

Hercules Metal Buildings



ENDWALL COLUMN:

Frm Line		Col Line		Dead		Collat		Live		Snow		Wind Left1		Wind Right1		Wind Left2		Wind Right2		Wind Press		Wind Suct		Wind Long1		Wind Long2				
Line	Line	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf		
1	A	0.3	0.1	1.4	0.2	-2.7	-1.3	-1.8	-0.4	-2.5	2.8	-2.8	-1.3																	
1	C	0.9	0.2	4.3	0.8	-6.4	-4.1	-4.1	-4.1	-5.0	5.5	-6.3	-6.3																	
1	E	0.3	0.1	1.4	0.2	-1.3	-2.7	-0.4	-1.8	-2.5	2.8	-2.8	-1.3																	

Frm Line		Col Line		Sels Left		Sels Right		-WIN_SNOW--		E1UNB_SL_L-		E1UNB_SL_R-	
Line	Line	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf
1	A	0.0	0.0	0.0	0.3	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0
1	C	0.0	0.0	0.0	0.9	0.0	0.9	0.0	0.0	0.0	0.0	0.0	0.0
1	E	0.0	0.0	0.0	0.3	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0

Frm Line		Col Line		Dead		Collat		Live		Snow		Wind Left1		Wind Right1		Wind Left2		Wind Right2		Wind Press		Wind Suct		Wind Long1		Wind Long2				
Line	Line	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf			
4	E	0.2	0.1	1.4	0.2	-2.3	-1.3	-1.4	-0.4	-1.6	1.9	-2.4	-1.4																	
4	D	0.5	0.2	3.7	0.6	-6.4	-4.1	-4.1	-4.1	-4.3	3.7	-6.3	-6.3																	
4	B	0.5	0.2	3.7	0.6	-4.1	-4.1	-4.1	-4.1	-4.3	3.7	-6.3	-6.3																	
4	A	0.2	0.1	1.4	0.2	-1.3	-2.3	-0.4	-1.4	-1.6	1.9	-1.4	-2.4																	

Frm Line		Col Line		Sels Left		Sels Right		-WIN_SNOW--		E2UNB_SL_L-		E2UNB_SL_R-	
Line	Line	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf	Verf
4	E	0.0	0.0	0.0	0.5	0.0	0.5	0.0	0.0	0.0	0.0	0.0	0.0
4	D	0.0	0.0	0.0	0.9	0.0	0.9	0.0	0.0	0.0	0.0	0.0	0.0
4	B	0.0	0.0	0.0	0.9	0.0	0.9	0.0	0.0	0.0	0.0	0.0	0.0
4	A	0.0	0.0	0.0	0.3	0.0	0.3	0.0	0.0	0.0	0.0	0.0	0.0

ENDWALL COLUMN:

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

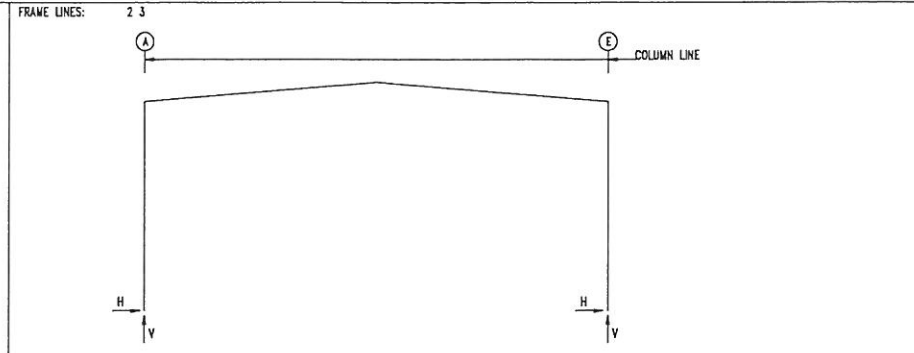
Frm Line		Col Line		Column_Reactions(k)		Hmin		V		Bolt(in)		Base_Plate(in)		Grout(in)	
Line	Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	Grout	Grout	
1	A	8	1.7	-1.5	9	-1.5	-1.5	2	0.625	3.500	8.000	0.250	0.0		
1	C	10	3.3	-3.3	11	-3.0	-3.3	4	0.625	5.000	8.000	0.250	0.0		
1	E	12	1.7	-1.5	11	-1.5	-1.5	2	0.625	3.500	8.000	0.250	0.0		
4	E	8	1.1	-1.3	9	-1.0	-1.3	2	0.625	3.500	8.000	0.250	0.0		
4	D	13	2.2	-3.5	9	-2.0	-3.5	2	0.625	3.500	8.000	0.250	0.0		
4	B	10	2.2	-3.5	11	-2.0	-3.5	2	0.625	3.500	8.000	0.250	0.0		
4	A	12	1.1	-1.3	11	-1.0	-1.3	2	0.625	3.500	8.000	0.250	0.0		

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Total Len (in)	Bend Len (in)	Proj (in)
8	Jamb	1/2"	A307	3.75		1.50
16	Endwall	5/8"	A307			2.50
16	Frame	3/4"	A307		3.00	2.50

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k)				Panel Shear (lb/H)	
		Wind	Seismic	Wind	Seismic	Wind	Seis
L_EW	1	4.6	4.3	0.3	0.2	79	5
R_SW	4	4.6	4.3	0.3	0.2	74	5



RIGID FRAME:

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line		Col Line		Column_Reactions(k)		Hmin		V		Bolt(in)		Base_Plate(in)		Grout(in)	
Line	Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	Grout	Grout	
2*	A	3	2.5	2.5	4	-4.0	-3.8	4	0.750	5.000	8.250	0.250	0.0		
2*	E	5	4.0	-3.8	2	-2.5	2.5	4	0.750	5.000	8.250	0.250	0.0		

RIGID FRAME:

BASIC COLUMN REACTIONS (k)

Frame Line		Column		Dead		Collateral		Live		Snow		Wind Left1		Wind Right1	
Line	Line	Horz	Verf	Horz	Verf	Horz	Verf	Horz	Verf	Horz	Verf	Horz	Verf	Horz	Verf
2*	A	0.4	1.3	0.1	0.4	1.6	5.1	0.5	1.5	-6.5	-12.7	1.7	-6.9	1.7	-6.9
2*	E	-0.4	1.3	-0.1	0.4	-1.6	5.1	-0.5	1.5	-1.7	-6.9	6.5	-12.7	1.7	-6.9

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 40.0
 - Length (ft) = 60.0
 - Eave Height (ft) = 18.0/ 18.0
 - Roof Slope (rise/run) = 1.0/ 1.0
 - Dead Load (psf) = 2.0
 - Collateral Load (psf) = 1.0
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 12.0
 - Snow Load (psf) = 3.5
 - Wind Speed (mph) = 146.0
 - Wind Code = IBC 15
 - Exposure = B
 - Closed/Open = C
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = B
 - Seismic Coeff (Fa/Ss) = 0.16
- Loading conditions are:
 - 1 Dead+Collateral+Live
 - 2 Dead+Collateral+0.75Live+0.45Wind_Left1
 - 3 Dead+Collateral+0.75Live+0.45Wind_Right1
 - 4 0.6Dead+0.6Wind_Left2
 - 5 0.6Dead+0.6Wind_Right2
 - 6 0.6Dead+0.6Wind_Long1L
 - 7 0.6Dead+0.6Wind_Long2L
 - 8 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
 - 9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
 - 10 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 - 11 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - 12 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
 - 13 0.6Dead+0.6Wind_Left1+0.6Wind_Suction



DESCRIPTION: ANCHOR BOLT REACTIONS

CUSTOMER: DLR **PROJECT:** Bouche Construction

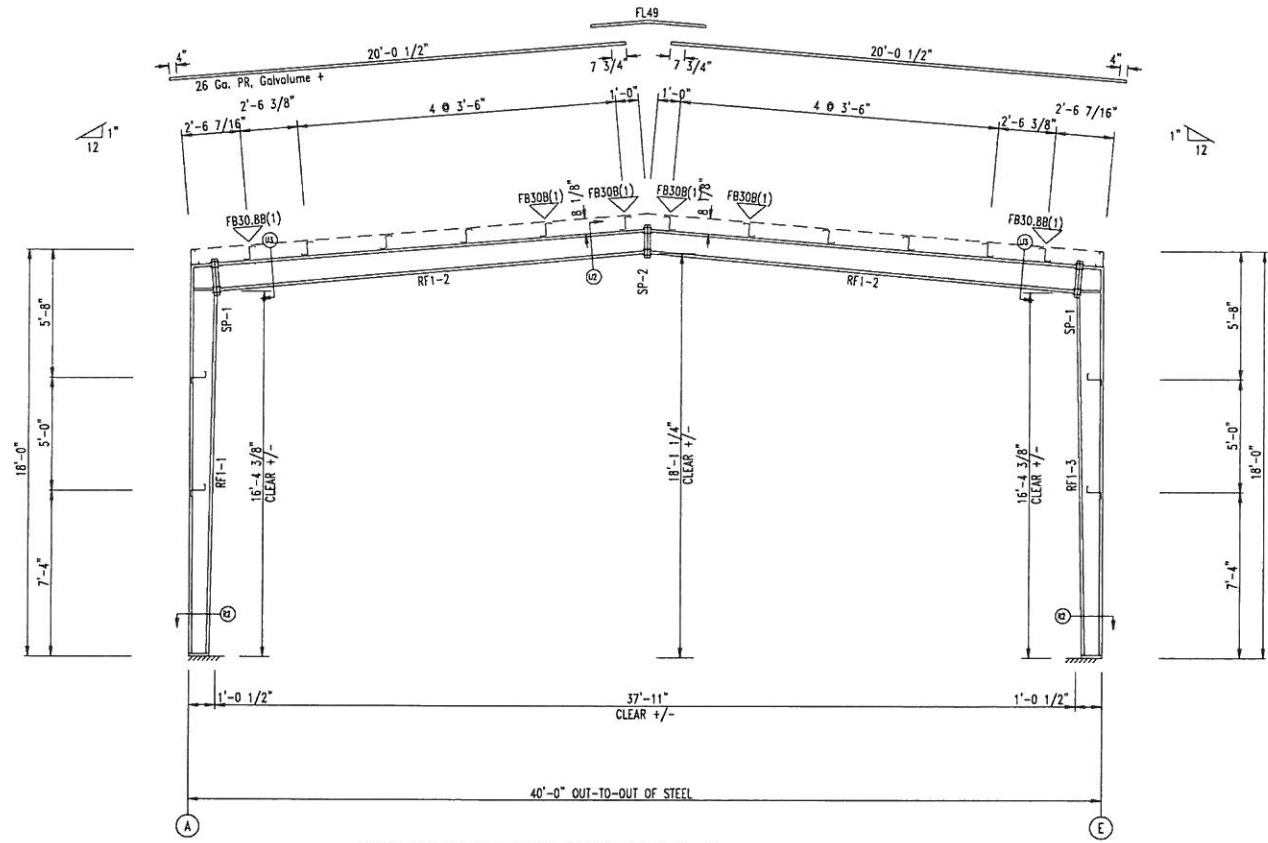
LOCATION: PRJ Address 2

DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.
DET	DES	2/28/19	N.T.S.	00	022219	OF

SPLICE PLATE & BOLT TABLE									
Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length
	Top	Bot							
SP-1	4	4	0	A325	0.750	2.00	6"	1/2"	1'-8 1/4"
SP-2	4	4	0	A325	0.750	2.00	6"	1/2"	1'-6 1/4"

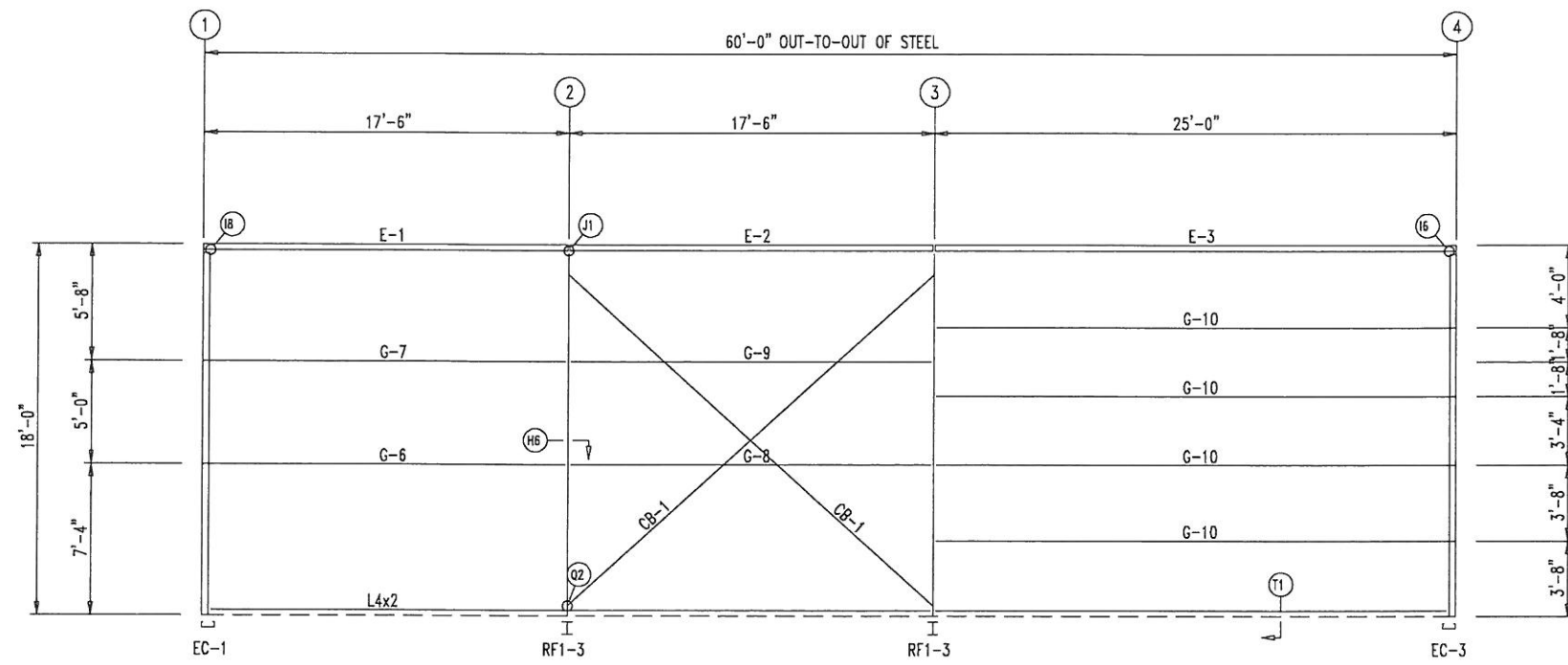
FLANGE BRACES: Both Sides(U.N.)
 FBxxB(1); xx=length(in)
 B - FBZK1/8

MEMBER TABLE									
Mark	Weight	Length	Web Depth		Web Plate		Outside Flange		Inside Flange
			Start/End	Thick	Thick	Length	W x Thk x Length	W x Thk x Length	
RF1-1	270	17'-3 7/8"	7.5/ 8.6	0.135	4'-0"	4'-0"	5 x 1/4" x 17'-3 3/8"	5 x 1/4" x 16'-0 1/4"	
			8.6/11.3	0.135	9'-4 1/4"	9'-4 1/4"	5 x 1/4" x 10 15/16"		
RF1-2	305	19'-1 3/8"	11.3/12.0	0.250	4'-0"	4'-0"	5 x 1/4" x 19'-0 3/8"	5 x 1/4" x 18'-11 9/16"	
			12.0/11.0	0.135	9'-11"	9'-11"			
RF1-3	273	17'-3 7/8"	11.0/10.0	0.135	9'-1 3/8"	9'-1 3/8"	5 x 1/4" x 10 15/16"	5 x 1/4" x 16'-0 1/4"	
			12.0/11.3	0.250	4'-0"	4'-0"	5 x 1/4" x 17'-3 3/8"		
			11.3/ 8.6	0.135	9'-4 1/4"	9'-4 1/4"			
			8.6/ 7.5	0.135	4'-0"	4'-0"			

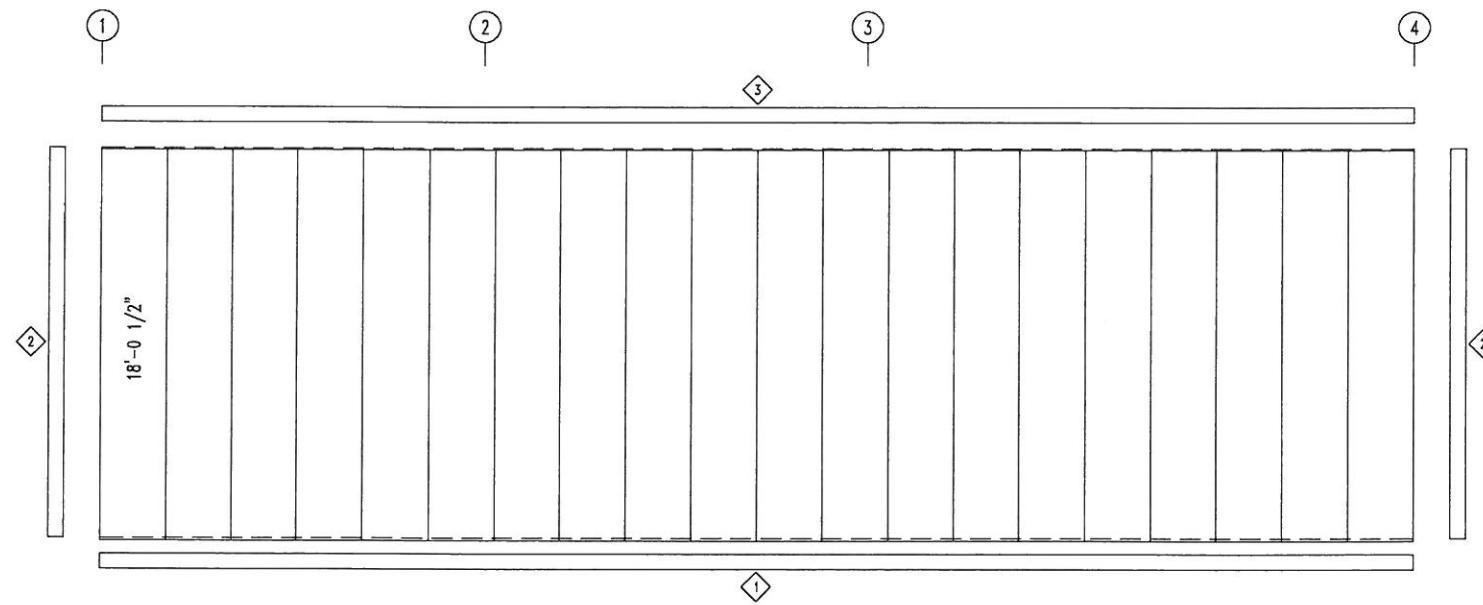


RIGID FRAME ELEVATION: FRAME LINE 2 3

<p>5001 Perle Road Chalmette, LA 70043 (800) 783-2647 (504) 277-7330 (fax)</p>	DESCRIPTION: RIGID FRAME ELEVATION						
	CUSTOMER: DLR			PROJECT: Bouche Construction			
	LOCATION: PRJ Address 2						
	DRN. BY: DET	CK'D BY: DES	DATE: 2/28/19	SCALE: N.T.S.	REV: 00	QUOTATION NO.: 022219	SHEET NO.: OF



SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 26 Ga. PR - NEED SIG 200

MEMBER TABLE FRAME LINE E		
MARK	PART	LENGTH
E-1	E085341L	17'-5 1/2"
E-2	E085341L	17'-5 1/2"
E-3	E085341L	24'-11 1/2"
G-6	8X25Z12	17'-1 5/8"
G-7	8X25Z16	17'-1 5/8"
G-8	8X25Z14	16'-9 3/4"
G-9	8X25Z16	16'-9 3/4"
G-10	8X25Z12	24'-7 5/8"
CB-1	HW374	24'-6 1/4"

TRIM TABLE FRAME LINE E		
ID	MARK	DETAIL
1	FL72	TRIM_74
2	FL834	TRIM_198
3	FL19A	TRIM_316



5001 Paris Road, Chalmette, LA 70043
 (800) 783-2647 (504) 277-7330 (fax)

DESCRIPTION: SIDEWALL FRAMING							
CUSTOMER: DLR				PROJECT: Bouche Construction			
LOCATION: PRJ Address 2							
DRN. BY DET	CK'D BY DES	DATE 2/28/19	SCALE N.T.S.	REV. 00	QUOTATION NO. 022219	SHEET NO. OF	



5001 Paris Road Chalmette, LA 70043
 (800) 783-2647 (504) 277-7330 (fax)

BUILDING LOADS / DESCRIPTION:

WIDTH: 40 LENGTH: 60 HEIGHT: 18 / 18
 (BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 15

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD: 2.000 PSF (ROOF PANELS & PURLINS)

COLLATERAL LOAD: 1 PSF

ROOF LIVE LOAD: 20.00 PSF

ROOF SNOW LOAD: 3.5 PSF

BASIC WIND SPEED: 146 MPH

SEISMIC ZONE: B

WIND EXPOSURE: B

IMPORTANCE FACTORS:

WIND LOAD: 1.00

SNOW LOAD 1.0000

SEISMIC LOAD 1.00

GENERAL NOTES:

- 1) MATERIALS : MINIMUM YIELD:
 HOT ROLLED BAR Fy = 50.0000 ksi MIN.
 STRUCTURAL STEEL SHEET Fy = 50.0000 ksi MIN.
 STRUCTURAL STEEL PLATE Fy = 50.0000 ksi MIN.
 COLD FORMED SHAPES Fy = 57.0000 ksi MIN.
 WALL SHEETING Fy = 60.0000 ksi MIN.
 ROOF SHEETING Fy = 60.0000 ksi MIN.
 BOLTS A307 & A325
 THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

- 2) BOLT TIGHTENING REQUIREMENTS:
 ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT" METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT ONLY.

- 3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

ROOF PANELS:

COLOR: Galvalume +

WALL PANELS:

COLOR: NEED SIG 200

TRIM COLORS:

CABLE: NEED SIG 200

CORNER: NEED SIG 200

EAVE: NEED SIG 200

FRAMED OPENINGS: NEED SIG 200

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A

DEFLECTION LIMTS:

- EW COL: 180
 EW RAF LIVE: 180
 EW RAF WIND: 180
 WALL GIRT: 90
 PURL LIVE: 180
 PURL WIND: 120
 WALL PANEL: 90
 ROOF PANEL LIVE: 180
 ROOF PANEL WIND: 120
 RF HORIZONTAL: 60
 RF VERTICAL: 180
 WIND BENT: 60
 RF CRANE: 100
 RF SEIS: 50
 WIND BENT SEIS: 50

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED. APPROVAL OF THE METAL BUILDING SYSTEM MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD PRACTICES, 9TH ED.)

WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION" DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1 AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS.

A SIGNATURE AND DATE IS REQUIRED ON ALL PAGES.

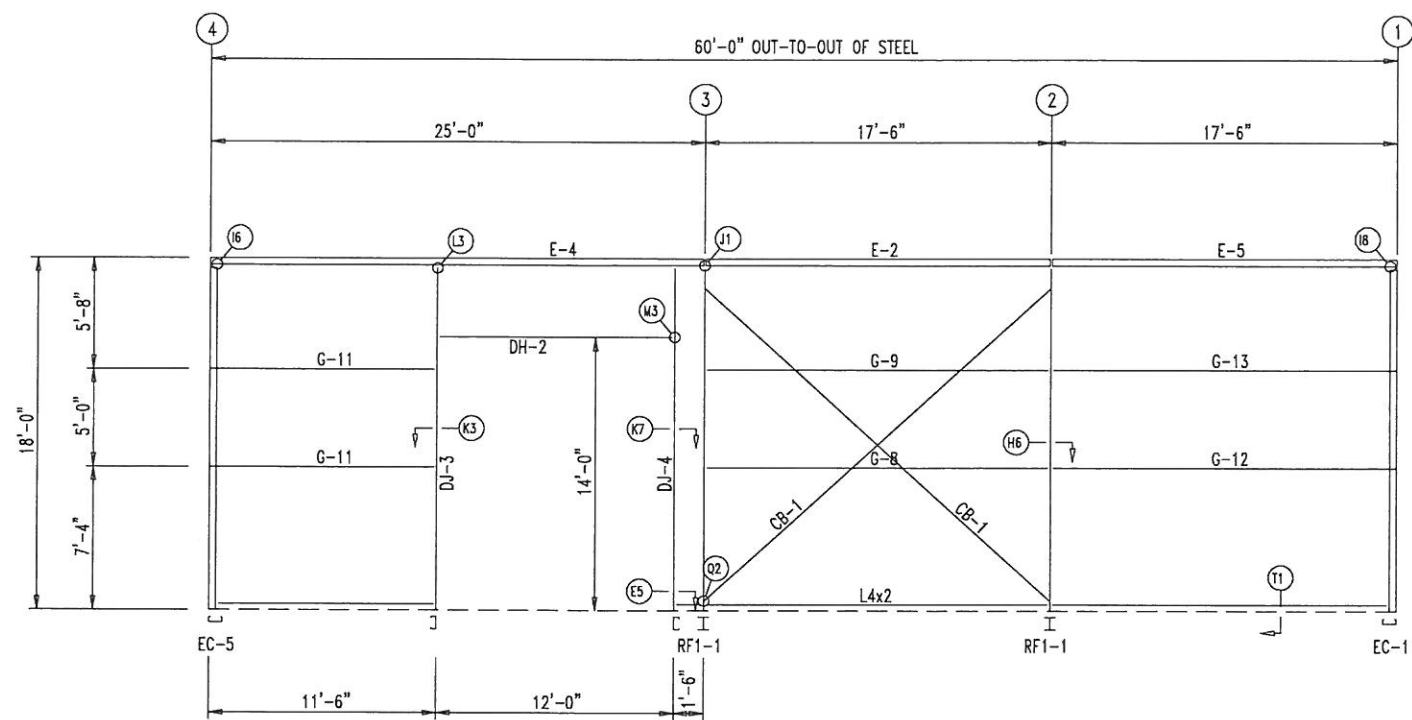
MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE METAL BUILDING SYSTEM MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER. ANY CHANGES NOTED ON THE DRAWINGS NOT IN COMFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATIOIN OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

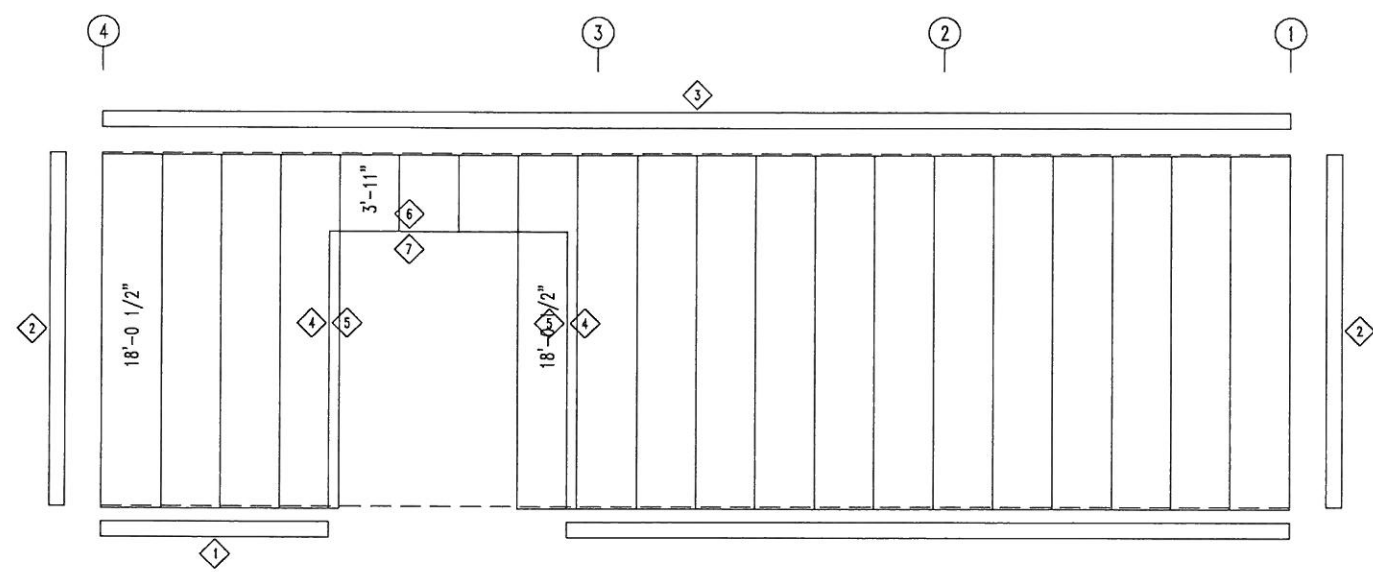
IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL BUILDING MANUFACTURER.

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△		
△		
△	.././.	FOR CONSTRUCTION
△	.././.	FOR APPROVAL
REV.	DATE	REVISION

PURCHASER: DLR
 PROJECT: Bouche Construction
 JOB NUMBER: 022219



SIDEWALL FRAMING: FRAME LINE A



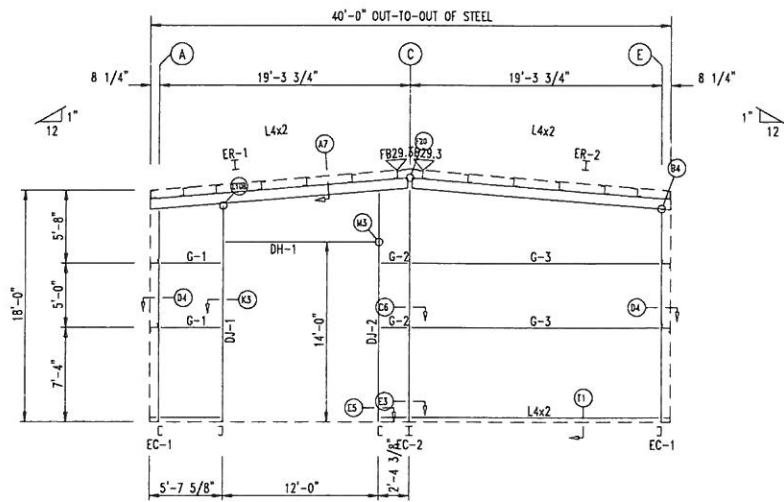
SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. PR - NEED SIG 200

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
DJ-3	8X35c12	17'-4 5/8"
DJ-4	8X35c14	17'-4 5/8"
DH-2	8X25c16	12'-0"
E-2	E085341L	17'-5 1/2"
E-4	E085341L	24'-11 1/2"
E-5	E085341L	17'-5 1/2"
G-8	8X25Z14	16'-9 3/4"
G-9	8X25Z16	16'-9 3/4"
G-11	8X25Z16	11'-2"
G-12	8X25Z12	17'-1 5/8"
G-13	8X25Z16	17'-1 5/8"
CB-1	HW374	24'-6 1/4"

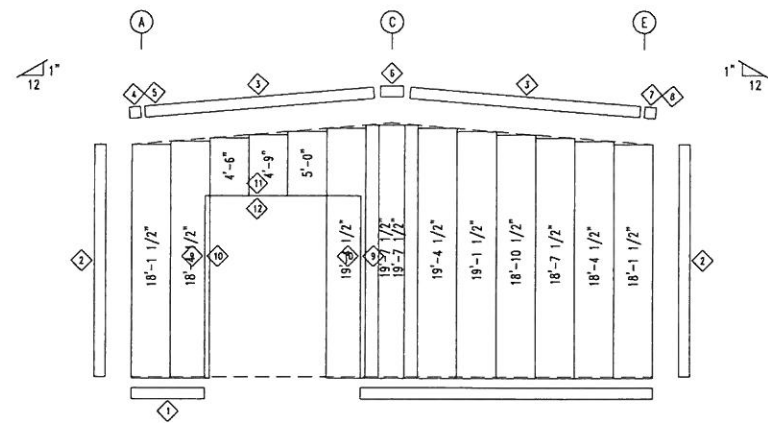
TRIM TABLE FRAME LINE A		
ID	MARK	DETAIL
1	FL72	TRIM_74
2	FL834	TRIM_198
3	FL19A	TRIM_316
4	FL-37	TRIM_242
5	FL23C	TRIM_239
6	FL-37	TRIM_235
7	FL26B	TRIM_232



DESCRIPTION: SIDEWALL FRAMING							
CUSTOMER: DLR				PROJECT: Bouche Construction			
LOCATION: PRJ Address 2							
DRN. BY DET	CK'D BY DES	DATE 2/28/19	SCALE N.T.S.	REV. 00	QUOTATION NO. 022219	SHEET NO. OF	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Gg. PR - NEED SIG 200

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	1 3/4"
Columns/Raf	2	A325	5/8"	2"
Jamb	2	A325	5/8"	2"

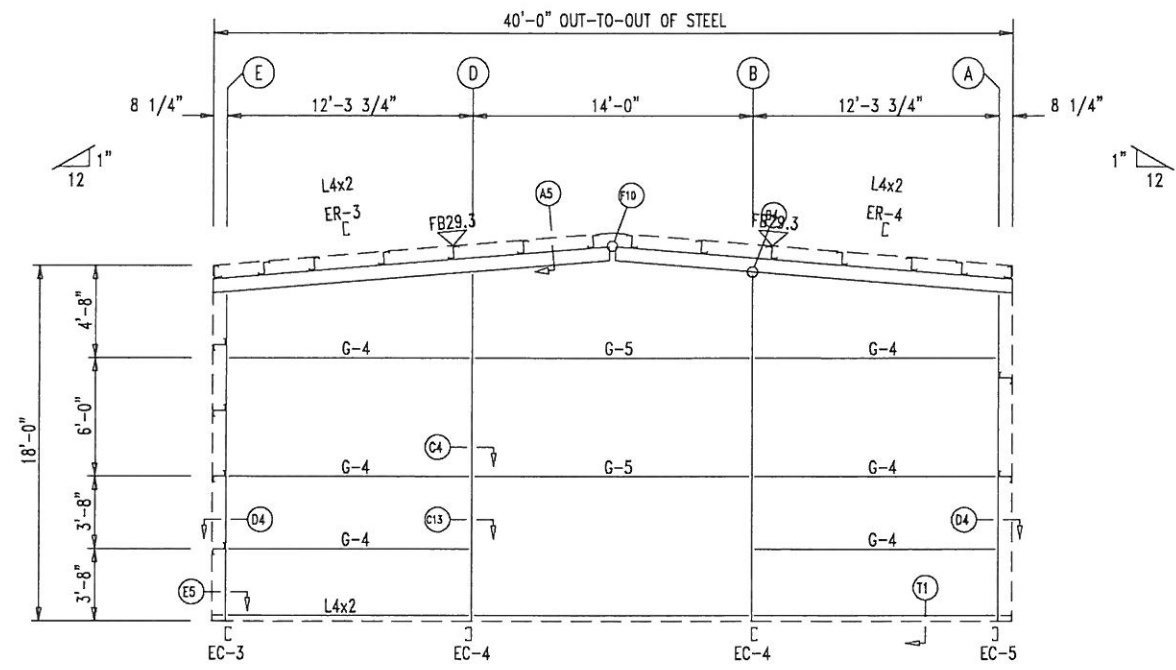
MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	8X35c12	16'-6 5/8"
EC-2	WBX10	17'-11 11/16"
ER-1	WBX10	20'-0 9/16"
ER-2	WBX10	20'-0 9/16"
DJ-1	10X35c14	16'-11 9/16"
DJ-2	10X35c14	17'-11 9/16"
DH-1	10X25c14	12'-0"
G-1	8X25Z16	4'-3 5/8"
G-2	8X25Z16	1'-8 5/8"
G-3	8X25Z12	18'-7 3/4"

TRIM TABLE		
FRAME LINE 1		
ID	MARK	DETAIL
1	FL72	TRIM_74
2	FLB34	TRIM_198
3	FL16D	TRIM_229
4	MTR LT	
5	LEFT	
6	FL16B	
7	MTR RT	
8	RIGHT	
9	SPCL01	TRIM_242
10	FL23C	TRIM_239
11	SPCL01	TRIM_235
12	FL28B	TRIM_232

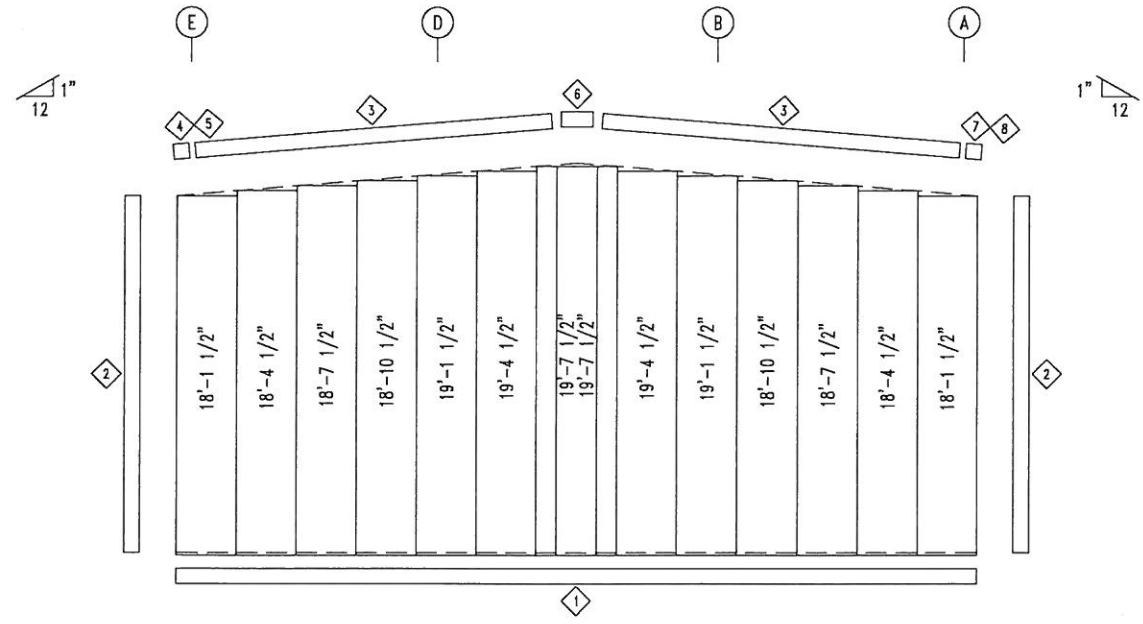
FLANGE BRACE TABLE		
FRAME LINE 1		
ID	MARK	LENGTH
1	FB29.3	2'-5 1/4"



DESCRIPTION: ENDWALL FRAMING							
CUSTOMER: DLR				PROJECT: Bouche Construction			
LOCATION: PRJ Address 2							
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.	
DET	DES	2/28/19	N.T.S.	00	022219	OF	



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4
PANELS: 26 Ga. PR - NEED SIG 200

BOLT TABLE				
FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-3/ER-4	4	A325	5/8"	1 3/4"
Columns/Raf	2	A325	5/8"	2"

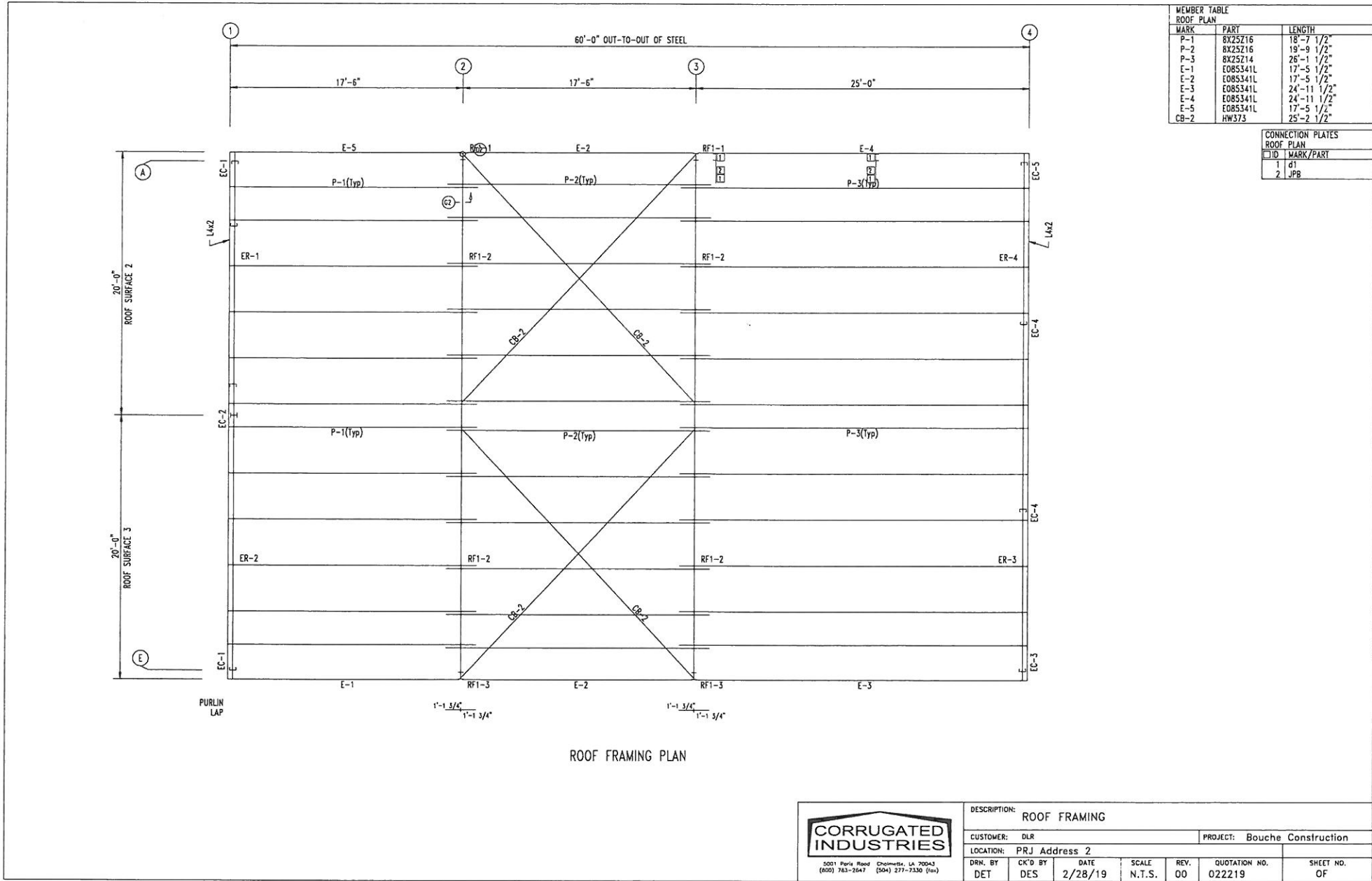
MEMBER TABLE		
FRAME LINE 4		
MARK	PART	LENGTH
EC-3	8X35c14	16'-6 1/2"
EC-4	8X35c12	17'-6 13/16"
EC-5	8X35c14	16'-6 1/2"
ER-3	8X35c14	20'-0 9/16"
ER-4	8X35c14	20'-0 9/16"
G-4	8X25Z16	11'-7 3/4"
G-5	8X25Z16	13'-11 1/2"

TRIM TABLE		
FRAME LINE 4		
ID	MARK	DETAIL
1	FL72	TRIM_74
2	FL834	TRIM_198
3	FL16D	TRIM_229
4	MTR LT	
5	LEFT	
6	FL16B	
7	MTR RT	
8	RIGHT	

FLANGE BRACE TABLE		
FRAME LINE 4		
ID	MARK	LENGTH
1	FB29.3	2'-5 1/4"



DESCRIPTION: ENDWALL FRAMING							
CUSTOMER: DLR				PROJECT: Bouche Construction			
LOCATION: PRJ Address 2							
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.	
DET	DES	2/28/19	N.T.S.	00	022219	OF	



MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z16	18'-7 1/2"
P-2	8X25Z16	19'-9 1/2"
P-3	8X25Z14	26'-1 1/2"
E-1	E085341L	17'-5 1/2"
E-2	E085341L	17'-5 1/2"
E-3	E085341L	24'-11 1/2"
E-4	E085341L	24'-11 1/2"
E-5	E085341L	17'-5 1/2"
CB-2	HW373	25'-2 1/2"

CONNECTION PLATES		
ROOF PLAN		
ID	MARK	PART
1	d1	
2	JPB	

ROOF FRAMING PLAN



DESCRIPTION: ROOF FRAMING							
CUSTOMER: DLR				PROJECT: Bouche Construction			
LOCATION: PRJ Address 2							
DRN. BY	CK'D BY	DATE	SCALE	REV.	QUOTATION NO.	SHEET NO.	
DET	DES	2/28/19	N.T.S.	00	022219	OF	