ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Louisiana Engineering Certificate of Authorization Number: EF.0004093
 Document ID:1V1H7634Z0119080357

Truss Fabricator:	Evergreen Lumber & Truss, Inc.
Job Identification:	MW730-/MW /MW730 -- Slidell, LA
Truss Count:	10
Model Code:	IRC
Truss Criteria:	IRC2009/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 12.03.
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration Floor - N/A Wind - 155 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. As shown on attached drawings; the drawing number is preceded by: HCUSR7634

Details: BRCLBSUB-GBLLETIN-

11/19/2013

-Truss Design Engineer-
William H. Krick

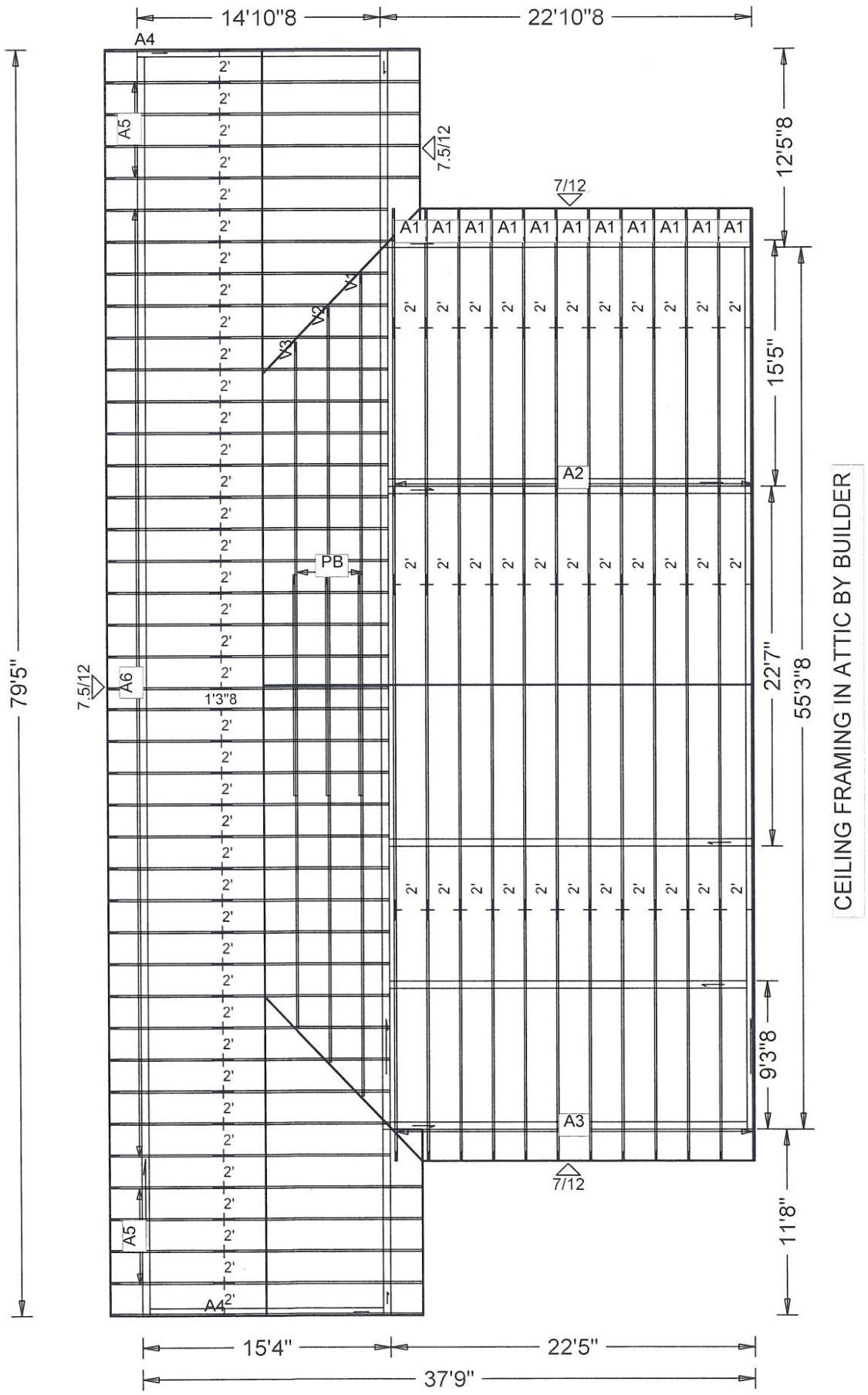
1950 Marley Drive
Haines City, FL 33844

#	Ref Description	Drawing#	Date
1	04583--A1	13323001	11/19/13
2	04584--A2	13323002	11/19/13
3	04585--A3	13323002	11/19/13
4	04586--A4	13323004	11/19/13
5	04587--A5	13323005	11/19/13
6	04588--A6	13323003	11/19/13
7	04589--PB	13323001	11/19/13
8	04590--V1	13323003	11/19/13
9	04591--V2	13323004	11/19/13
10	04592--V3	13323005	11/19/13

SUBMITTALS REVIEWED AND APPROVED
MCDONALD CONSTRUCTION INC. OF SLIDELL

S. M. Cmol

DATE 22 Nov 13



CUSTOMER: Fill in later
DESCRIPTION: St. Tammany Fire
DESIGNER: Michael Whitmire
Date Printed:: 11-19-2013
ADDRESS:
SALESMAN: Fill in later

JOB NO:
MW730

PAGE NO:
1 OF 1

(MW730-/MW /MW730 -- Slidell, LA - A1)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

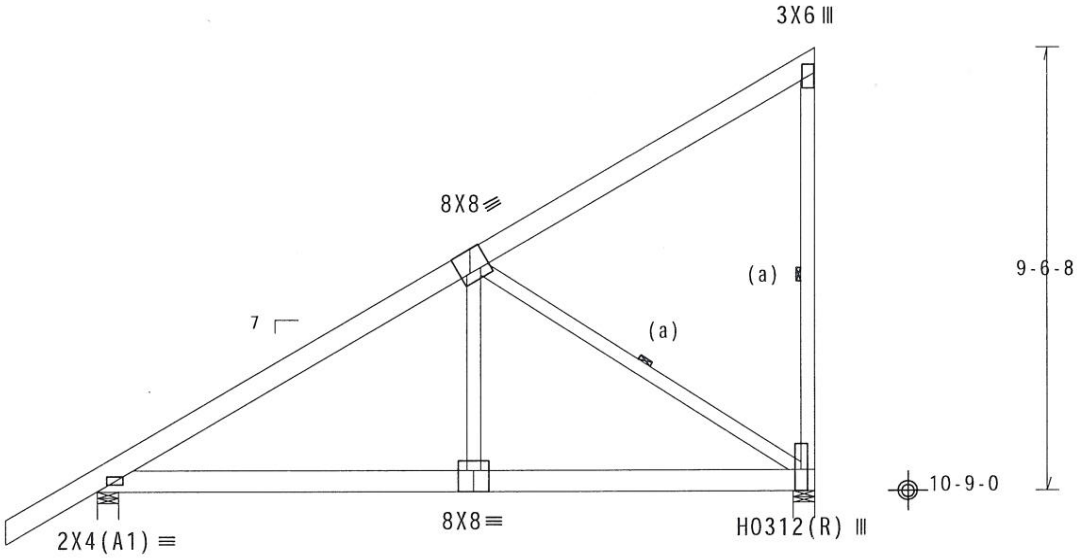
Unbalanced snow loads have not been considered.

155 mph wind, 15.22 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.



(MW730-/MW /MW730 -- Slidell, LA - A1)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave | FT/RT=20%(0%)/10(0) | 12.03.05.0416.16 | QTY:22 LA/-/1/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4583
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323001
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	192206
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - A2)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

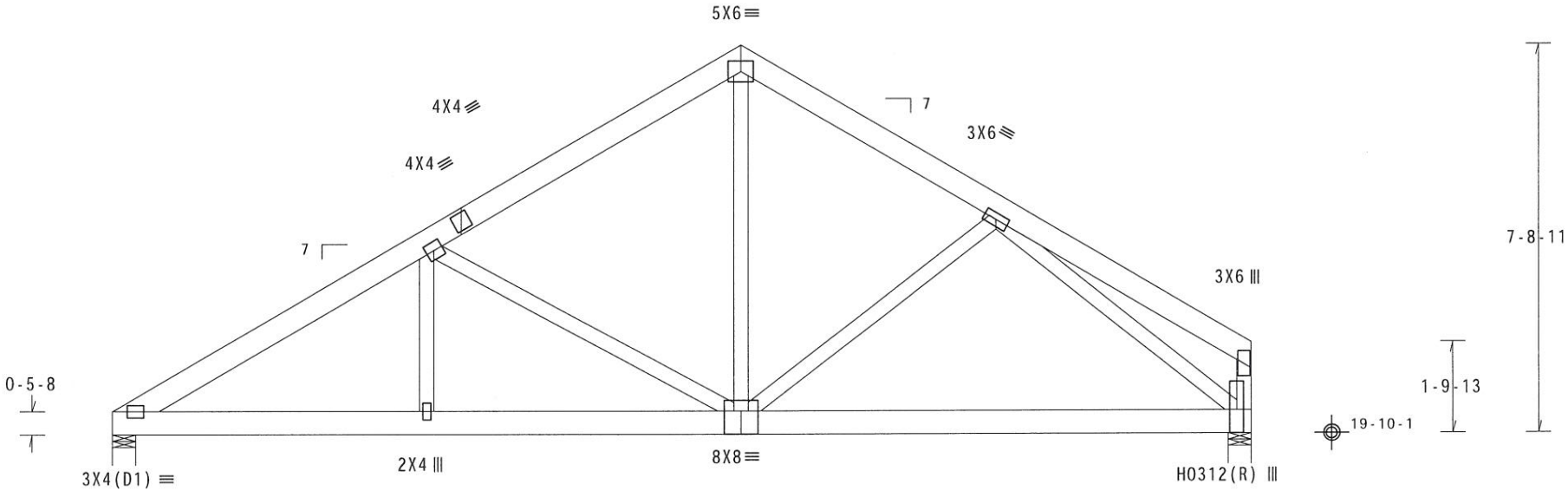
Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

155 mph wind, 23.93 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.



(MW730-/MW /MW730 -- Slidell, LA - A2)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.17 QTY:12 LA/-/1/-/-/R/- Scale =.3125"/Ft.

ALPINE
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Orlando FL, 32837

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WILLIAM H. KRICK
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PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4584
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323002
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	184041
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - A3)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Right cantilever is exposed to wind

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

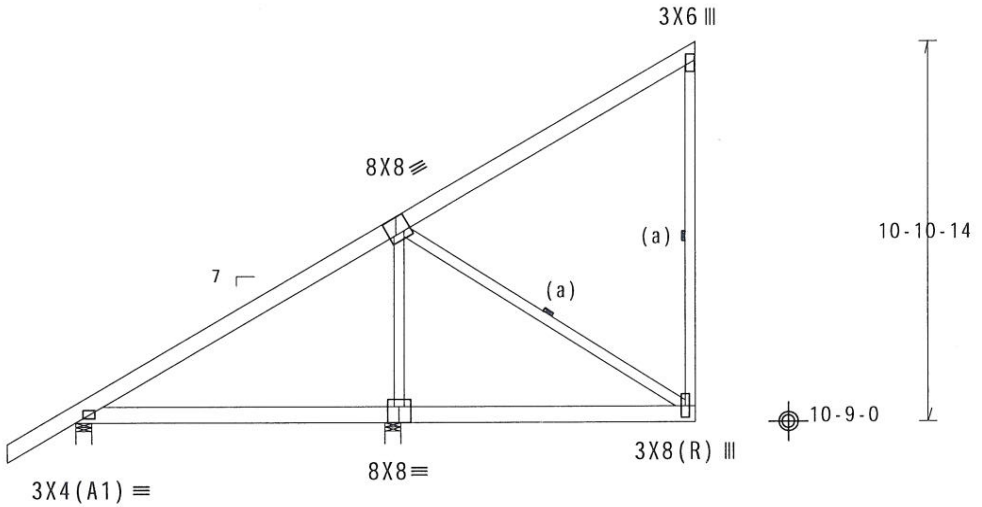
155 mph wind, 15.90 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Right end vertical not exposed to wind pressure.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.



1-11-12 | 8-5-8 | 17-9-0 Over 2 Supports

R=1398 U=3145 W=5.5"

R=370 Rw=837 U=164 W=5.5"
RL=1279/-288

(MW730-/MW /MW730 -- Slidell, LA - A3)

Design Crit: IRC2009/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

12.03.05.0416.16

QTY:12 LA/-/1/-/-/R/-

Scale =.1875"/Ft.

ALPINE
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Orlando FL, 32837

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PROFESSIONAL ENGINEER
Nov 19 2013

TC LL	20.0 PSF	REF	R7634- 4585
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323002
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	192201
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - A4)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

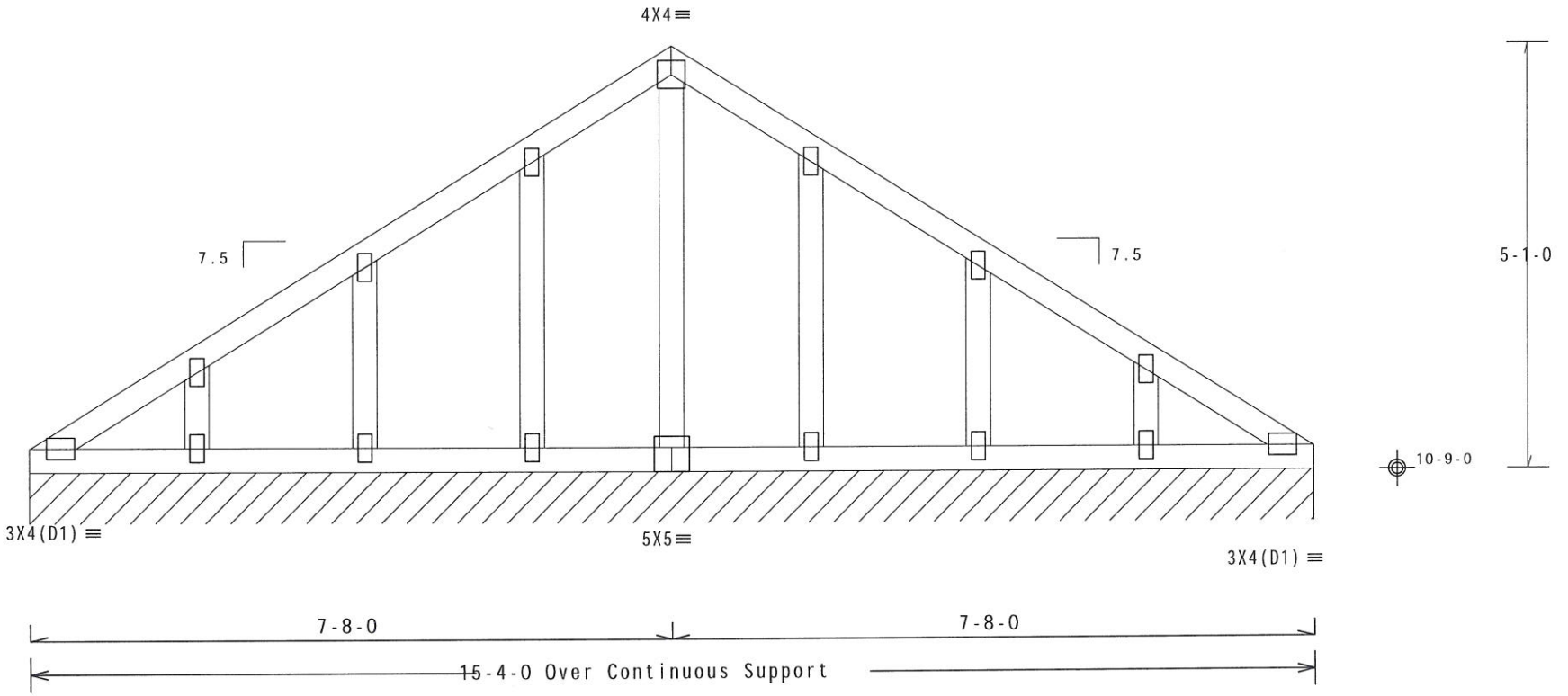
Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.

155 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

See DWG GBLLETIN0212 for more requirements.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.



R=84 PLF U=99 PLF W=15-4-0
RL=22/-22 PLF

Note: All Plates Are 2X4 Except As Shown.

(MW730-/MW /MW730 -- Slidell, LA - A4)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0)

12.03.05.0416.17

QTY:2

LA/-/1/-/-/R/-

Scale =.5"/Ft.

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STATE OF LOUISIANA
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PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4586
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323004
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	184101
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - A5)

Top chord 2x6 SP #2-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

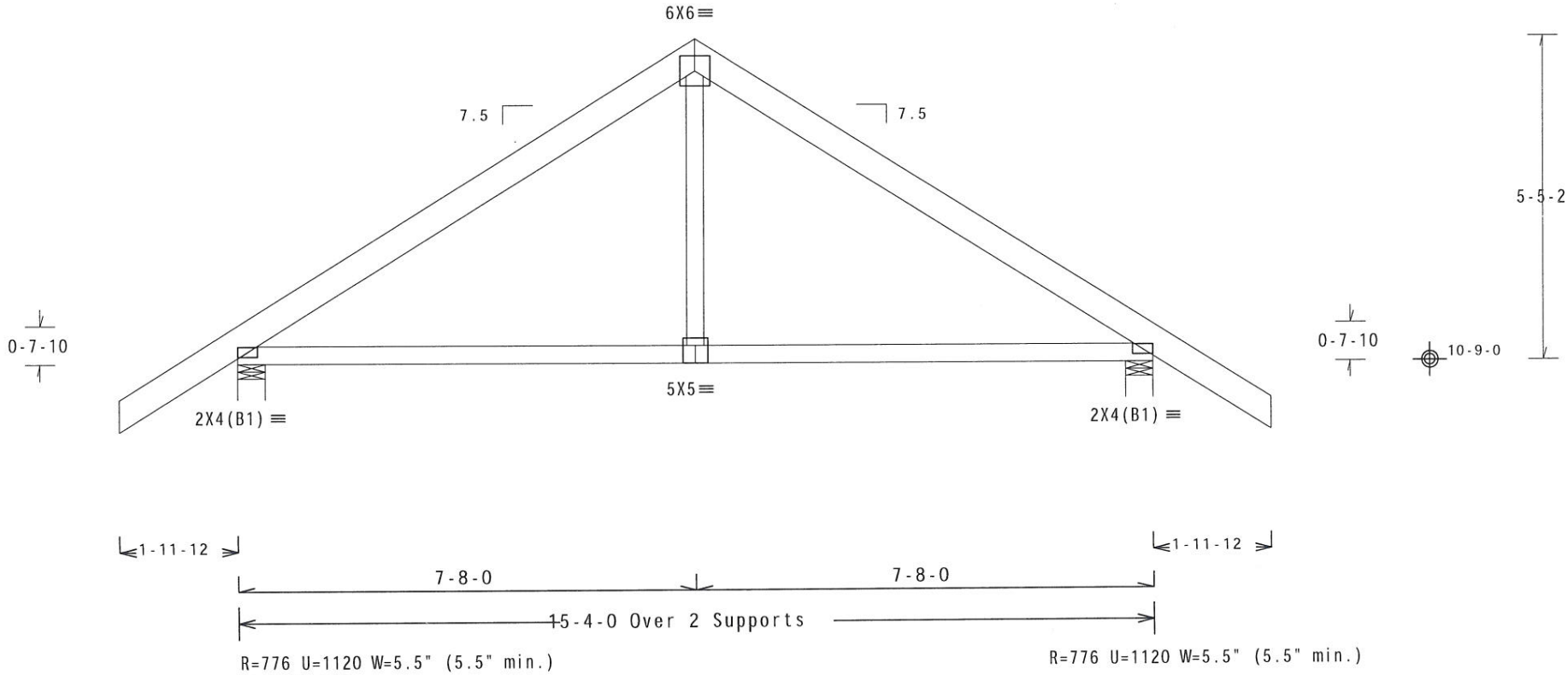
Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

155 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.



(MW730-/MW /MW730 -- Slidell, LA - A5)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.17 QTY:8 LA/-/1/-/-/R/- Scale =.375"/Ft.

ALPINE
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PROFESSIONAL ENGINEER
Nov 19, 2013

TC LL	20.0 PSF	REF	R7634- 4587
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323005
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	184051
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - A6)

Top chord 2x6 SP #2-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

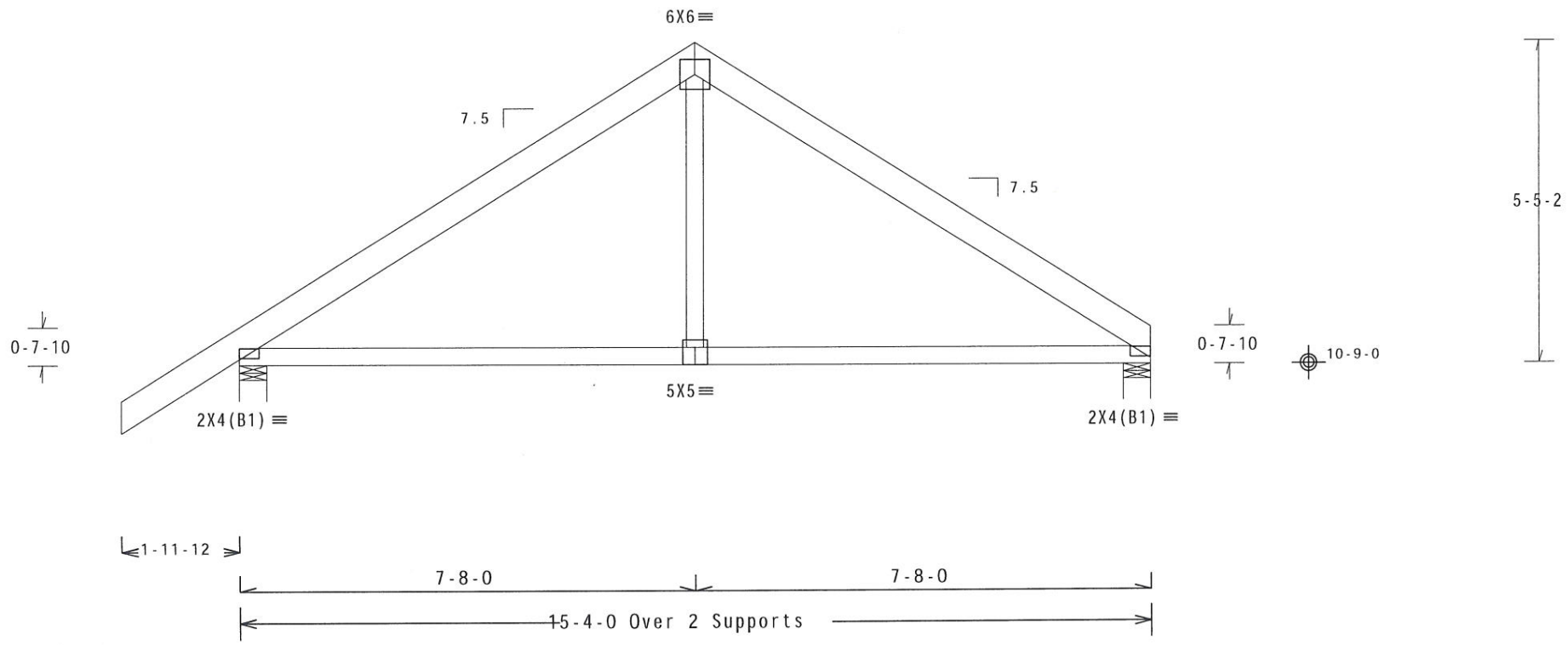
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

155 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.



(MW730-/MW /MW730 -- Slidell, LA - A6)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.17 QTY:31 LA/-/1/-/-/R/- Scale =.375"/Ft.

ALPINE
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Orlando FL, 32837

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19, 2013

TC LL	20.0 PSF	REF	R7634- 4588
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323003
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	184055
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - PB)

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Negative reaction(s) of -230# MAX. (See below) from a non-wind load case requires uplift connection.

Wind loads based on MWFRS with additional C&C member design & reactions.

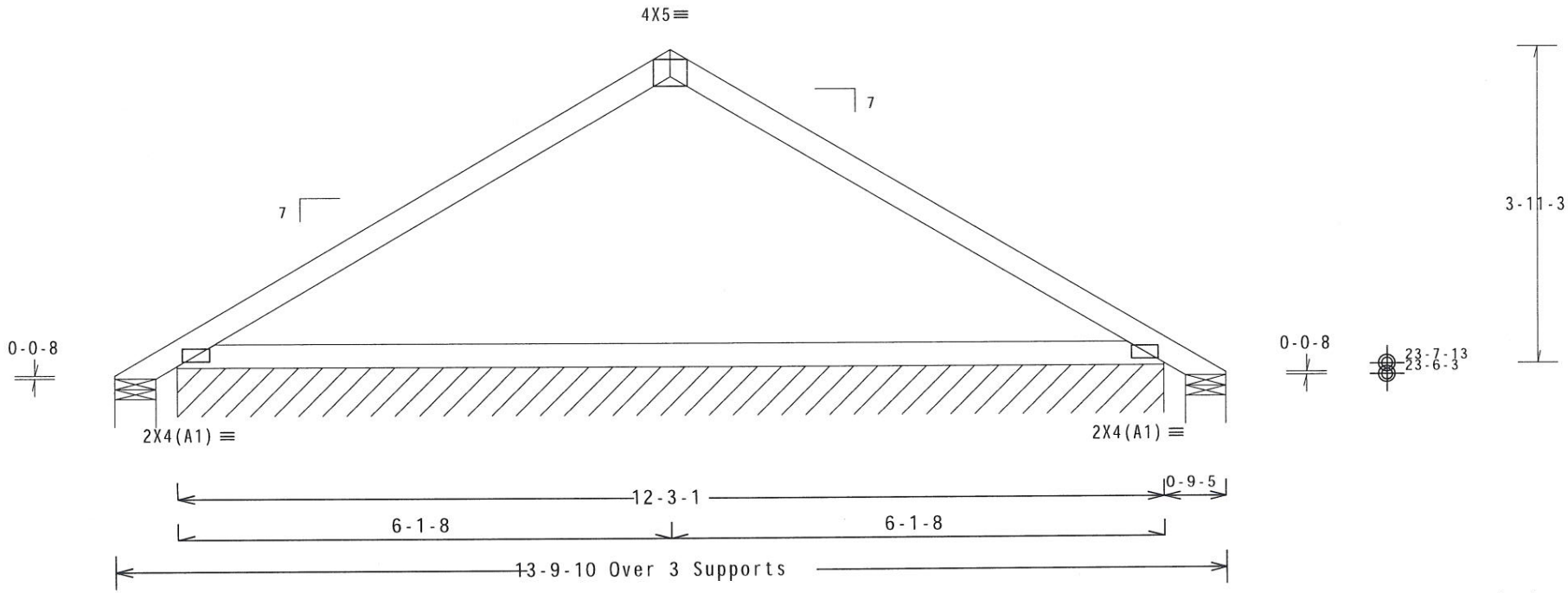
Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 63 plf at -0.78 to 63 plf at 6.13
TC- From 63 plf at 6.13 to 63 plf at 13.03
BC- From 4 plf at -0.78 to 4 plf at 13.03

155 mph wind, 25.57 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.



R=-230 Rw=446 U=441 W=6.089" (6.089" min.)
RL=303 R=306 PLF U=203 PLF W=12-3-1

R=-230 Rw=443 U=305 W=6.089" (6.089" min.)

(MW730-/MW /MW730 -- Slidell, LA - PB)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.17 QTY:3 LA/-/1/-/-/R/- Scale =.5"/Ft.

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4589
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323001
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	184095
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - V1)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

155 mph wind, 18.23 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

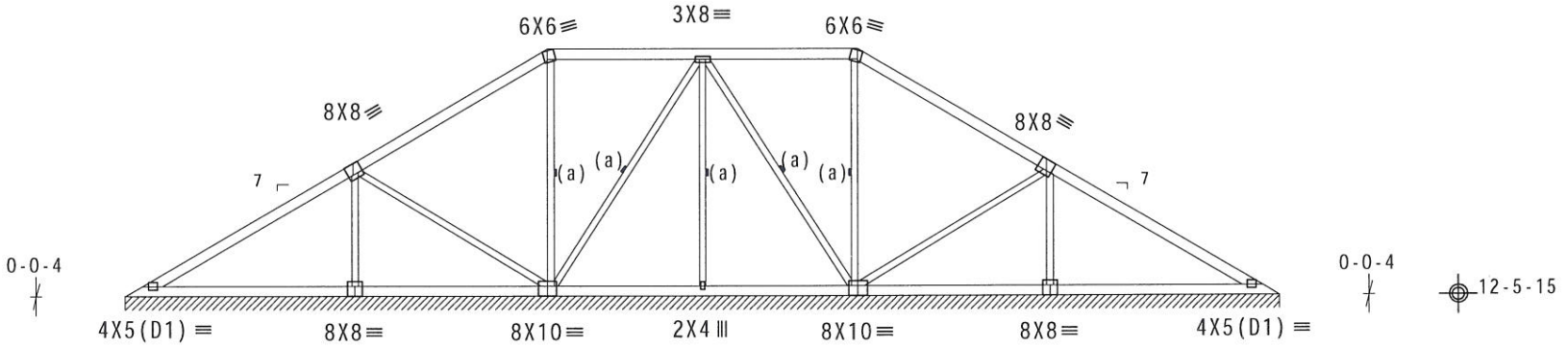
TC-	From	53 plf	at	0.75	to	53 plf at 18.86
TC-	From	53 plf	at	18.86	to	53 plf at 32.73
TC-	From	53 plf	at	32.73	to	53 plf at 50.84
BC-	From	70 plf	at	0.00	to	70 plf at 0.46
BC-	From	20 plf	at	0.46	to	20 plf at 51.13
BC-	From	70 plf	at	51.13	to	70 plf at 51.59

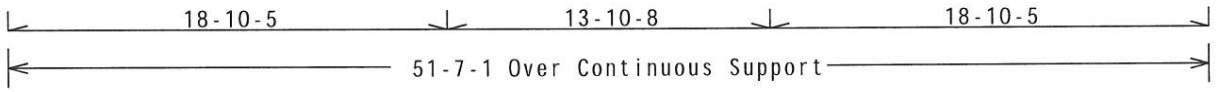
(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 9.12 ft. from roof edge.





R=89 PLF U=89 PLF W=51-7-1
RL=16/-16 PLF

(MW730-/MW /MW730 -- Slidell, LA - V1)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.16 QTY:1 LA/-/1/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4590
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323003
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	192225
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - V2)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

155 mph wind, 18.86 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC-	From	53 plf	at	0.75	to	53 plf at 16.71
TC-	From	53 plf	at	16.71	to	53 plf at 30.59
TC-	From	53 plf	at	30.59	to	53 plf at 46.55
BC-	From	70 plf	at	0.00	to	70 plf at 0.46
BC-	From	20 plf	at	0.46	to	20 plf at 46.84
BC-	From	70 plf	at	46.84	to	70 plf at 47.30

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

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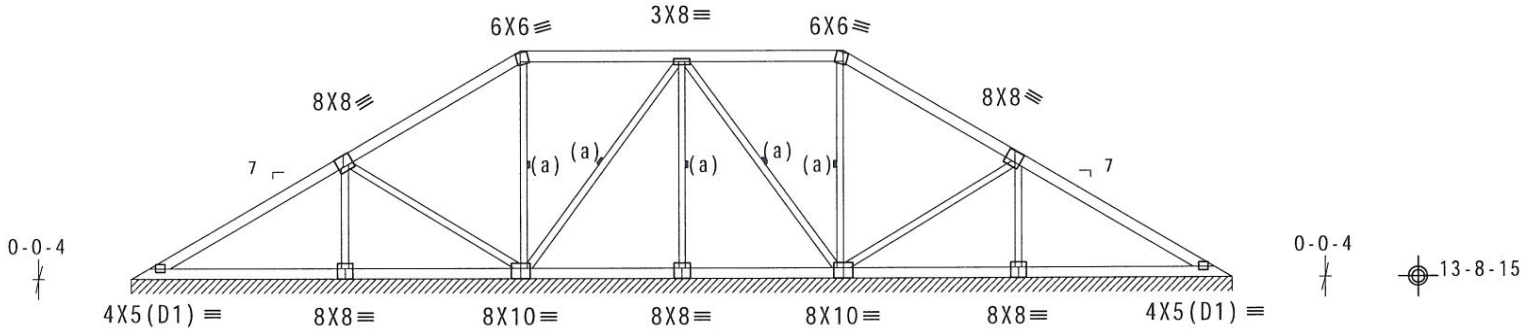
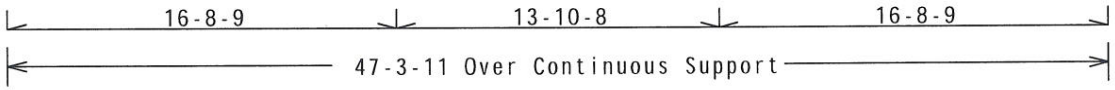
MWFRS loads based on trusses located at least 9.43 ft. from roof edge.





R=88 PLF U=90 PLF W=47-3-11
RL=15/-15 PLF

(MW730-/MW /MW730 -- Slidell, LA - V2)
Design Crit: IRC2009/TPI-2007(STD)
PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.16 QTY:1 LA/-/1/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837

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STATE OF LOUISIANA
WILLIAM HAKRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19 '13

TC LL	20.0 PSF	REF	R7634- 4591
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323004
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	192221
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

(MW730-/MW /MW730 -- Slidell, LA - V3)

Top chord 2x6 SP #2-13B
Bot chord 2x6 SP #2-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

155 mph wind, 19.48 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT III OR IV, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind loads based on MWFRS with additional C&C member design & reactions.

Deflection meets L/240 live and L/480 total load. Creep increase factor for dead load is 1.50.

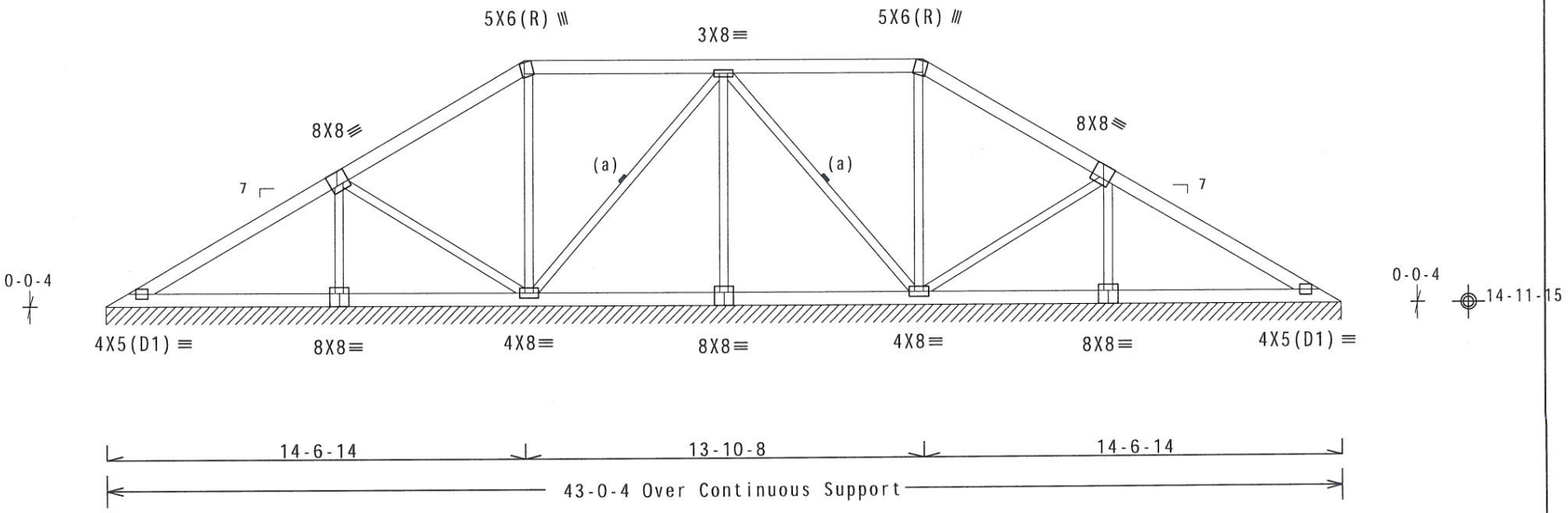
MWFRS loads based on trusses located at least 9.74 ft. from roof edge.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 53 plf at 0.75 to 53 plf at 14.57
TC- From 53 plf at 14.57 to 53 plf at 28.45
TC- From 53 plf at 28.45 to 53 plf at 42.27
BC- From 70 plf at 0.00 to 70 plf at 0.46
BC- From 20 plf at 0.46 to 20 plf at 42.55
BC- From 70 plf at 42.55 to 70 plf at 43.02

(a) Continuous lateral restraint equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Truss designed for unbalanced snow load based on Pg=20.00 psf, Ct=1.10, Ce=1.00, CAT III & Pf=16.94 psf.



R=87 PLF U=91 PLF W=43-0-4
RL=14/-14 PLF

(MW730-/MW /MW730 -- Slidell, LA - V3)

Design Crit: IRC2009/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.16 QTY:1 LA/-/1/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Orlando FL, 32837

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER
Nov 19, 2013

TC LL	20.0 PSF	REF	R7634- 4592
TC DL	10.0 PSF	DATE	11/19/13
BC DL	10.0 PSF	DRW	HCUSR7634 13323005
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	192217
DUR.FAC.	1.25	FROM	MW
SPACING	24.0"	JREF-	1V1H7634Z01

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

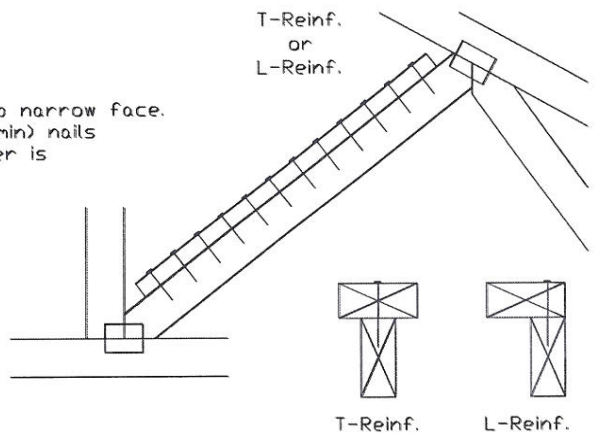
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

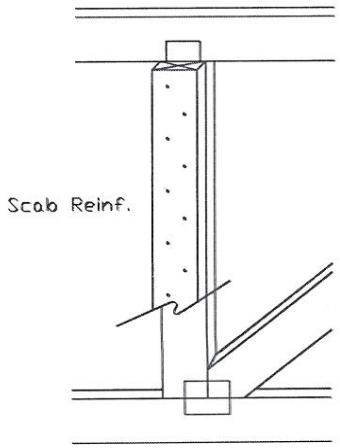
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Building Components Group Inc.

Earth City, MO 63045

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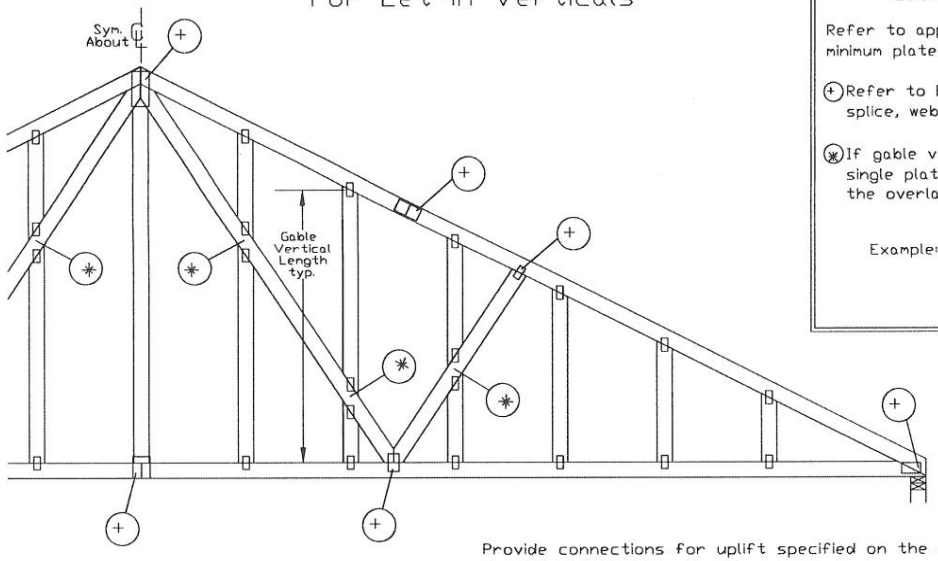
STATE OF LOUISIANA
WILLIAM M. KRICK
License No. 36852
PROFESSIONAL ENGINEER
8/15/13

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 8/15/13
BC DL	PSF	DRWG BRCLBSUB0813
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Detail
For Let-in Verticals



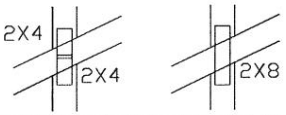
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

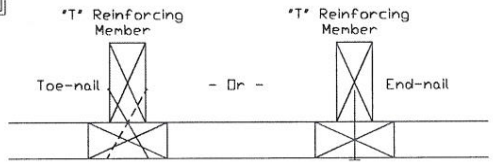
(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail

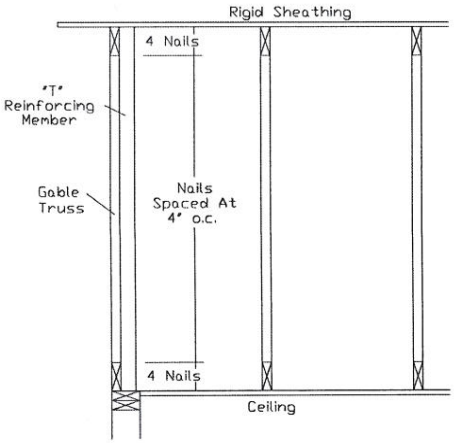


Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, A16015ENC100212,
A18015ENC100212, A20015ENC100212, A20015END100212, A20015PED100212,
A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212,
A18030ENC100212, A20030ENC100212, A20030END100212, A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Building Components Group Inc.

Earth City, MO 63045

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STATE OF LOUISIANA
WILLIAM H. KRICK
License No. 36852
PROFESSIONAL ENGINEER

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212