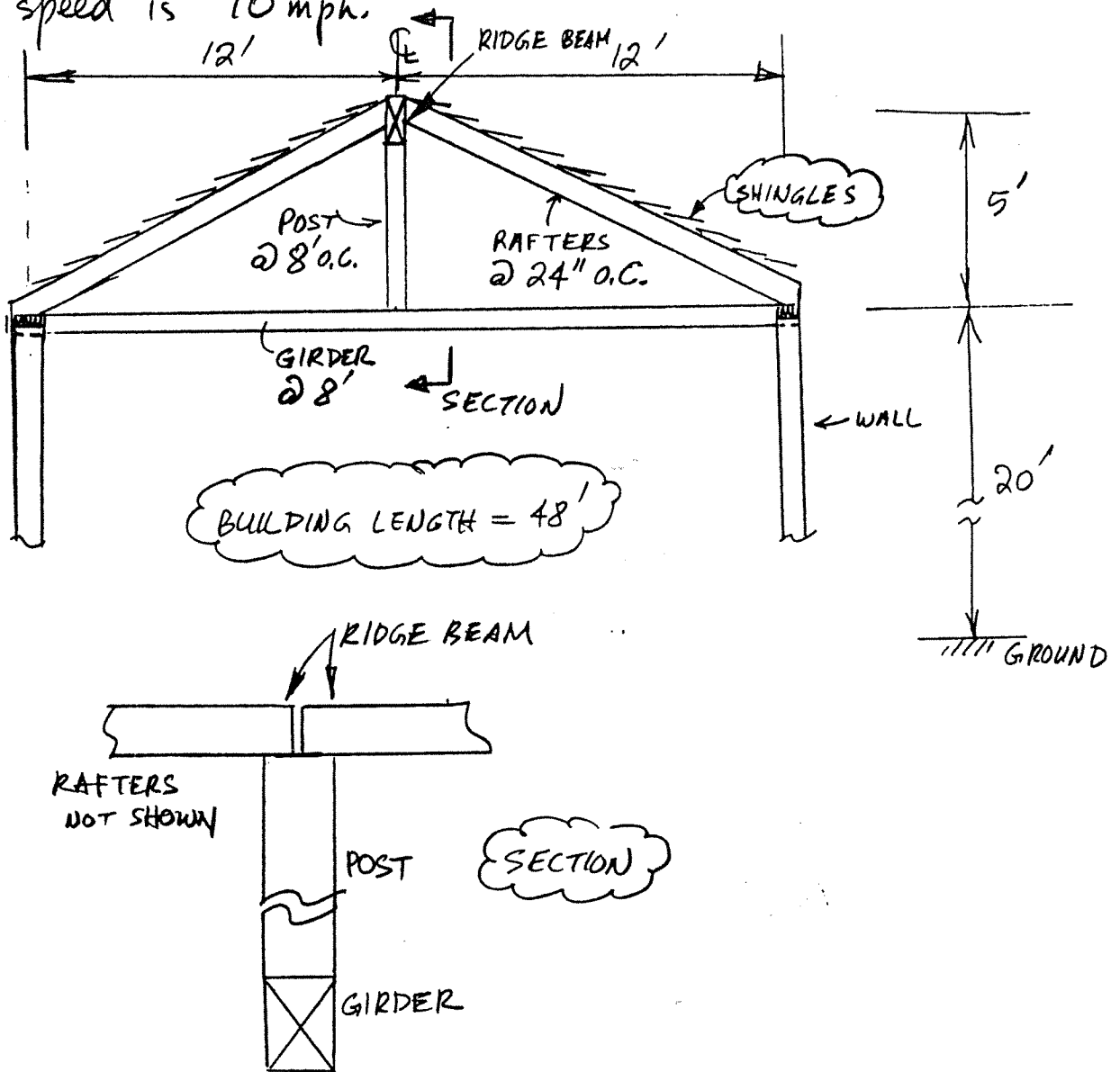


EXAMPLE

The shown timber dwelling has a roof consisting of rafters placed at 24 inch intervals and supported at the centerline by a ridge beam. The ridge beam is supported by posts placed at 8 foot intervals. Each post is supported by a girder. All connections are simple, that is, they do not transfer moment.

Roof dead load = 20 psf (includes self weight of framing).
The structure is located in an area that is densely forested with ground snow load of 40 psf. Basic wind speed is 70 mph.



Determine:

- a) Maximum bending moment in rafters
- b) Maximum axial load in post
- c) Maximum bending moment in girder

Consider dead load, dead load + snow load, or dead load + wind load whichever dominates in each case.

Directions:

Load is collected by the rafters and transferred to the ridge beam and supporting wall. Ridge beam is supported by posts, which are supported by girders. This represents the load path. It will be useful in sequencing your analysis.

Note that different loading combinations may be critical for the various members. For example, dead plus unbalanced snow load may be critical for one member, whereas dead plus balanced snow load may be critical for another member.

You do not have to do calculations for every single case of loading and combination of loadings. Use engineering judgement to exclude some of these loadings (however, explain the rationale).

SNOW LOAD CALCULATIONS (PER ASCE 7-93)

$p_g = 40 \text{ psf}$, $C_e = 1.2$ (densely forested) , $C_t = 1.0$ (dwelling)
 $I = 1$ regular structure

$p_f = 0.7 C_e C_t I p_g = 0.7 \times 1.2 \times 1.0 \times 1.0 \times 40 = 33.6 \text{ psf}$

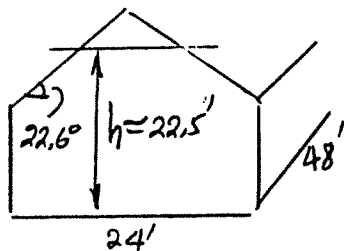
SLOPE 5 ON 12 OR 22.6° , NON-SLIPPERY - WARM ROOF $\Rightarrow C_s = 1.0$

BALANCED SNOW LOAD $P_s = \underline{33.6 \text{ psf}}$

UNBALANCED $\frac{1.5 p_s}{C_e} = \frac{1.5 \times 33.6}{1.2} = \underline{42 \text{ psf}}$

WIND LOAD CALCULATIONS (PER ASCE 7-93)

CALCULATE ONLY ROOF PRESSURE/SUCTION



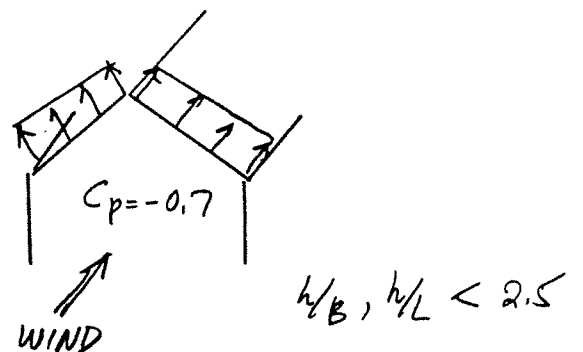
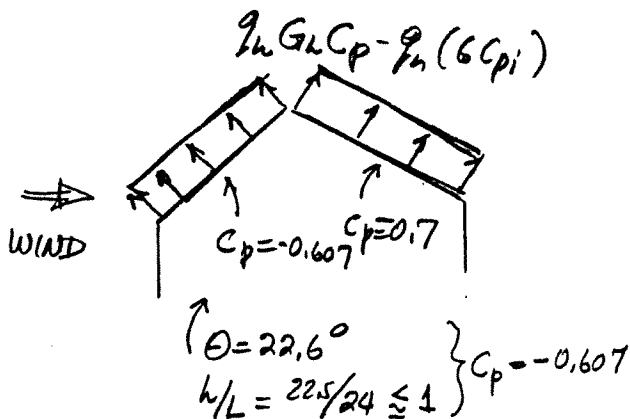
EXPOSURE B : WOODED AREA

$I = 1$ (NO HURRICANES)

$q_w = q_z = q_h = 0.00256 K_z (I V)^2 = 5.52 \text{ psf}$
 (where $K_z = 0.44$ and $V = 70 \text{ mph}$)

$G_w = 1.565$

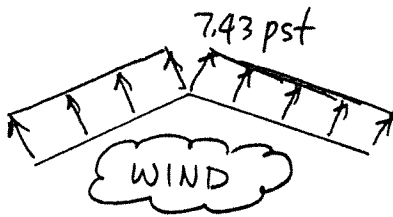
$G C_{pi} = \pm 0.25 \Rightarrow q_w (G C_{pi}) = \pm 1.38 \text{ psf}$



SINCE WIND LOADING ON ROOF IS SUCTION (THAT IS, IT COUNTERACTS EFFECT OF WEIGHT), OF INTEREST IS ITS MAXIMUM VALUE. IT MAY BE LARGE ENOUGH TO REVERSE MOMENT IN RAFTERS AND THUS DOMINATE OVER DEAD + SNOW LOAD (THOUGH UNLIKELY).

WORST CASE IS WIND BLOWING PARALLEL TO RIDGE WITH OVERPRESSURE INTERNALLY:

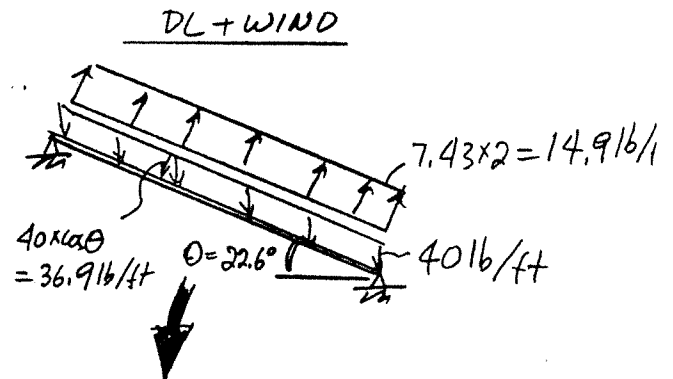
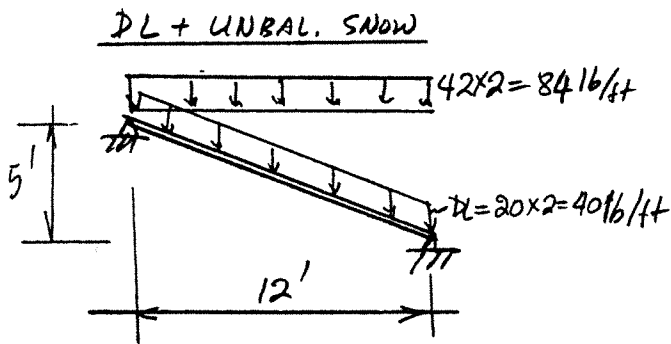
$$\text{SUCTION} = q_h G_h C_p + q_h (G R p_i) = 5.52 \times 1.565 \times 0.7 + 1.38 = \underline{\underline{7.43 \text{ psf}}}$$



THE MINIMUM 10 psf WIND LOADING CAUSED PRESSURE ON ROOF RAFTERS BUT THE EFFECT IS MUCH LESS THAN UNBALANCED SNOW LOAD \Rightarrow NO NEED TO CONSIDER.

RAFTERS

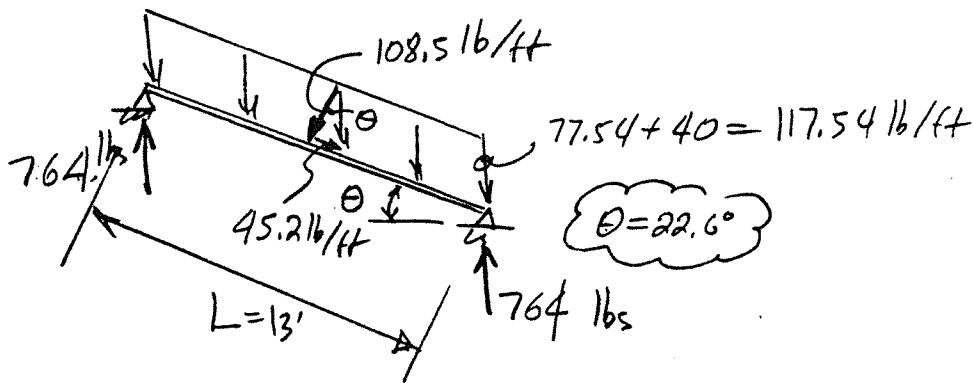
LOADINGS TO CONSIDER



NET EFFECT IS DOWNWARD LOAD ON RAFTER \Rightarrow LESS THAN DL + UN. SNOW.

DONOT CONSIDER WIND.

UNBALANCED S.L. / ACTUAL LENGTH = $84 \times 12 / 13 = 77.54 \text{ lb/ft}$

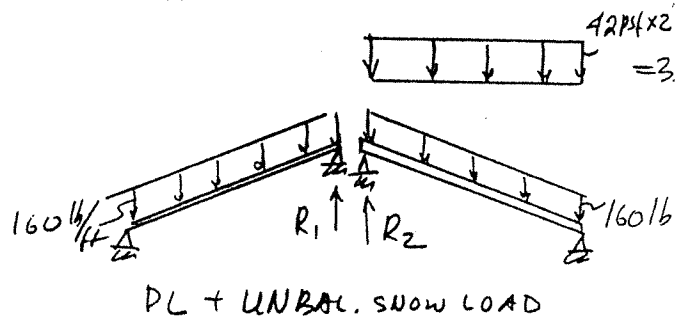
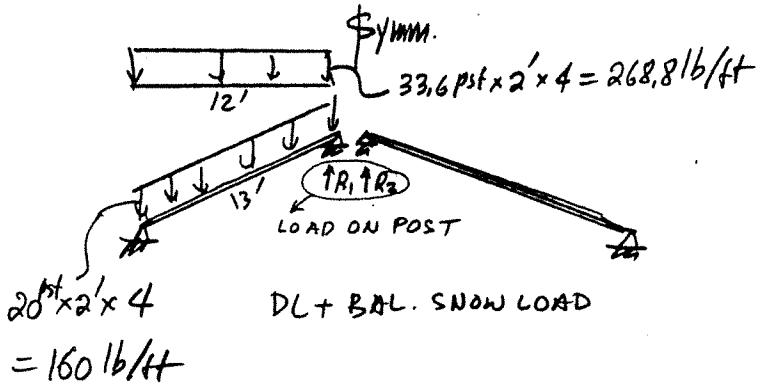


RAFTER SUBJECTED TO PERPENDICULAR LOAD OF $108.5 \text{ lb/ft} = W$

$\Rightarrow M_{\max} = M_c = \frac{WL^2}{8} = \frac{108.5 \times 13^2}{8} = \underline{\underline{2.3 \text{ 1-K}}}$
 MAX. MOMENT IN RAFTER

POST

POST RECEIVES LOAD FROM 4 RAFTERS ON LEFT & 4 RAFTER ON RIGHT
 SPACED AT 8' SPACED AT 2'



$R_1 = R_2 = 268.8 \times 12/2 + 160 \times 13/2$
 $= 2652.8 \text{ lbs}$

$R_1 = 160 \times 13/2 = 1040 \text{ lbs}$
 $R_2 = 160 \times 13/2 + 336 \times 12/2 = 2016$

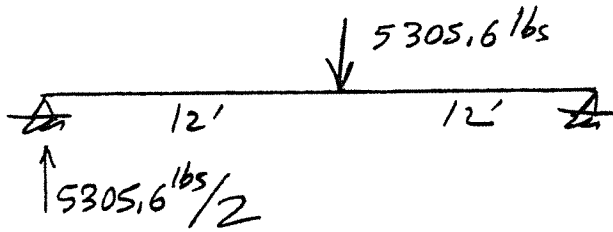
LOAD ON POST = $R_1 + R_2 = 5305.6 \text{ lbs}$

LOAD ON POST = $R_1 + R_2 = 3056 \text{ lbs.}$

\Rightarrow AXIAL FORCE IN POST = 5305.6 lbs COMPR.

GIRDER

GIRDER IS LOADED BY POST AXIAL FORCE



$$\Rightarrow M_{\max} = M_{\text{center}} = \frac{5305.6 \text{ lb} \times 12'}{2} = \underline{\underline{31.8 \text{ k}}}$$

MAX. MOMENT IN GIRDER.