



DAMMON
ENGINEERING, INC.
Architects & Engineers
554 Old Spanish Trail Slidell, LA 70458 985-649-5832

SHOP DRAWING and SAMPLE TRANSMITTAL

DATE: 12/21/2015

TO: Steve Hart
 C.G. Favret Co., Inc.
 4524 Shores Dr., Suite 25
 Metairie, LA 70006

FROM: David Dammon

TRANSMITTAL No.: 13125.01.01

REFERENCE: **TEXTRON Stone Road Facility**
New Office Building
 DE Project: 2203

WE TRANSMIT:

- enclosed under separate cover _____

FOR YOUR:

- use record approval
 review and comment information drafting

THE FOLLOWING:

- drawing(s) contracts specifications
 shop drawings samples change order(s)
 product information warranty substitution request

# COPIES	DESCRIPTION	ACTION
4	Anchor Bolt Drawings	B

ACTION CODES:

- | | | | |
|----|---------------------------|----|---|
| A. | Reviewed/No Exceptions | D. | No Action Required |
| B. | Reviewed/Exceptions Noted | E. | For Signature and Return to this Office |
| C. | Revise and Resubmit | F. | See Remarks Below |
| G. | Rejected | | |

REMARKS:

See attached page F-1 for comments

COPIES TO: File

SHOP DRAWING / SUBMITTAL REVIEW

REVIEWED REVIEWED AS NOTED
 REVISE AND RESUBMIT REJECTED

Project No.: 2203 Submittal No.: 13125.01.01

Corrections or comments made on the shop drawings during this review do not relieve the contractor from compliance with requirements of the drawings and specifications. This check is only for review of the general conformance with the design concept of the project and general compliance with the information given in the contract documents. This contractor is responsible for: confirming and correlating all quantities and dimensions; selecting fabrication processes and techniques of construction; coordinating his or her work with that and other trades and performing all in a safe and satisfactory manner.

By: David Dammon Date: 12/21/2015
DAMMON ENGINEERING, INC.
 Slidell, LA

Revision	Date	Description
1	12/11/15	REDRAWN THRU SALES ORDER REVISION 6.

STAR BUILDING SYSTEMS
 ANNECUMANN
 P.O. BOX 94810 OKLAHOMA CITY, OK 73143
 (405) 636-2000

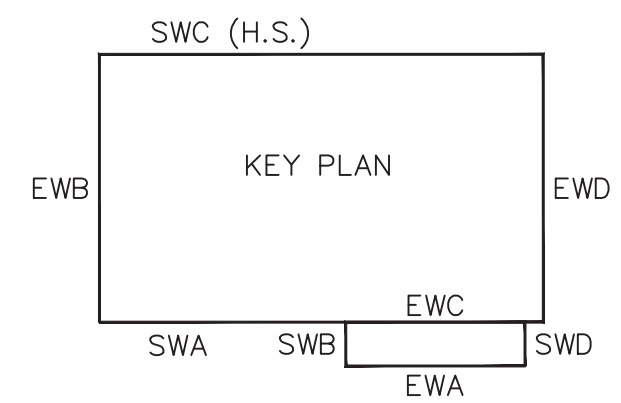
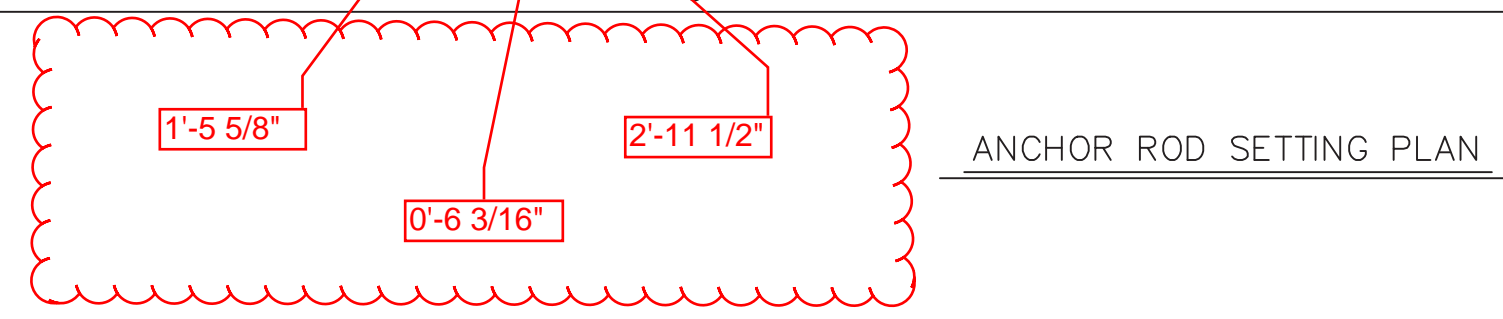
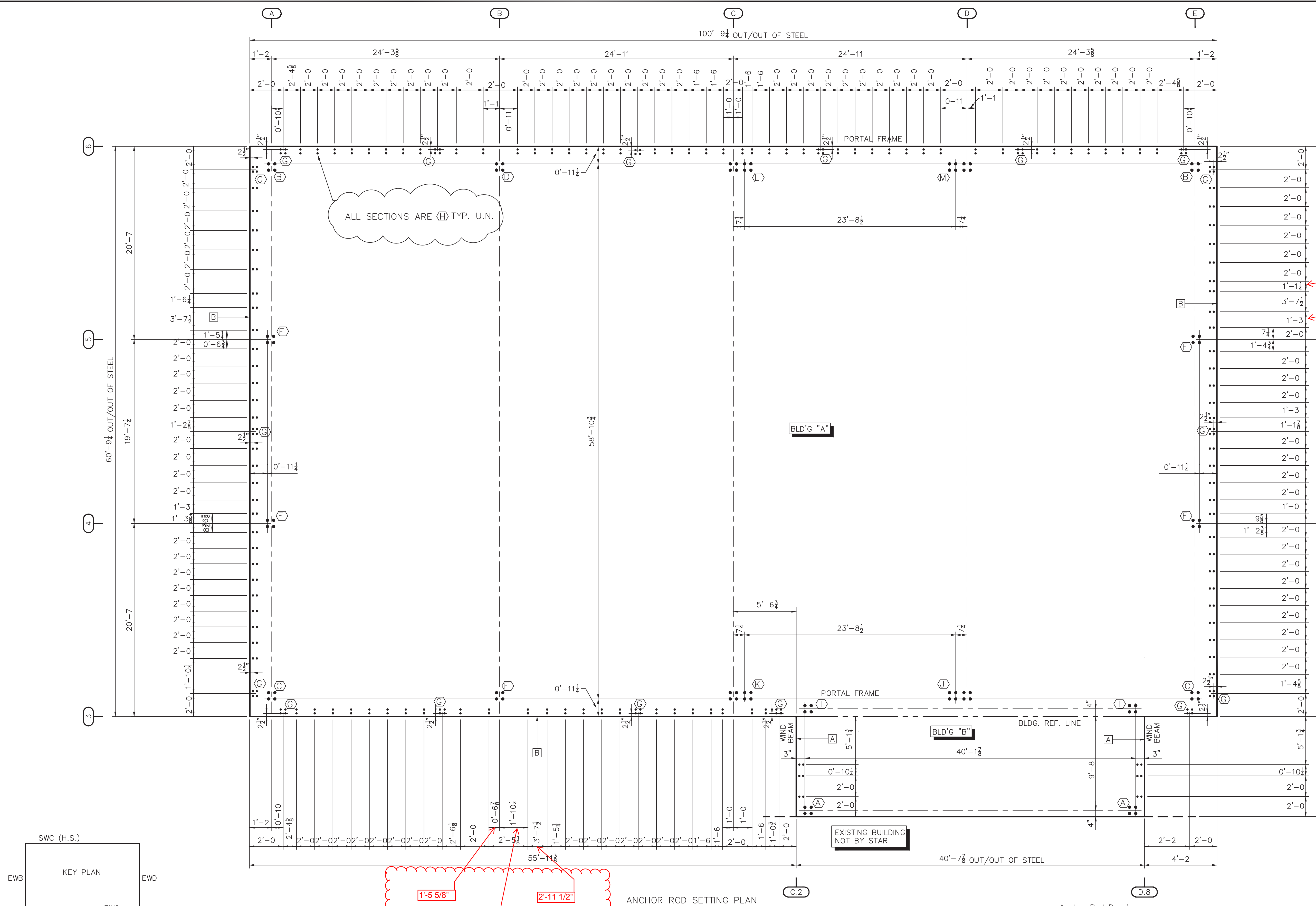
Customer: ECHO VENTURES INC DESTREHAN, LA
 Project Name & Location: TEXTRON MARINE & LAND SYSTEMS SLIDELL, LA

Drawing Status: Preliminary (Not For Construction) For Construction Permit
 REVISED For Erector Installation

Scale: NOT TO SCALE
 Drawn by: KRK 9/23/15
 Checked by: FER 9/29/15
 Project Engineer: AK
 Job Number: 14-B-97231
 Sheet Number: F1 of 4

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Brian A. Carmichael, P.E.
 Louisiana P.E. 33110
 DRSTIA ENSTIA



ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.

ANCHOR ROD DESCRIPTION	QUANTITY
5/8" Ø DIAMETER X	326
3/4" Ø DIAMETER X	88

ACCESSORY SCHEDULE

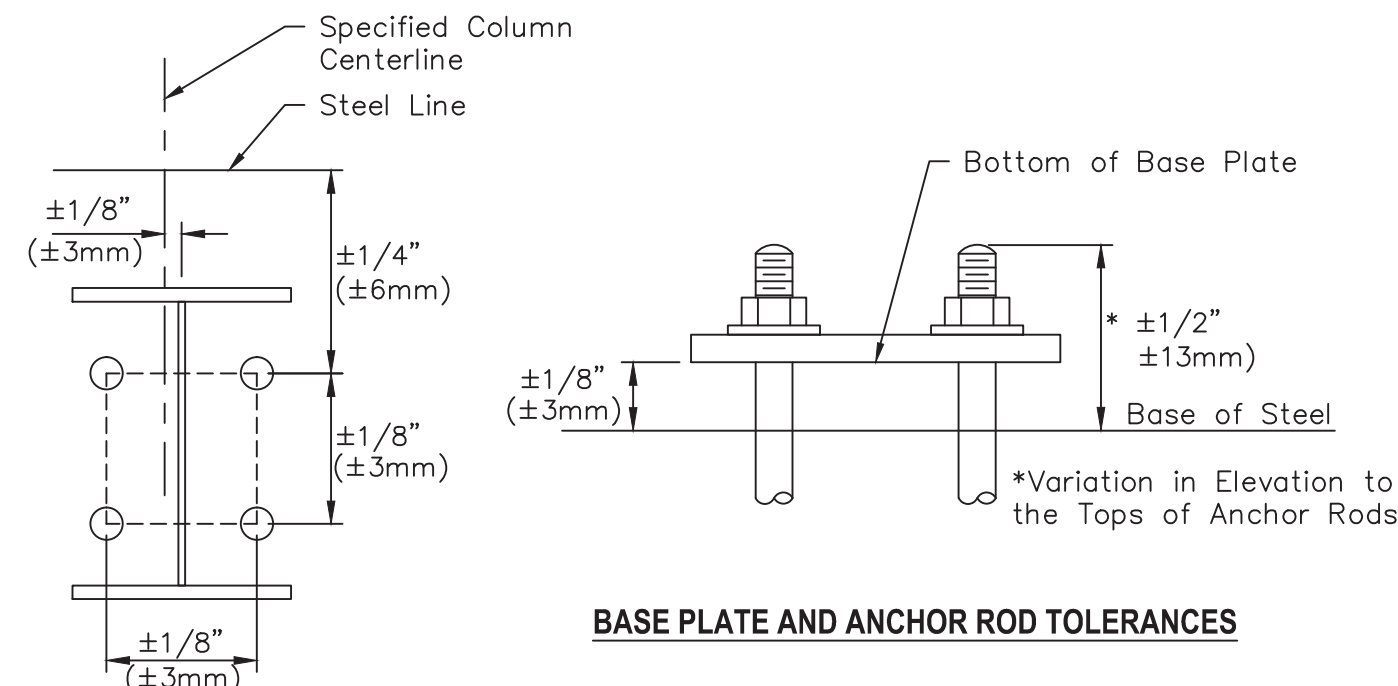
MARK	DESCRIPTION	DETAIL	QUAN.
A	5'-8 X 9'-0 FRAMED OPENINGS	(H)	2
B	3'-4 X 7'-2 FRAMED OPENINGS	(H)	3

FINISH FLOOR AT ELEVATION 100'-0"

- Anchor Rod Drawings
- This drawing is for anchor rod placement only and is not foundation design.
 - Foundation must be square and level with all anchor rods true in size, location, and projection.
 - Projection shown must be held to keep threads clear of finished concrete.
 - This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
 - Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
 - Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.

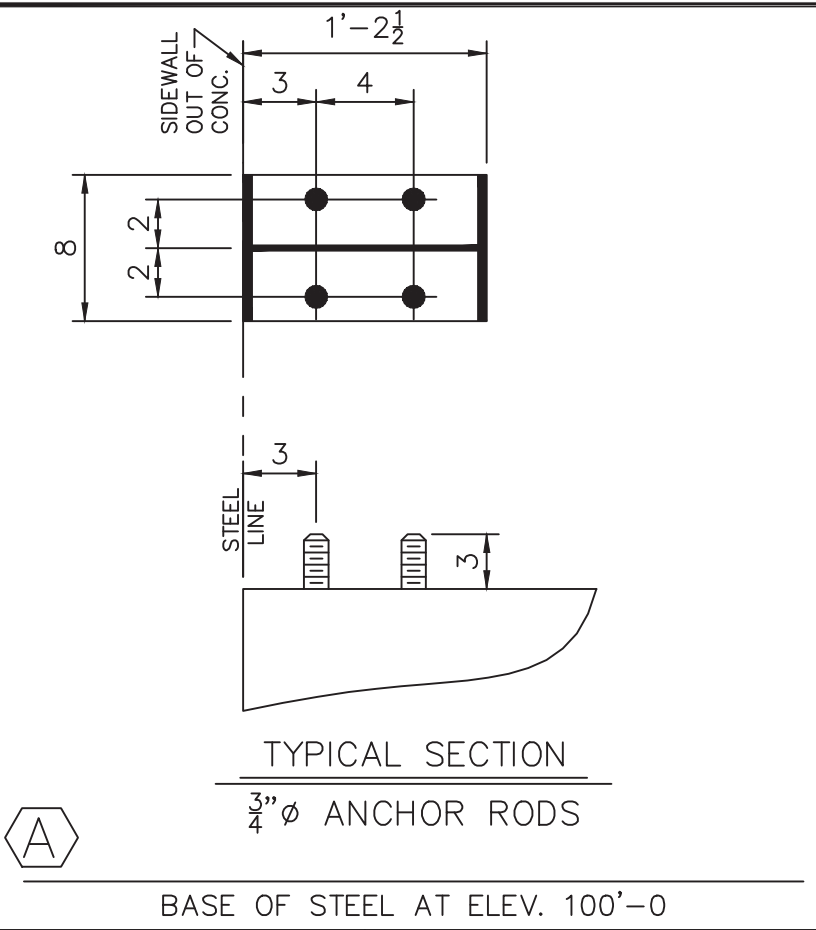


AISC CODE OF STANDARD PRACTICE TOLERANCES FOR SETTING ANCHOR RODS



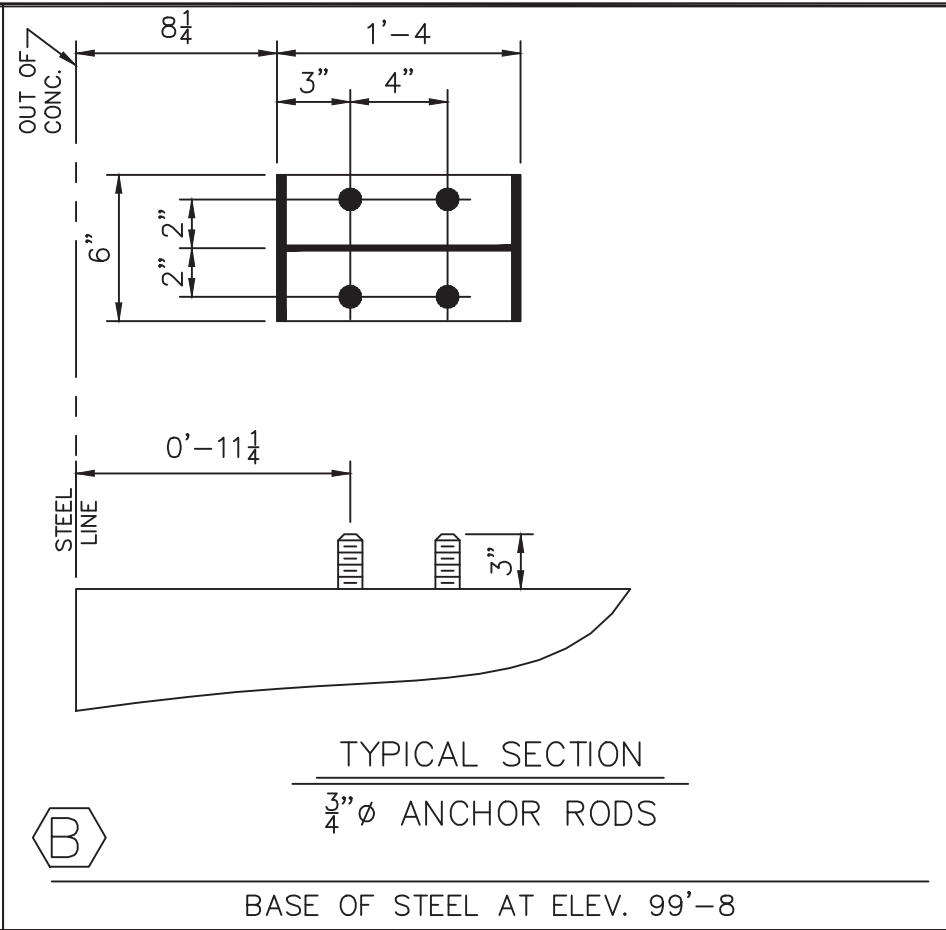
BASE PLATE AND ANCHOR ROD TOLERANCES

ANCHOR ROD SETTING TOLERANCES



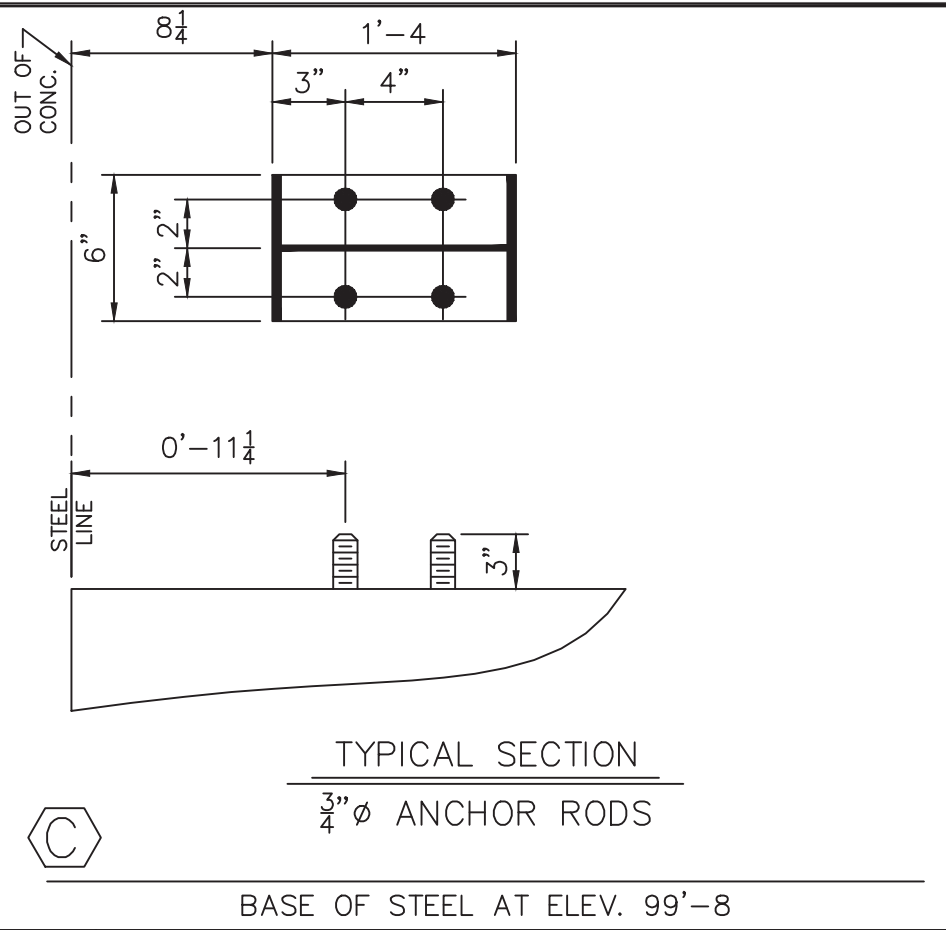
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 100'-0



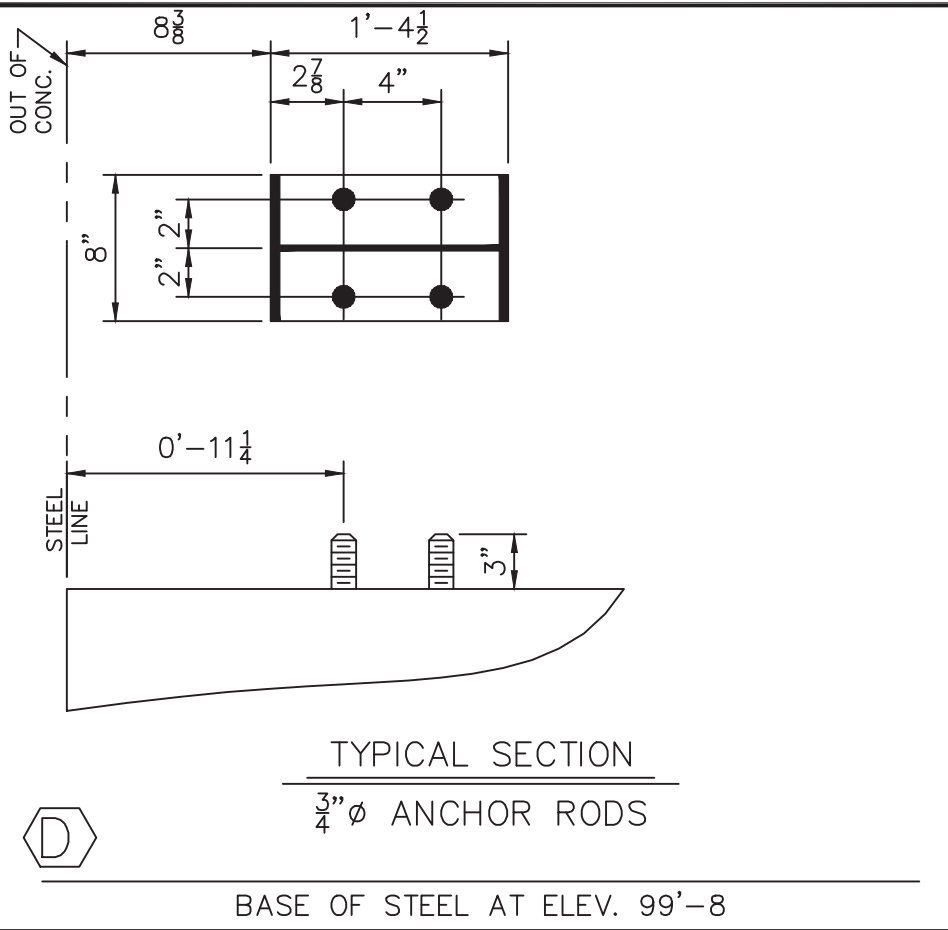
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



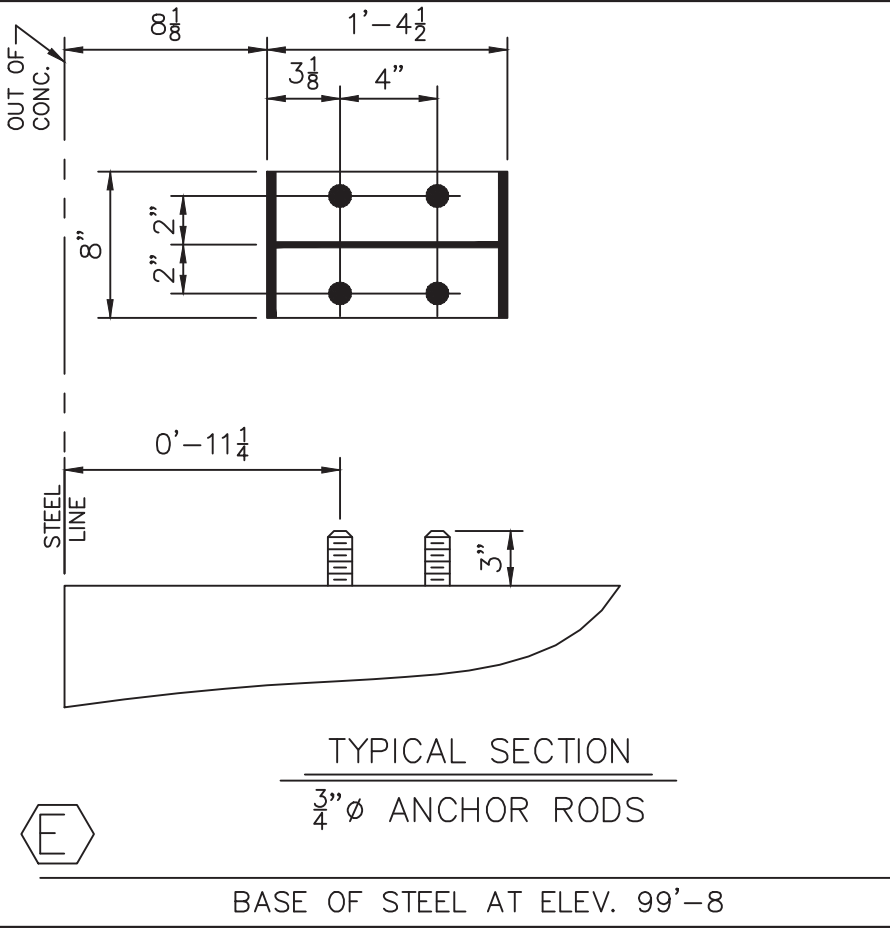
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



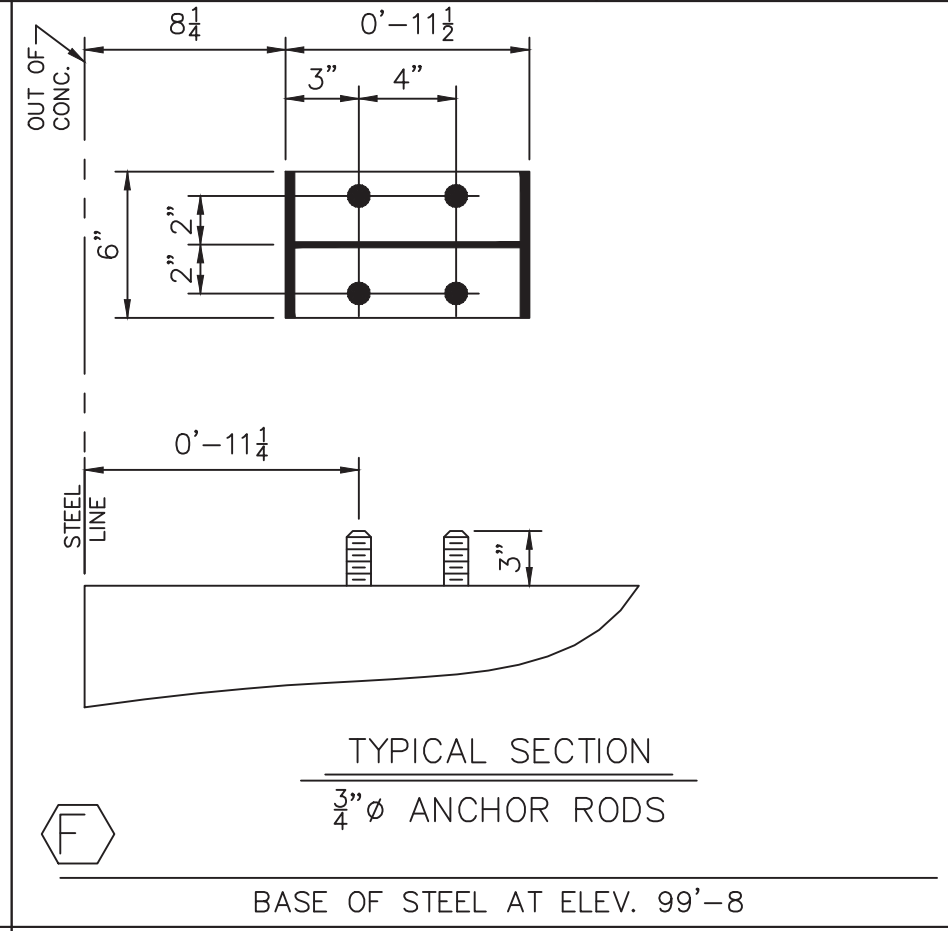
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



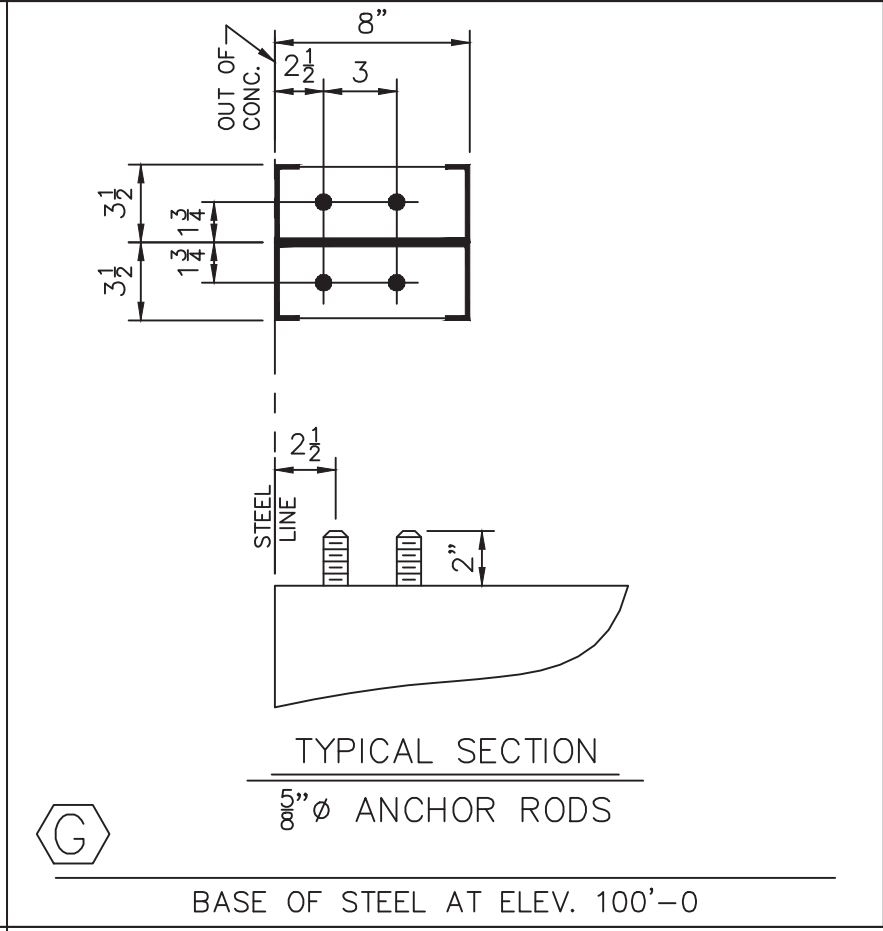
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



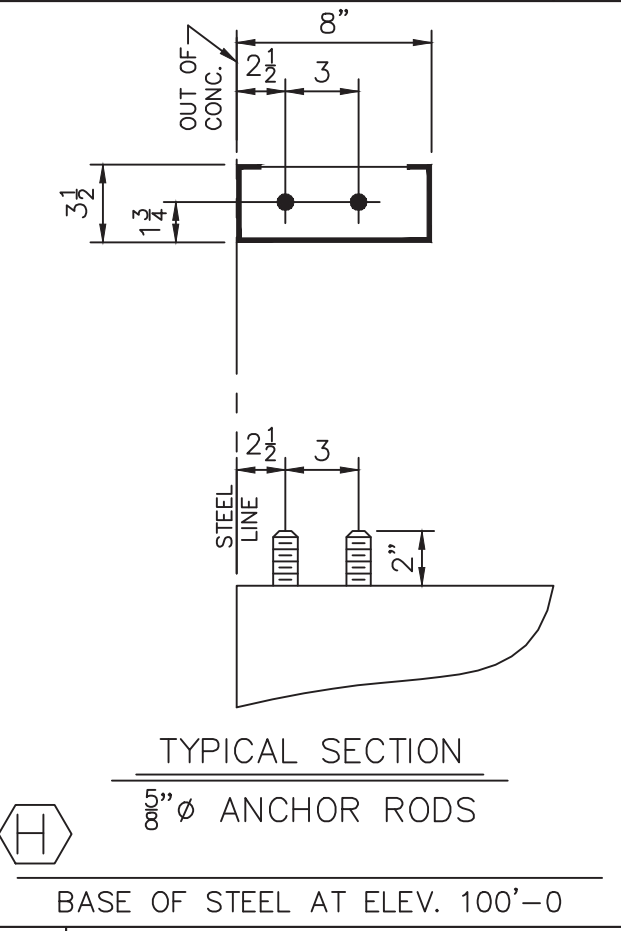
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



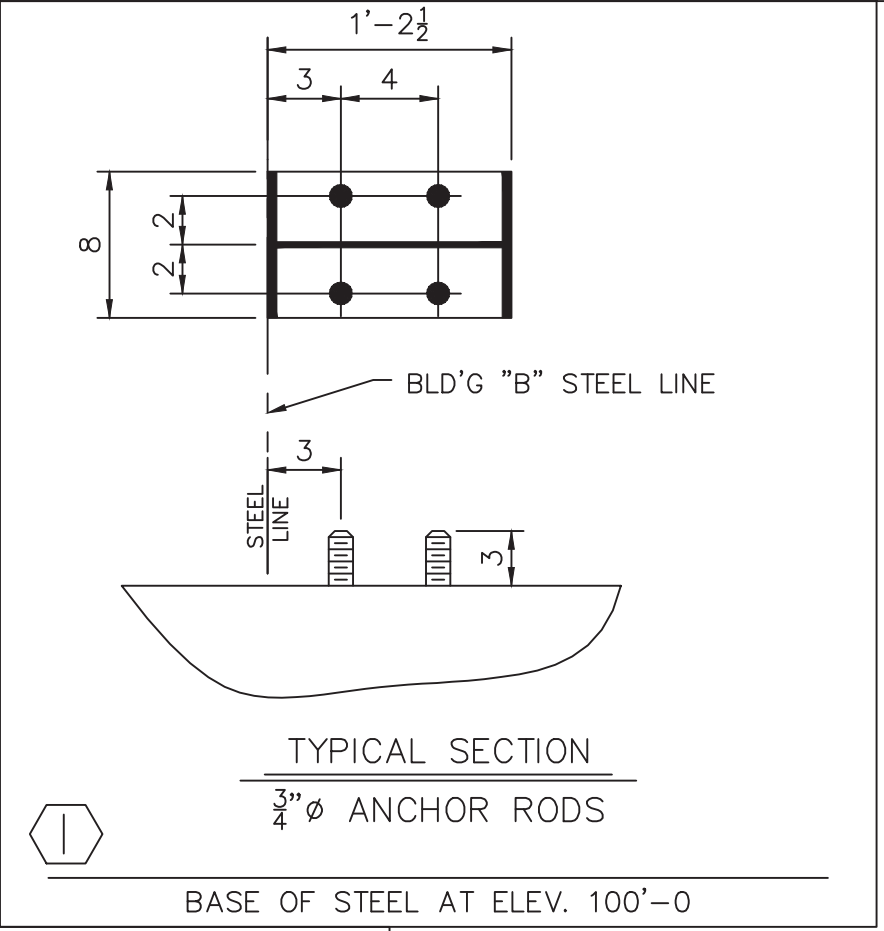
TYPICAL SECTION
5/8" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 100'-0



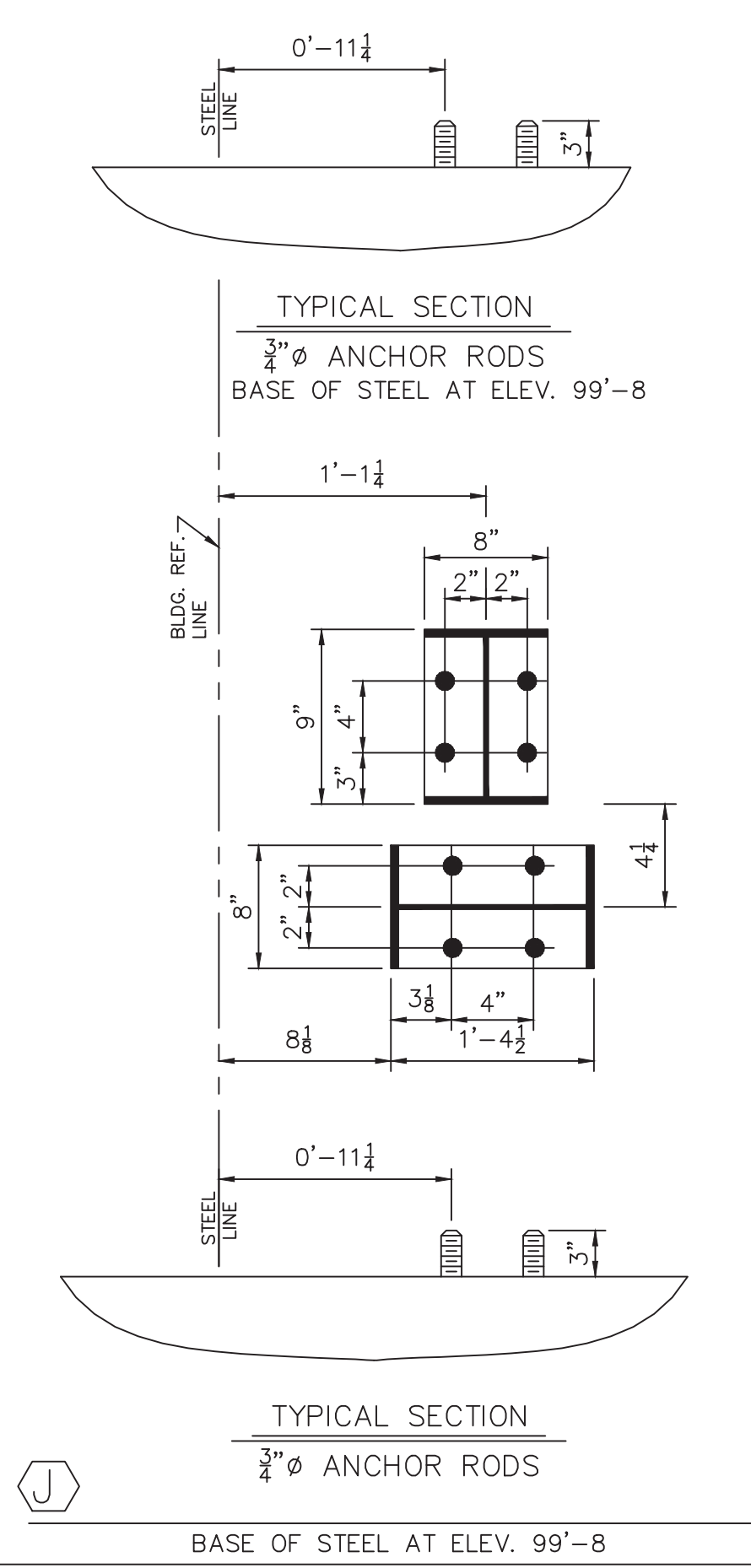
TYPICAL SECTION
5/8" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 100'-0



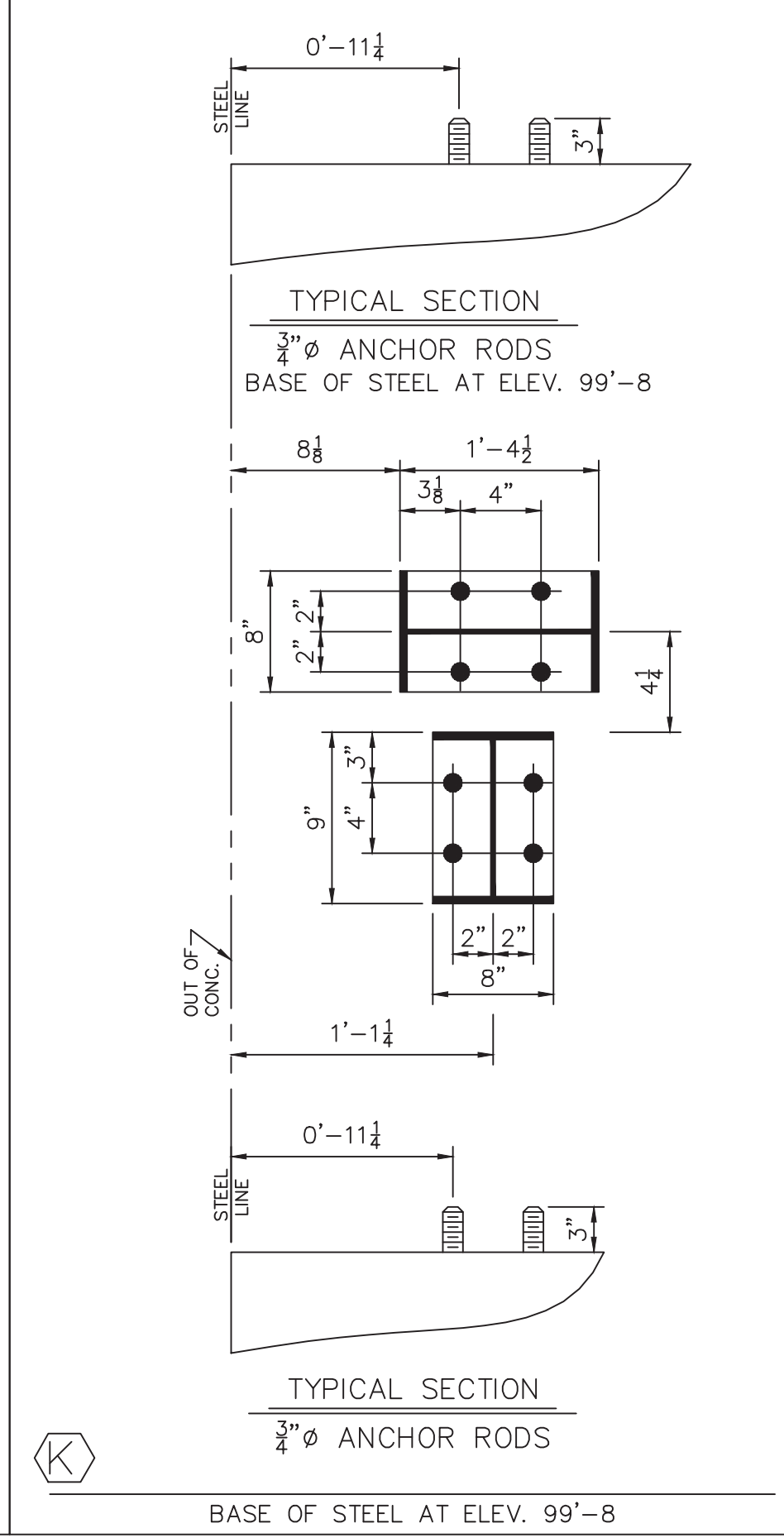
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 100'-0



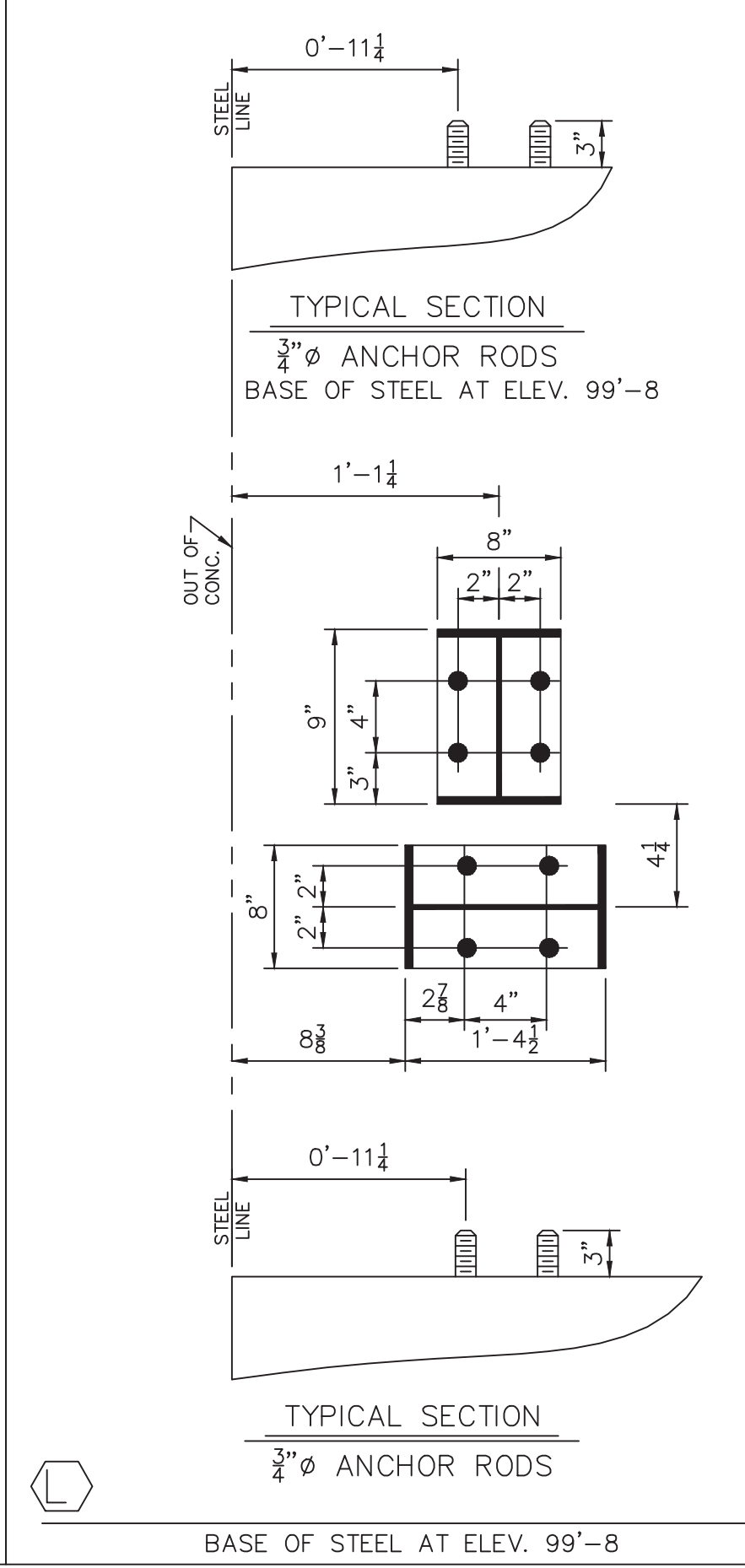
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



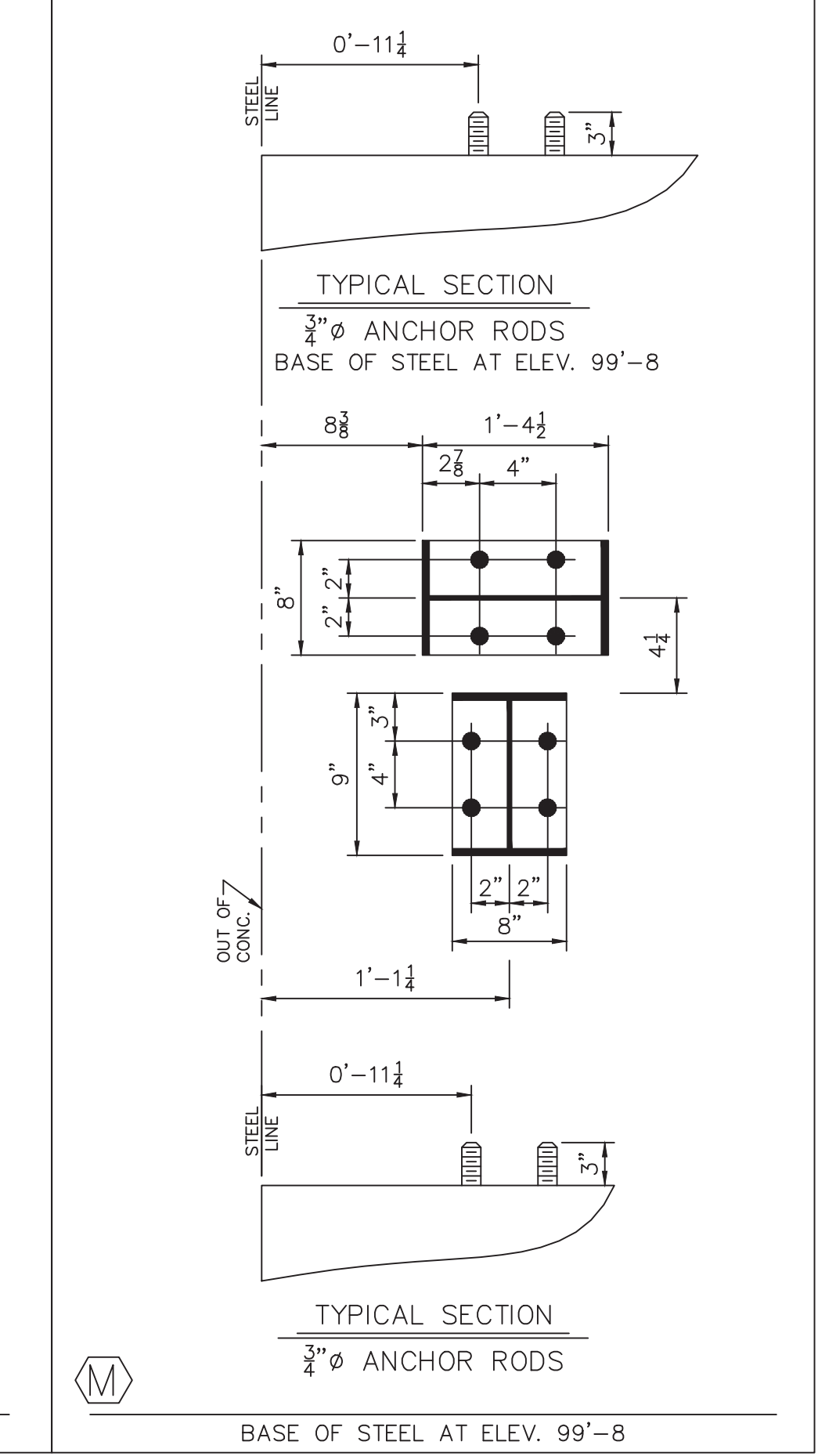
TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8



TYPICAL SECTION
3/4" ϕ ANCHOR RODS

BASE OF STEEL AT ELEV. 99'-8

Revision	Date	Description
1	12/11/15	REDRAWN THRU SALES ORDER REVISION 6.

P.O. BOX 94910
OKLAHOMA CITY, OK 73143
ANN ARBOR, MI (408) 636-2010

STAR BUILDING SYSTEMS
AN RECOMBANT

Customer: ECHO VENTURES INC
DESTREHAN, LA

Project Name & Location: TEXTRON MARINE & LAND SYSTEMS
SLIDELL, LA

Drawing Status: Preliminary (Not For Construction) For Construction Permit For Erector Installation

REVISED

Scale: NOT TO SCALE

Drawn by: KRK 9/23/15

Checked by: FER 9/29/15

Project Engineer: AK

Job Number: 14-B-97231

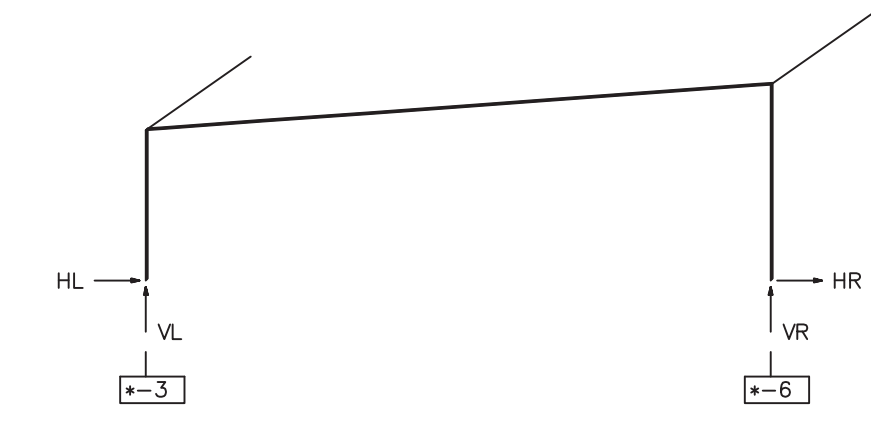
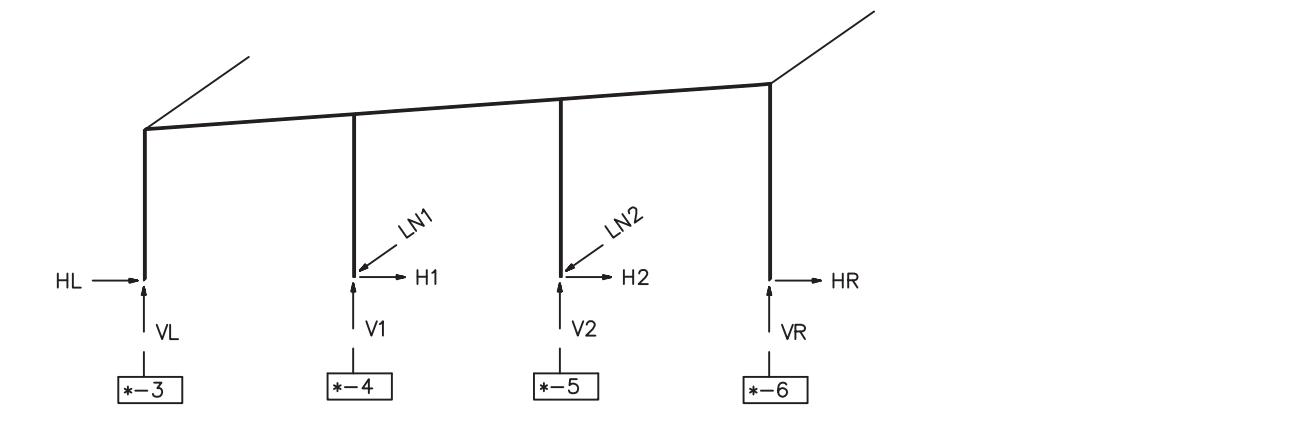
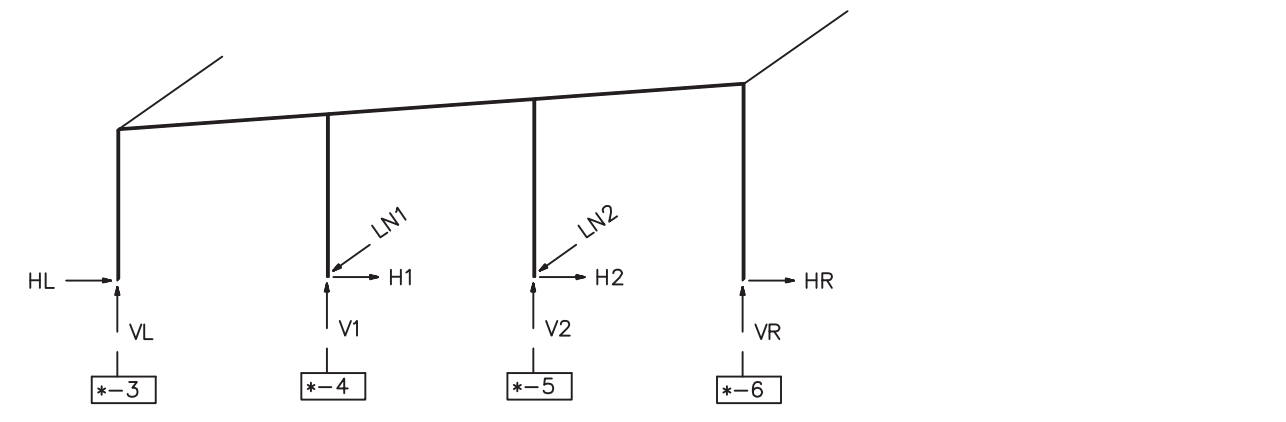
Sheet Number: F2 of 4

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Brian A. Carmichael, P.E.
Louisiana P.E. 33110

DRS11A ENST1A

Brian A. Carmichael
License No. 33110
PROFESSIONAL ENGINEER
9/12/14/15
ENGINEERING



LOAD GROUP REACTION TABLE * = A E

COLUMN	--3			--4			--5					
LOAD GROUP	HL	VL	LNL	HR	VR	LNR	H1	V1	LN1	H2	V2	LN2
DL	0.1	1.3	0.0	-0.1	1.4	0.0	0.0	2.4	0.0	0.0	2.3	0.0
COLL	0.2	1.3	0.0	-0.2	1.4	0.0	0.0	2.7	0.0	0.0	2.6	0.0
PSL1	0.0	0.6	0.0	-0.0	0.1	0.0	0.0	0.7	0.0	0.0	-0.2	0.0
PSL2	-0.0	-0.1	0.0	0.0	-0.1	0.0	-0.0	0.6	0.0	-0.0	0.6	0.0
PSL3	0.0	0.1	0.0	-0.0	0.6	0.0	-0.0	-0.1	0.0	-0.0	0.6	0.0
LL	0.3	2.7	0.0	-0.3	2.7	0.0	0.0	5.5	0.0	0.0	5.3	0.0
SNOW	0.1	0.7	0.0	-0.1	0.8	0.0	0.0	1.5	0.0	0.0	1.5	0.0
EQ	-0.5	-0.5	0.0	-0.3	0.4	0.0	0.0	0.7	0.0	0.0	-0.6	0.0
WL1	-3.7	-11.6	0.0	-4.8	-4.6	0.0	0.0	-15.0	0.0	0.0	-14.2	0.0
WL2	-7.3	-5.4	0.0	-1.2	2.3	0.0	0.0	-1.7	0.0	0.0	-3.6	0.0
LWL1	3.4	-8.6	0.0	-3.0	-7.0	0.0	0.2	-19.4	-8.3	0.2	-10.3	-9.1
LWL2	3.4	-5.9	0.0	-2.9	-9.9	0.0	0.2	-13.5	-8.3	0.2	-16.1	-9.1
LWL3	-0.6	-3.0	0.0	0.3	0.2	0.0	-0.2	-5.5	8.3	-0.2	-0.2	9.1
LWL4	-0.6	-0.2	0.0	0.3	-2.6	0.0	-0.2	0.5	8.3	-0.2	-6.0	9.1
WL3	8.7	-0.4	0.0	4.7	-13.5	0.0	-0.0	-20.6	0.0	-0.0	-10.9	0.0
WL4	5.1	5.8	0.0	8.2	-6.6	0.0	-0.0	-7.4	0.0	-0.0	-0.2	0.0
P2K1	0.1	1.0	0.0	-0.1	0.2	0.0	0.0	1.3	0.0	0.0	-0.4	0.0
P2K2	-0.1	-0.2	0.0	0.1	-0.2	0.0	-0.0	1.2	0.0	-0.0	1.2	0.0

LOAD GROUP REACTION TABLE * = A E

COLUMN	--3			--4			--5					
LOAD GROUP	HL	VL	LNL	HR	VR	LNR	H1	V1	LN1	H2	V2	LN2
P2K3	0.1	0.2	0.0	-0.1	1.0	0.0	-0.0	-0.4	0.0	-0.0	1.2	0.0
SBAL	0.1	0.6	0.0	-0.1	0.6	0.0	0.0	1.2	0.0	0.0	1.1	0.0
DSNW	0.1	0.9	0.0	-0.1	1.1	0.0	0.0	1.8	0.0	0.0	1.8	0.0

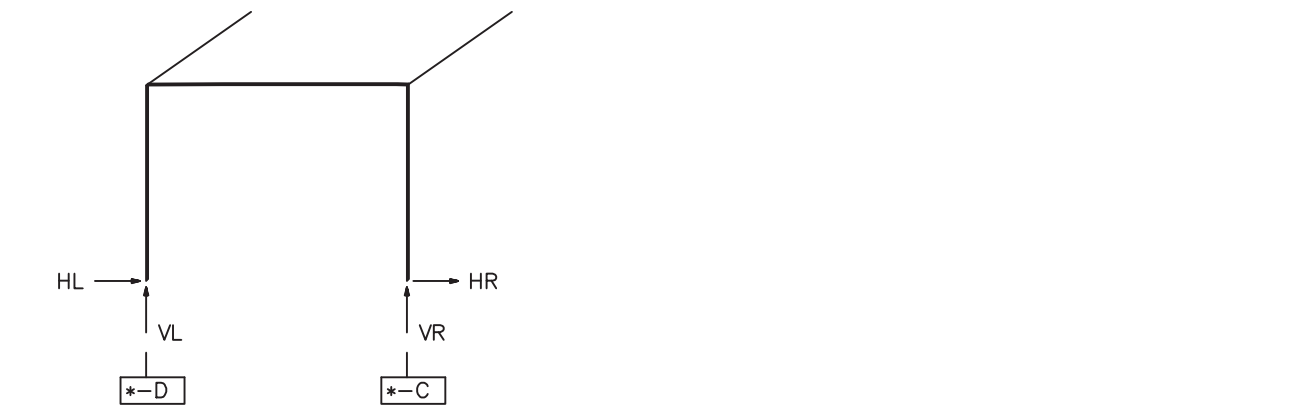
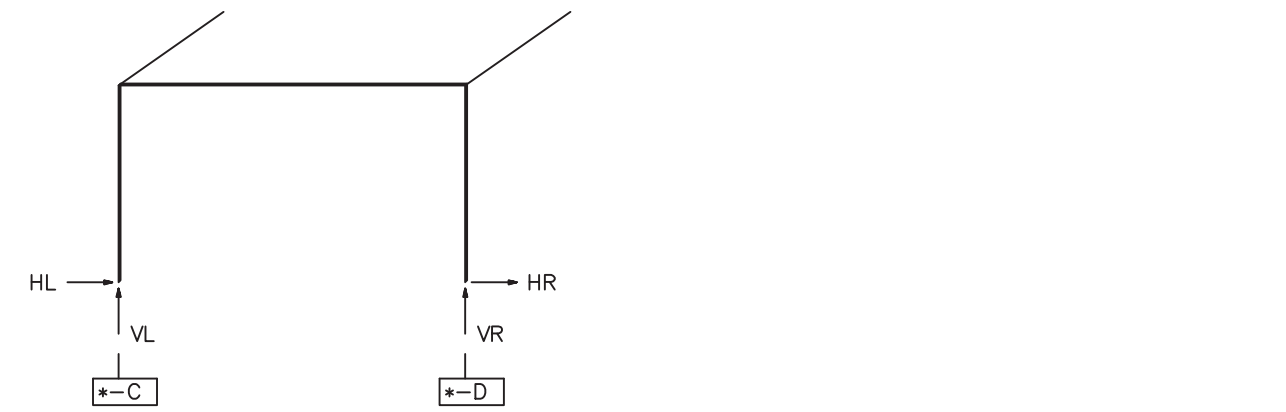
LOAD GROUP REACTION TABLE * = B C D

COLUMN	--3			--6		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	3.3	5.8	0.0	-3.3	6.0	0.0
LL	9.3	15.1	0.0	-9.3	15.2	0.0
COLL	4.6	7.6	0.0	-4.6	7.6	0.0
SNOW	2.5	4.2	0.0	-2.5	4.2	0.0
EQ	-0.8	-0.4	0.0	-0.6	0.4	0.0
WL1	-22.9	-38.4	0.0	11.7	-29.5	0.0
WL2	-10.0	-3.7	0.0	-1.2	5.0	0.0
LWL1	-13.8	-37.1	0.0	12.9	-30.8	0.0
LWL2	-13.9	-31.7	0.0	12.9	-36.2	0.0
LWL3	-0.9	-2.4	0.0	-0.0	3.6	0.0
LWL4	-1.0	3.0	0.0	0.0	-1.7	0.0
WL3	-7.7	-27.9	0.0	25.2	-40.1	0.0
WL4	5.2	6.8	0.0	12.3	-5.6	0.0
P2K1	1.0	1.0	0.0	-1.0	1.0	0.0
SBAL	2.0	3.2	0.0	-2.0	3.2	0.0
DSNW	0.2	0.3	0.0	-0.2	1.0	0.0

LOAD GROUP DESCRIPTION
 P2K3 : 2000# Point Load
 SBAL : Code Calculated Balanced Roof Snow Load
 DSNW : Drifting Snow

LOAD GROUP DESCRIPTION
 DL : Roof Dead Load
 LL : Roof Live Load
 COLL : Roof Collateral Load
 SNOW : Roof Snow Load
 EQ : Lateral Seismic Load [parallel to plane of frame]
 WL1 : Lateral Primary Wind Load
 WL2 : Lateral Primary Wind Load
 LWL1 : Longitudinal Primary Wind Load
 LWL2 : Longitudinal Primary Wind Load
 LWL3 : Longitudinal Primary Wind Load
 LWL4 : Longitudinal Primary Wind Load
 WL3 : Lateral Primary Wind Load
 WL4 : Lateral Primary Wind Load
 P2K1 : 2000# Point Load
 SBAL : Code Calculated Balanced Roof Snow Load
 DSNW : Drifting Snow

LOAD GROUP DESCRIPTION
 DL : Roof Dead Load
 COLL : Roof Collateral Load
 PSL1 : Pattern Snow Load [PSLxx]
 PSL2 : Pattern Snow Load [PSLxx]
 PSL3 : Pattern Snow Load [PSLxx]
 LL : Roof Live Load
 SNOW : Roof Snow Load
 EQ : Lateral Seismic Load [parallel to plane of frame]
 WL1 : Lateral Primary Wind Load
 WL2 : Lateral Primary Wind Load
 LWL1 : Longitudinal Primary Wind Load
 LWL2 : Longitudinal Primary Wind Load
 LWL3 : Longitudinal Primary Wind Load
 LWL4 : Longitudinal Primary Wind Load
 WL3 : Lateral Primary Wind Load
 WL4 : Lateral Primary Wind Load
 P2K1 : 2000# Point Load
 P2K2 : 2000# Point Load



LOAD GROUP REACTION TABLE * = 3

COLUMN	--C			--D		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.0	0.9	0.0	-0.0	0.9	0.0
EQ	-1.3	-1.5	0.0	-1.3	1.5	0.0
WL1	-12.3	-14.8	0.0	-12.9	14.8	0.0
WL2	12.9	14.8	0.0	12.3	-14.8	0.0

LOAD GROUP REACTION TABLE * = 6

COLUMN	--D			--C		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.0	0.8	0.0	-0.0	0.8	0.0
EQ	-1.3	-2.0	0.0	-1.3	2.0	0.0
WL1	-9.2	-14.6	0.0	-9.6	14.6	0.0
WL2	9.6	14.6	0.0	9.2	-14.6	0.0

LOAD GROUP DESCRIPTION
 DL : Roof Dead Load
 EQ : Lateral Seismic Load [parallel to plane of frame]
 WL1 : Lateral Primary Wind Load
 WL2 : Lateral Primary Wind Load

LOAD GROUP DESCRIPTION
 DL : Roof Dead Load
 EQ : Lateral Seismic Load [parallel to plane of frame]
 WL1 : Lateral Primary Wind Load
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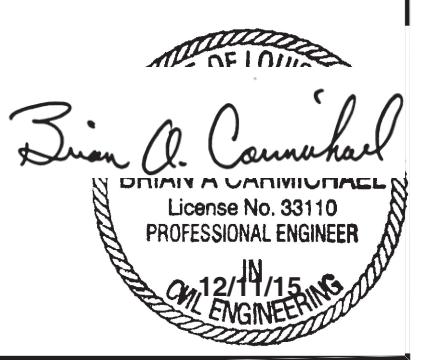
NOTES
 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
 a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 b) RIGID FRAMES
 (1) GABLED BUILDINGS
 (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 (2) SINGLE SLOPE BUILDINGS
 (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 c) ENDWALLS
 (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 f) X-BRACING
 (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, R_b .
 (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $I_p S_a(0.2)$ IS GREATER THAN 0.45.
 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC OR 2010 FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
 THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

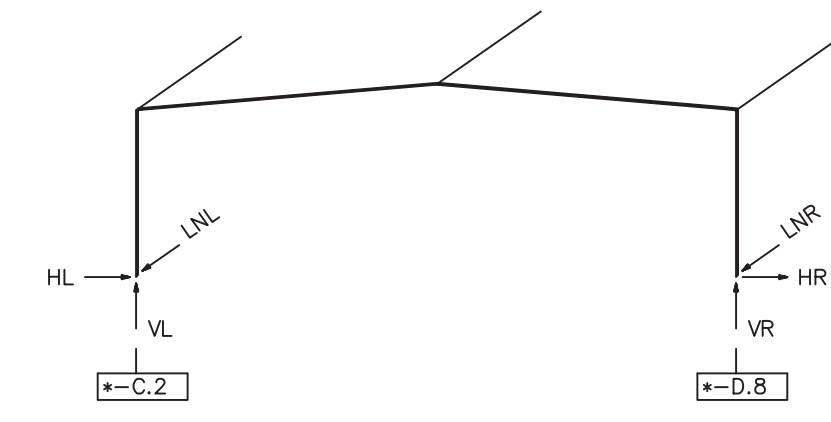
By	THC	Revision	Date
	HGF	1	12/11/15

STAR BUILDING SYSTEMS®
 AN OKLAHOMA COMPANY
 P.O. BOX 94810
 OKLAHOMA CITY, OK 73143
 (405) 636-2010
 Project Name & Location:
 ECHO VENTURES INC
 TEXTRON MARINE & LAND SYSTEMS
 SLIDELL, LA
 DESTREHAN, LA
 Drawing Status:
 Preliminary
 (Not For Construction)
 For Approval
 (Not For Construction)
 For Construction Permit
 For Erector Installation
REVISED

Scale: NOT TO SCALE
 Drawn by: KRK 9/23/15
 Checked by: FER 9/29/15
 Project Engineer: AK
 Job Number: 14-B-97231
 Sheet Number: F3 of 4
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 Louisiana P.E. 33110
 DRSTIA ENSTIA



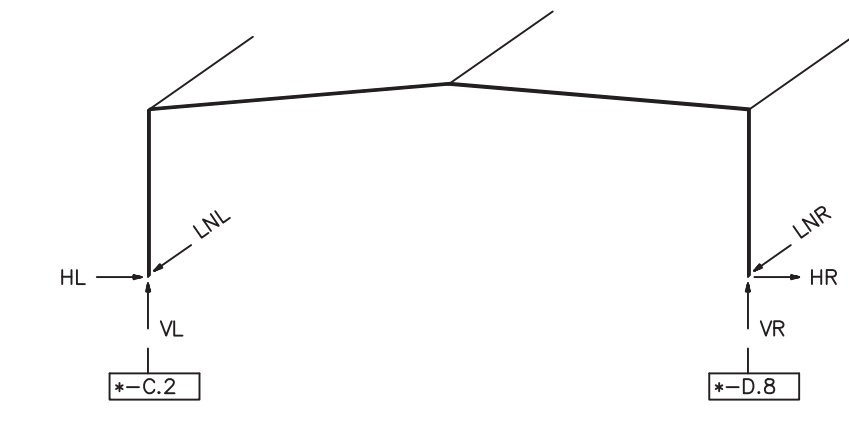


LOAD GROUP REACTION TABLE * = 1 2

LOAD GROUP	*-C.2			*-D.8		
	HL	VL	LNL	HR	VR	LNR
DL	0.4	1.7	0.0	-0.4	1.7	0.0
LL	1.0	2.6	0.0	-1.0	2.6	0.0
COLL	0.5	1.1	0.0	-0.5	1.1	0.0
SNOW	0.3	0.7	0.0	-0.3	0.7	0.0
EQ	-0.1	-0.1	-0.3	-0.1	0.1	-0.3
WL1	-3.4	-8.6	-3.8	1.0	-5.5	-3.8
WL2	-2.4	-3.6	-3.8	-0.1	-0.6	-3.8
LWL1	-1.4	-7.6	0.0	1.6	-6.0	0.0
LWL2	-1.6	-6.0	0.0	1.4	-7.6	0.0
LWL3	-0.6	-1.7	0.0	0.8	-0.0	0.0
LWL4	-0.8	-0.0	0.0	0.6	-1.7	0.0
WL3	-1.0	-5.5	3.8	3.4	-8.6	3.8
WL4	0.1	-0.6	3.8	2.4	-3.6	3.8
RS	0.2	0.3	0.0	-0.2	0.5	0.0
LS	0.2	0.5	0.0	-0.2	0.4	0.0
P2K1	0.8	1.0	0.0	-0.8	1.0	0.0
SBAL	0.2	0.5	0.0	-0.2	0.5	0.0
DSNW	1.1	2.3	0.0	-1.1	2.4	0.0

LOAD GROUP DESCRIPTION

- DL : Roof Dead Load
- LL : Roof Live Load
- COLL : Roof Collateral Load
- SNOW : Roof Snow Load
- EQ : Lateral Seismic Load [parallel to plane of frame]
- WL1 : Lateral Primary Wind Load
- WL2 : Lateral Primary Wind Load
- LWL1 : Longitudinal Primary Wind Load
- LWL2 : Longitudinal Primary Wind Load
- LWL3 : Longitudinal Primary Wind Load
- LWL4 : Longitudinal Primary Wind Load
- WL3 : Lateral Primary Wind Load
- WL4 : Lateral Primary Wind Load
- RS : Unbalanced Right Roof Snow Load
- LS : Unbalanced Left Roof Snow Load
- P2K1 : 2000# Point Load
- SBAL : Code Calculated Balanced Roof Snow Load
- DSNW : Drifting Snow



LOAD GROUP REACTION TABLE * = 1 2

LOAD GROUP	*-C.2			*-D.8		
	HL	VL	LNL	HR	VR	LNR
DSNL	-0.1	1.4	0.0	0.1	-0.1	0.0
DSNR	-0.2	-0.1	0.0	0.2	1.8	0.0

LOAD GROUP DESCRIPTION

- DSNL : Drifting Snow Left
- DSNR : Drifting Snow Right

NOTES

- THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).
 - A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - RIGID FRAMES
 - GABLED BUILDINGS
 - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - SINGLE SLOPE BUILDINGS
 - LEFT COLUMN IS THE LOW SIDE COLUMN.
 - RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - ENDWALLS
 - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 - X-BRACING
 - ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Ω_b .
 - FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $I_p/S_s(0.2)$ IS GREATER THAN 0.45.
- REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC OR 2010 FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0. THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

Ck'd	By	THC	HGF	REVISION	THRU SALES ORDER	REVISION	DATE	DESCRIPTION
				1			12/11/15	REDRAWN THRU SALES ORDER

STAR BUILDING SYSTEMS
ANN ARBOR, MI 48106
P.O. BOX 94910 OKLAHOMA CITY, OK 73143
(405) 636-2010

Customer: ECHO VENTURES INC
DESTREHAN, LA

Project Name & Location: TEXTRON MARINE & LAND SYSTEMS
SLIDELL, LA

Drawing Status: Preliminary (Not For Construction) For Construction Permit For Erector Installation

REVISED

Scale: NOT TO SCALE
 Drawn by: KRK 9/23/15
 Checked by: FER 9/29/15
 Project Engineer: AK
 Job Number: 14-B-97231
 Sheet Number: F4 of 4

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Brian A. Carmichael, P.E.
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