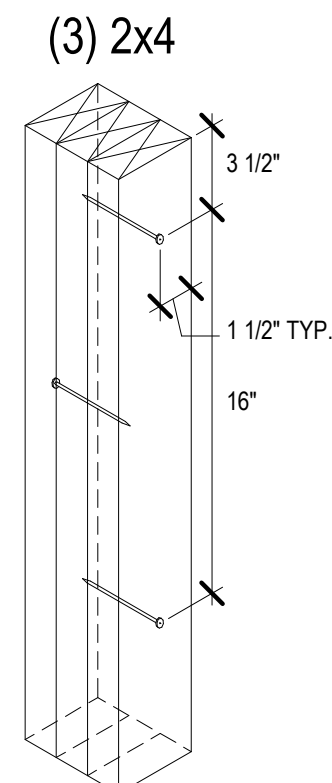
**FOR CONSTRUCTION**

COLUMN MK.	C1	C2	C3	C4	C5	C6	C7
# OF LEVELS SUPPORTED							
ROOF	2-2x4	2-2x4	2-2x4	-	-	3-2x4	4-2x4
ROOF + 1	2-2x4	2-2x4	3-2x4	3-2x4	4-2x4	4-2x4	4-2x4
ROOF + 2	2-2x4	3-2x4	4-2x4	5-2x4	3 1/2 x 8 1/4 GLULAM	5-2x4	5-2x4

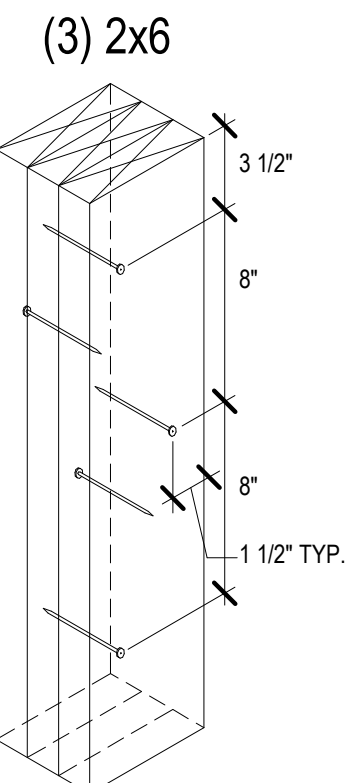
COLUMN MK.	C101	C102	C103	C104	C105	C106	C107	C108	C109	C110	C111
COLUMN OCCURS ONLY AT LEVELS INDICATED IN PLAN	2-2x4	3-2x4	4-2x4	5-2x4	2-2x6	3-2x6	4-2x6	5-2x6	3 1/2 x 6 7/8 GLULAM	3 1/2 x 8 1/4 GLULAM	3 1/2 x 9 5/8 GLULAM



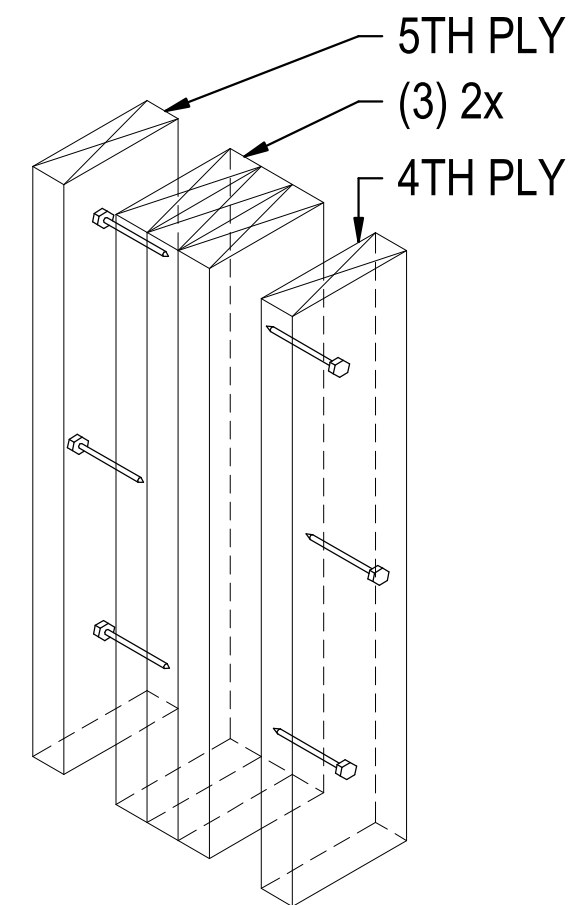
(2) 2x  
2x LAMINATIONS FASTENED W/ 10d COMMON WIRE NAILS @ 12" O.C. EACH FACE STAGGERED (e=0.148", L=3")



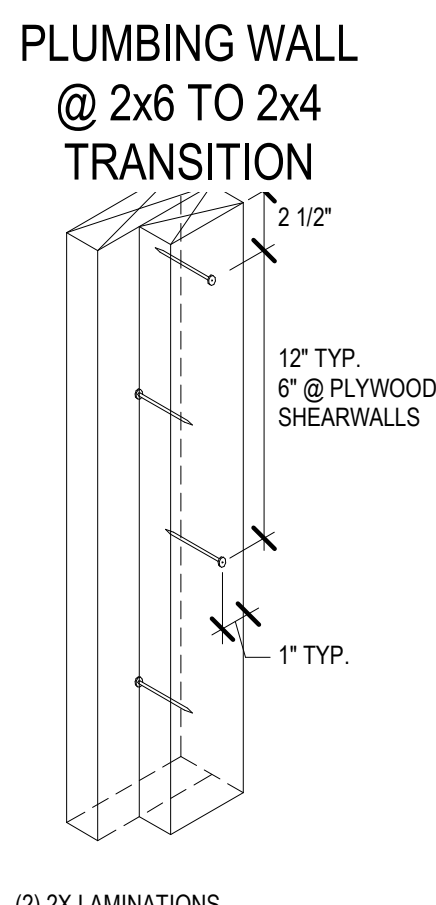
(3) 2x4  
3x 2x4 LAMINATIONS FASTENED W/ 10d COMMON WIRE NAILS @ 16" O.C. EACH FACE STAGGERED (e=0.148", L=3")



(3) 2x6  
3x 2x6 LAMINATIONS FASTENED W/ 10d COMMON WIRE NAILS @ 8" O.C. EACH FACE STAGGERED (e=0.148", L=3")



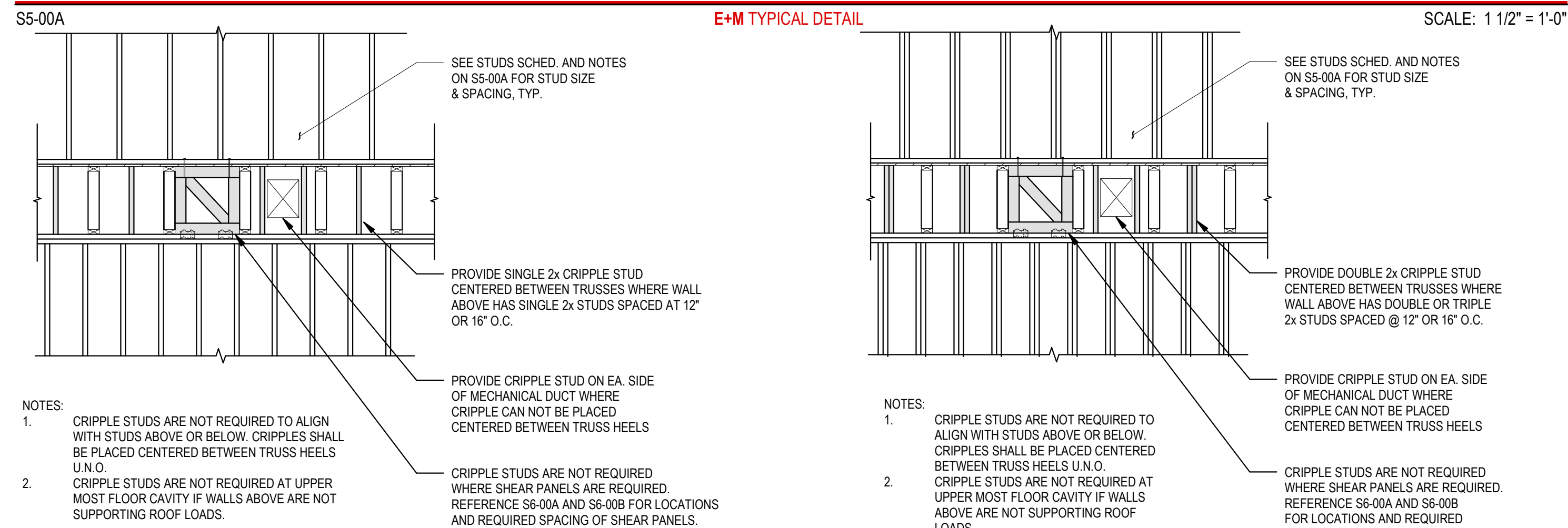
FASTEN 4TH AND 5TH LAMINATIONS W/ 1 ROW OF 8 6x3 1/2" WOOD SCREWS STAGGERED



(2) 2x LAMINATIONS FASTENED W/ 10d COMMON WIRE NAILS @ 12" O.C. EACH FACE STAGGERED (e=0.148", L=3")

- NOTES:
- ALL POSTS, BUILT-UP STUD COLUMNS AND STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING AT FLOOR CAVITY AND CONTINUE TO FOUNDATION OR BEAM SUPPORT BELOW.
  - DENOTES ENGINEERED LUMBER STUD AT EXTERIOR OPENING TAKING WIND LOAD. WIND STUDS NEEDED PER DETAIL IN 714 S4-03
  - \*\* DENOTES QUANTITY OF LEVELS OF WOOD FRAMING ABOVE THE CONCRETE SUPPORT SLAB (I.E. SLAB-ON-GRADE OR PODIUM).

### 15 BUILT-UP COLUMN SCHEDULE



### 16 CRIPPLE STUDS - SINGLE 2x STUD WALL ABOVE

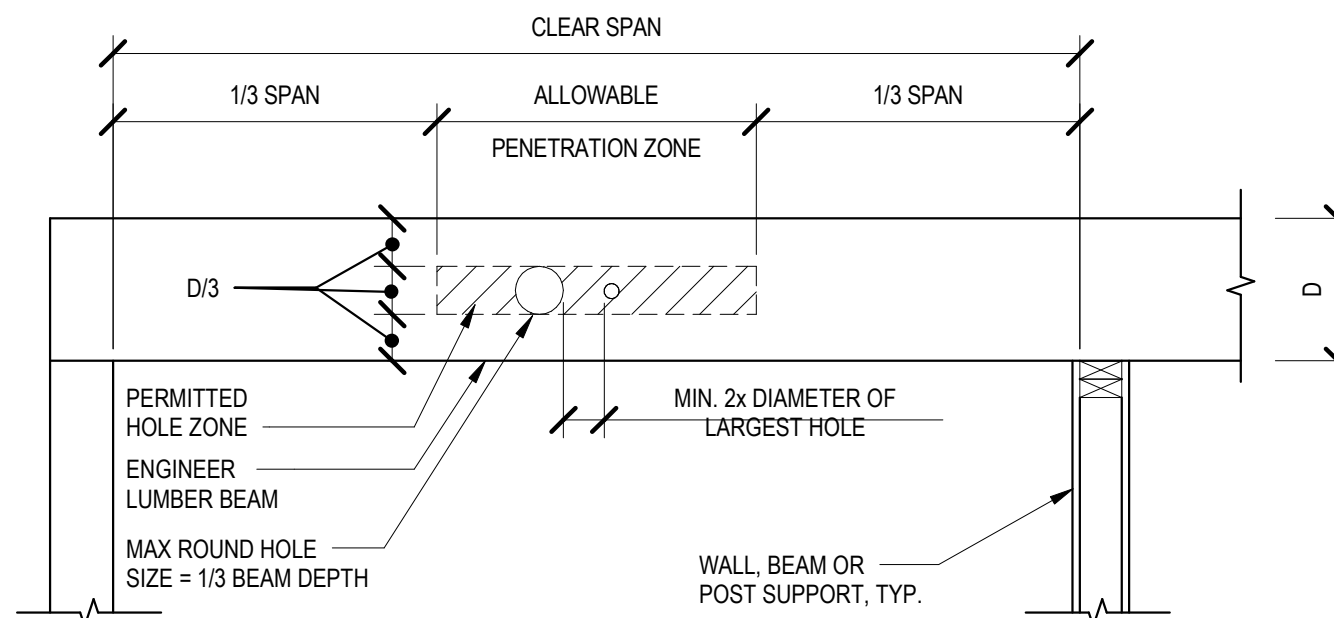
S5-00A E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"

### 12 CRIPPLE STUDS - DOUBLE 2x STUD WALL ABOVE

S5-00A E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"

### 8 STUD SCHEDULE AND NOTES

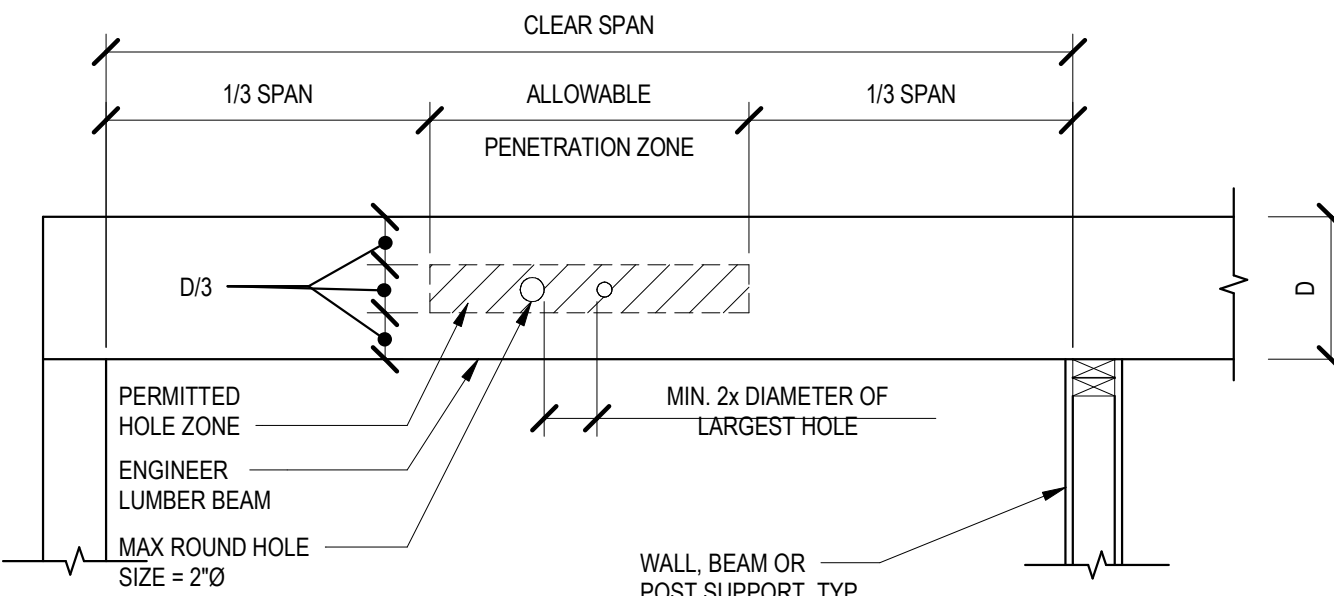
S5-00A E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"



- NOTES:
- DETAIL APPLIES TO LVL AND PSL BEAMS WITH DEPTHS GREATER THAN 7 1/4".
  - ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADS ONLY.
  - HOLES MUST BE ROUND. SQUARE HOLES ARE NOT PERMITTED.
  - HOLES ARE PERMITTED TO BE PLACED WITHIN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM DEPTH.
  - HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS.
  - DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
  - ANY CONDITION NOT COVERED BY THIS DETAIL MUST BE SUBMITTED TO THE SEOR FOR REVIEW PRIOR TO CUTTING ANY HOLES IN THE FIELD.

### 5 ALLOWABLE HOLES IN SOLID SAWN LUMBER BEAMS AND HEADERS

S5-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



- NOTES:
- DETAIL APPLIES TO LVL AND PSL BEAMS WITH DEPTHS GREATER THAN 7 1/4".
  - ALLOWABLE HOLES INDICATED ARE SUITABLE FOR HEADERS AND BEAMS WITH UNIFORM LOADS ONLY.
  - HOLES MUST BE ROUND. SQUARE HOLES ARE NOT PERMITTED.
  - HOLES ARE PERMITTED TO BE PLACED WITHIN THE MIDDLE 1/3 OF THE SPAN IN THE MIDDLE 1/3 OF THE BEAM DEPTH.
  - HOLES ARE NOT PERMITTED TO BE PLACED IN CANTILEVERS.
  - DETAIL APPLIES TO BOTH SIMPLE AND CONTINUOUS SPANS.
  - ANY CONDITION NOT COVERED BY THIS DETAIL MUST BE SUBMITTED TO THE SEOR FOR REVIEW PRIOR TO CUTTING ANY HOLES IN THE FIELD.

### 6 ALLOWABLE HOLES IN ENGINEERED LUMBER BEAMS AND HEADERS

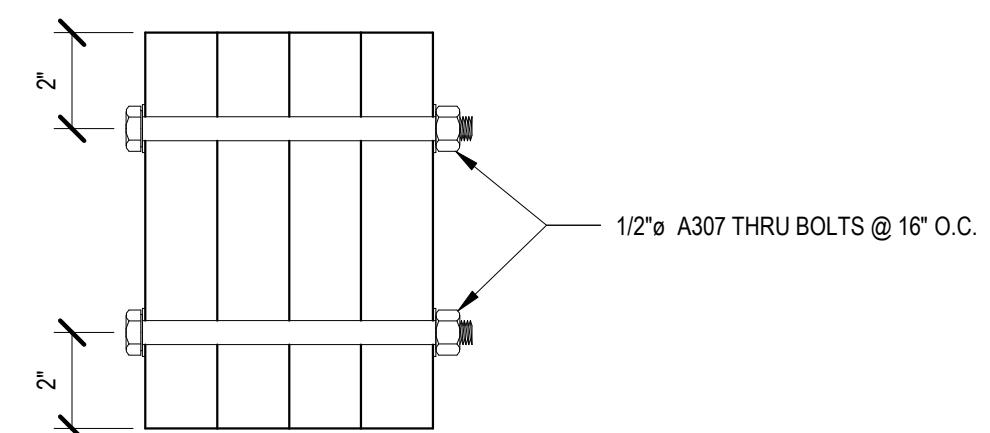
S5-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"

### TYPICAL FLOOR FRAMING NOTES:

- TRUSS MANUFACTURER TO COORDINATE LOCATION OF HORIZONTAL CHASES WITH MECHANICAL DRAWINGS.
- SEE GENERAL NOTES (S6-02) FOR LUMBER SPECIES AND GRADE U.N.O.
- SEE BUILDING PLAN FOR BEAM/HEADER TAGS, STUD FRAMING, AND ADD'L INFO.
- SEE S4-0F SERIES FOR TYPICAL FLOOR FRAMING DETAILS AND INFO.
- FOR EXTERIOR/BEARING WALLS, ANY WOOD STUD IS PERMITTED TO BE CUT/NOTCHED TO A DEPTH NOT EXCEEDING 25% OF ITS WIDTH. FOR NON-LOAD BEARING WALLS, STUDS MAY BE CUT/NOTCHED TO A DEPTH NOT EXCEEDING 40% OF ITS WIDTH. SEE S5-00B FOR ADDITIONAL INFORMATION.
- ALL POSTS, BUILT-UP STUD COL'S AND STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING AT FLOOR CAVITY LOCATIONS AND CONTINUE TO FOUNDATION OR BEAM SUPPORT BELOW.
- UNLESS NOTED OTHERWISE ON THE FRAMING PLANS OR IN 4'SS-00A, THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2x DIMENSION LUMBER BEAMS SHALL MATCH THE NUMBER OF MEMBERS IN THE BEAM. ALL LVL AND LSL BEAMS SHALL A (3) STUD MIN. BEARING (U.N.O.). THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS.
- ALL CONNECTOR TYPES REFER TO SIMPSON STRONG-TIE SPECIFICATIONS. SEE 8P/S0-03 FOR ADDITIONAL INFORMATION.
- PROVIDE SIMPSON TT JOIST HANGERS AT ALL FLUSH BEAM SUPPORT CONDITIONS. ALL HANGERS DESIGNATED IN DRAWINGS SHALL BE MAXIMUM AVAILABLE IF MANUFACTURER GIVES MIN/MAX OPTIONS.
- FOR NON-BALCONY CONDITIONS, CONNECT ALL WOOD BEAMS TO COLUMNS USING SIMPSON ECCO OR CCO U.N.O. CONTRACTOR SHALL COORDINATE SIZES WITH CONNECTION MANUFACTURER.
- WHERE PSL AND/OR GLULAM COLUMNS ARE INDICATED IN THE COLUMN SCHEDULE, A SQUASH BLOCK OR CRIPPLE STUD SHALL BE PROVIDED AT THE INTERSTITIAL FLOOR SPACE THAT IS OF THE SAME MATERIAL AS THE COLUMN. USE OF SAWN LUMBER SQUASH BLOCKING OR SAWN LUMBER CRIPPLE STUDS IS NOT ACCEPTABLE FOR PSL/GLULAM COLUMNS.

### 1 TYPICAL FLOOR FRAMING NOTES

S5-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



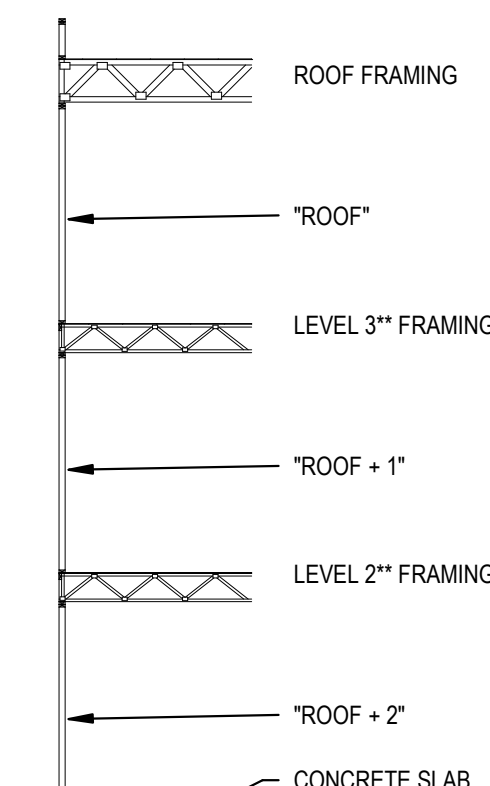
### 2 BOLTING PATTERN FOR TYPICAL 4-PLY LVL BEAM

S5-00A E+M TYPICAL DETAIL SCALE: 3" = 1'-0"

BUILDING STUD WALL SCHEDULE				
WALL MK.	AA	BB	CC	DD
# OF LEVELS SUPPORTED **				
ROOF	2x4 @ 16"	2x4 @ 16"	2x6 @ 16"	2x4 @ 16"
ROOF + 1	2x4 @ 16"	2x4 @ 16"	2x6 @ 16"	2x4 @ 12"
ROOF + 2	2x4 @ 16"	2x4 @ 12"	2x6 @ 16"	(2-)2x4 @ 16"

- NOTES:
- ALL LOAD BEARING STUDS SHALL BE: FINGER JOINTED SYP. GRADE: NO.2
  - CRIPPLE STUDS SHALL BE PROVIDED AT ALL FLOORS CAVITIES BELOW LOAD BEARING WALLS WHERE LOAD BEARING LADDER TRUSSES ARE NOT PROVIDED, E.G. INTERIOR LOAD BEARING WALLS, LOAD BEARING PARTY WALLS, ETC. CRIPPLE STUDS SHALL BE INSTALLED PER THE REQUIREMENTS OF 18 AND 19 ON S5-00B.
  - ALL VOLUME AREAS SHALL BE BALLOON FRAMED AS FOLLOWS UNLESS NOTED OTHERWISE IN THE STRUCTURAL FRAMING PLANS: 2x4 @ 16" O.C. UP TO 10 FT, 2x6 @ 16" O.C. UP TO 14 FT, (2) 2x6 @ 16" O.C. UP TO 18 FT.
  - ALL SHEAR WALLS SHALL HAVE STUDS @ 16" O.C. MAX.
  - ALL PLUMBING WALL SHALL HAVE 2x6 @ 16" U.N.O.
  - NON-LOAD BEARING INTERIOR PARTITIONS - SEE ARCH. DWGS.
  - ALL EXTERIOR WALLS SHALL BE TYPE "BB" U.N.O.
  - ALL NON-UNIT BEARING CORRIDOR WALLS SHALL BE TYPE "BB" U.N.O.
  - IT IS RECOMMENDED THAT THE STUDS @ EVERY LEVEL BE TEMPORARILY BLOCKED OR BRACED @ MID HEIGHT.
  - \*\* DENOTES QUANTITY OF LEVELS OF WOOD FRAMING ABOVE THE CONCRETE SUPPORT SLAB (I.E. SLAB-ON-GRADE OR PODIUM).
  - ALL LOAD BEARING WALLS SUPPORTING ROOF FRAMINGS SHALL BE FRAMED WITH 2x4 @ 16" O.C. UNLESS NOTED OTHERWISE.
  - WHERE ARCHITECTURAL DRAWINGS INDICATE 2x6 WALLS, PROVIDE 2x6 STUDS IN LIEU OF THE 2x4 STUDS INDICATED IN THE WALL SCHEDULE. 2x6 STUDS MAY BE SUBSTITUTED FOR 2x4 STUDS PER THE CONVERSION TABLE BELOW. THIS TABLE MAY NOT BE USED IN REVERSE TO SUBSTITUTE 2x4 STUDS IN LIEU OF SCHEDULED 2x6 STUDS.

2x4 STUD WALL PER WALL SCHED.	2x6 STUD WALL EQUIVALENT
2x4 @ 16" O.C.	2x6 @ 16" O.C.
2x4 @ 12" O.C.	2x6 @ 16" O.C.
(2) 2x4 @ 16" O.C.	2x6 @ 16" O.C.
(2) 2x4 @ 12" O.C.	2x6 @ 12" O.C.
(3) 2x4 @ 16" O.C.	(2) 2x6 @ 16" O.C.
(3) 2x4 @ 12" O.C.	(2) 2x6 @ 16" O.C.



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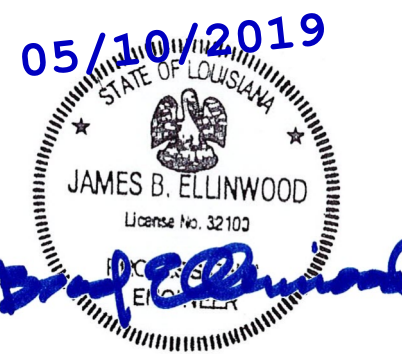
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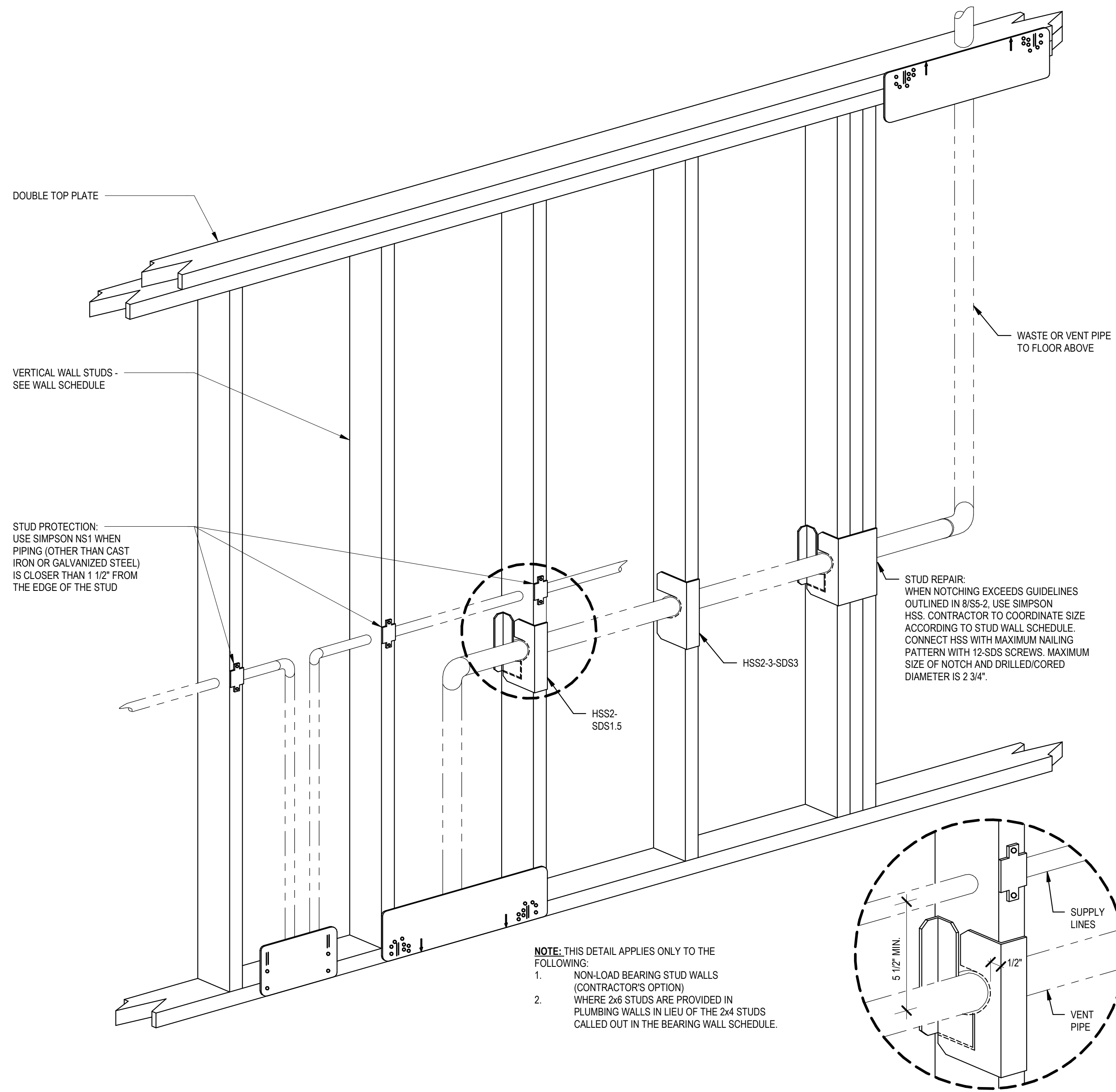
DRAWING TITLE WOOD WALL SCHEDULE AND DETAILS

SHEET NUMBER

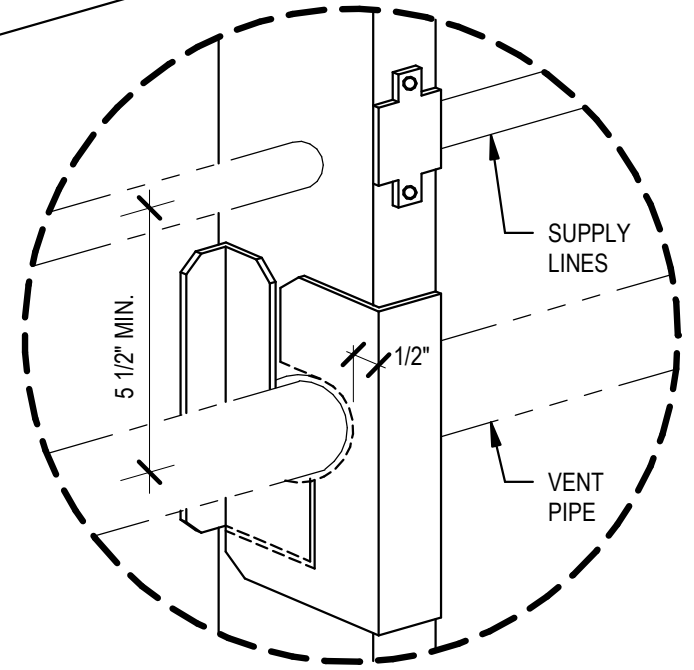
S5-00A

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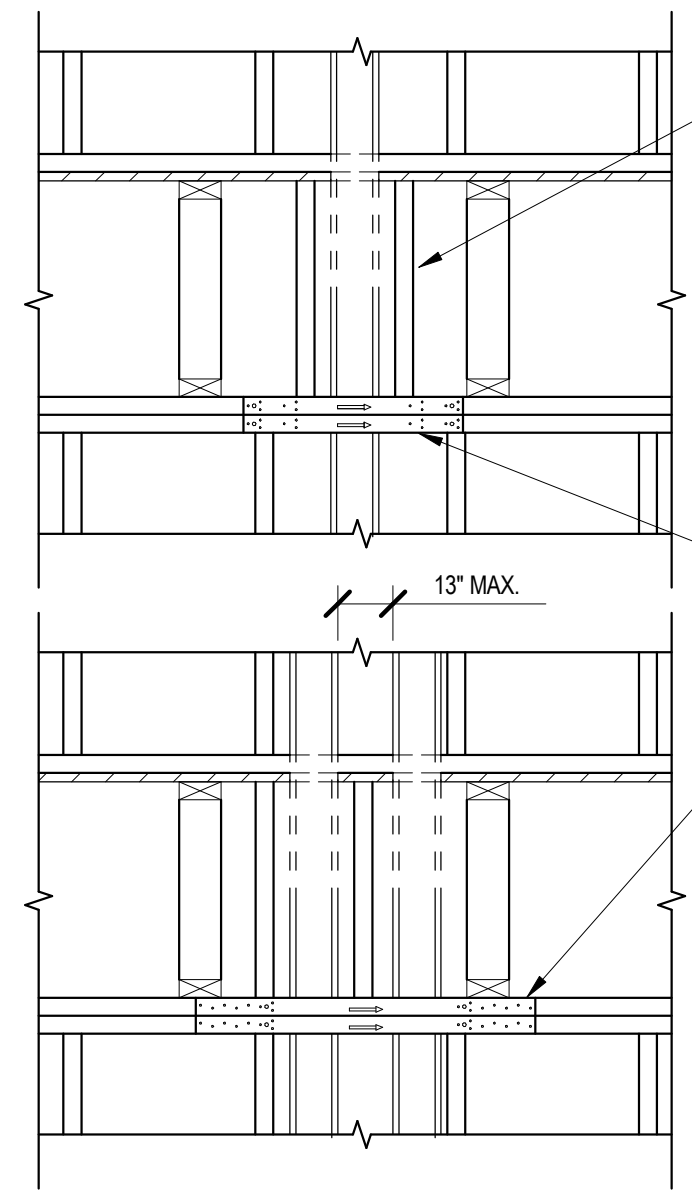
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NOTE: THIS DETAIL APPLIES ONLY TO THE FOLLOWING:  
 1. NON-LOAD BEARING STUD WALLS (CONTRACTOR'S OPTION)  
 2. WHERE 2x6 STUDS ARE PROVIDED IN PLUMBING WALLS IN LIEU OF THE 2x4 STUDS CALLED OUT IN THE BEARING WALL SCHEDULE.



STUD REPAIR:  
 WHEN NOTCHING EXCEEDS GUIDELINES OUTLINED IN 8/SS-2, USE SIMPSON HSS. CONTRACTOR TO COORDINATE SIZE ACCORDING TO STUD WALL SCHEDULE. CONNECT HSS WITH MAXIMUM NAILING PATTERN WITH 12-SDS SCREWS. MAXIMUM SIZE OF NOTCH AND DRILLED/CORED DIAMETER IS 2 3/4\"/>



PROVIDE 2-RSP18 STRAPS AT ALL TOP PLATE PENETRATIONS (HOLES OR NOTCHES) IN WHICH 40% OF THE PLATE WIDTH IS REMOVED.

NOTE: DETAIL APPLIES TO ALL INTERIOR LOAD BEARING WALLS AND SHEAR WALLS.

**15 PLUMBING WALL PENETRATIONS WITH REPAIRS**

S5-00B E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"

**6 STRAP REPAIR AT TOP PLATE PENETRATION HEADERS**

S5-00B E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"

**2 BEAM AND HEADER SCHEDULE**

S5-00B E+M TYPICAL DETAIL SCALE: 1 1/2" = 1'-0"

FLOOR LEVEL**	UP TO 3'-0" OPENING	UP TO 3'-0" OPENING	UP TO 6'-6" OPENING
	INTERIOR	EXTERIOR	EXTERIOR
ROOF	(1) 2x4 JAMB	(1) 2x4 JAMB	(1) 2x4 JAMB
	(1) 2x4 KING	(2) 2x4 KING	(3) 2x4 KING
ROOF + 1	(1) 2x4 JAMB	(1) 2x4 JAMB	(1) 2x4 JAMB
	(1) 2x4 KING	(2) 2x4 KING	(3) 2x4 KING
ROOF + 2	(1) 2x4 JAMB	(1) 2x4 JAMB	(1) 2x4 JAMB
	(2) 2x4 KING	(2) 2x4 KING	(4) 2x4 KING

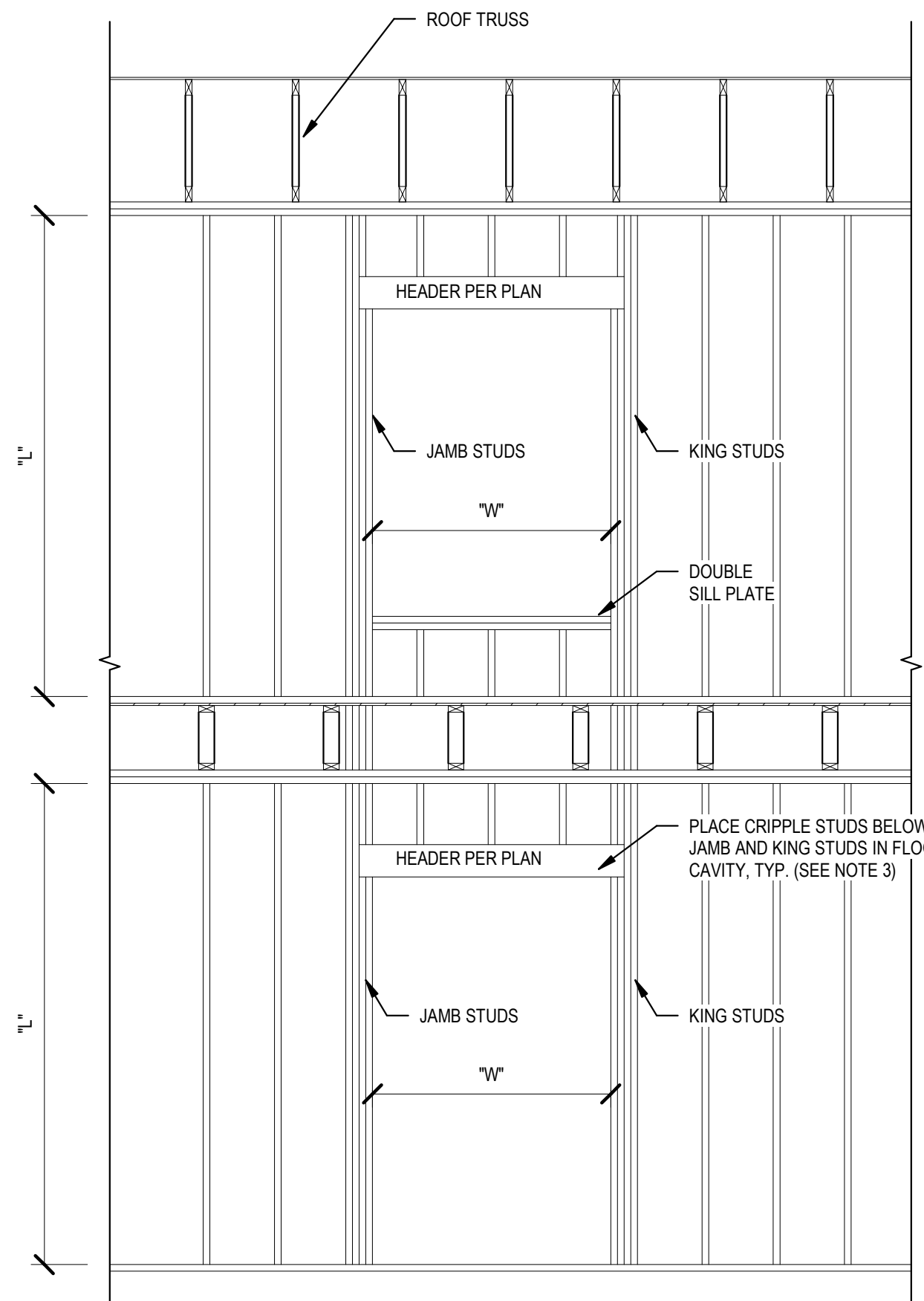
NOTES:  
 1. \*\* - SEE GRAPHIC IN DETAIL 8/SS-00A FOR CLARIFICATION ON "FLOOR LEVEL" TERMINOLOGY.  
 2. CRIPPLE STUDS SHALL BE PLACED BELOW JACK STUDS WITHIN THE FLOOR CAVITY. QUANTITY OF CRIPPLE STUDS SHALL MATCH QUANTITY OF JACK AND KING STUDS ABOVE.  
 3. WHERE 2x6 WALLS ARE REQUIRED PER ARCHITECTURAL DRAWINGS AND/OR 8/SS-00A STUD SCHEDULE, USE 2x6 KING & JAMB STUDS IN LIEU OF 2x4'S.  
 4. USE 2x6 STUDS WHERE 2x6 POSTS, SPLY HEADERS OR 2x6 JAMB/KING STUDS ARE INDICATED.  
 5. WHERE COLUMNS ARE INDICATED AT HEADER SUPPORTS IN PLAN, PROVIDE QUANTITY OF KING PLUS JAMB STUDS INDICATED IN TABLE FOR OPENING SIZE.  
 6. SCHEDULES APPLY TO WALL OPENINGS INCLUDING DOORS, WINDOWS AND CASED OPENINGS ONLY. FLUSH BEAM SUPPORT POSTS SHALL BE CALLED OUT IN PLAN OR AS INDICATED IN NOTE 7 OF 1/SS-00A WHERE MULTIPLE ADJACENT WINDOW OPENINGS ARE REQUIRED IN THE EXTERIOR WALL (DOUBLE OR TRIPLE WINDOWS), PROVIDE JAMB STUDS PER THE WINDOW WIDTH BETWEEN JAMB STUD PACKS, PROVIDE KING STUDS PER THE TOTAL OPENING WIDTH BETWEEN THE KING STUDS.

**8 JAMB/KING STUD SCHEDULE AT WALL OPENINGS (APPLIES TO ALL WALLS SUPPORTING FLOOR TRUSSES AND EXTERIOR WALLS UNO)**

S5-00B E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"

BEAM/HEADER MARK	MATERIAL TYPE	NO. OF PLYS	PLY WIDTH	DEPT H
B1	DIMENSION LUMBER	2	1 1/2"	7 1/4"
B2	DIMENSION LUMBER	3	1 1/2"	7 1/4"
B3	DIMENSION LUMBER	2	1 1/2"	9 1/4"
B4	DIMENSION LUMBER	3	1 1/2"	9 1/4"
B5	DIMENSION LUMBER	2	1 1/2"	11 1/4"
B6	DIMENSION LUMBER	3	1 1/2"	11 1/4"
B10	MICROLLAM	2	1 3/4"	9 1/2"
B11	MICROLLAM	3	1 3/4"	9 1/2"
B12	MICROLLAM	2	1 3/4"	11 7/8"
B13	MICROLLAM	3	1 3/4"	11 7/8"
B14	MICROLLAM	2	1 3/4"	14"
B15	MICROLLAM	3	1 3/4"	14"
B15a*	MICROLLAM	4	1 3/4"	14"
B16	MICROLLAM	2	1 3/4"	16"
B17	MICROLLAM	3	1 3/4"	16"
B18	MICROLLAM	2	1 3/4"	18"
B19	MICROLLAM	3	1 3/4"	18"
B20*	MICROLLAM	4	1 3/4"	18"
B21	GLULAM	1	5 1/8"	16 1/2"

NOTES:  
 1. DIMENSION LUMBER SHALL BE SOUTHERN YELLOW PINE, NO. 2.  
 2. SEE GENERAL NOTES FOR MICROLLAM (LVL) PROPERTIES.  
 3. \* DENOTES SPECIAL BOLTING PATTERN FOR LAMINATION. SEE 2/SS-00A FOR INFORMATION.  
 4. SEE GENERAL NOTES SECTION 9 NOTE M FOR TYPICAL LAMINATION REQUIREMENTS FOR MULTI-PLY WOOD BEAMS.



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ISSUE & REVISION RECORD

#	DATE	DESCRIPTION
	09/24/2018	PERMIT SET
	08/08/2018	DESIGN DEVELOPMENT
D	12/05/18	COORDINATION REVIEW
F	02/07/2019	CONSTRUCTION REVIEW SET
G	03/22/2019	CONSTRUCTION RELEASE
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PROJECT

**FREMAUX PARK APARTMENTS**

SLIDELL, LA

CLIENT



PHILLIPS JOB NUMBER 1867709

ISSUE DATE 05/10/2019

DRAWN BY/CHECKED BY DKJ / RAFAEL, RIM

APPROVED BY BRAD, JBE

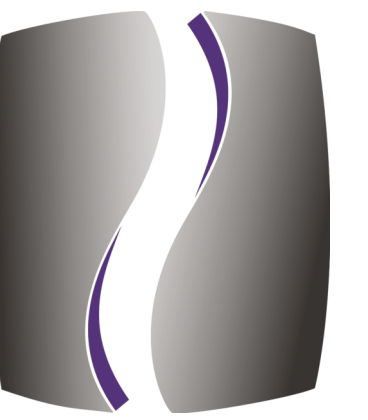
DRAWING TITLE

**WOOD WALL SCHEDULE AND DETAILS**

SHEET NUMBER

**S5-00B**

5901 PEACHTREE DUNWOODY RD.  
 BUILDING A, SUITE 450  
 ATLANTA, GEORGIA 30328  
 PHILLIPSPART.COM 770-394-1616



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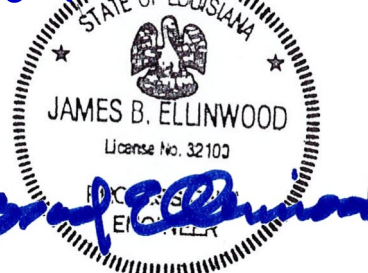


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 DRAWING TITLE

**WOOD WALL SCHEDULE AND DETAILS**

SHEET NUMBER

**S5-00C**

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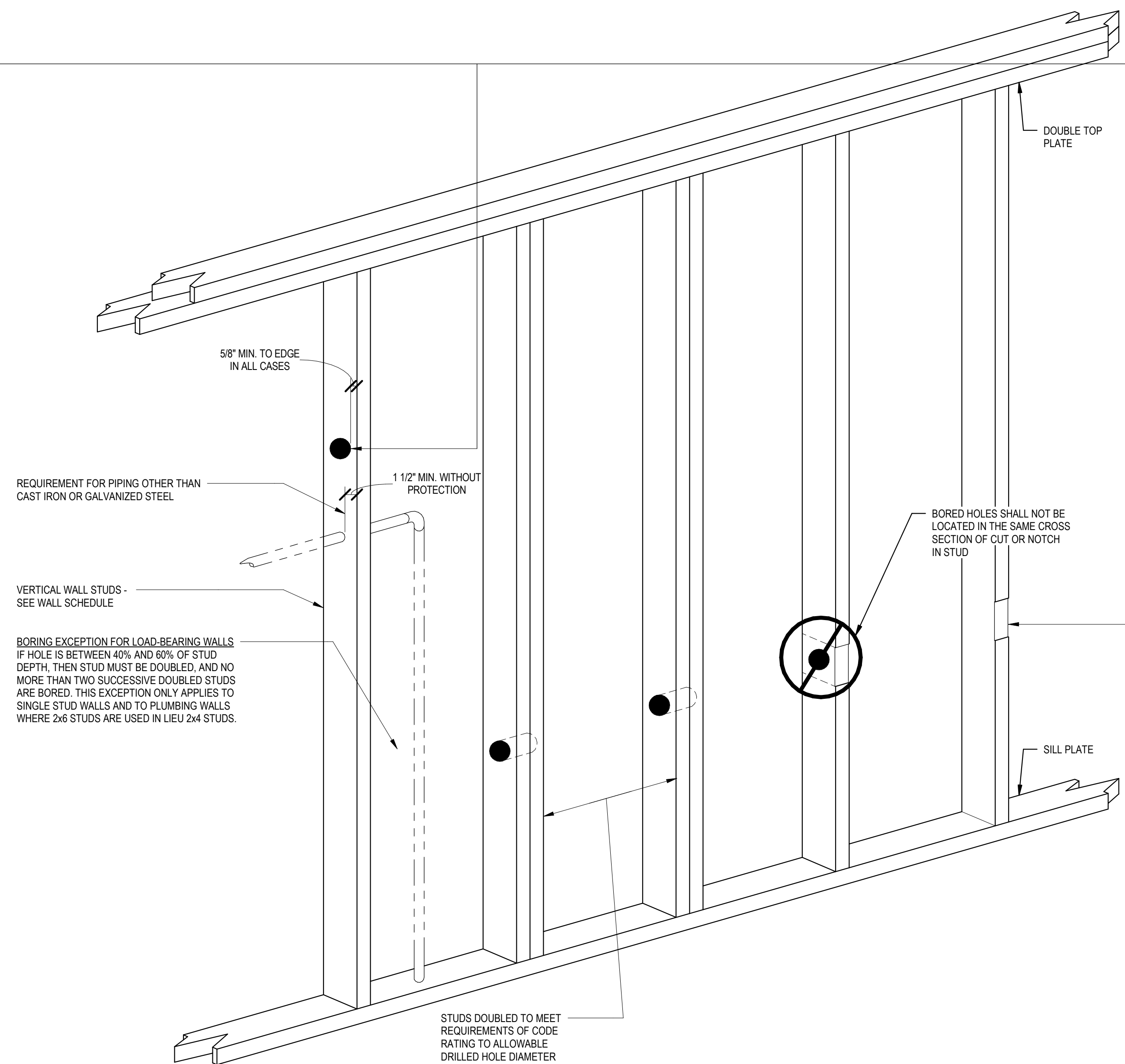
**MAXIMUM BORED HOLE DIAMETER/NOTCH DEPTH WITH NO REPAIR/PROTECTION**

STUD SIZE (IN)	APPLICATION	MAXIMUM HOLE DIAMETER (IN)	MAXIMUM NOTCH DEPTH (IN)
2x4	NON LOAD-BEARING STUDS (INTERIOR ONLY)	2	1 3/8
	ALL EXTERIOR STUDS & LOAD-BEARING STUDS	1 3/8 *	7/8
2x6	NON LOAD-BEARING STUDS (INTERIOR ONLY)	3 1/4	2 3/16
	ALL EXTERIOR STUDS & LOAD-BEARING STUDS	2 3/16 *	1 3/8

\* DENOTES BORED HOLE DIAMETER EXCEPTION FOR INTERIOR LOAD-BEARING STUDS ONLY. SEE TYPICAL NOTE 1 FOR INFORMATION.

**TYPICAL NOTES:**

- EXCEPTION FOR HOLE BORING IN INTERIOR LOAD-BEARING STUDS:  
IF REQUIRED BORED HOLE DIAMETER EXCEEDS VALUE GIVEN ABOVE, BUT IS LESS THAN 60% OF THE STUD WIDTH, THEN BORED STUDS SHALL BE DOUBLED, PROVIDED NO MORE THAN TWO SUCCESSIVE DOUBLED STUD GROUPS ARE BORED.  
- WHERE MULTIPLE STUDS ARE SPECIFIED IN STUD WALL SCHEDULE ON SHEET S5-00A, DOUBLE STUD GROUPS SHALL EQUAL TWICE THE PRESCRIBED STUDS.
- WHERE NOTCHES AND/OR BORED HOLES DO NOT COMPLY WITH THIS DETAIL, THE CONTRACTOR SHALL REPORT THE CONDITIONS TO THE STRUCTURAL ENGINEER OF RECORD (E.O.R.) CONTRACTOR SHALL NOT CONTINUE BORING AND/OR NOTCHING ACTIVITIES UNTIL ALTERNATIVE SOLUTION IS PROVIDED BY E.O.R.



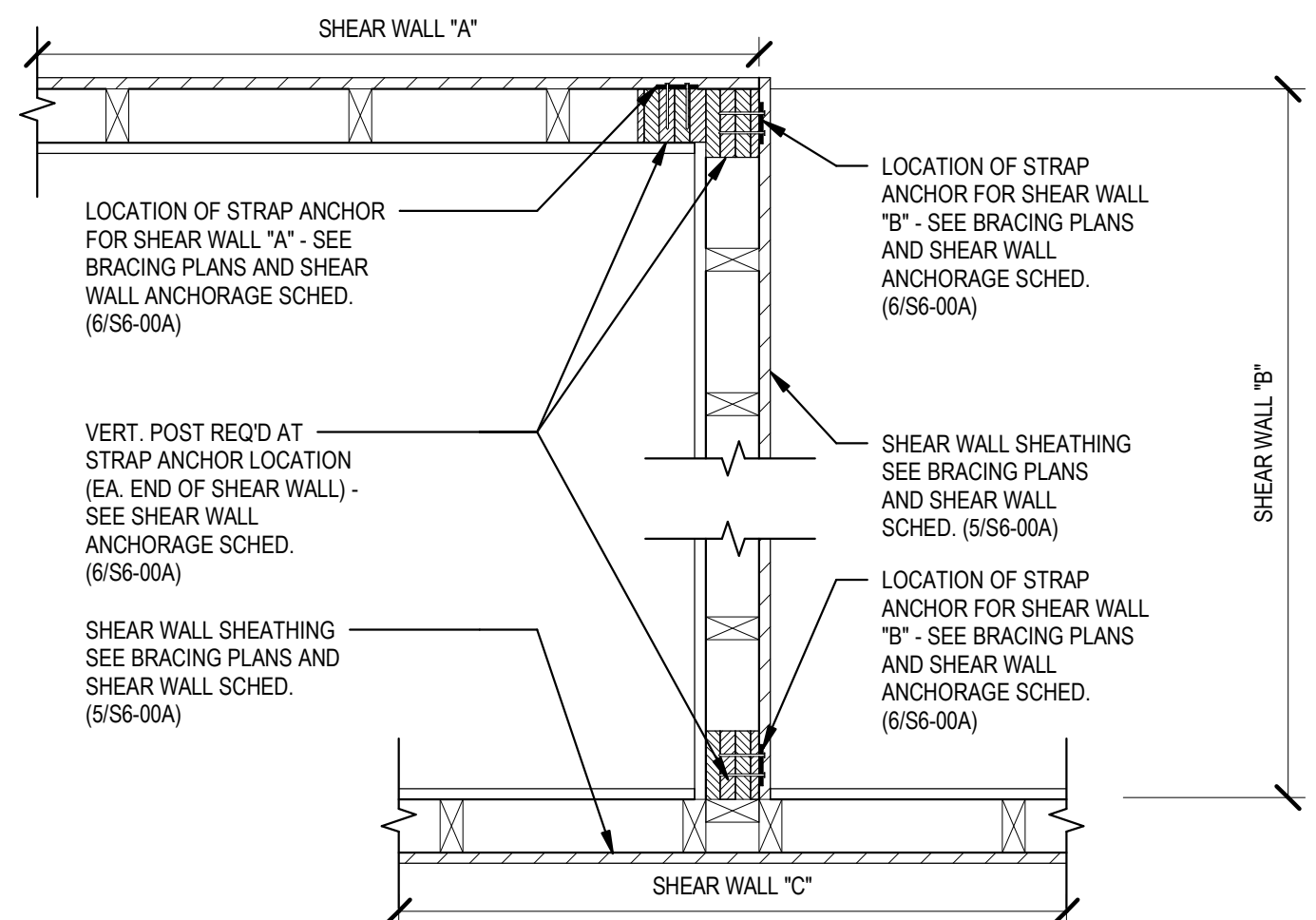
**11 ALLOWABLE STUD WALL PENETRATIONS PER IBC 2012**

S5-00C

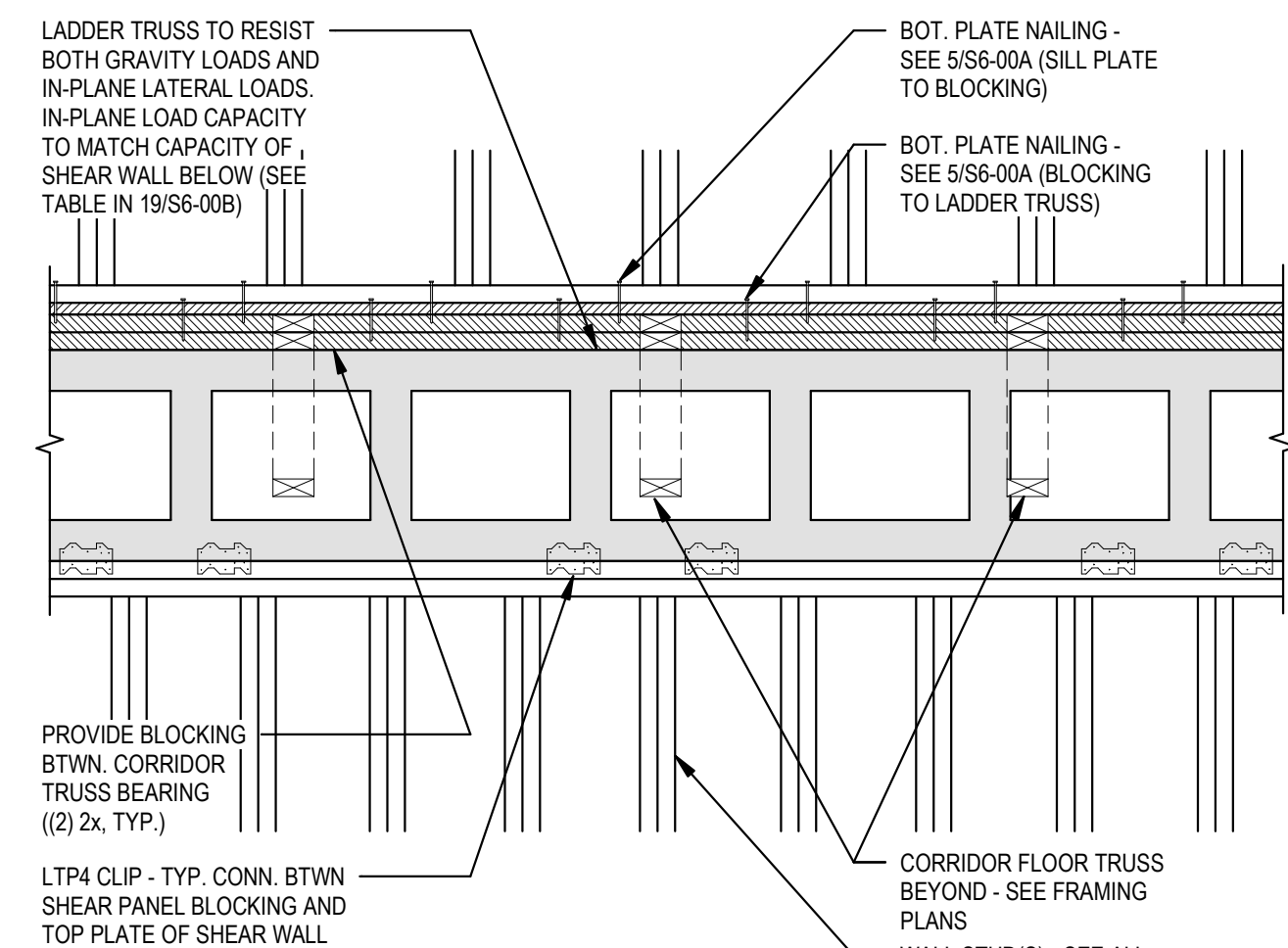
E+M TYPICAL DETAIL

SCALE: 1 1/2" = 1'-0"

FOR CONSTRUCTION



**13 (ELEVATED)**  
S6-00A E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



**5 SHEAR PANEL DETAILING AT CORRIDOR WALL**  
S6-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"

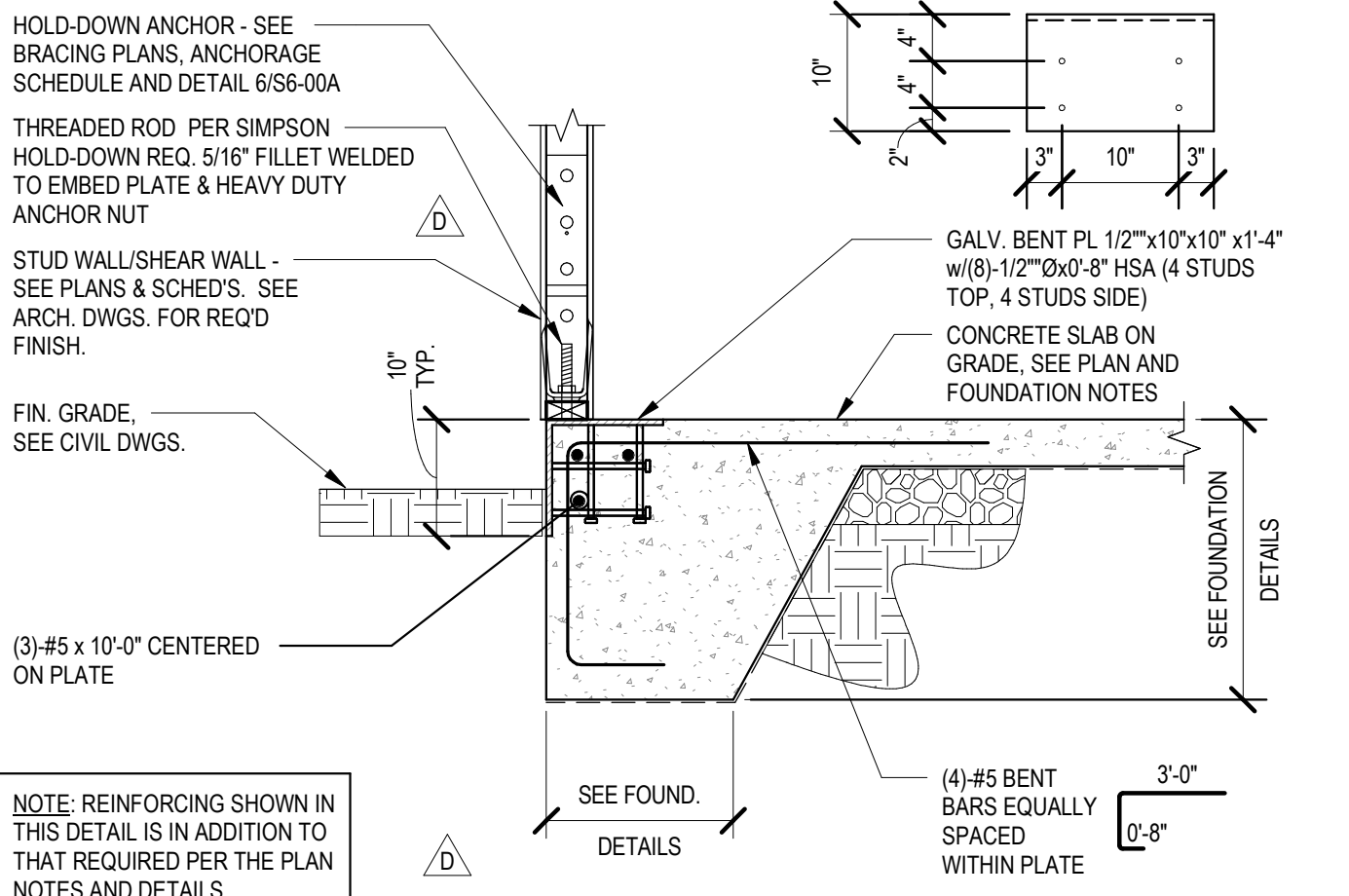
- 1 SHEAR WALL NOTES**
- SHEAR WALL FRAMING SHALL CONSIST OF WOOD STUDS AT A MAXIMUM SPACING OF 16" O.C. THE ACTUAL SIZE AND SPACING OF STUDS SHALL BE PER THE STUD WALL SCHEDULE ON SHEET S5-00A. SEE GENERAL NOTES FOR MATERIAL REQUIREMENTS FOR WOOD FRAMING, TYP.
  - SHEAR WALL SHEATHING SHALL BE PER THE SHEAR WALL SCHEDULE (DETAIL 5/S6-00A). SEE GENERAL NOTES FOR ADDITIONAL INFORMATION.
  - REQUIRED FASTENING PATTERNS FOR SHEAR WALL SILL PLATES, TOP PLATES AND SHEATHING SHALL BE PER THE SHEAR WALL SCHEDULE (5/S6-00A), SHEAR WALL ANCHORAGE SCHEDULE (6/S6-00A) AND NAILING REQUIREMENTS IN THE GENERAL NOTES (5/0-0).
  - REQUIRED UPLIFT HARDWARE (VERTICAL STRAPPING AND FOUNDATION ANCHORS) AND END POSTS SHALL BE PER THE SHEAR WALL ANCHORAGE SCHEDULE (6/S6-00A). SEE TYPICAL DETAILS FOR LAYOUT AND ADDITIONAL REQUIREMENTS.
  - REQUIRED HORIZONTAL STRAPPING AT PERFORATIONS IN SHEAR WALLS SHALL BE PER TYPICAL DETAILS ON SHEET S6-00A.
  - IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE SPECIFIED MATERIALS AND HARDWARE. SUBSTITUTION OF THE MATERIALS AND/OR HARDWARE SPECIFIED IN THESE DRAWINGS MUST BE APPROVED BY THE ENGINEER OF RECORD PRIOR TO INSTALLATION. USE OF ALTERNATE HARDWARE AND/OR MATERIALS MAY RESULT IN A REQUEST FOR THE SHOP DRAWINGS TO BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
  - IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE PROPER INSTALLATION OF ALL HARDWARE, SUCH THAT IT IS IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND IN COMPLIANCE WITH ALL APPLICABLE BUILDING CODES. IF REQUIRED BY THE AUTHORITY HAVING JURISDICTION, IT SHALL BE THE OWNER'S RESPONSIBILITY TO PROVIDE SPECIAL INSPECTIONS. SEE SCHEDULE OF SPECIAL INSPECTIONS ON SHEET S0-10.
  - WHERE PLUMBING WALLS ARE PRESENT WITHIN A PORTION OF A SHEAR WALL, THE FOLLOWING SHALL BE PROVIDED:
    - 2x6 PLUMBING WALLS SHALL BE FRAMED SUCH THAT A MINIMUM OF (2)-2x6 VERTICAL STUDS ARE PROVIDED AT THE END OF THE PLUMBING WALL (AT THE TRANSITION TO THE 2x4 WALL). SEE DETAIL 4/S6-00B.
    - A MINIMUM OF (2)-2x4 VERTICAL STUDS SHALL BE PROVIDED AT THE END OF THE PLUMBING WALL (AT THE TRANSITION TO THE PLUMBING WALL). SEE DETAIL 4/S6-00B.
    - STUDS AT WALL TRANSITION SHALL BE FASTENED TOGETHER PER DETAIL 4/S6-00B.
    - SHEAR WALL SHEATHING SHALL BE FASTENED TO BOTH (2)-2x6 AT END OF PLUMBING WALL AND (2)-2x4 AT END OF PLUMBING WALL USING FASTENING PATTERN SPECIFIED IN SHEAR WALL SCHEDULE. SEE DETAIL 4/S6-00B AND SHEAR WALL SCHEDULE (5/S6-00A).
  - SHEAR PANELS OR CONTINUOUS SHEATHING SHALL BE PROVIDED THROUGH THE FLOOR CAVITY. SEE DETAILS ON SHEET S6-00A. WHERE SHEAR PANELS ARE PROVIDED, SEE DETAIL 19/S6-00B.

**1 SHEAR WALL NOTES**  
S6-00A E+M TYPICAL DETAIL SCALE: 12" = 1'-0"

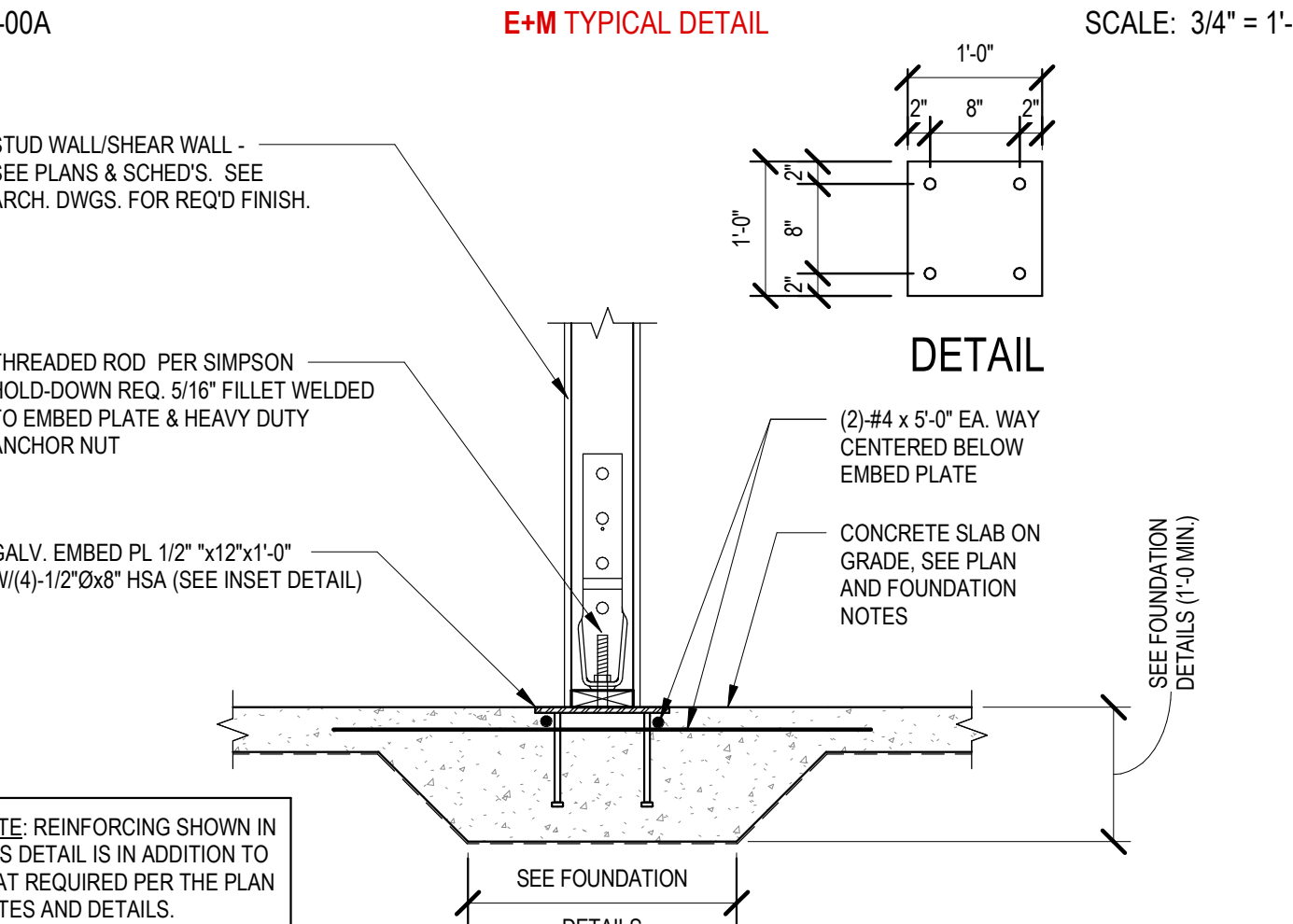
SHEAR WALL TYPE	SHEATHING		EDGE NAILING (2), (3), (4)	FIELD NAILING (2), (3), (4)	BLOCKED PANEL EDGES	BOTTOM PLATE NAILING	SILL PLATE AT BASE ANCHORAGE TYPE				
	15/32" PLYWOOD OR OSB (8)	5/8" GYPSUM WALLBOARD OR GYPSUM SHEATHING					RECOMMENDED	ALTERNATES			
A	*		10d @ 2" O.C.	10d @ 12" O.C.	YES	2" O.C.	5/8" @ 16" O.C.	1/2" x 2 1/4" EMBED @ 24" O.C.	-	-	9"
B	*		10d @ 3" O.C.	10d @ 12" O.C.	YES	3" O.C.	5/8" @ 20" O.C.	1/2" x 2 1/4" EMBED @ 30" O.C.	-	-	12"
C	*		10d @ 4" O.C.	10d @ 12" O.C.	YES	4" O.C.	1/2" @ 16" O.C. OR 5/8" @ 24" O.C.	1/2" x 2 1/4" EMBED @ 40" O.C.	-	-	14"
D	*		10d @ 6" O.C.	10d @ 12" O.C.	YES	6" O.C.	1/2" @ 24" O.C. OR 5/8" @ 30" O.C.	1/2" x 2 1/4" EMBED @ 48" O.C.	-	-	24"
E		*	6d @ 4" O.C.	6d @ 4" O.C.	YES	10" O.C. (4)	1/2" @ 16" O.C. (4)	1/2" x 2 1/4" EMBED @ 48" O.C.	6" O.C.	-	48"
F		*	6d @ 4" O.C.	6d @ 4" O.C.	NO	12" O.C. (4)	1/2" @ 48" O.C. (4)	1/2" x 2 1/4" EMBED @ 48" O.C.	7" O.C.	-	48"
G		*	6d @ 7" O.C.	6d @ 7" O.C.	NO	15" O.C. (4)	1/2" @ 48" O.C. (4)	1/2" x 2 1/4" EMBED @ 48" O.C.	9" O.C.	-	48"

- NOTES:**
- FRAMING AT ADJOINING PANEL EDGES SHALL BE 3-INCH NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED.
  - NAIL REQUIREMENT: INTERIOR 5/8" GYPSUM REQUIRES 6d COOLER NAILS (0.092" @ 1.78" LONG, 1/4" HEAD) OR TYPE W OR S NO. 6 DRYWALL SCREWS 1 1/4" LONG.
  - NAIL REQUIREMENT: ALL EXTERIOR SHEATHING SHALL BE FASTENED WITH CORROSION RESISTANT NAILS.
  - REDUCE SILL PLATE ANCHOR SPACING BY 1/2 WHEN SHEAR WALL TYPES ARE SPECIFIED ON BOTH SIDES OF WALL.
  - NOT RECOMMENDED FOR USE ON POST-TENSIONED AND/OR ELEVATED SLAB. CONTRACTOR SHALL COORDINATE BOLT SPACING WITH SLAB REINFORCING PRIOR TO POUR.
  - PROVIDE 2" MINIMUM EDGE DISTANCE AND 4" MINIMUM EMBEDMENT AT SLAB EDGE CONDITIONS, TYP.
  - SEE GENERAL NOTES FOR REQUIRED GRADE AND EXPOSURE RATING FOR SHEAR WALL SHEATHING.
  - ALL WOOD SHEATHING SHALL BE STRUCTURAL NO. 1 GRADE.

**6 SHEAR WALL SCHEDULE**  
S6-00A E+M TYPICAL DETAIL SCALE: 12" = 1'-0"



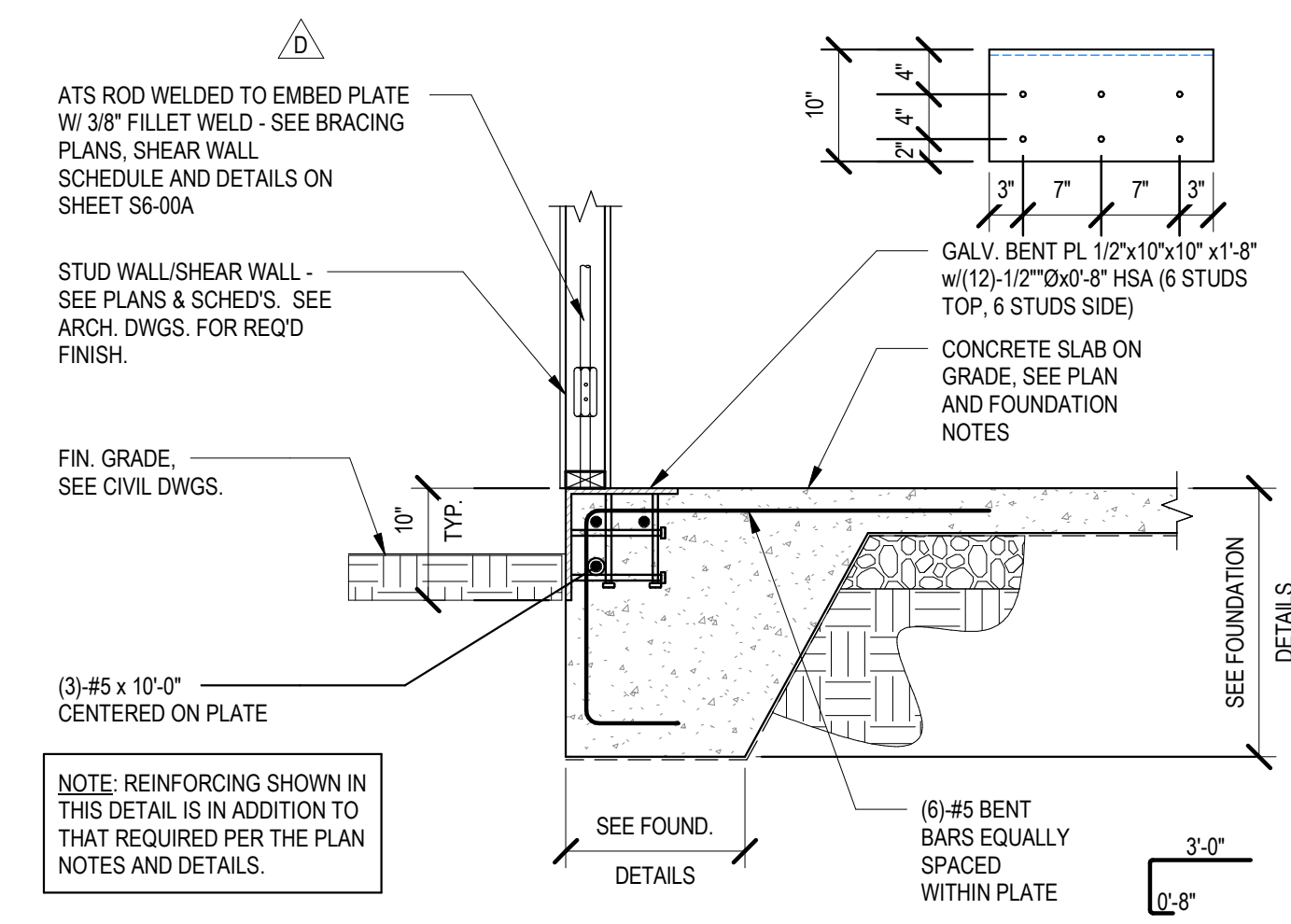
**15 EMBED PLATE @ SLAB EDGE CONDITION**  
S6-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**16 PLATE INTERIOR CONDITION**  
S6-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**12 EDGE CONDITION**  
S6-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



**12 EDGE CONDITION**  
S6-00A E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"

**8 SHEAR WALL ANCHORAGE SCHEDULE**

ANCHOR NO.	ANCHOR TYPE (1)	VERTICAL COLUMN REQUIREMENTS AT END (4) OF SHEARWALL	ANCHOR TO VERT. WOOD MEMBER CONNECTION REQUIREMENTS	FOUNDATION ANCHORAGE REQUIREMENT (3)			ANCHOR CAPACITY (LBS)
				RECOMMENDED	ALTERNATE		
1	(1) CS16	(2) 2x4 (MIN)	(26) 8d	-	-	-	1,705
2	(2) CS16	(2) 2x4 (MIN)	(52) 8d	-	-	-	3,410
3	(1) MST60/72	(2) 2x4 (MIN)	(68) 16d	-	-	-	4,830
4	(2) MST60/72	6x6 NOMINAL POST (MIN)	(136) 16d	-	-	-	9,660
5	LTT19	(2) 2x4 (MIN)	(8) 10d	5/8" @ 7" MIN. EMBED.	5/8" x 4" MIN. EMBED.	5/8" @ 3" MIN. EMBED.	1,150
6	LTT20B	(2) 2x4 (MIN)	(10) 10d	5/8" @ 7" MIN. EMBED.	5/8" @ 4" MIN. EMBED.	5/8" @ 5 5/8" MIN. EMBED.	1,290
7	HTT4	(2) 2x4 (MIN)	(26) 16d x 1/2"	5/8" @ 7" MIN. EMBED.	3/4" @ 7" MIN. EMBED.	5/8" @ 5 5/8" MIN. EMBED.	3,640
8	HTT5	(2) 2x4 (MIN)	(3) 3/4" BOLTS	5/8" @ 7" MIN. EMBED.	3/4" @ 6 1/2" MIN. EMBED.	5/8" @ 5 5/8" MIN. EMBED.	4,375
9	HD7B	(3) 2x4 OR (3) 2x6 (MIN)	(3) 3/4" BOLTS	7/8" @ 7" MIN. EMBED.	-	7/8" @ 7 7/8" MIN. EMBED.	6,245
10	HD9B	4x6 NOMINAL POST (MIN)	(3) 7/8" BOLTS	7/8" @ 7" MIN. EMBED.	-	7/8" @ 10 1/2" MIN. EMBED.	9,920
11	HD12	4x8 OR 6x6 NOMINAL POST (MIN)	(4) 1" BOLTS	1 1/8" @ 11" MIN. EMBED.	-	1 1/8" @ 12" MIN. EMBED.	15,435
12	HD19	4x8 OR 6x6 NOMINAL POST (MIN)	(5) 1" BOLTS	1 1/4" @ 11" MIN. EMBED.	-	1 1/4" @ 15" MIN. EMBED.	19,070
ATS-3	ATS-SR9	(2) 4x8 OR (2) 6x6 NOMINAL POST (MIN)	-	-	-	-	21,205
ATS-5	ATS-SR7H	(2) 6x6 NOMINAL POST (MIN)	-	-	-	-	27,060
ATS-6	ATS-SR9H	(2) 6x6 NOMINAL POST (MIN)	-	-	-	-	44,730

- NOTES:**
- ANCHOR TYPE REFERS TO SIMPSON STRONG-TIE HARDWARE SPECIFICATION.
  - USE MST50 STRAP FOR CLEAR SPANS < 16'. USE MST72 STRAP FOR ALL CLEAR SPANS > 16'. "CLEAR SPAN" IS DEFINED AS THE LENGTH OF UNFASTENED STRAP OCCURRING AT THE FLOOR CAVITY (SEE SIMPSON STRONG-TIE SPECIFICATION FOR ADDITIONAL INFORMATION).
  - REFER TO TYPICAL DETAILS ON SHEET S6-00C FOR ANCHORAGE TO ELEVATED CONCRETE SLAB.
  - WHERE VERTICAL COLUMN MEMBERS (EACH END OF SHEAR WALL) CONSIST OF BUILT-UP STUD PACKS, INDIVIDUAL STUDS SHALL BE LAMINATED TO ONE ANOTHER AS SHOWN IN DETAIL 3/S5-00A.
  - EXPANSION ANCHORS AND EPOXY ANCHORS SHALL BE PER THE RECOMMENDATIONS IN THE GENERAL NOTES. EXPANSION ANCHORS MAY ONLY BE PROVIDED AS AN ALTERNATE IF THE MINIMUM EDGE DISTANCE (CENTER OF ANCHOR TO EDGE OF CONCRETE) IS AT LEAST 0'-6".
  - HD19 REQUIRES A 3"x3" PLATE WASHER.
  - PROVIDE A STANDARD CUT WASHER UNDER THE ANCHOR NUT.

**8 SHEAR WALL ANCHORAGE SCHEDULE**  
S6-00A E+M TYPICAL DETAIL SCALE: 12" = 1'-0"



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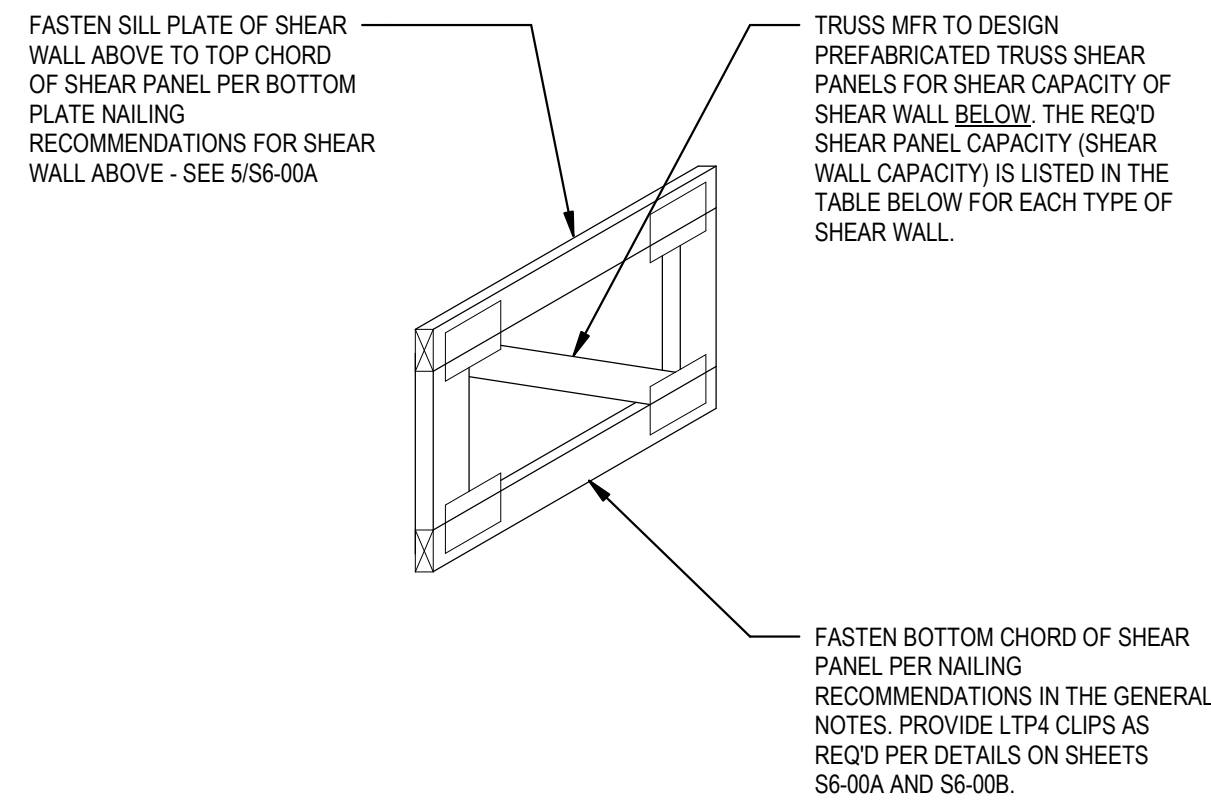
DRAWING TITLE WOOD SCHEDULES AND DETAILS - LATERAL

SHEET NUMBER

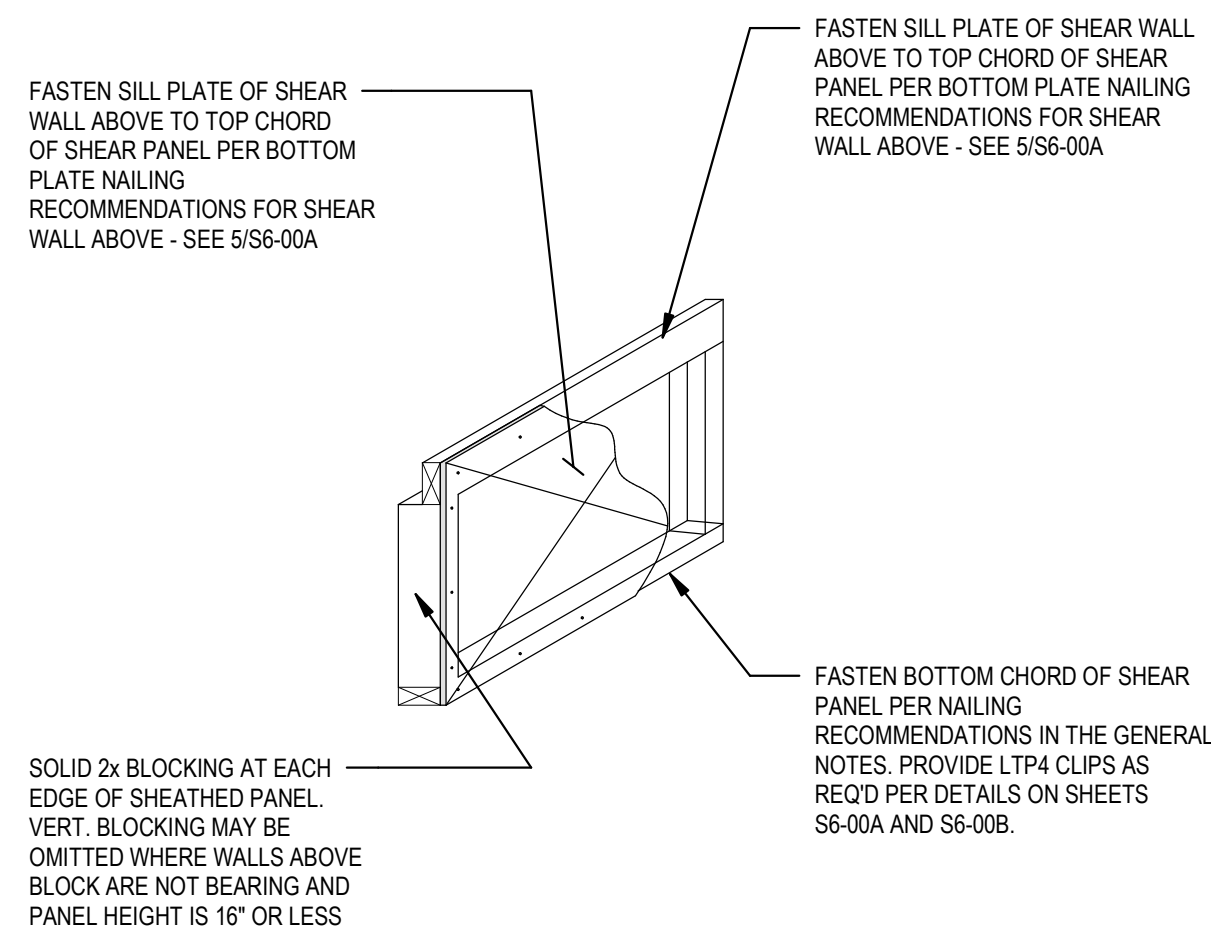
**S6-00A**

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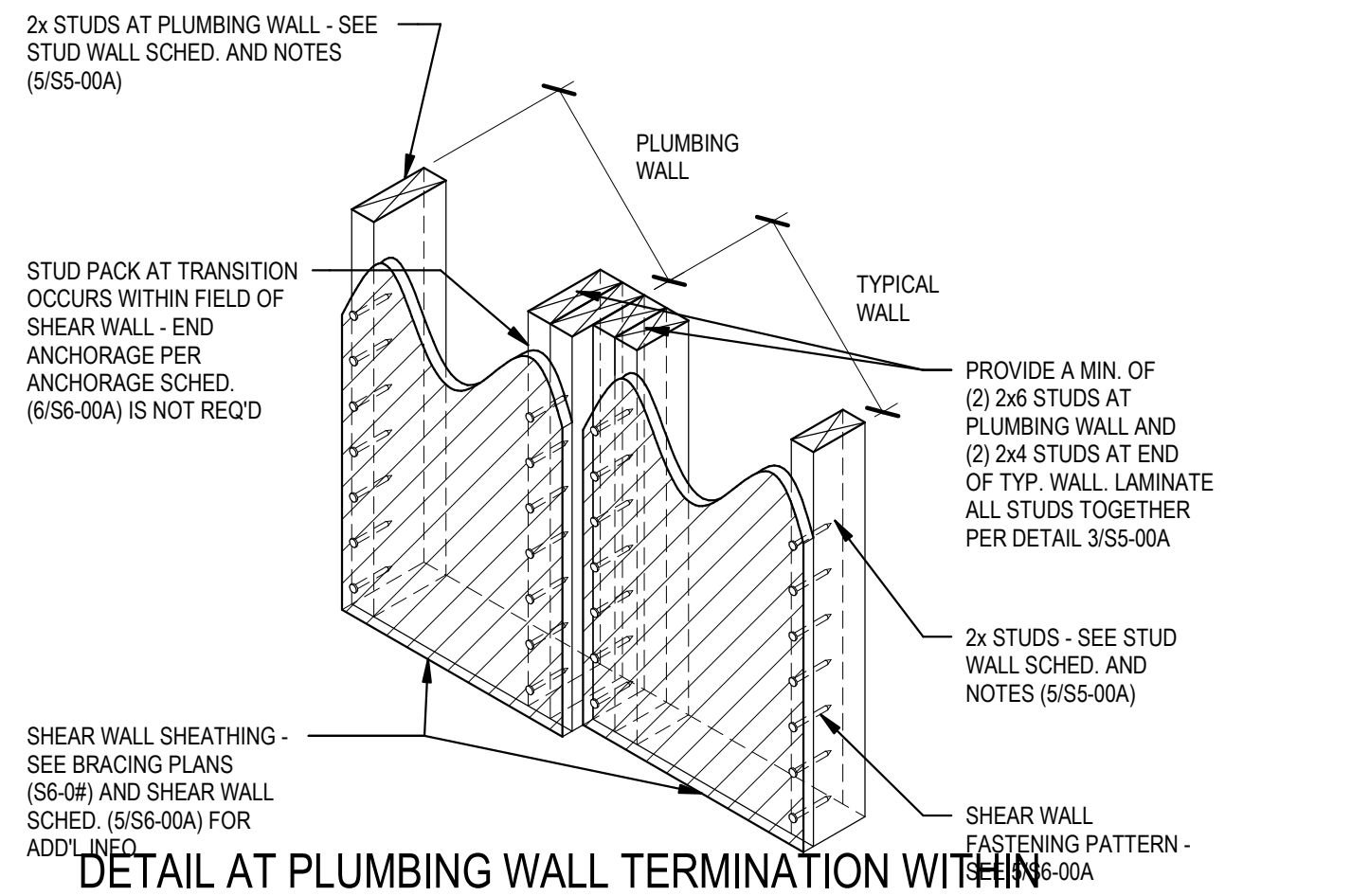
**PREFABRICATED TRUSS SHEAR BLOCKING OPTION #1**



**SHEATHED SHEAR PANEL BLOCKING OPTION #2**

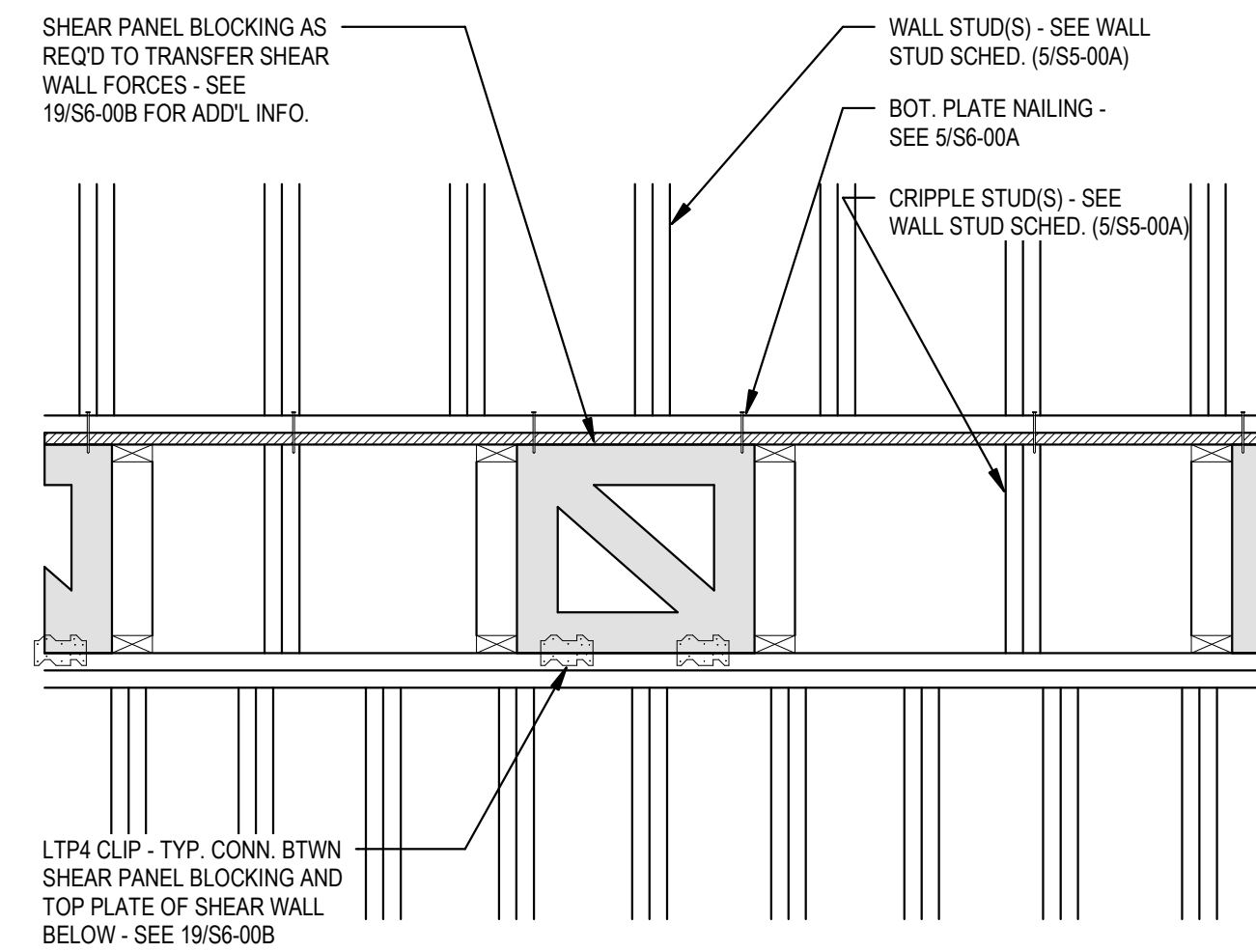
PREFABRICATED TRUSS SHEAR BLOCKING DESIGN FORCE AND SPACING				
SHEAR WALL TYPE	SHEAR WALL CAPACITY (LBS/FT)	PANEL SPACING	LTP4 CONN. PER PANEL (6), (7)	SHEAR PANEL DESIGN FORCE (LBS PER PANEL)
A	992	2'-0" O.C.	4	1984
B	773	2'-0" O.C.	3	1546
C	592	2'-0" O.C.	2	1184
D	399	3'-0" O.C.	2	1197
E	350	4'-0" O.C.	2	1400
F	290	4'-0" O.C.	2	1160
G	230	6'-0" O.C.	2	1380

- NOTES:**
- TOP CHORD OF SHEATHED SHEAR PANEL BLOCKING SHALL BE VERTICALLY ORIENTED WHEN INSTALLED UNDER LOAD BEARING WALLS. WHERE SHEAR PANELS ARE USED IN NON-LOAD-BEARING WALLS, THE TOP CHORD MAY BE VERTICALLY OR HORIZONTALLY ORIENTED.
  - TRUSS MANUFACTURER TO DESIGN PREFABRICATED TRUSS SHEAR PANELS FOR SHEAR CAPACITY OF SHEAR WALL BELOW. THE REQUIRED SHEAR PANEL CAPACITY (SHEAR WALL CAPACITY) IS LISTED IN THE TABLE ABOVE FOR EACH TYPE OF SHEAR WALL, ALONG WITH REQUIRED MAXIMUM SPACING OF SHEAR PANELS AND REQUIRED QUANTITY OF LTP4 CONNECTORS.
  - LATERAL FORCES MAY NOT NECESSITATE SHEAR PANELS IN EVERY TRUSS BAY. EMPTY TRUSS BAYS SHALL HAVE CRIPPLE STUDS PER STUD SCHEDULE ON S6-00A, OR SHALL HAVE A LADDER OR PONY TRUSS.
  - WHERE OPTION #2 IS USED, SHEAR BLOCKING SHALL BE SHEATHED WITH 15/32" PLYWOOD OR O.S.B. SHEATHING (SEE GENERAL NOTES FOR ADDITIONAL INFORMATION REGARDING SHEATHING FOR SHEAR WALLS). SHEATHING SHALL BE FASTENED TO PANEL EDGES WITH FASTENERS AND A FASTENER PATTERN THAT MEETS (OR EXCEEDS) THAT REQUIRED FOR THE SHEAR WALL BELOW. SEE SHEAR WALL SCHEDULE (5/S6-00A) FOR ADDITIONAL INFORMATION.
  - AVERAGE SPACING OF SHEAR PANELS TO EQUAL 3'-0" O.C. (MAX). THIS IS EQUIVALENT TO PROVIDING TWO (2) SHEAR PANELS IN A 3-BAY AREA (ONE OPEN BAY IN EVERY 3 TOTAL BAYS). SIMPSON A35 MAY BE SUBSTITUTED FOR LTP4 WHERE WIDTH OF SHEAR PANEL WILL ALLOW FOR PROPER CONNECTION OF A35 TO BE MADE.
  - LTP4 AND/OR A35 CONNECTORS MAY BE PROVIDED ON EACH SIDE OF THE SHEAR BLOCKING PANEL. DEPENDING UPON THE LENGTH OF THE SHEAR PANEL, THIS MAY BE NECESSARY. THE STATED SHEAR CAPACITY OF THESE WALLS ASSUMES GYPSUM SHEATHING ON TWO (2) SIDES OF THE WALL.



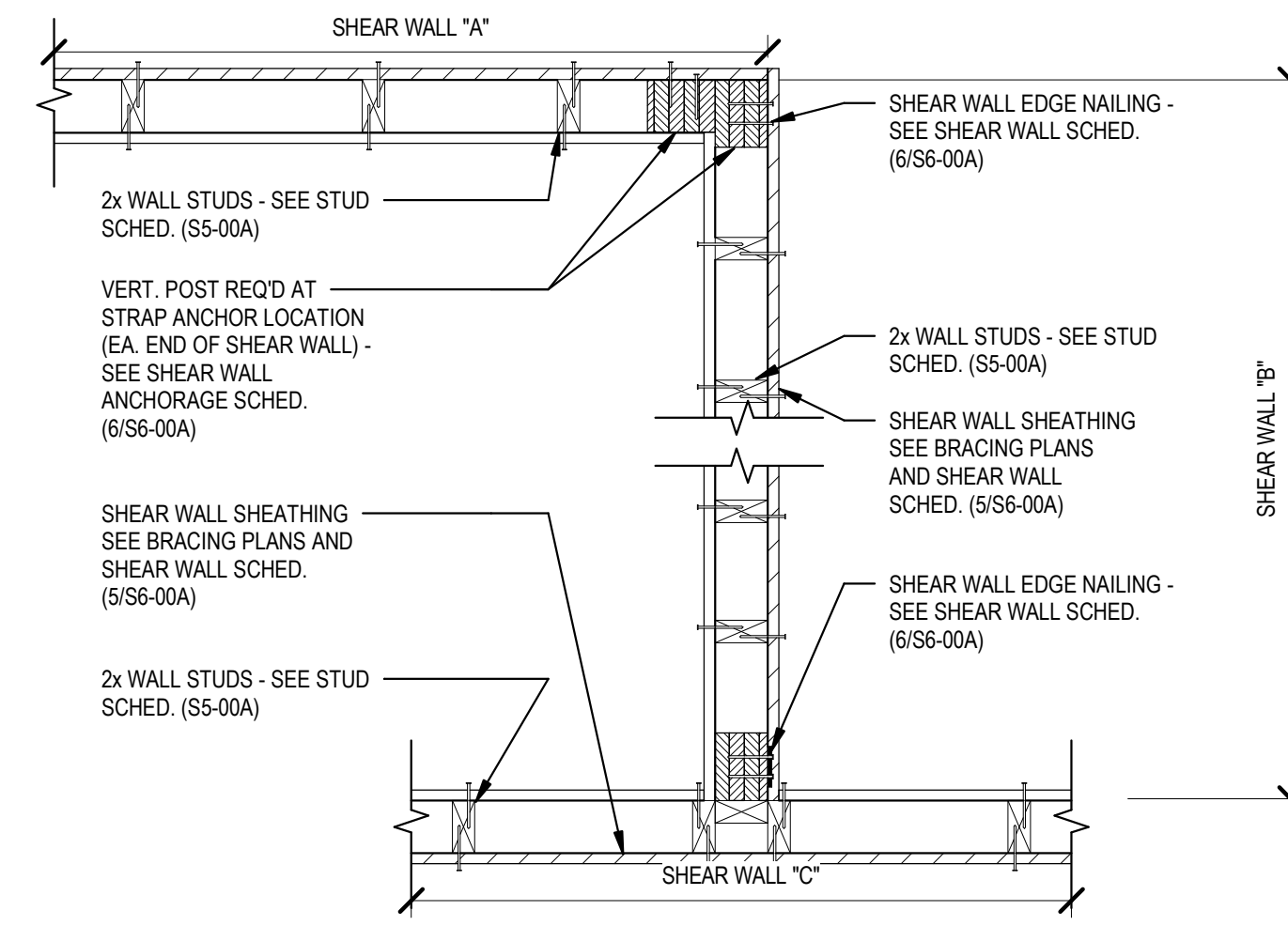
**9 SHEARWALL DETAIL AT PLUMBING WALL TERMINATION WITHIN SHEARWALL**

S6-00B E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



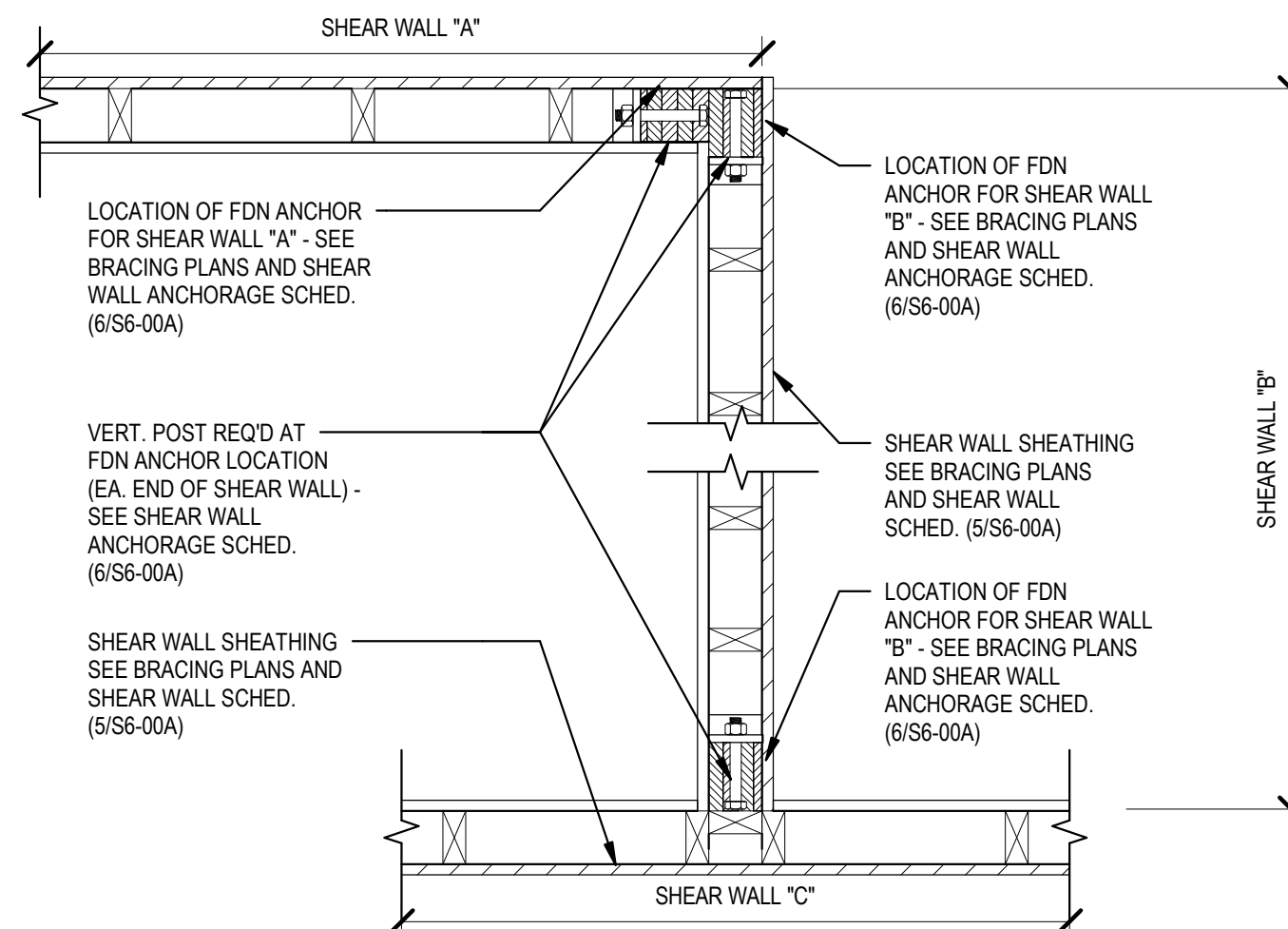
**5 SHEAR PANEL DETAILING AT INTERIOR BEARING WALL**

S6-00B E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



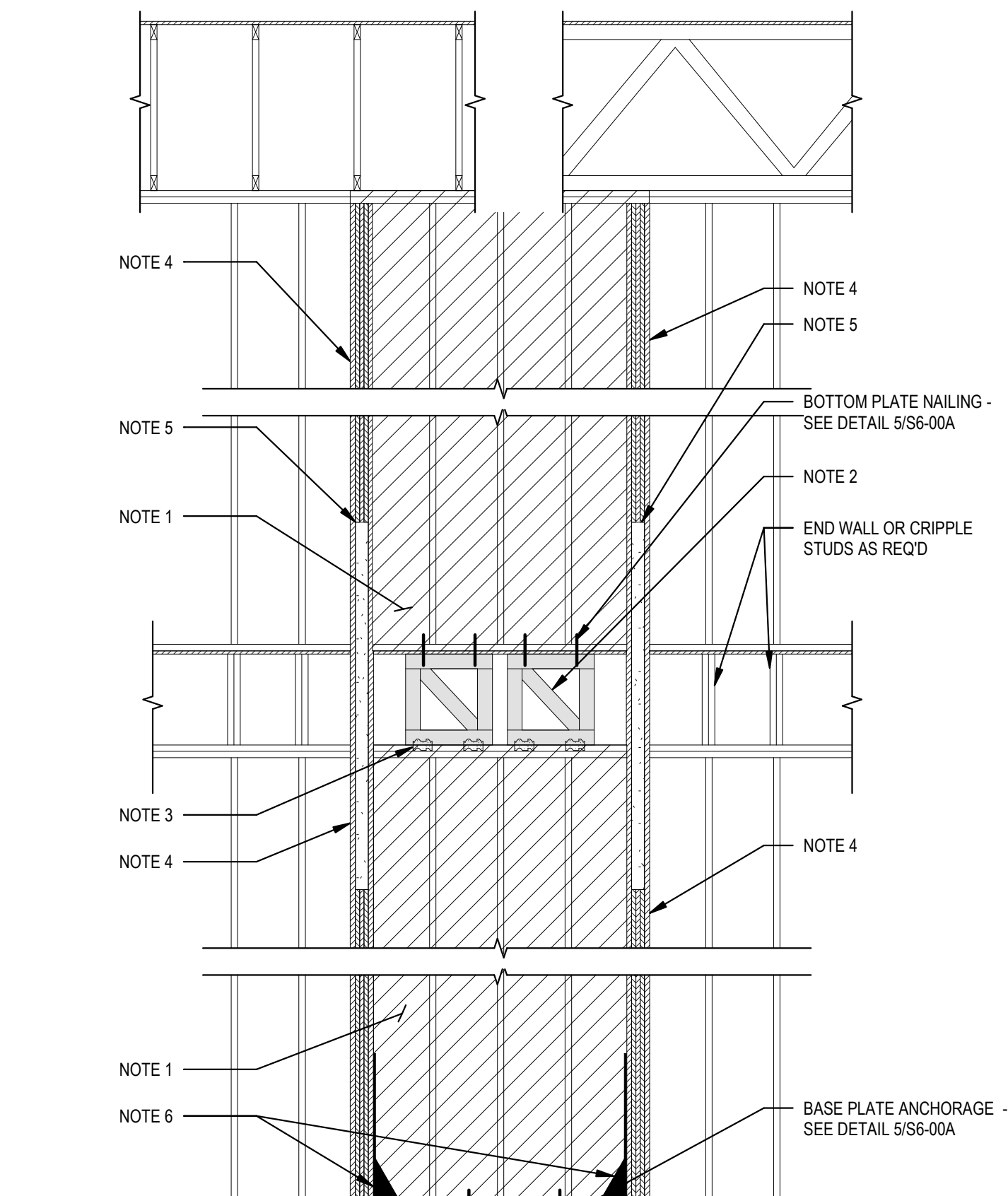
**1 FRAMING & ANCHOR. REQ. AT SHEARWALL INTERSECTION**

S6-00B E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



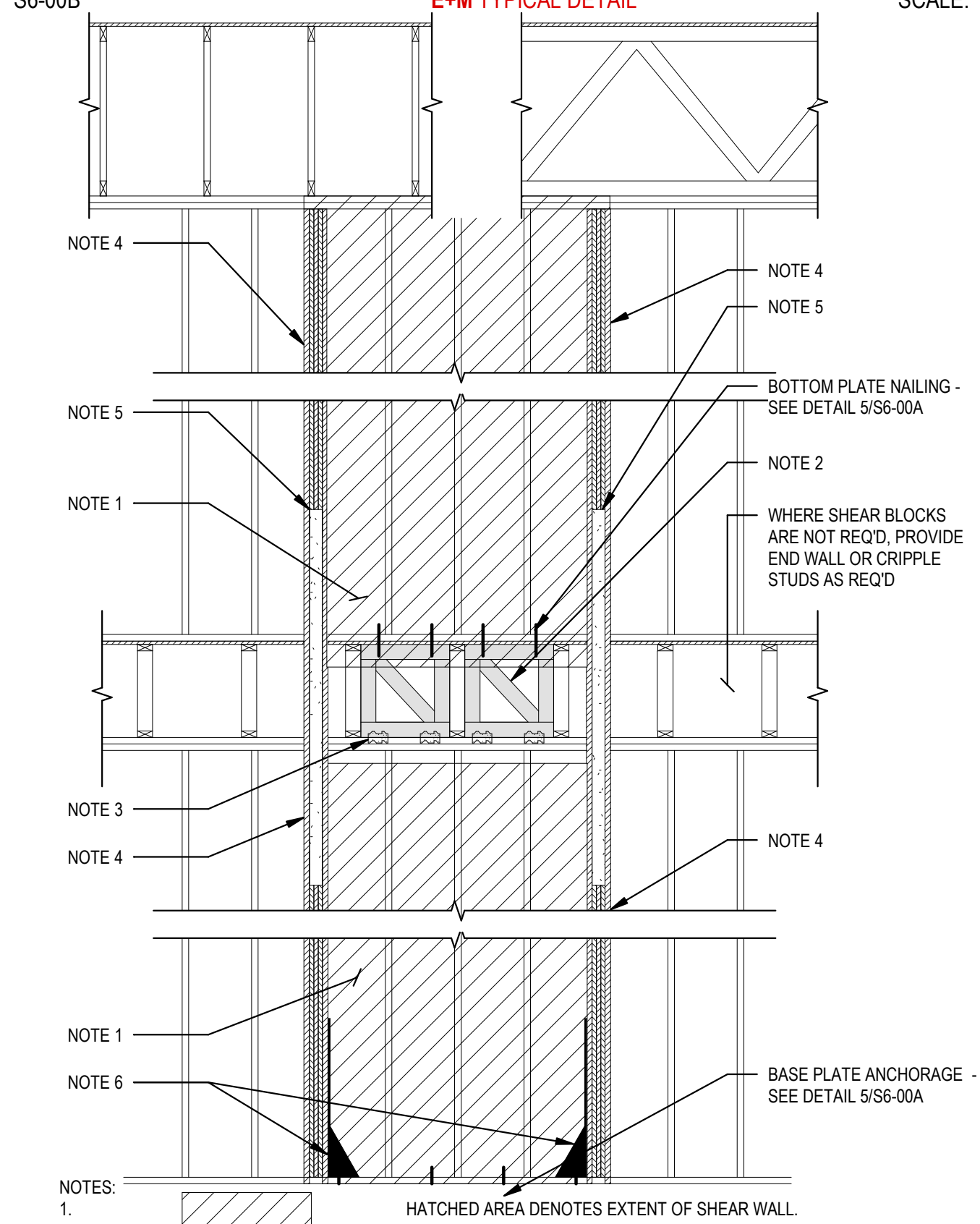
**6 FRAMING & ANCHOR. REQ. AT SHEARWALL INTER. (FNDN)**

S6-00B E+M TYPICAL DETAIL SCALE: 1" = 1'-0"



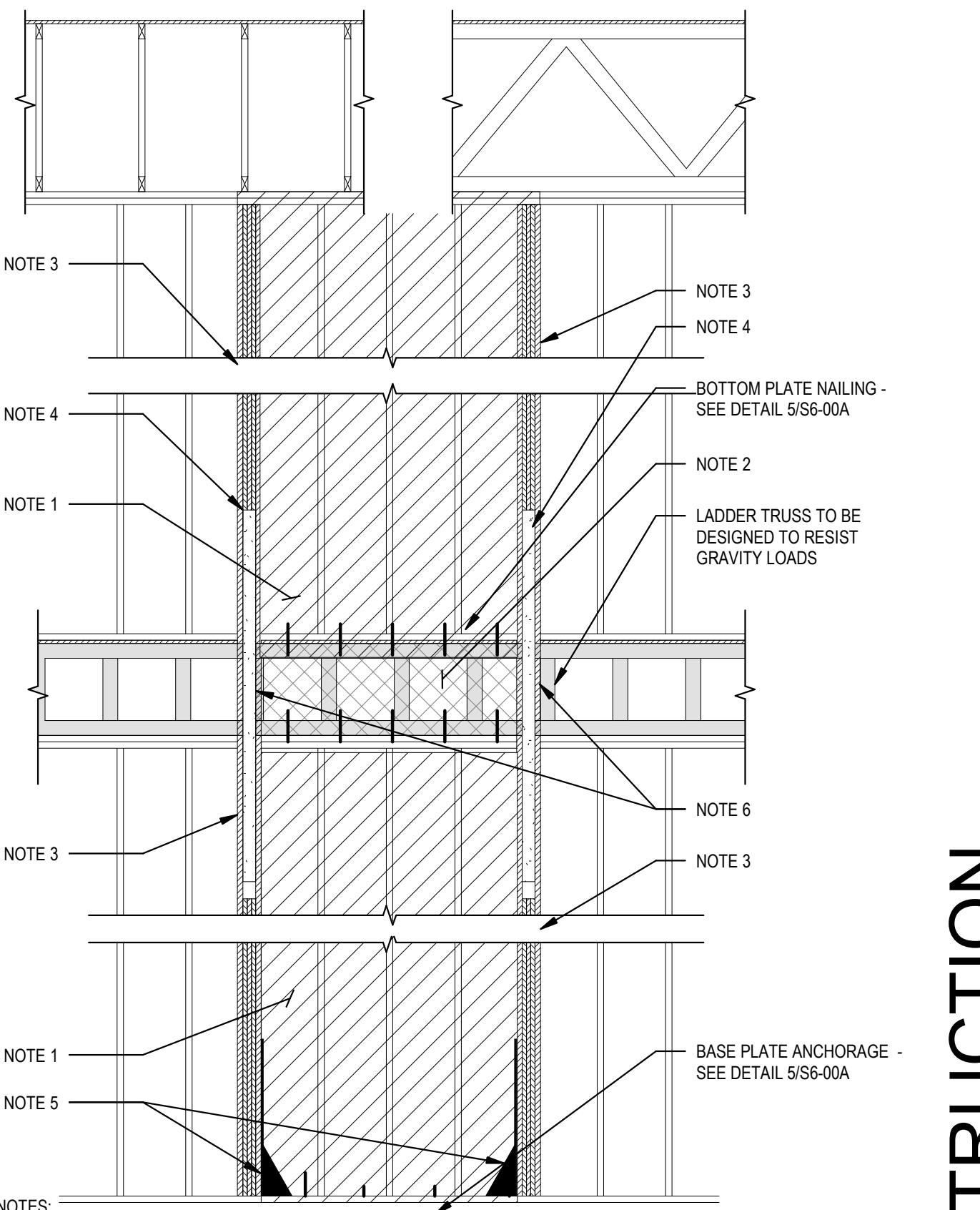
**12 WALL SHEAR WALL ELEVATION - INTERIOR NON-UNIT-BEARING**

S6-00B E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"



**8 SHEAR WALL ELEVATION - INTERIOR BEARING WALL**

S6-00B E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"

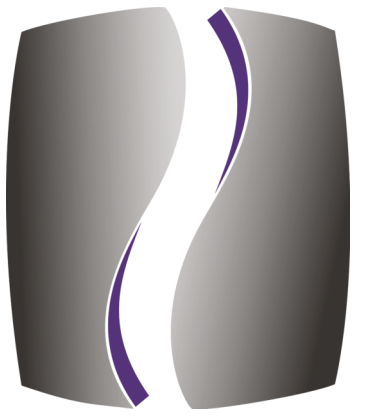


**4 CONDITION SHEAR WALL ELEVATION - EXTERIOR NON-UNIT-BEARING**

S6-00B E+M TYPICAL DETAIL SCALE: 3/8" = 1'-0"

**16 SHEAR BLOCK PANELS**

S6-00B E+M TYPICAL DETAIL SCALE: 3/4" = 1'-0"



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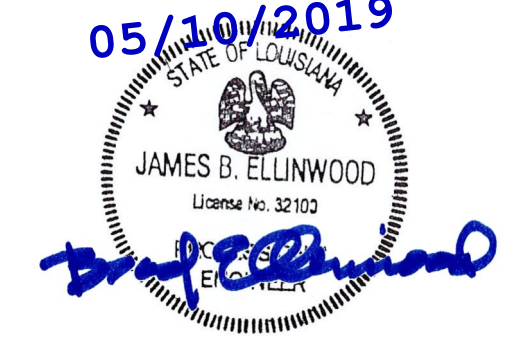


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**FREMAUX PARK APARTMENTS**

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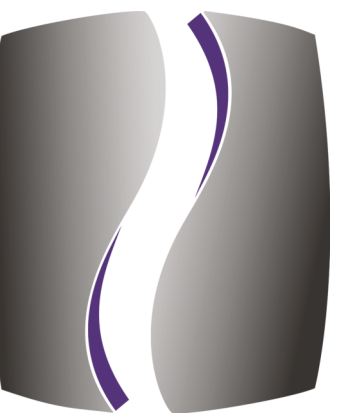


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ISSUE DATE \_\_\_\_\_ 1867709  
DRAWN BY/CHECKED BY \_\_\_\_\_ 05/10/2019  
APPROVED BY \_\_\_\_\_ DKJ / RAFAEL, RIM  
DRAWING TITLE \_\_\_\_\_ BRAD, JBE  
WOOD SCHEDULES AND DETAILS - LATERAL  
SHEET NUMBER \_\_\_\_\_

**S6-00B**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328  
PHILLIPSPART.COM 770-394-1616

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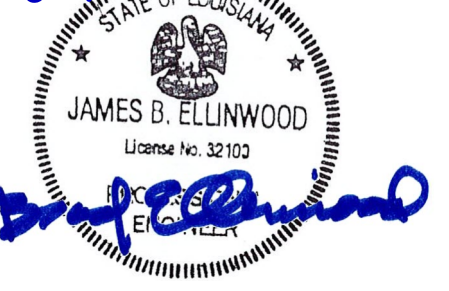


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ISSUE DATE 05/10/2019

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WOOD SCHEDULES AND DETAILS - LATERAL

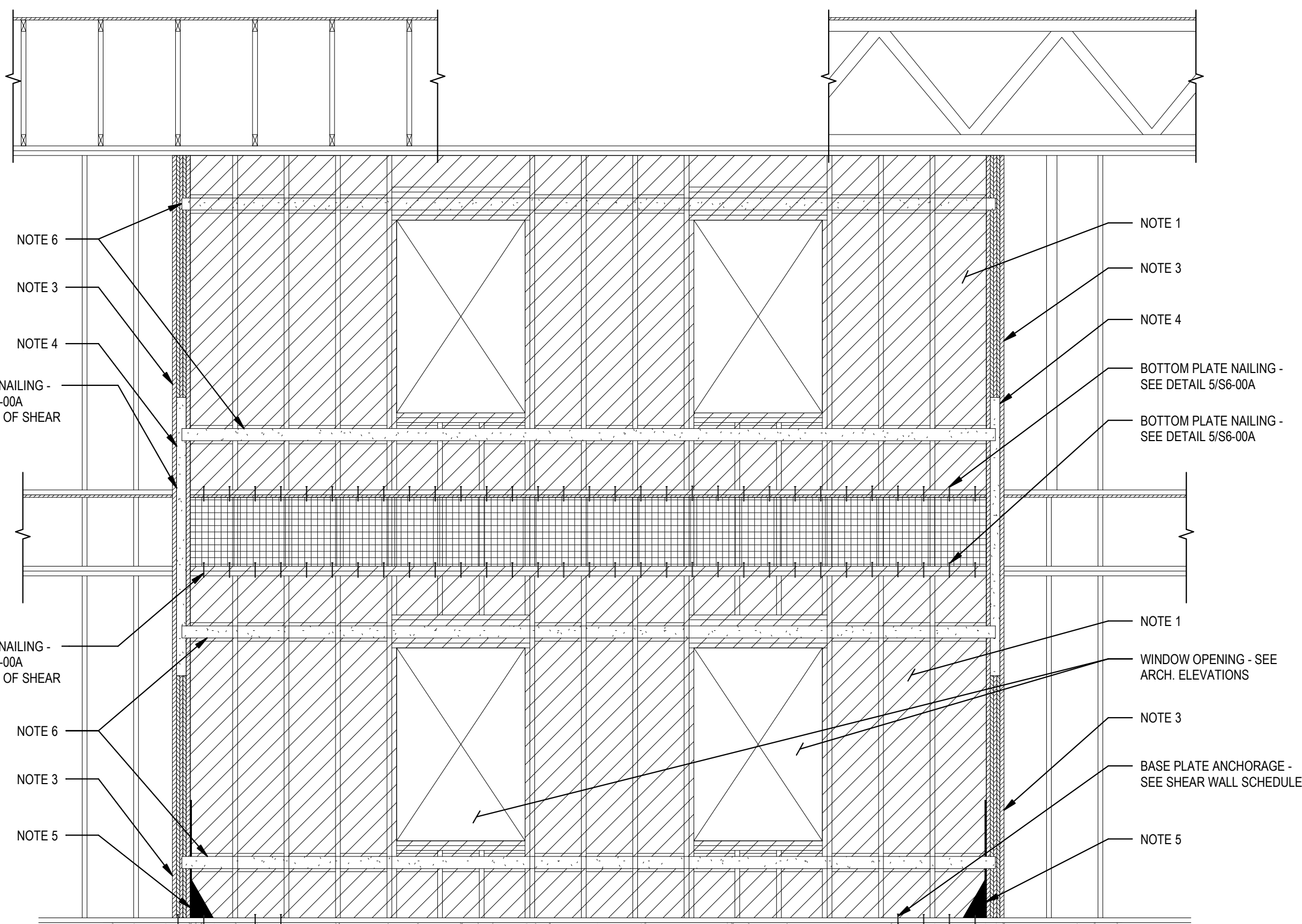
SHEET NUMBER

S6-00C

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BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

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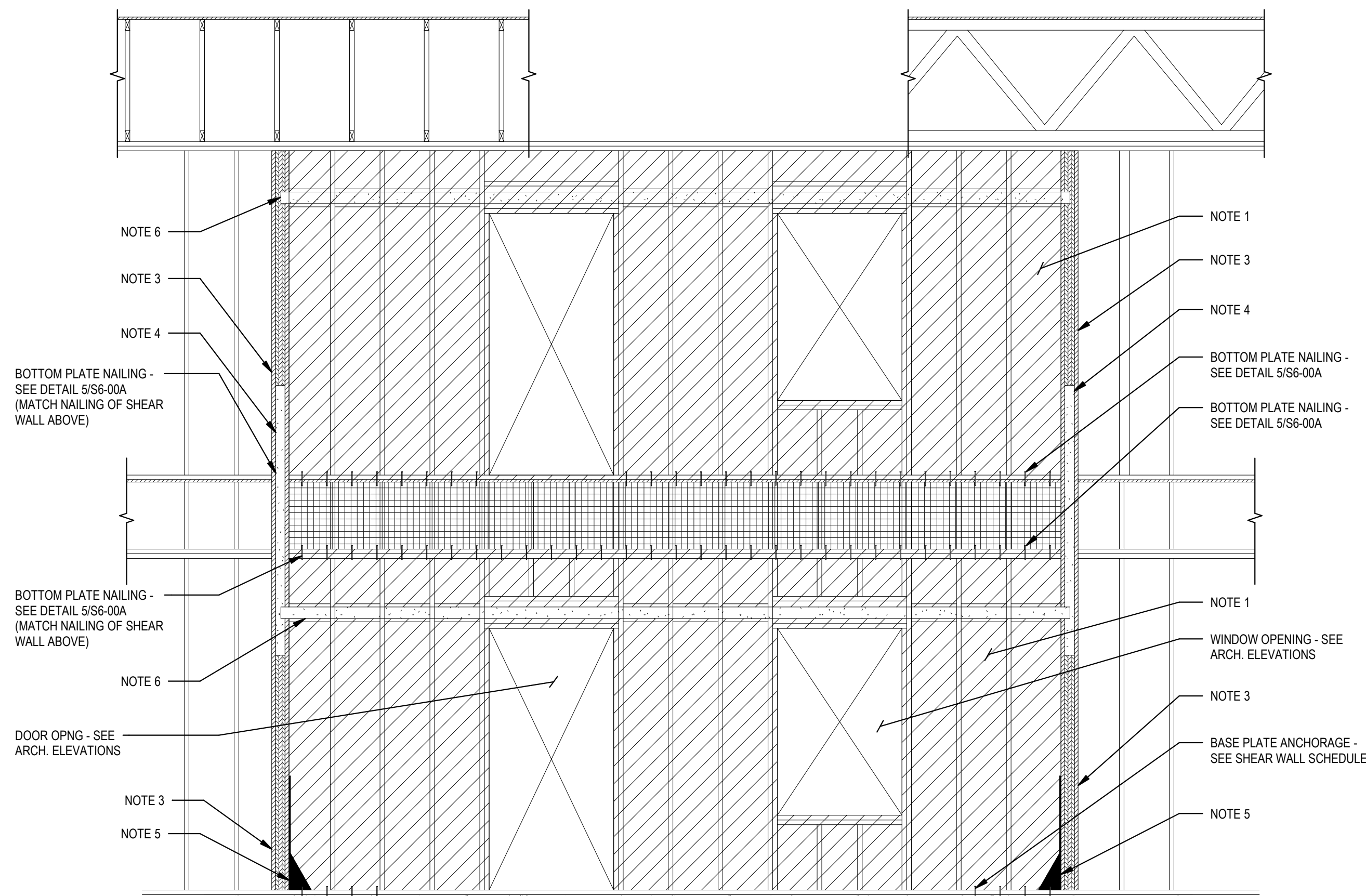
- NOTES:
- HATCHED AREA DENOTES EXTENT OF SHEAR WALL.
  - PROVIDE CONTINUOUS SHEATHING THRU FLOOR CAVITY. SHEATHING TYPE, SHEATHING THICKNESS, AND NAILING PATTERN(S) SHALL MATCH THAT REQUIRED FOR THE SHEAR WALL AT THE FLOOR BELOW, TYP.
  - SHEAR PANEL BLOCKING MAY BE PROVIDED IN LIEU OF CONTINUOUS SHEATHING TO TRANSFER SHEAR WALL FORCES - SEE DETAIL 16/S6-00B FOR ADDITIONAL INFORMATION AND SCHEDULE OF REQUIRED SHEAR PANEL CAPACITIES. SPACING/QUANTITY OF SHEAR PANELS REQUIRED SHALL BE PER SCHEDULE IN DETAIL 16/S6-00B.
  - VERTICAL POST REQUIRED AT EACH END OF SHEAR WALL PER SHEAR WALL ANCHORAGE SCHEDULE - SEE DETAIL 6/S6-00A. THE SCHEDULED POST SHALL BE PROVIDED IN ADDITIONAL TO THE VERTICAL WALL STUDS REQUIRED WITHIN THE SHEAR WALL AND SHALL BE CONTINUOUS FROM FLOOR TO FLOOR (CRIPPLES WITHIN THE FLOOR CAVITY TO MATCH SCHEDULED SIZE OF POST, TYP.).
  - FLOOR-TO-FLOOR STRAPPING AT EACH END OF SHEAR WALL - SEE DETAIL 6/S6-00A.
  - FOUNDATION ANCHOR AT EACH END OF SHEAR WALL - SEE DETAIL 6/S6-00A.
  - CMST14 COILED STRAP TO SPAN FULL WIDTH OF WALL (ANCHOR TO ANCHOR). PROVIDE STRAP AT ALL LEVELS CONTAINING OPENING(S) IN SHEAR WALL. WHERE DOUBLE SHEATHING IS REQUIRED, PROVIDE STRAP ON BOTH SIDES OF WALL, TYP. PROVIDE SOLID BLOCKING BEHIND STRAP (3" MIN. WIDTH), TYP. PROVIDE MAXIMUM NAILING RECOMMENDED BY MANUFACTURER.

15 EXTERIOR SHEAR WALL WITH WINDOW OPENINGS ONLY

S6-00C

E+M TYPICAL DETAIL

SCALE: 3/8" = 1'-0"



- NOTES:
- HATCHED AREA DENOTES EXTENT OF SHEAR WALL.
  - PROVIDE CONTINUOUS SHEATHING THRU FLOOR CAVITY. SHEATHING TYPE, SHEATHING THICKNESS, AND NAILING PATTERN(S) SHALL MATCH THAT REQUIRED FOR THE SHEAR WALL AT THE FLOOR BELOW, TYP.
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  - FLOOR-TO-FLOOR STRAPPING AT EACH END OF SHEAR WALL - SEE DETAIL 6/S6-00A.
  - FOUNDATION ANCHOR AT EACH END OF SHEAR WALL - SEE DETAIL 6/S6-00A.
  - CMST14 COILED STRAP TO SPAN FULL WIDTH OF WALL (ANCHOR TO ANCHOR). PROVIDE STRAP AT ALL LEVELS CONTAINING OPENING(S) IN SHEAR WALL. WHERE DOUBLE SHEATHING IS REQUIRED, PROVIDE STRAP ON BOTH SIDES OF WALL, TYP. PROVIDE SOLID BLOCKING BEHIND STRAP (3" MIN. WIDTH), TYP. PROVIDE MAXIMUM NAILING RECOMMENDED BY MANUFACTURER.

7 EXTERIOR SHEAR WALL WITH WINDOW AND DOOR OPENINGS

S6-00C

E+M TYPICAL DETAIL

SCALE: 3/8" = 1'-0"





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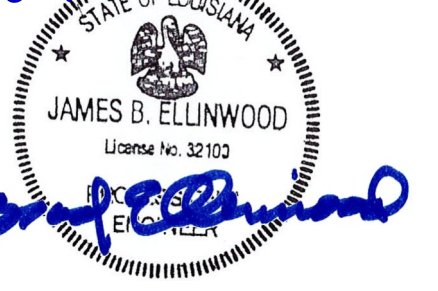


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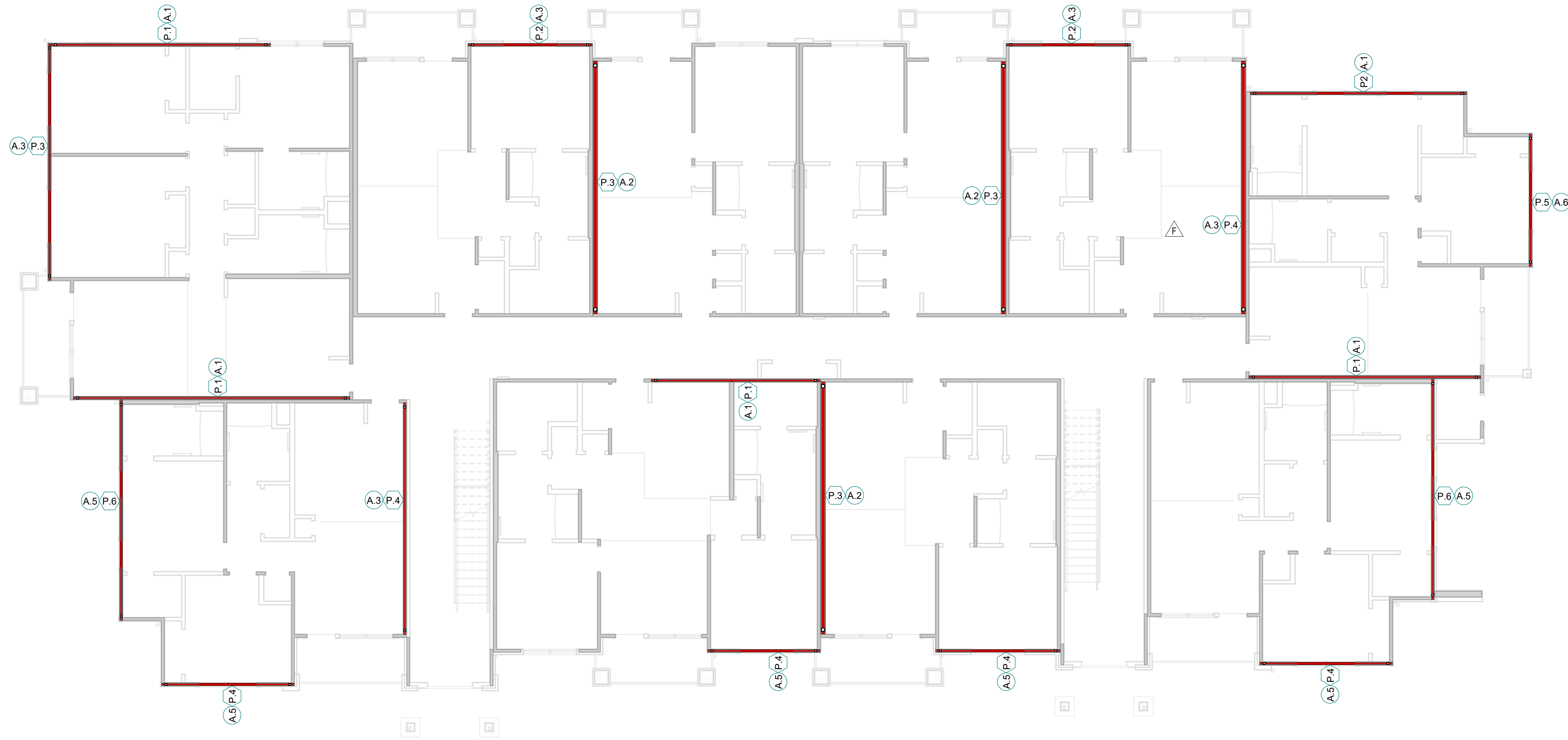
**BRACING PLAN – BUILDING A**

SHEET NUMBER

**S6-01**

5901 PEACHTREE DUNWOODY RD.  
BUILDING A, SUITE 450  
ATLANTA, GEORGIA 30328

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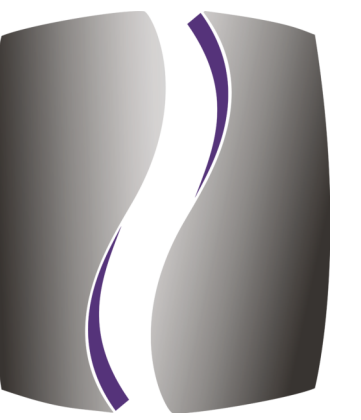
**SHEARWALL PLAN NOTES:**

- ALL SHEARWALL MATERIALS SHALL COMPLY WITH NOTES AND SCHEDULES PER SHEET S0-03 AND S6-00A.
  - FOR SHEARWALL AND ANCHORAGE TYPES, REFERENCE SHEET S6-00A.
  - DENOTES SHEAR WALL EXTENTS.
  - DENOTES SHEAR WALL TYPE WITH PLYWOOD ON REFERENCED FACE OF WALL. SEE SCHEDULE ON S6-00A FOR SHEATHING DESIGNATIONS
- P.#** DENOTES ANCHOR DESIGNATION. SEE SCHEDULE ON S6-00A FOR REQUIRED DESIGN CAPACITIES AND TYPICAL NOTES.

	SHEAR WALL MARK					
LEVEL	P.1	P.2	P.3	P.4	P.5	P.6
ROOF	D	D	D	D	D	D
ROOF+1	D	D	D	C	C	B
ROOF+2	D	C	B	B	A	A

	ANCHORAGE MARK					
LEVEL	A.1	A.2	A.3	A.4	A.5	A.6
ROOF	1	2	2	2	2	2
ROOF+1	2	2	3	4	4	4
ROOF+2	9	10	11	11	12	ATS-5

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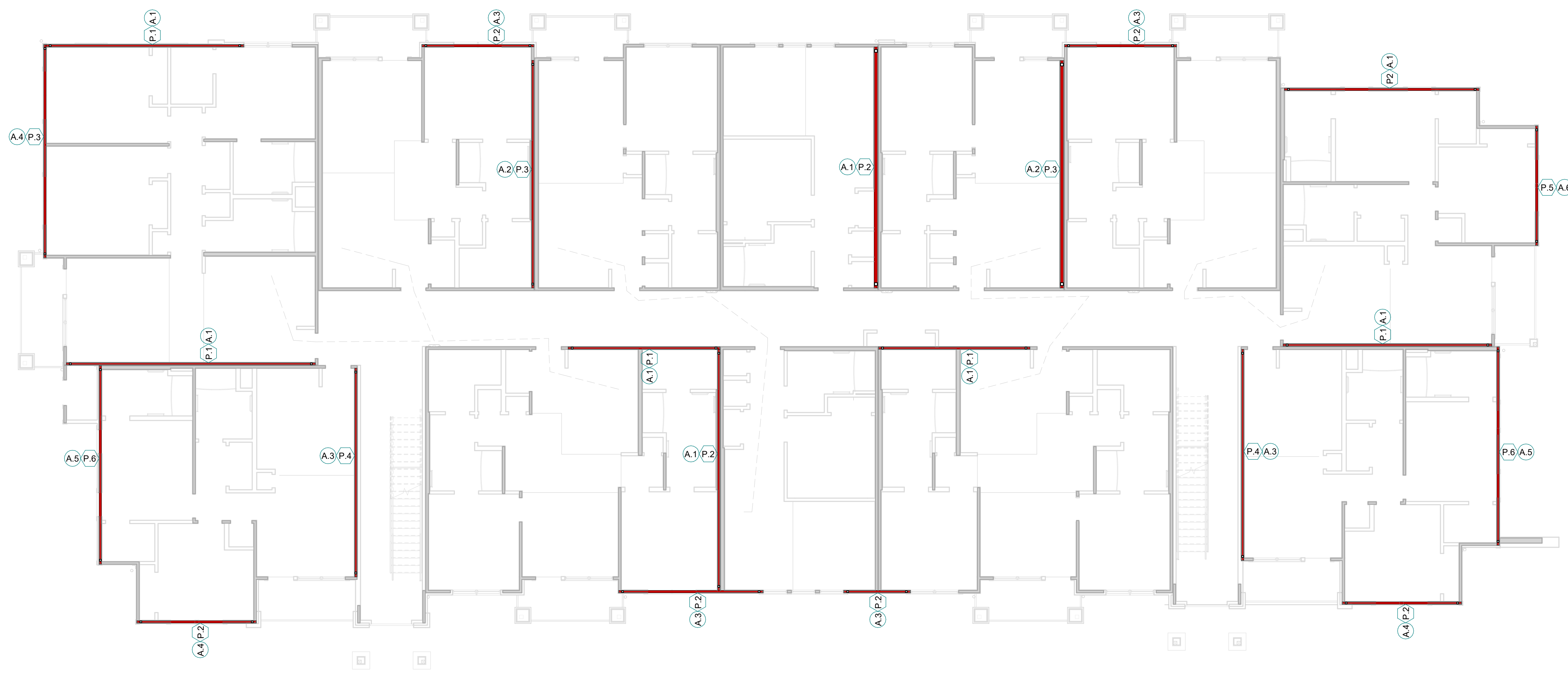
DRAWING TITLE

**BRACING PLAN – BUILDING B**

SHEET NUMBER

**S6-02**

5901 PEACHTREE DUNWOODY RD.  
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**SHEARWALL PLAN NOTES:**




- ALL SHEARWALL MATERIALS SHALL COMPLY WITH NOTES AND SCHEDULES PER SHEET S0-03 AND S6-00A.
  - FOR SHEARWALL AND ANCHORAGE TYPES, REFERENCE SHEET S6-00A.
  - DENOTES SHEAR WALL EXTENTS.
  - DENOTES SHEAR WALL TYPE WITH PLYWOOD ON REFERENCED FACE OF WALL. SEE SCHEDULE ON S6-00A FOR SHEATHING DESIGNATIONS
- P.#** DENOTES ANCHOR DESIGNATION. SEE SCHEDULE ON S6-00A FOR REQUIRED DESIGN CAPACITIES AND TYPICAL NOTES.

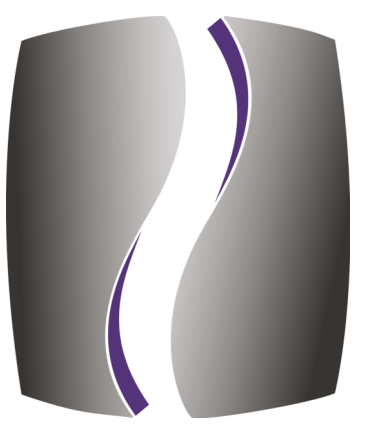
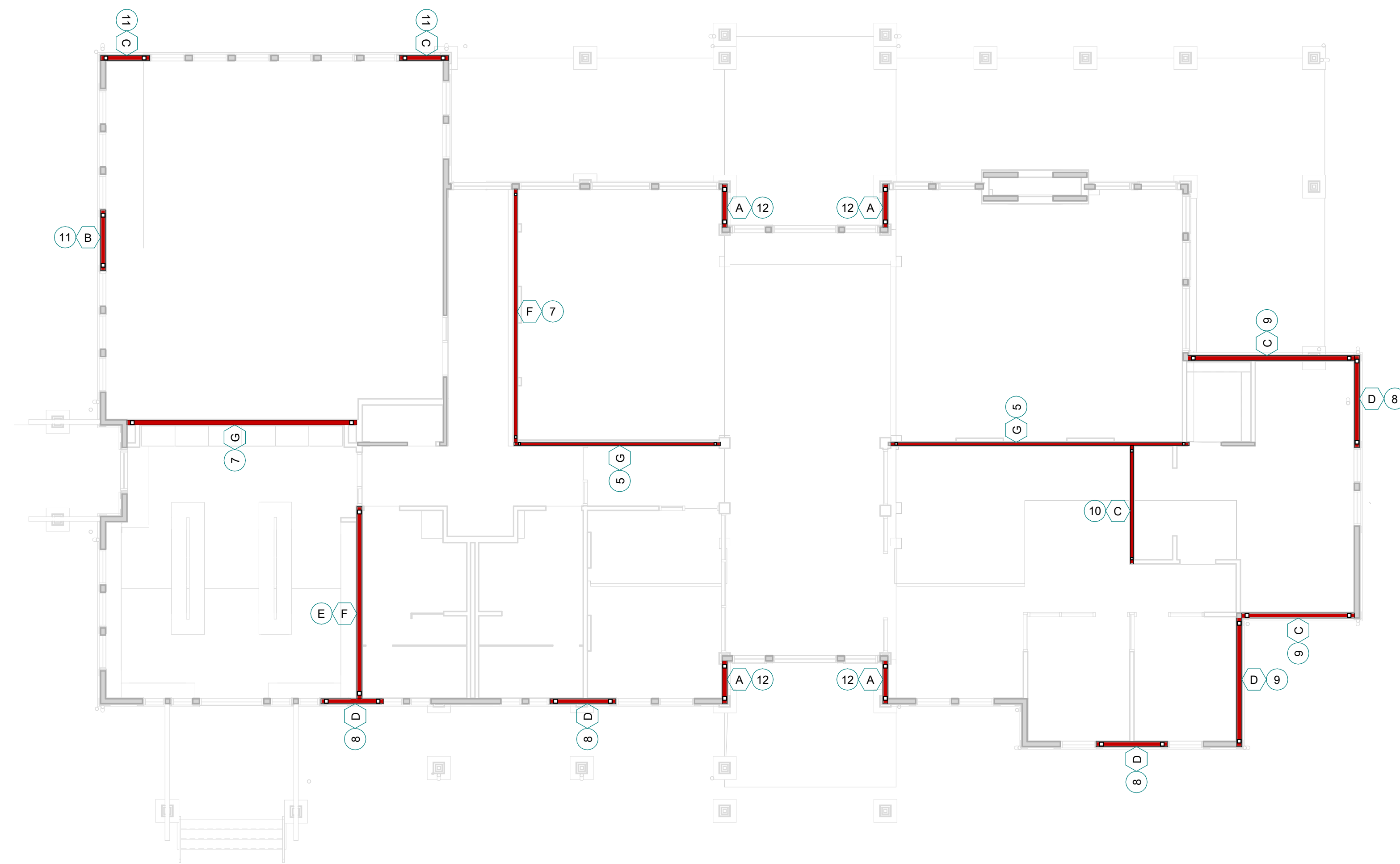
LEVEL	SHEAR WALL MARK					
	P.1	P.2	P.3	P.4	P.5	P.6
ROOF	D	D	D	D	D	D
ROOF+1	D	D	D	C	C	B
ROOF+2	D	C	B	B	A	A

LEVEL	ANCHORAGE MARK					
	A.1	A.2	A.3	A.4	A.5	A.6
ROOF	1	2	2	2	2	2
ROOF+1	2	2	3	4	4	4
ROOF+2	9	10	11	11	12	ATS-5

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**SINGLE STORY BUILDING SHEAR WALL PLAN NOTES:**

1. ALL SHEARWALL MATERIALS SHALL COMPLY WITH NOTES AND SCHEDULES PER SHEET S0-03 AND S6-00A.
2. FOR SHEARWALL AND ANCHORAGE TYPES, REFERENCE SHEET S6-00A.
4.  DENOTES SHEAR WALL EXTENTS.
5.  DENOTES SHEAR WALL TYPE ON REFERENCED FACE OF WALL. SEE SCHEDULE ON S6-00A FOR SHEATHING DESIGNATIONS.
-  DENOTES ANCHOR DESIGNATION. SEE SCHEDULE ON S6-00A FOR REQUIRED DESIGN CAPACITIES AND TYPICAL NOTES.



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**BRACING PLAN - CLUBHOUSE**

SHEET NUMBER

**S6-03**

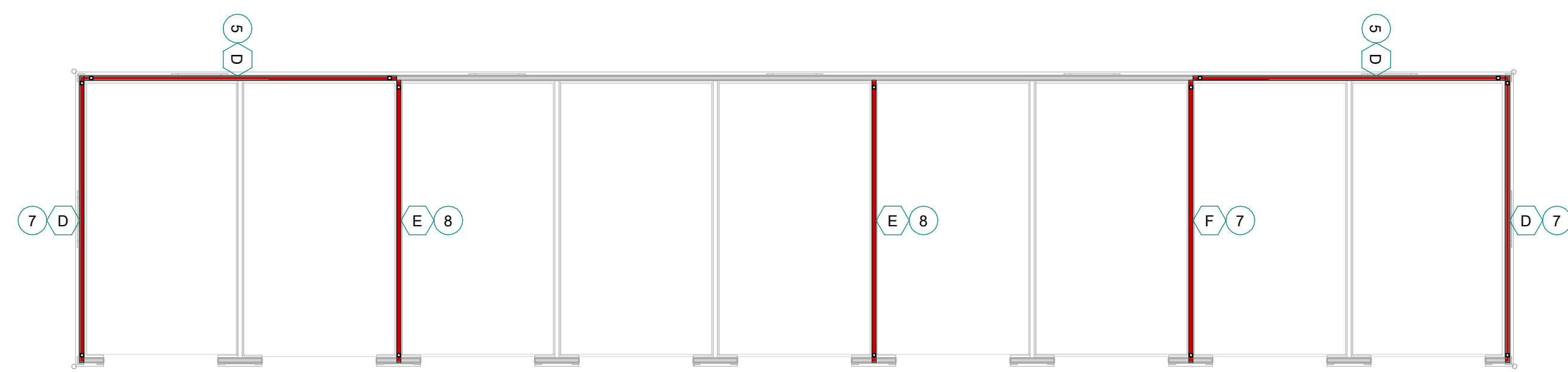
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**SINGLE STORY BUILDING SHEAR WALL PLAN NOTES:**

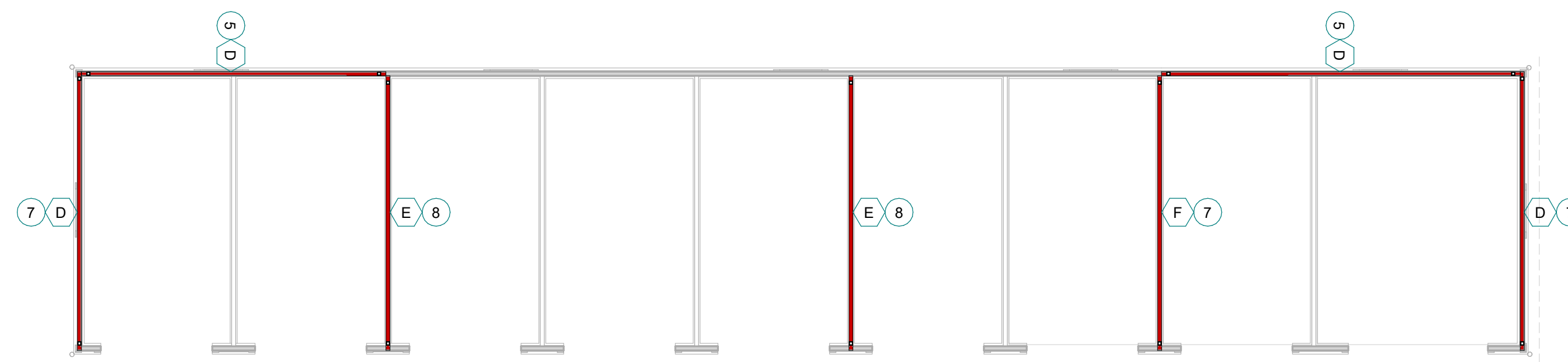
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4.  DENOTES SHEAR WALL EXTENTS.
5. X DENOTES SHEAR WALL TYPE ON REFERENCED FACE OF WALL. SEE SCHEDULE ON S6-00A FOR SHEATHING DESIGNATIONS
6. # DENOTES ANCHOR DESIGNATION. SEE SCHEDULE ON S6-00A FOR REQUIRED DESIGN CAPACITIES AND TYPICAL NOTES.



**1 BRACING PLAN - GARAGE BUILDING 1**

S6-04

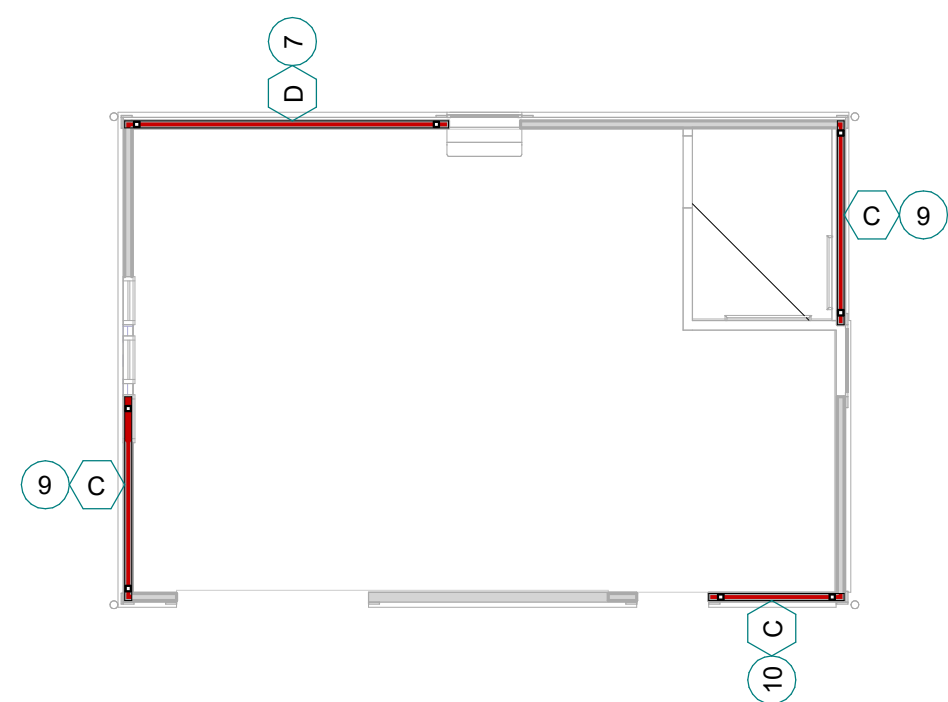
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**2 BRACING PLAN - GARAGE BUILDING 2**

S6-04

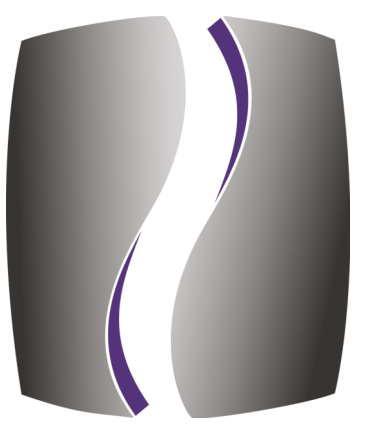
SCALE: 1/8" = 1'-0"



**3 BRACING PLAN - MAINTENANCE BUILDING**

S6-04

SCALE: 1/8" = 1'-0"



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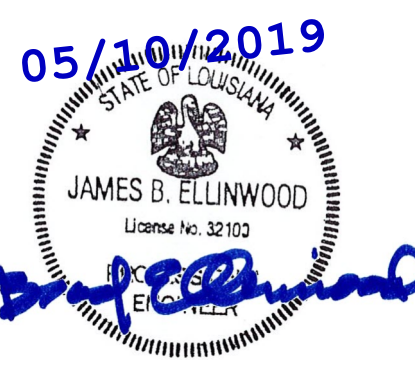


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**BRACING PLANS - ANCILLARY BUILDINGS**

SHEET NUMBER

**S6-04**

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