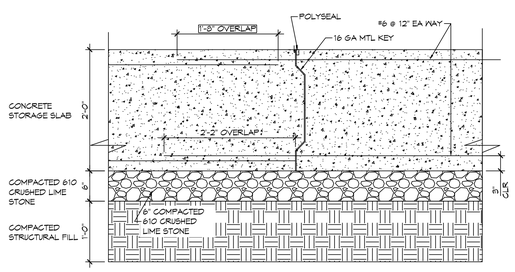
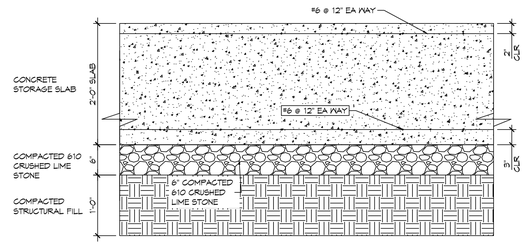
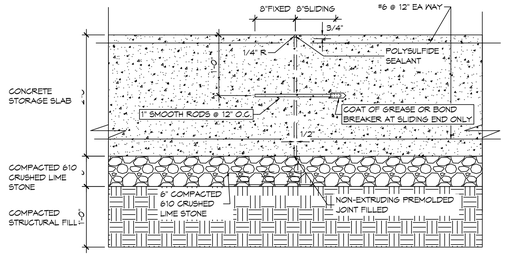
FILE NAME: J:\- HOUSTON, SITES\2025 - Jindal Tubular - Material Storage Site\Dwgs\Current\C101 Haward Site Plan.dwg PLOT DATE & TIME: Friday, September 26, 2025 11:05:13 AM



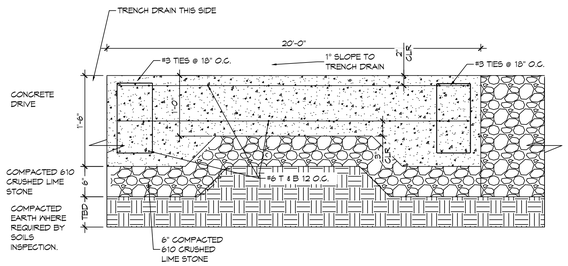
E TYPICAL CONSTRUCTION JOINT



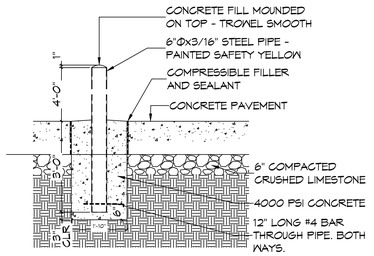
D TYPICAL STORAGE SLAB



A TYPICAL DOWELED EXPANSION JOINT



C TYPICAL DRIVE



E TYPICAL BOLLARD

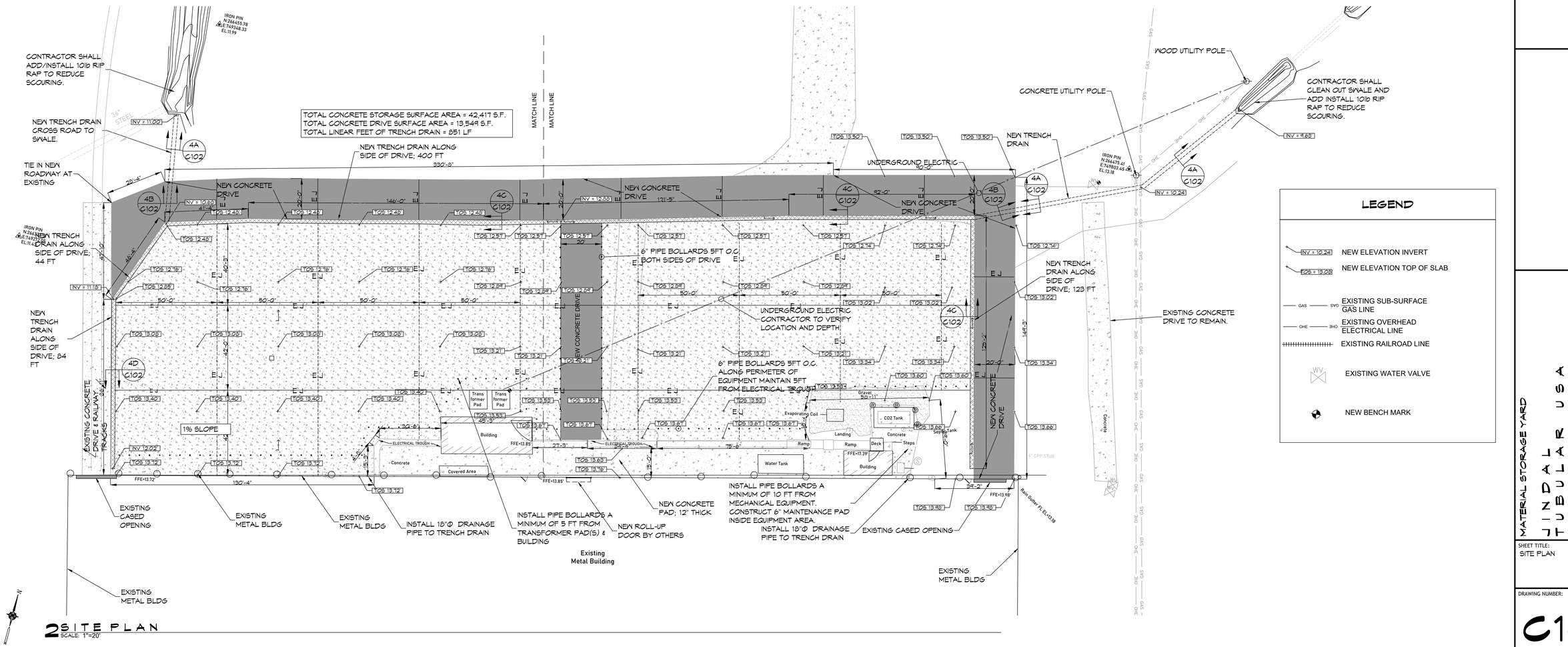
GENERAL PAVING NOTES

1. OWNER SHALL EMPLOY A GEO-TECHNICAL ENGINEER TO MONITOR THE POURING OF ALL CONCRETE. THE GEO-TECHNICAL ENGINEER SHALL COLLECT SAMPLES FROM CONCRETE DELIVERY TRUCKS AND TEST THE CONCRETE AT 7-DAYS AND 28-DAYS FOR COMPRESSIVE STRENGTH.
2. ALL NEW CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS AND A MINIMUM THICKNESS OF 6". CONCRETE MIX SHALL BE IN ACCORDANCE WITH THE LATEST REVISION OF ASTM C-180 TYPE 1.
3. CONCRETE PAVING THICKNESS SHALL VARY AS FOLLOWS:
   a. STORAGE AREAS = 24" THICKNESS
   b. DRIVE LANES & PARKING AREAS = 12" THICKNESS
   c. MAINTENANCE PADS = 6" THICKNESS
3. ALL REINFORGING STEEL SHALL MEET ASTM-A615 (GRADE 60).
4. ALL REINFORGING STEEL SHALL BE SECURELY SUPPORTED TO PREVENT BOTH VERTICAL AND HORIZONTAL MOVEMENT DURING CONCRETE PLACEMENT. ALL CONTROL AND EXPANSION JOINTS SHALL BE LOCATED AND INSTALLED AS SHOWN ON THE PAVING PLAN AND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
5. ALL SUB GRADE STRUCTURAL FILL SHALL BE SELECT GRANULAR MATERIAL COMPACTED TO 95% STANDARD PROCTOR DENSITY IN A MAXIMUM OF 6" LIFTS. OWNER'S REPRESENTATIVE SHALL PERFORM ONSITE TESTING.

GENERAL SITEPREP NOTES

1. THE OWNER SHALL EMPLOY A GEOTECHNICAL ENGINEER TO MONITOR SITE CONDITIONS DURING THE PREP WORK OF THE SITE FOUNDATION.
2. IN THE AREA FOR STORAGE SLAB REMOVE EXISTING NEAR SURFACE LIMESTONE AND MIXTURE OF SANDY SOILS WITH GRAVEL APPROXIMATELY 8 - 10 INCHES THICK. ONCE THE TOP LAYER HAS BEEN REMOVED, EXCAVATE TO A DEPTH OF 36 INCHES OVERALL DEPTH, HOWEVER THE ACTUAL EXCAVATION DEPTH SHALL BE DETERMINED BY A GEOTECHNICAL ENGINEER. THE EXPOSED SUBGRADE IN THE STORAGE AREA SHALL BE PROOF-ROLLED WITH A RUBBER TIRED VEHICLE WEIGHING ABOUT 20 TONS, PROOF-ROLLING SHALL BE MONITORED BY A GEOTECHNICAL ENGINEER. ANY SOILS WHICH ARE OBSERVED TO RUT OR DEFLECT EXCESSIVELY UNDER THE MOVING LOAD SHOULD BE UNDERCUT AND REPLACED WITH COMPACTED STRUCTURAL FILL. THE PROOFROLLING AND UNDERCUTTING ACTIVITIES SHALL BE WITNESSED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER AND SHOULD BE PERFORMED DURING A PERIOD OF DRY WEATHER.
3. AFTER THE SUBGRADE PREPARATION AND OBSERVATION HAVE BEEN COMPLETED, THE STRUCTURAL FILL SHOULD BE PLACED IN A RELATIVELY UNIFORM HORIZONTAL LIFT AND SHOULD BE ADEQUATELY KEYED INTO THE STRIPPED AND SCARIFIED SOILS. THE STRUCTURAL FILL SHOULD CONSIST OF SANDY CLAYS OR CLAYEY SANDS HAVING A MAXIMUM LIQUID LIMIT OF 40 PERCENT AND A MAXIMUM PLASTICITY INDEX OF 20. THE FILL SHOULD BE COMPACTED TO AT LEAST 95 PERCENT OF THE FILL'S MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698 (STANDARD PROCTOR).
4. THE STRUCTURAL FILL SHOULD BE PLACED IN MAXIMUM LIFTS OF 8 INCHES OF LOOSE MATERIAL AND SHOULD BE COMPACTED WITHIN THE RANGE OF ONE (1) PERCENTAGE POINT BELOW TO THREE (3) PERCENTAGE POINTS ABOVE THE OPTIMUM MOISTURE CONTENT VALUE. IF WATER MUST BE ADDED, IT SHOULD BE UNIFORMLY APPLIED AND THOROUGHLY MIXED INTO THE SOIL BY DISKING OR SCARIFYING. EACH LIFT OF COMPACTED STRUCTURAL FILL SHOULD BE TESTED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF SUBSEQUENT LIFTS. IN-PLACE DENSITY MEASUREMENTS SHOULD BE TAKEN TO ASSURE THAT THE ABOVE DEGREE OF COMPACTION IS ACHIEVED. THE COMPACTED STRUCTURAL FILL SHOULD EXTEND 5 FEET BEYOND THE PERIMETER OF THE BUILDING PRIOR TO SLOPING. ADEQUATE DRAINAGE SHOULD BE PROVIDED PRIOR TO AND DURING SITE WORK. THE SITE SHOULD BE GRADED TO PROMOTE RAPID RUNOFF.
5. ALL RUNOFF WATER MUST BE CARRIED AWAY FROM THE SLAB TO PREVENT SATURATION OF THE SUB-BASE.
6. ALL TREES WITHIN CLOSE PROXIMITY SHALL BE REMOVED TO PREVENT THE ROOTS FROM EXTENDING UNDER THE SLAB.
7. PROVIDE AND MAINTAIN IMMEDIATE SITE DRAINAGE BEFORE, DURNG, AND AFTER CONSTRUCTION. PROVIDE GRADING, SWELLS, AND SUMP PUMPS AS MAY BE REQUIRED TO IMMEDIATELY DRAIN ALL RAINWATER FROM THE CONSTRUCTION AREA. FOOTING EXCAVATIONS SHOULD BE OBSERVED AND CONCRETE PLACED AS QUICKLY AS POSSIBLE TO AVOID EXPOSURE OF THE FOOTING BOTTOMS TO WETTING AND DRYING. SURFACE RUNOFF WATER SHOULD BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND PRIOR OR AFTER CONCRETE PLACEMENT. IF IT IS REQUIRED THAT A FOOTING EXCAVATIONS BE LEFT OPEN FOR MORE THAN ONE DAY, THEY SHOULD BE PROTECTED TO REDUCE EVAPORATION OR ENTRY OF MOISTURE.

3 DETAILS
SCALE: NTS



2 SITE PLAN
SCALE: 1"=20'

LEGEND

- NV = 10.24 — NEW ELEVATION INVERT
- TOS = 13.08 — NEW ELEVATION TOP OF SLAB
- GAS — EXISTING SUB-SURFACE GAS LINE
- OHE — EXISTING OVERHEAD ELECTRICAL LINE
- EXISTING RAILROAD LINE
- WV — EXISTING WATER VALVE
- NEW BENCH MARK

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Chief Engineer: Brian Mistich, PE
Civil Engineer: Brett Hall
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#	DESCRIPTION	REVISIONS	DATE

SEAL:

MATERIAL STORAGE YARD
JINDAL TUBULAR USA
13042 SEA PLANE RD.
BAY ST LOUIS, MS 39520

JOB No:	2510	DATE:	9/2/2025
DRAWN BY:	LP	CHECKED BY:	DMM

SHEET TITLE:
SITE PLAN

DRAWING NUMBER:
C101

SHEET No: 1 of 1