

Date: 11/12/2025

Customer:
 HOPKINS CONSTRUCTION & MAINTENANCE
 19197 COMMISSION RD
 LONG BEACH, MS 39560

Pinnacle Job #: 251599
 Project: Pride Fire District-Indian Mound Station
 Project Location: Central, LA 70739 (East Baton Rouge Parish)
 Project Description: Width Length L.EH R.EH L. Slope
 Multiple Buildings

This is to certify that the above referenced metal building and its components have been designed and fabricated by the metal building manufacturer, Pinnacle Structures Inc., in accordance with the information specified on the order documents. The specified design loads and criteria are applied in accordance with the **2021 International Building Code**. Pinnacle Structures Inc. is an IAS accredited manufacturer maintaining a quality system in compliance with both IAS AC472 criteria and the requirements of Chapter 17 of the International Building Code.

In addition to the dead load of the building components, the members are designed to the following basis:

Building Risk Category	IV - Post								
Collateral Loads*	C	6.00	psf	Others:					
Roof Live Load	L _r	20.00	psf	(4) Roof Hoods each weighing maximum of 400lbs					
(Reducible as permitted by code)									
Roof Snow Load Data									
Ground Snow Load	P _g	0.00	psf						
Flat-Roof Snow Load	P _f	0.00	psf						
Snow Exposure Factor	C _e	1.00							
Snow Importance Factor	I _s	1.20		Drift Surcharge Load(s)	P _d	N/A			
Thermal Factor	C _t	1.00		Width of Snow Drift(s)	w	N/A			
Wind Design Data									
Ultimate Design Wind Speed (3-second gust)	V _{ult}	136	mph			Wind Exposure	B		
Nominal Design Wind Speed	V _{asd}	105	mph			Internal Pressure Coefficient	±0.18		
Rain Intensity									
	i	10.0	in/hr						
Earthquake Design Data									
Analysis Procedure				<i>Equivalent Lateral Force Procedure</i>					
Seismic Importance Factor				I _e	1.50	Design Base Shear V			
Mapped Spectral Response Acceleration Parameters				S _s	0.089	S ₁	0.058	Transverse Direction	2.58 kips
Design Spectral Response Acceleration Parameters				S _{DS}	0.095	S _{D1}	0.093	Longitudinal Direction	1.46 kips
Site Class D				Seismic design category C					
Basic Seismic Force-Resisting Systems (SFRS)				C _s		R			
Transverse	Steel Ordinary Moment Frame(s)			0.048	3.00				
Left Endwall	Steel Ordinary Moment Frame			0.048	3.00		Cs: <i>Seismic Response Coefficient.</i>		
Right Endwall	Steel Ordinary Moment Frame			0.048	3.00		R: <i>Response Modification Coefficient.</i>		
Front Sidewall	Steel Ordinary Moment Frame			0.048	3.00				
Back Sidewall	Steel Ordinary Moment Frame(s)			0.048	3.00				

The buyer and/ or Engineer of Record for the Project is responsible to verify specified loads are in compliance with the local regulatory authorities and report any changes or deviations from the order documents to metal building manufacturer.

This project is designed as **enclosed**. Exterior wall component and cladding materials not specifically supplied by Pinnacle Structures, Inc. should be designed to withstand 24.73/-26.79 psf in the field zone. Additional wind pressure / suction for other zones are available upon request.

*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.

This project is designed using metal building manufacturer's standard serviceability standards in accordance with 2012 MBMA Manual criteria unless specified otherwise on the order documents.

This Letter of Certification applies solely to the structural framing and its component parts as furnished by the metal building manufacturer and as specified in the contract.

The undersigned engineer does not serve as or represent the Engineer of Record for the overall project.

Sincerely,



11/25/2025

GENERAL NOTES

1. This structure has been designed in accordance with the 2016 AISI NAUS Cold Formed Steel Design Manual and the AISI (16th Edition, ASD) Steel Construction Manual.

2. Fabrication shall be accordance with Pinnacle Standards in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1".

3. Materials	ASTM Designation	Minimum Yield
Hot Rolled Angle	A36	Fy = 36 ksi
Structural Steel Plate	A572, A529, A1011	Fy = 55 ksi
Cold Formed Shapes	A1011/(A653 Galvanized)	Fy = 55 ksi
Cable Bracing	A475 (7-Wire Strand)	Ex. High Strength
Rod Bracing	A529 - GR 50	Fy = 50 ksi
Roof & Wall Sheeting	A792 26 GA	80 ksi, Class 1
	A792 24 and 22 GA	50 ksi, Class 2
High Strength Bolts	A325-Group A/(A490-Group B)	
Pipe	A53, Gr. B	Fy = 35 ksi
Round Structural Tubing	A500, Gr. B	Fy = 42 ksi
Shaped Structural Tubing	A500, Gr. B	Fy = 46 ksi
Hot Rolled Shapes	A572, A992, A529 Gr. 50	Fy = 50 ksi
Hot Rolled Shapes	A36	Fy = 36 ksi

4. Shop primer paint is a rust inhibitive primer which meets the end performance of SSPC-Paint 15: Steel Joist Shop Primer/Metal Building Primer and is maroon oxide in color. This paint is not intended for long term exposure to the elements. Pinnacle Structures, Inc. is not responsible for any deterioration of the shop primer as a result of improper handling or storage. Pinnacle will not be responsible for any field applied paint and or coatings. (Section 7.17 AISC code of Standard Practice for Steel Buildings & Bridges, 13th Edition).

5. Bolts for the construction of Pinnacle Structures, Inc. material shall be as follows:
 All secondary member connections - 1/2" x 1 1/4" A307 unless noted
 Bearing frame endwall connections - A325
 Main frame connections - A325 as shown on drawings

6. Connections Using High Strength Structural Bolts:

All high strength bolts are A325-N, unless noted otherwise. High strength structural bolts are supplied without washers, unless noted otherwise. Bolt length shall be such that the end of the bolt extends beyond or is at least flush with the outer face of the nut, when properly installed. All bolted connections, unless noted, are designed as bearing type connection with bolt threads not excluded from the shear plane.

A325-N High Strength Structural Bolts:

- Snug-Tightened connections are permitted with A325-N bolts, except for these cases:
- Where crane beams and rigid frame connections in crane buildings are present
 - In Slip-Critical Connections
 - If noted in the erection drawings otherwise
- For these exceptions, Turn-of-the-Nut method must be used.

A490 High Strength Structural Bolts:

A490 structural bolts shall be tightened using the Turn-of-the-Nut method. Snug-Tightened connections are not permitted with A490 bolts.

Tightening Methods:

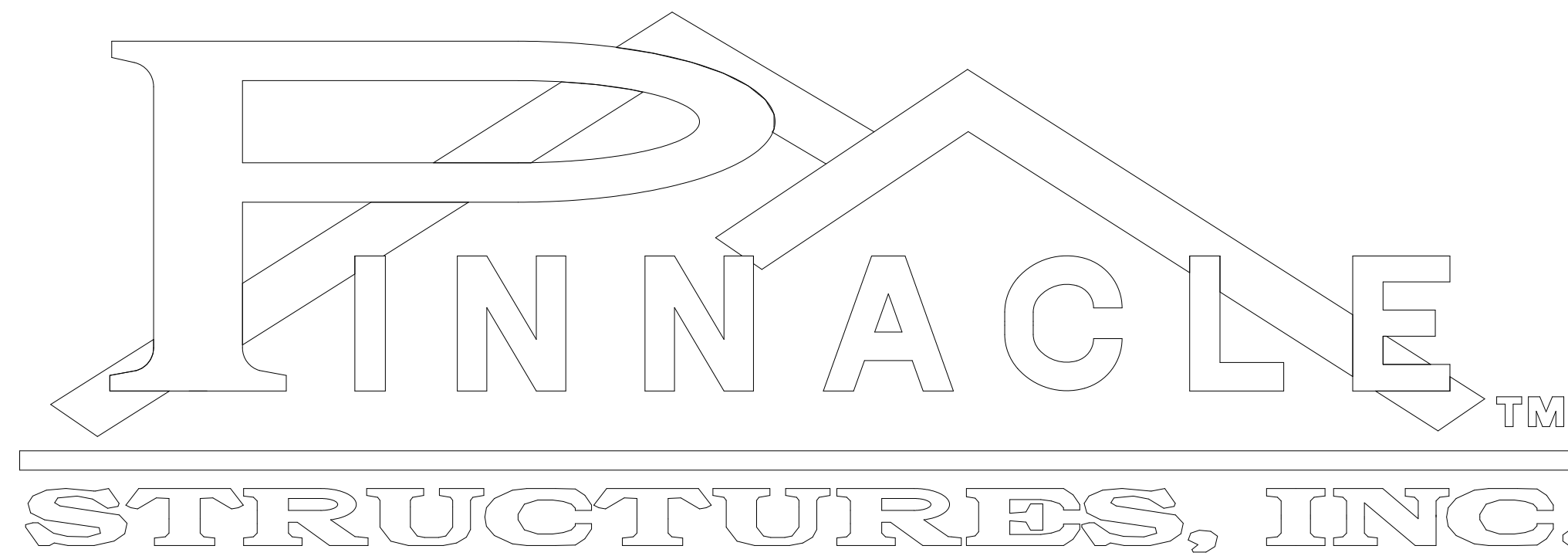
Snug-Tightened Joint: A condition in which the tightness that exists when all of the plies in a connection have been pulled into firm contact by the bolts in the joint and all of the bolts in the joint have been tightened sufficiently to prevent the removal of the nuts without the use of a wrench, in accordance with the 14th Edition of AISI "Specification for Structural Joints Using High-Strength Bolts", per Section 8.1.

Turn-of-the-Nut method in is to be performed in accordance with the 14th Edition AISI "Specification for Structural Joints Using High-Strength Bolts" per Section 8.2.1.

7. All Bracing shown and provided by Pinnacle for this building is required for transferring building loads to the foundation and shall be installed by the erector as a permanent part of the structure. Cable/Rod bracing is designed for structural loads only and is not designed to plumb the building. The cable/rod bracing shall be taut, tighten to remove sag only. Bracing shall not be over-tighten. If additional bracing is required for stability during erection, it shall be the erectors responsibility to determine the amount of such bracing and to procure and install as necessary.

8. Soil profile type is determined by the foundation Engineer per local code.

9. Building Codes Require Consideration of Snow Surcharges for Any Lower Roof of a Structure Located within 20 Feet of a Higher Structure. Information Supplied to Pinnacle Structures Does Not Indicate the Presence of a Shadowing Structure within this 20 Foot Envelope. Therefore Snow Surcharges Have Not Been Considered in this Design Unless Noted Otherwise.



P.O. Box 1268
 Cabot, AR 72023
 Phone: (501) 941-3929 or (800) 201-1534
 Fax: (501) 941-2675



FOR APPROVAL ONLY



These drawings and the metal building they represent are the product of Pinnacle Structures Inc. - Cabot, AR. The engineer whose seal appears hereon is employed by Pinnacle Structures Inc. and is not the engineer of record for this project.

DRAWING PACKAGE FOR:

Customer: HOPKINS CONSTRUCTION & MAINTENANCE
 Job Number: 251599
 Project: Pride Fire District-Indian Mound Station
 Project Location: Central, LA (East Baton Rouge CO)
 Project Description: MULTIPLE BUILDINGS

DESIGN REQUIREMENTS

**** PRE-PRODUCTION DESIGN REVIEW IS REQUIRED AFTER APPROVAL RETURNED ****

Building Code: IBC 2021

Building Risk Category: IV - Post
 Collateral Load: * 6.00 psf
 Roof Live Load: 20.00 psf
 Tributary Reduction: Yes

**** PLEASE VERIFY LOADS, CODE & DEFLECTIONS ****

Roof Snow Load Data
 Ground Snow Load (Pg): 0.00 psf
 Flat Roof Snow Load (Pf): 0.00 psf
 Snow Exposure Factor (Ce): 1.00
 Snow Importance Factor (Is): 1.20
 Thermal Factor (Ct): 1.00

Rain Data:
 Rain Intensity (i): 10.0 in/hr

Wind Design Data
 Ultimate Design Wind Speed : 136 mph
 (3 Second Gust)
 Nominal Design Wind Speed : 105 mph
 Internal Pressure Coefficient: ± 0.18
 Wind Exposure: B

Earthquake Design Data:
 Analysis Procedure - Equivalent Lateral Force Procedure
 Seismic Importance Factor: Ie 1.50

Mapped Spectral Response Acceleration Parameters: Ss 0.089 S1 0.058

Design Spectral Response Acceleration Parameters: S0s 0.095 S01 0.093

Site Class : D Seismic Design Category : C

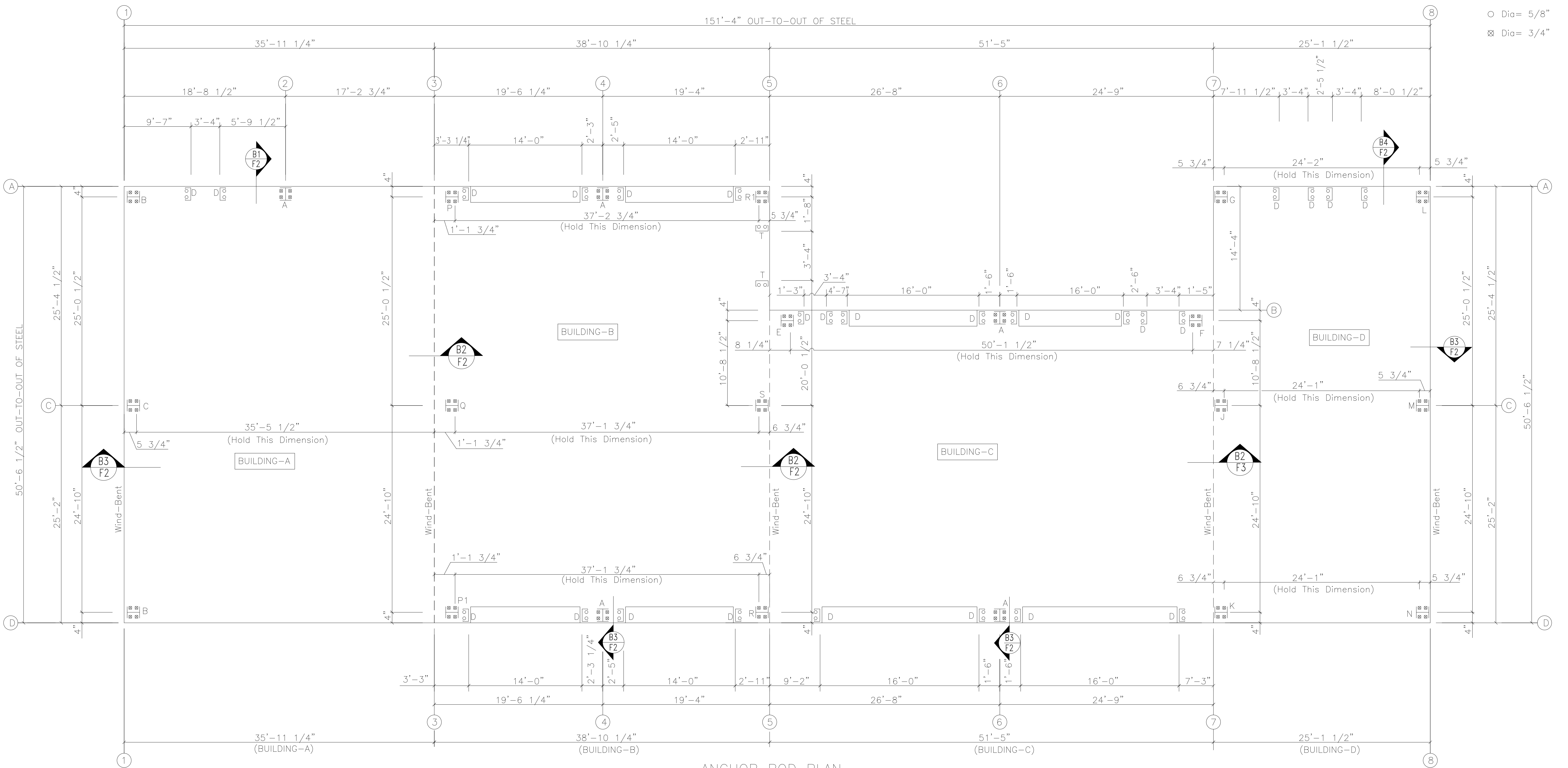
Design Base Shear V
 Transverse Direction : 3.06 kips
 Longitudinal Direction : 1.47 kips

Basic Seismic Force- Resisting Systems (SFRS)		Cs	R
Transverse	Steel Ordinary Moment Frame(s)	0.048	3.00
Left Endwall	Steel Ordinary Moment Frame	0.048	3.00
Right Endwall	Steel Ordinary Moment Frame	0.048	3.00
Front Sidewall	Steel Ordinary Moment Frame	0.048	3.00
Back Sidewall	Steel Ordinary Moment Frame(s)	0.048	3.00

Other: (4) Roof Hoods each weighing maximum of 400lbs

Exterior wall component & cladding materials not specifically supplied by P.S.I. should be designed to withstand 24.73 /-26.79 psf in the field zone.

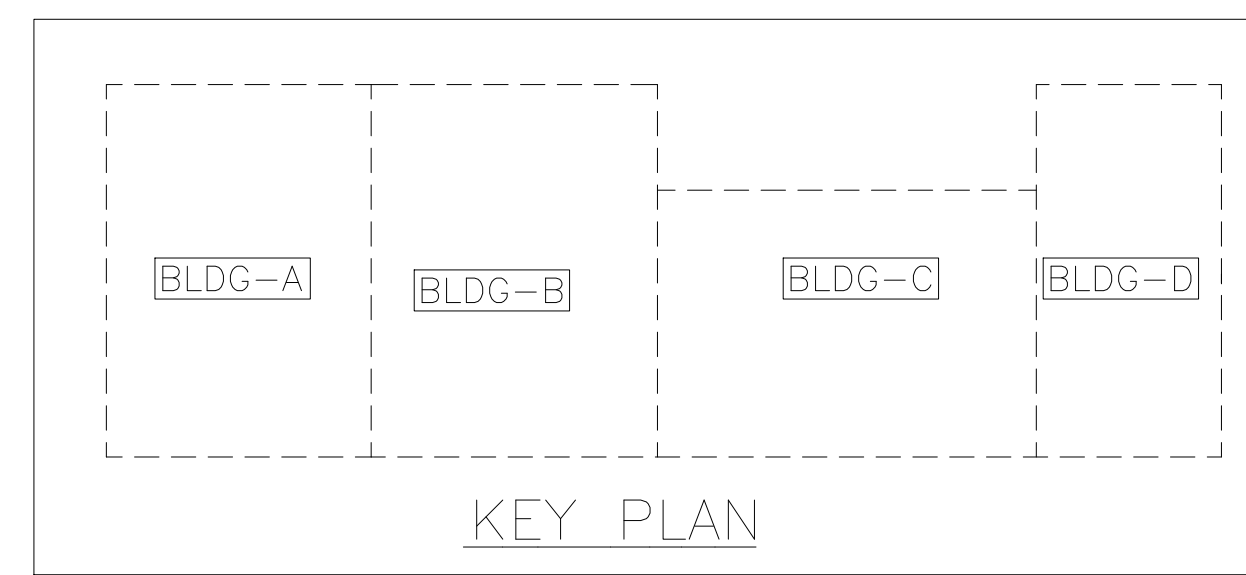
*This project is designed for this collateral loading. Suspension of any load-inducing system in excess of this loading is prohibited without consultation with the manufacturer to determine structural reinforcement, if required, to safely support supplemental loads.



ANCHOR ROD PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

ANCHOR ROD SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 56	Jamb	5/8"	F1554 - GR36	2.00
⊗ 20	Endwall	3/4"	F1554 - GR36	2.50
⊗ 68	Frame	3/4"	F1554 - GR36	2.50



GENERAL NOTES

- These drawings are NOT to scale.
- Pinnacle's steel line is shown.
- A sheeting notch or brick ledge, if used, must be added to determine the out of concrete.
- Wall panels shall be held 1/4" above the sheet notch and/or base trim.
- Attachment of material by others to Pinnacle steel is the responsibility of others.

**** Please verify building dimensions, loading data, framed opening locations and clearances on approval drawings ****

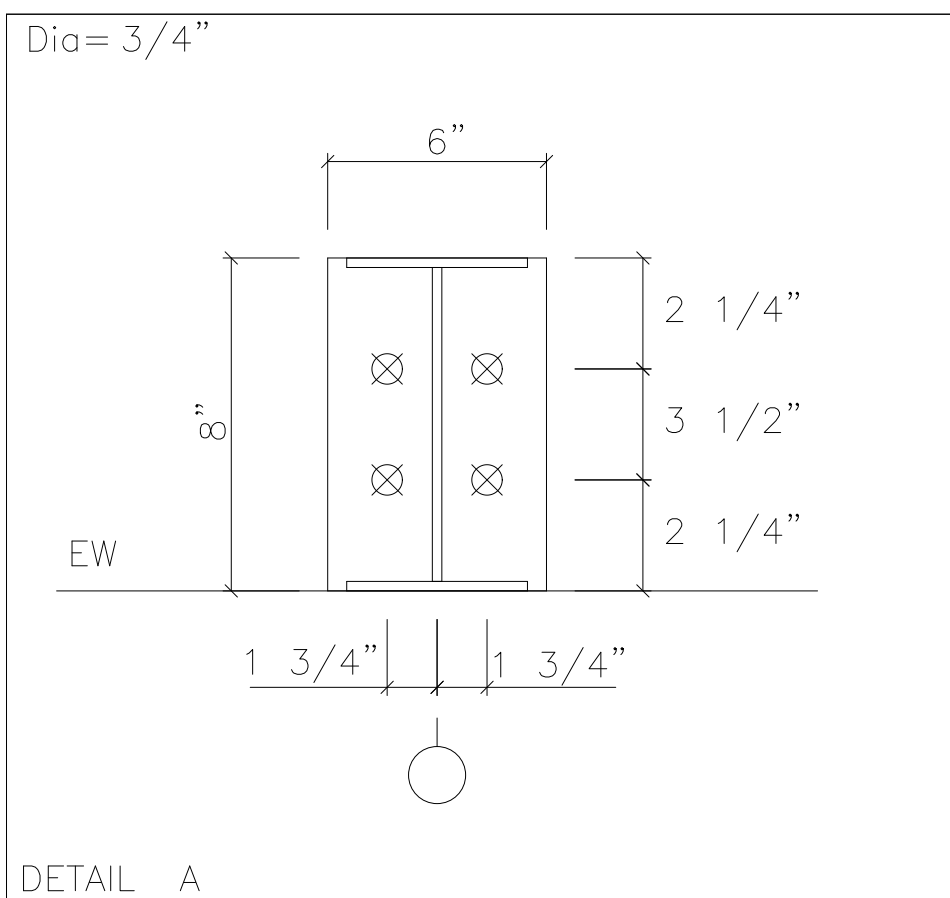
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
STRUCTURES, INC.
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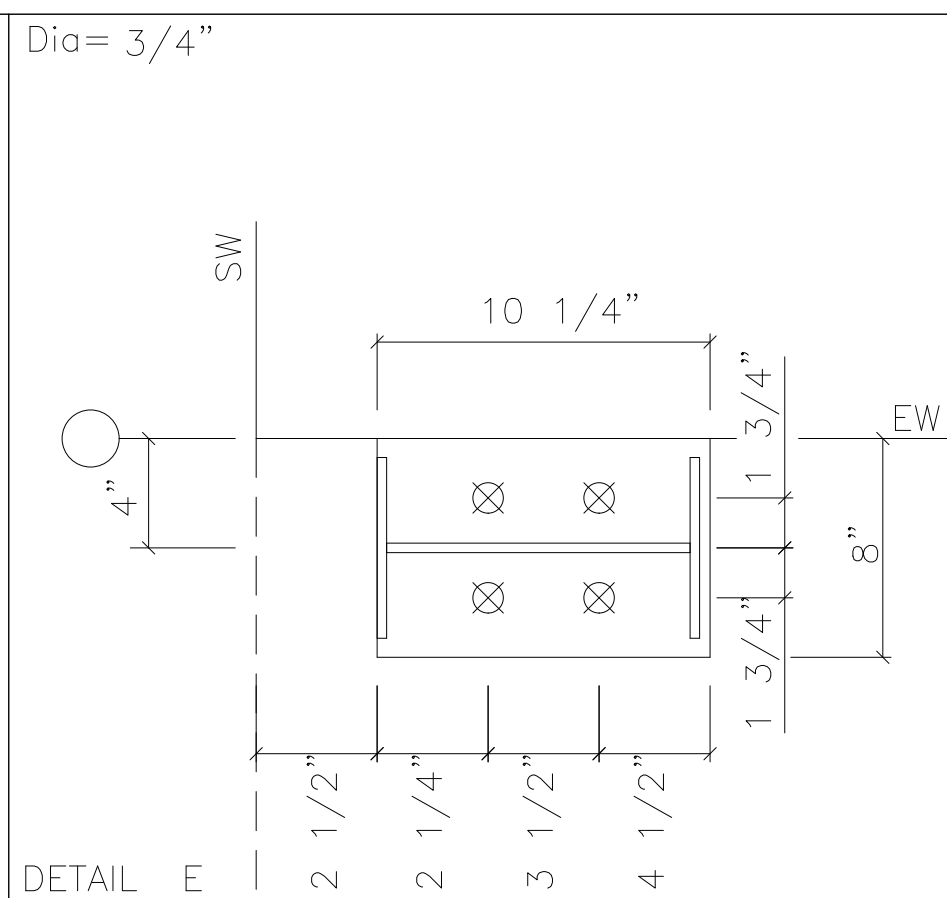
DESCRIPTION: ANCHOR ROD PLAN
CUSTOMER: HOPKINS CONSTRUCTION & MAINTENANCE
LOCATION: Central, LA (East Baton Rouge Parish)

STATE OF LOUISIANA
Professional Engineer
11/25/2025

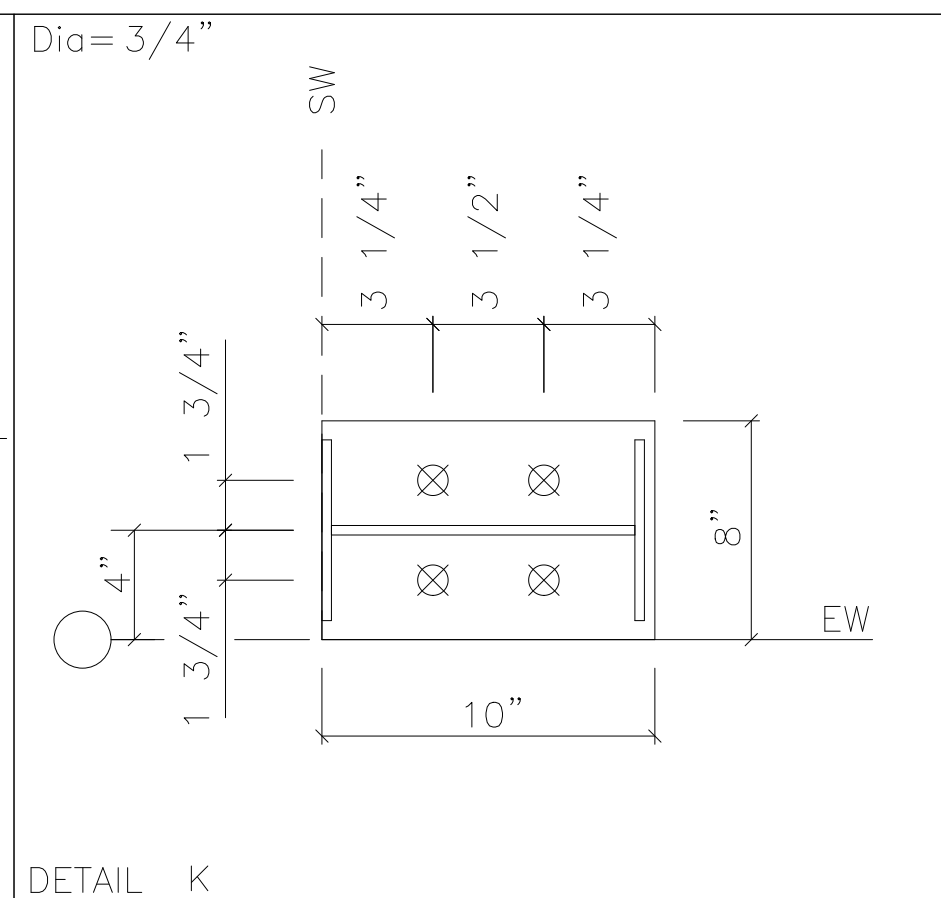
Detailer	MVU	Checker	JJ	Designer	HA
Job No.	251599	Sheet	F1	Issue	A



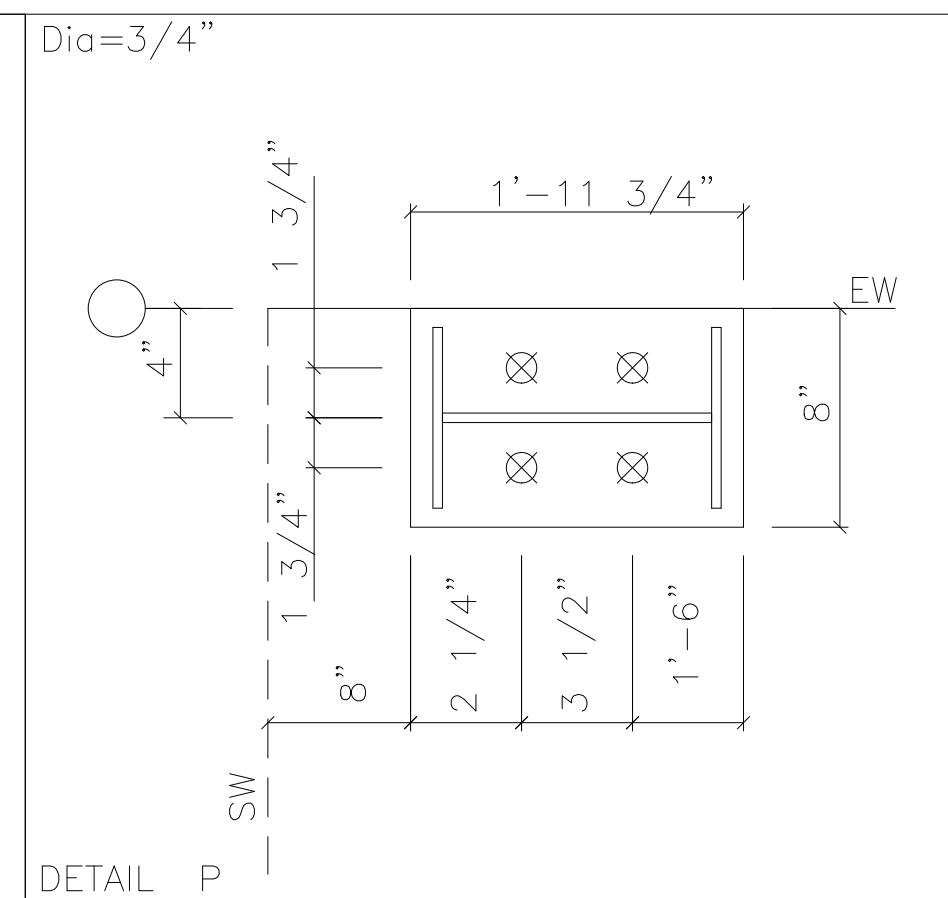
DETAIL A



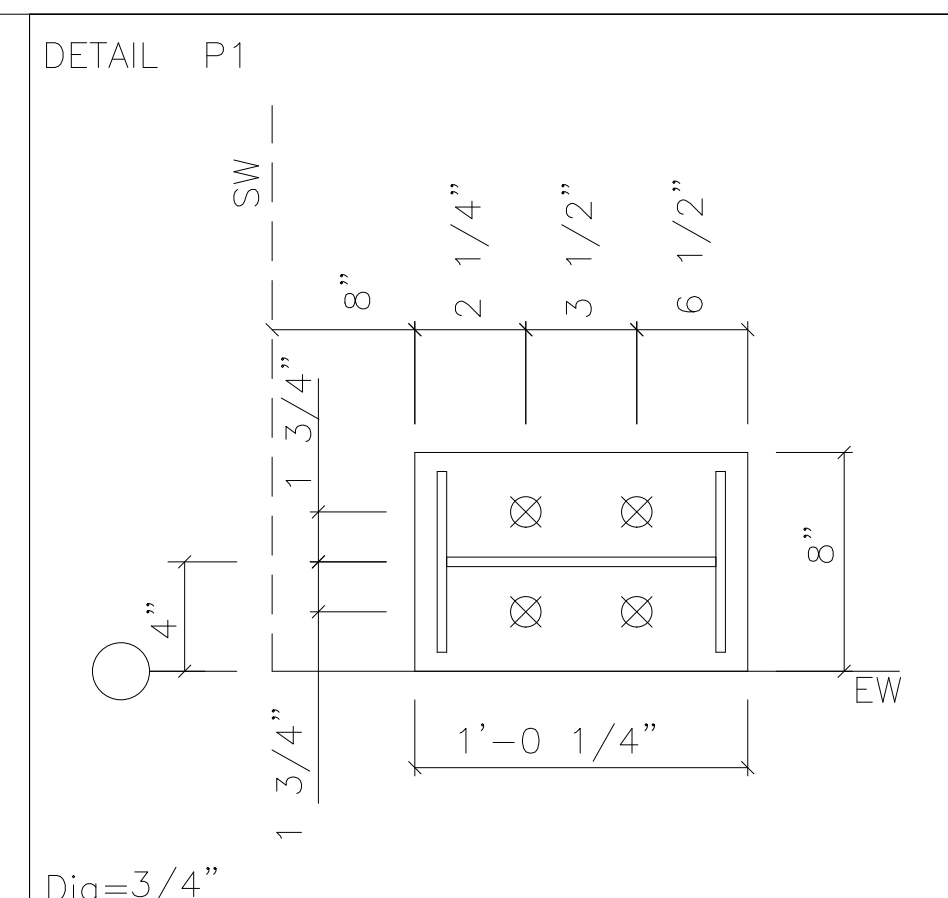
DETAIL E



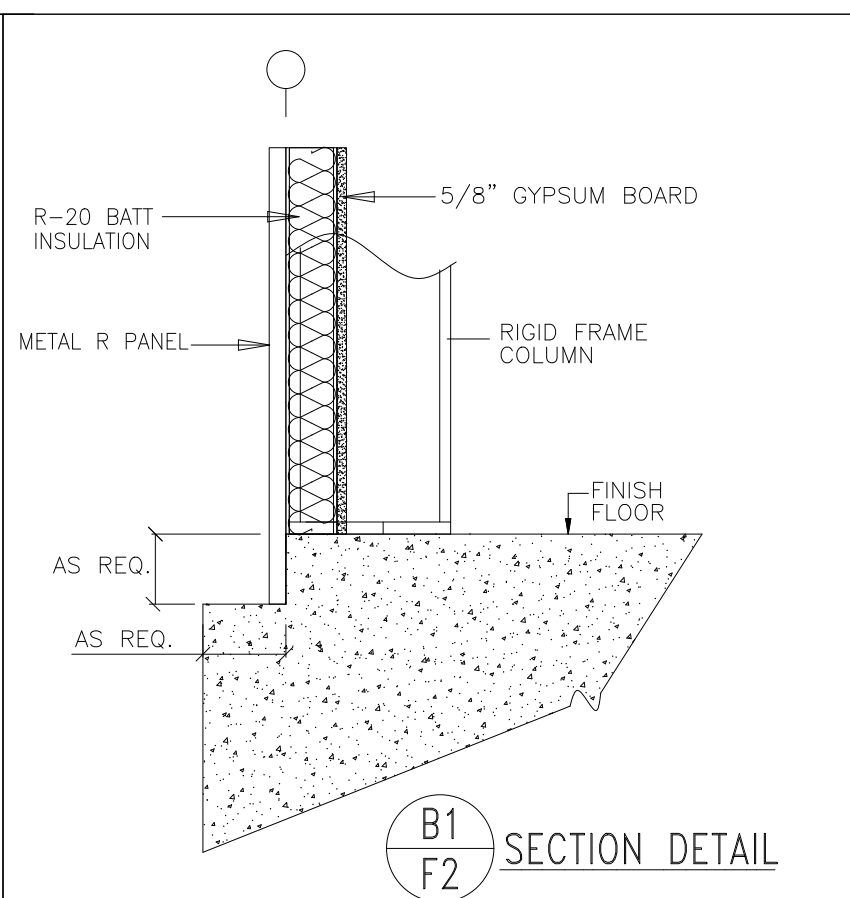
DETAIL K



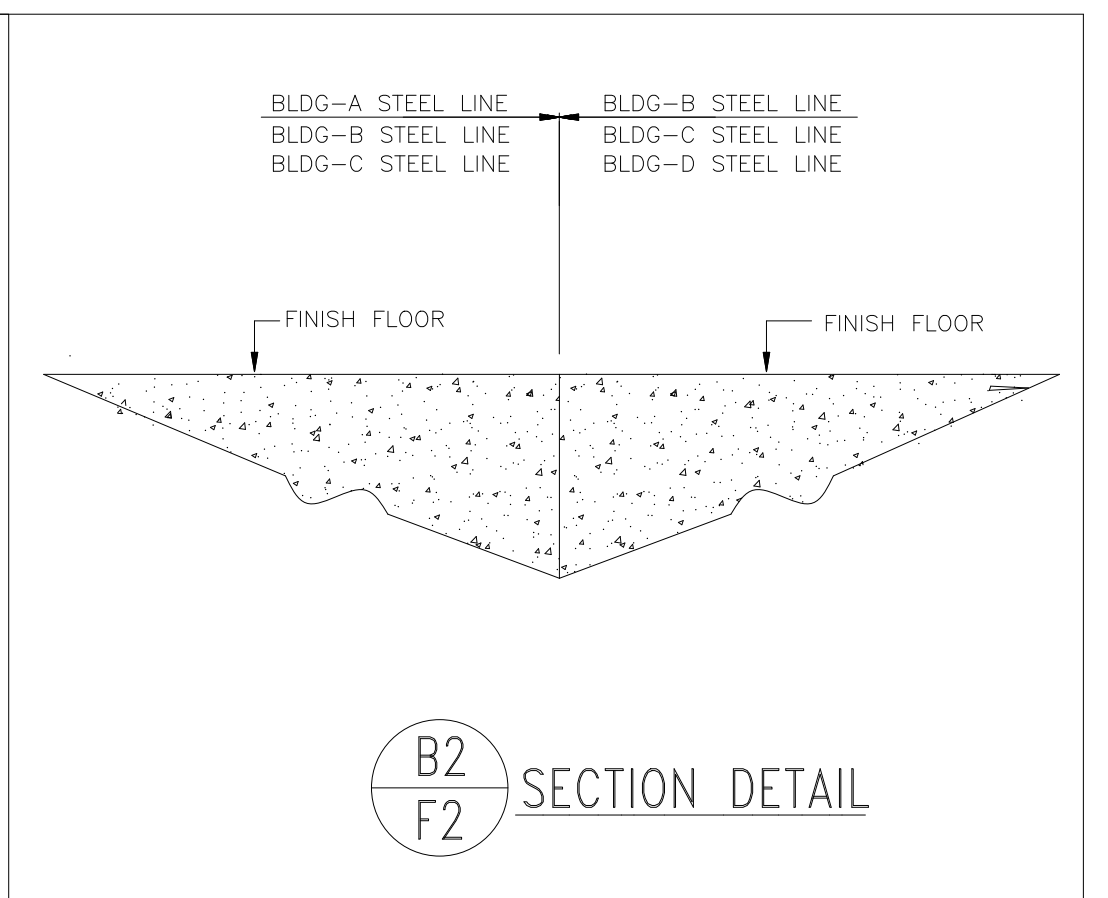
DETAIL P



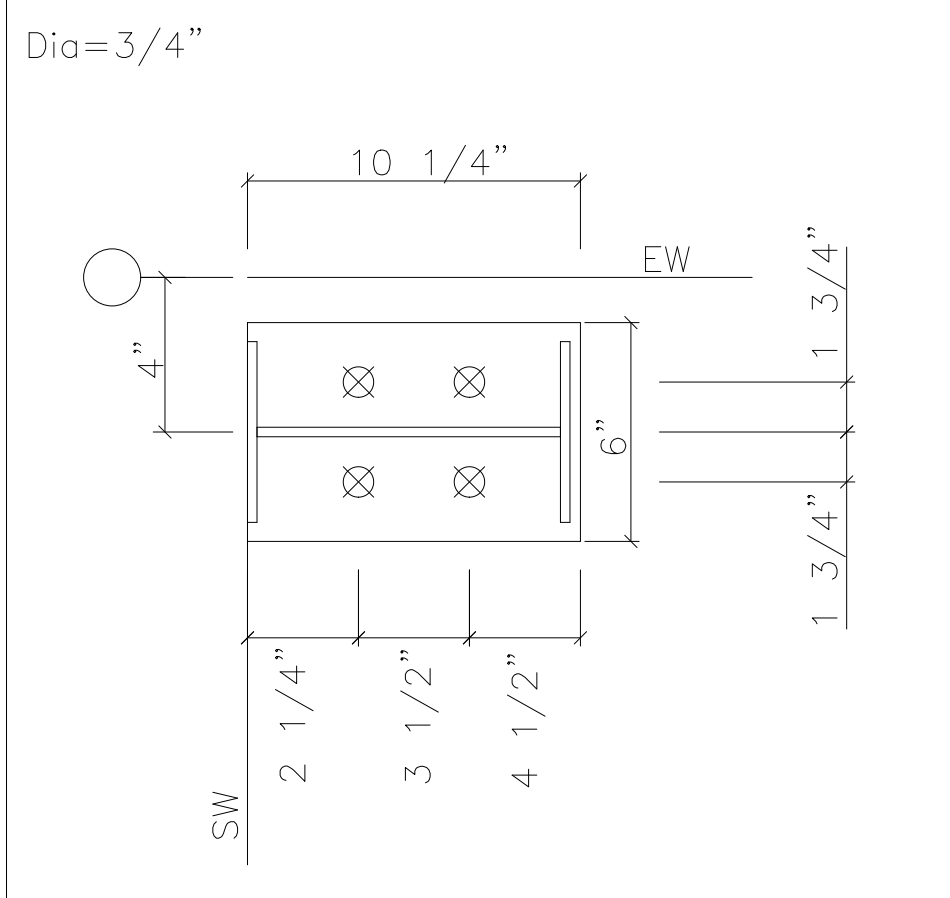
DETAIL P1



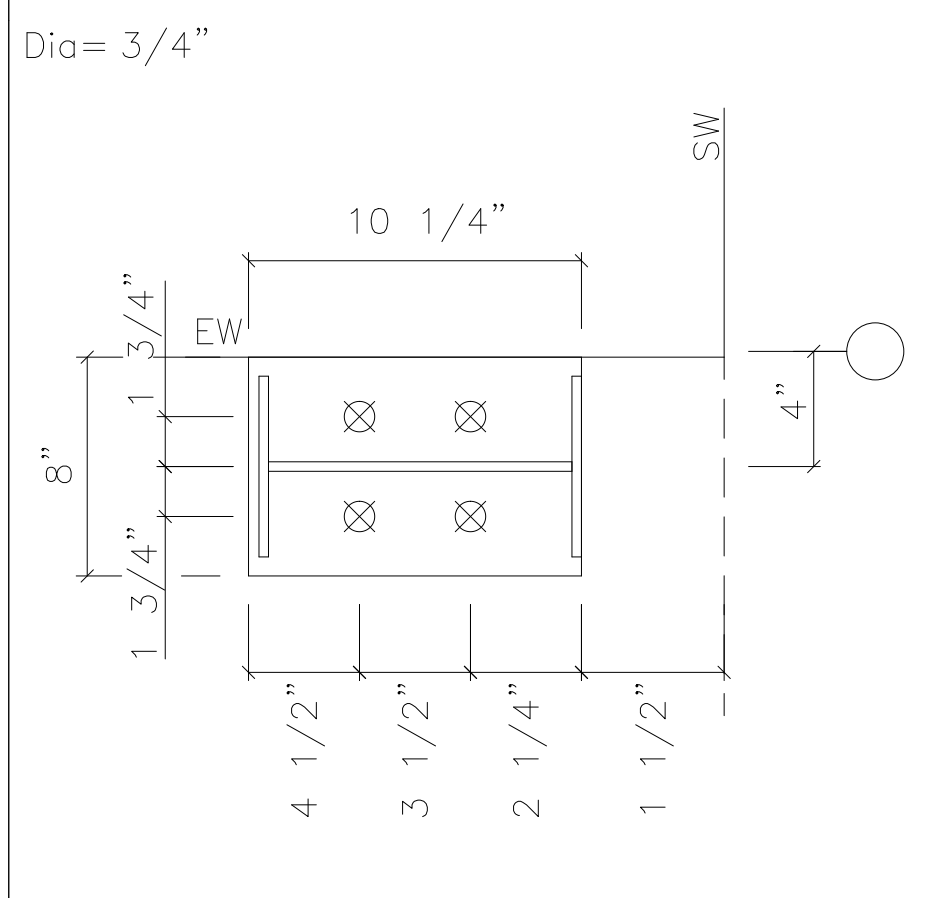
B1 F2 SECTION DETAIL



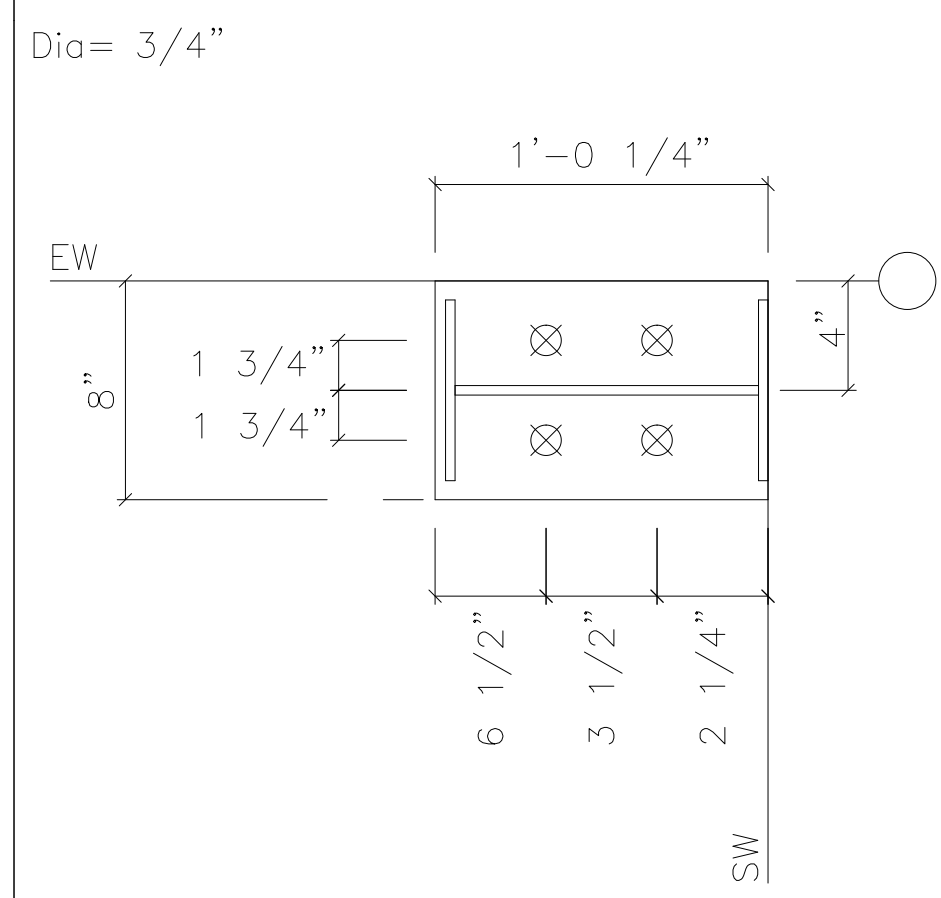
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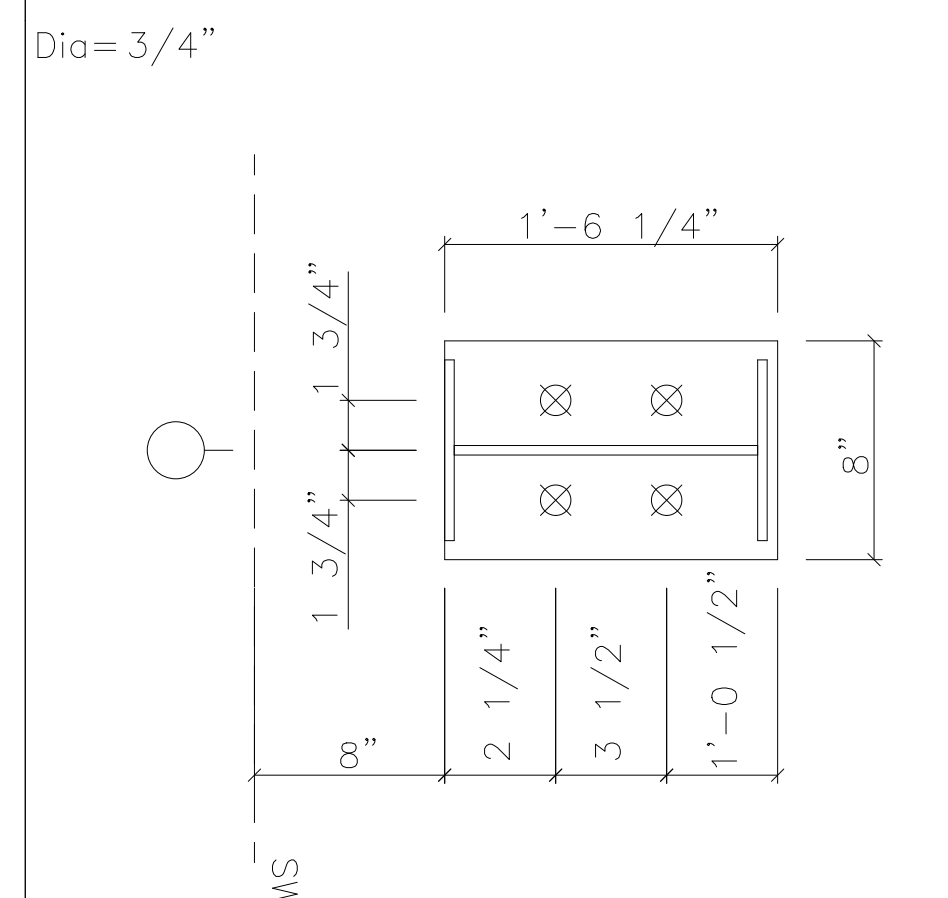
DETAIL B



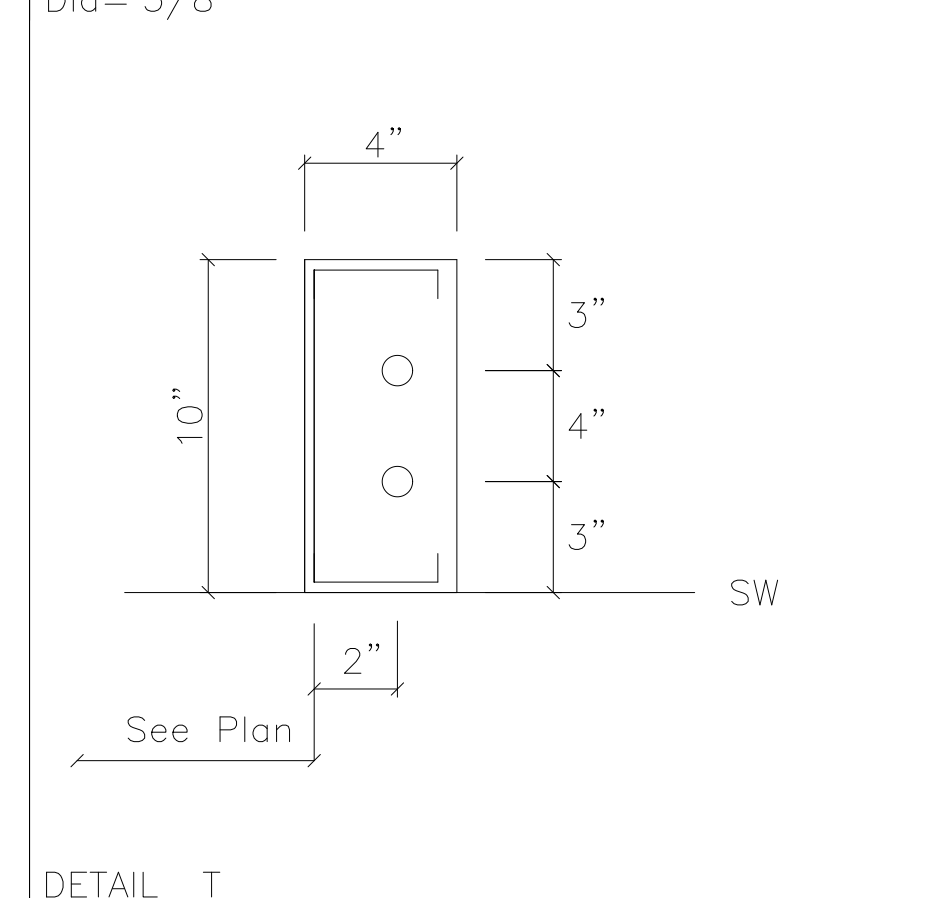
DETAIL F



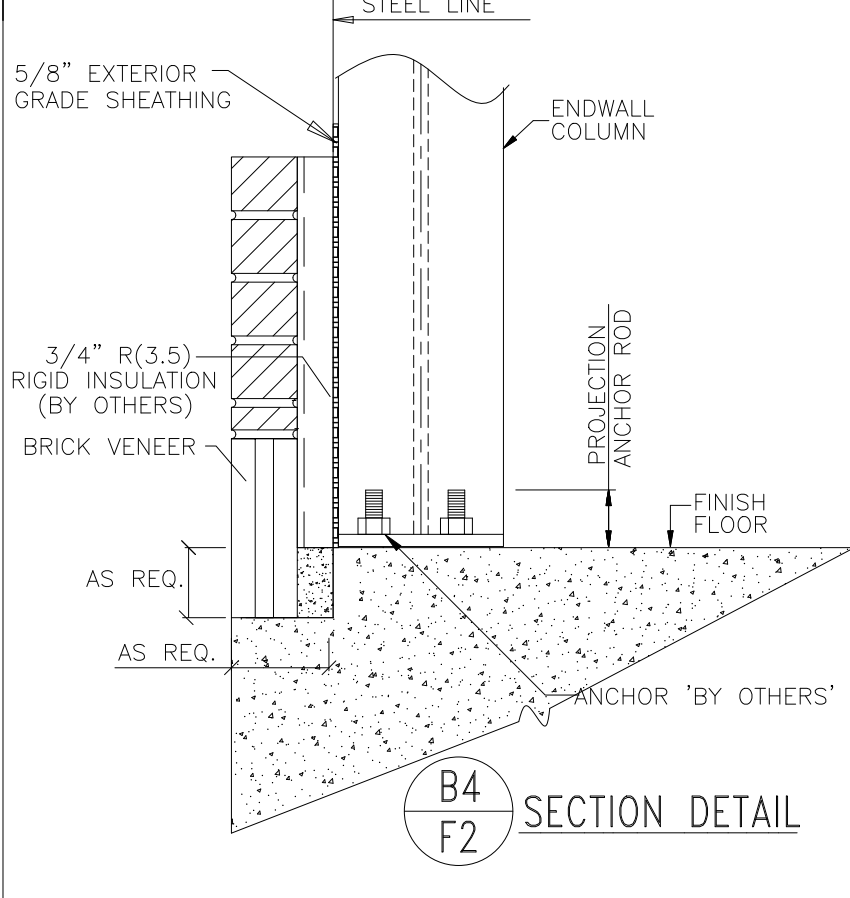
DETAIL L



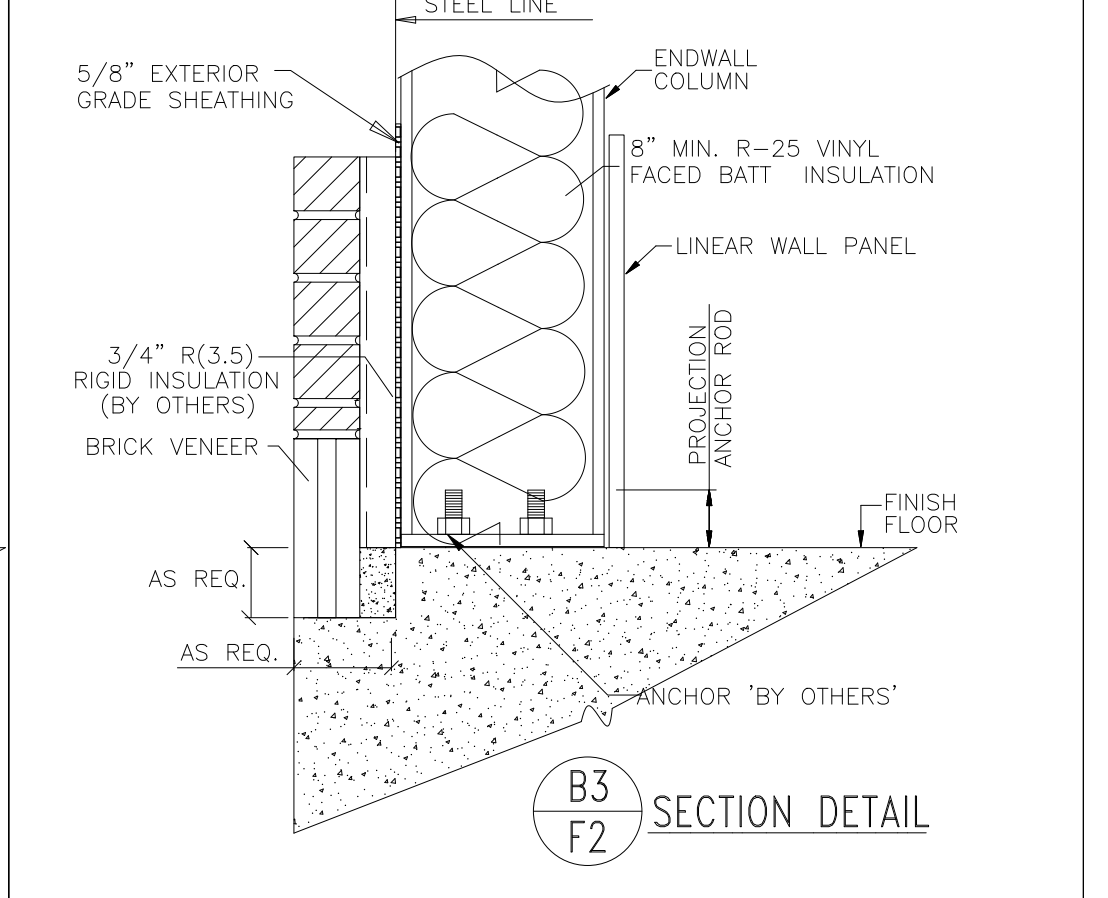
DETAIL Q



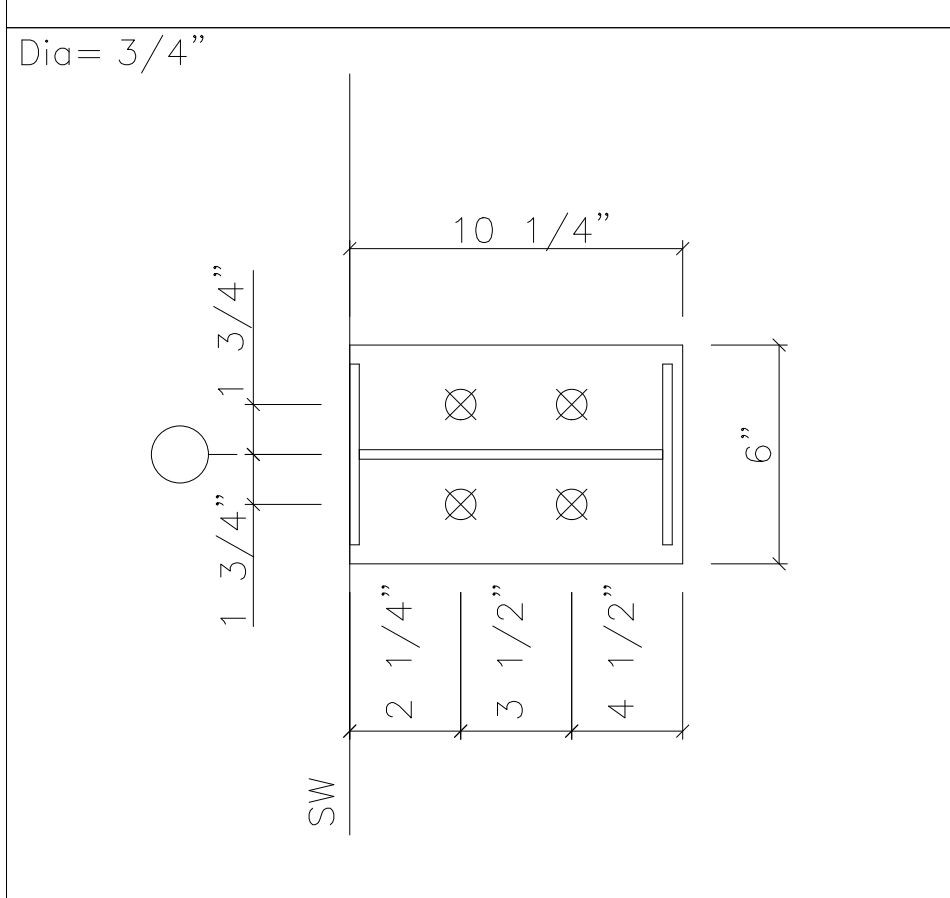
DETAIL T



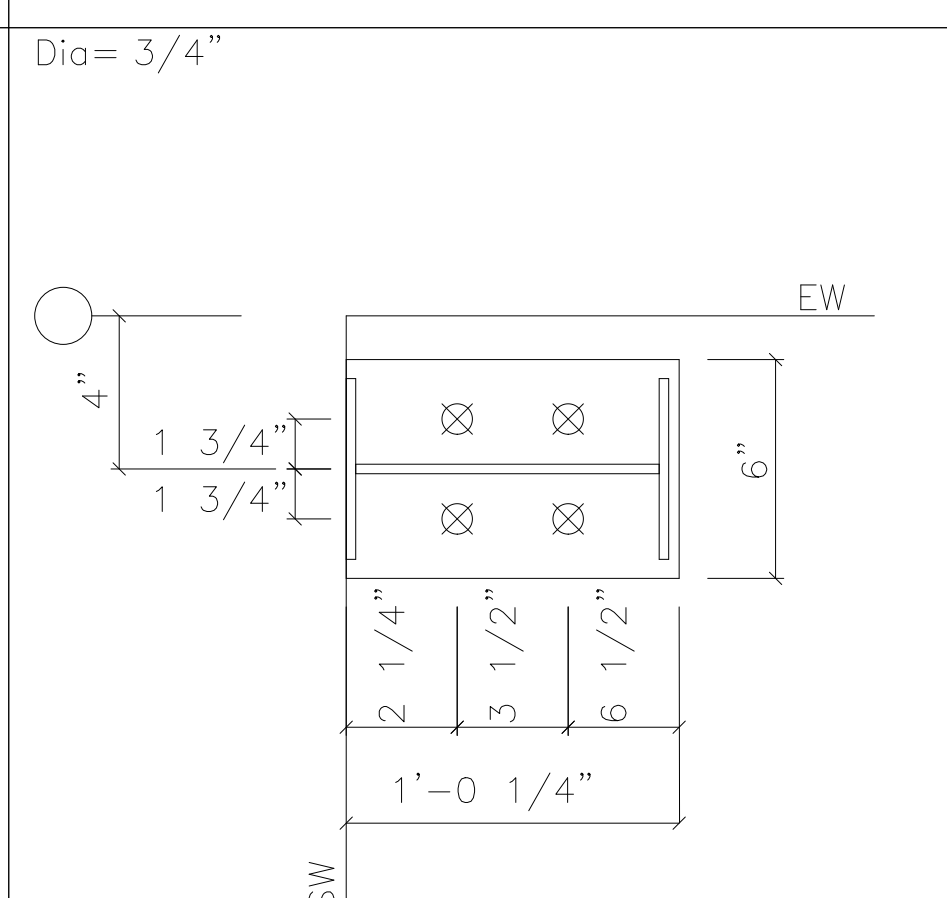
B4 F2 SECTION DETAIL



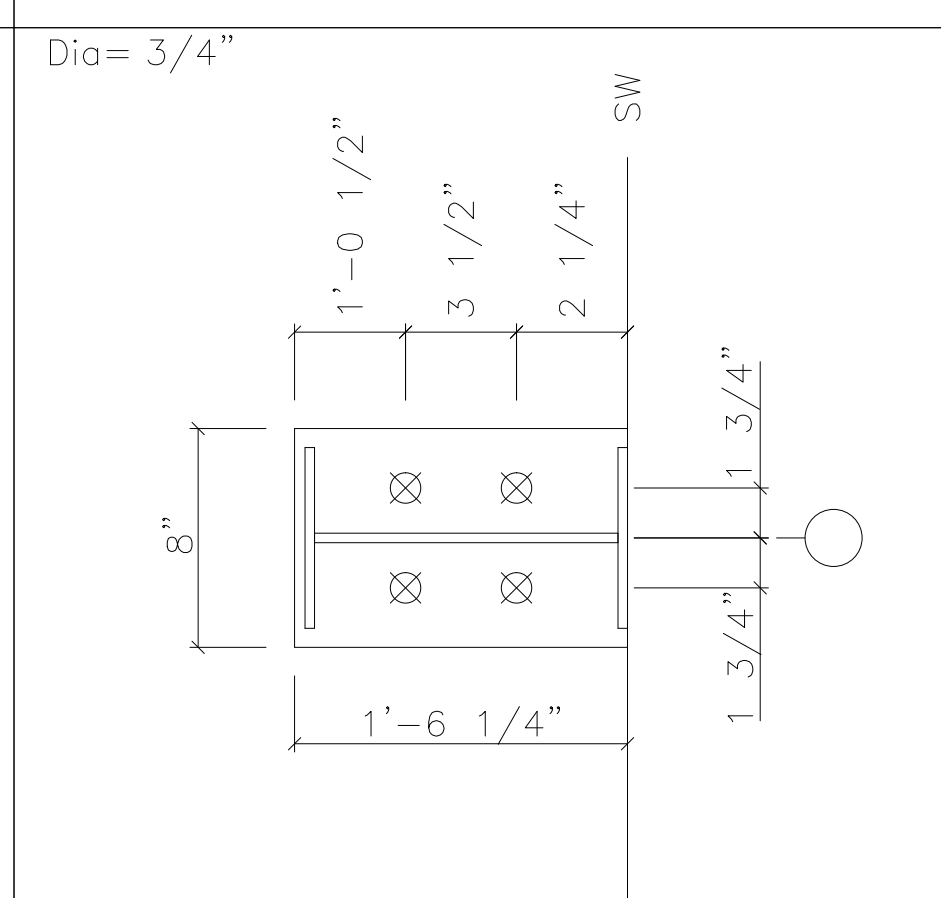
B3 F2 SECTION DETAIL



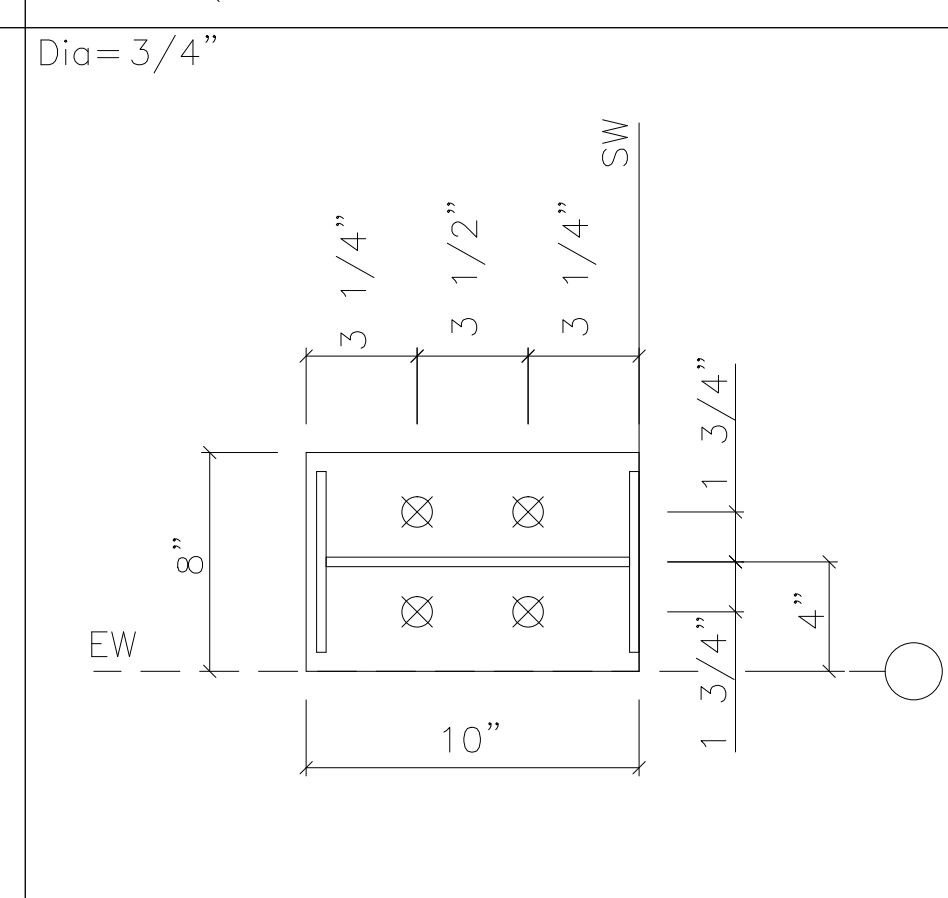
DETAIL C



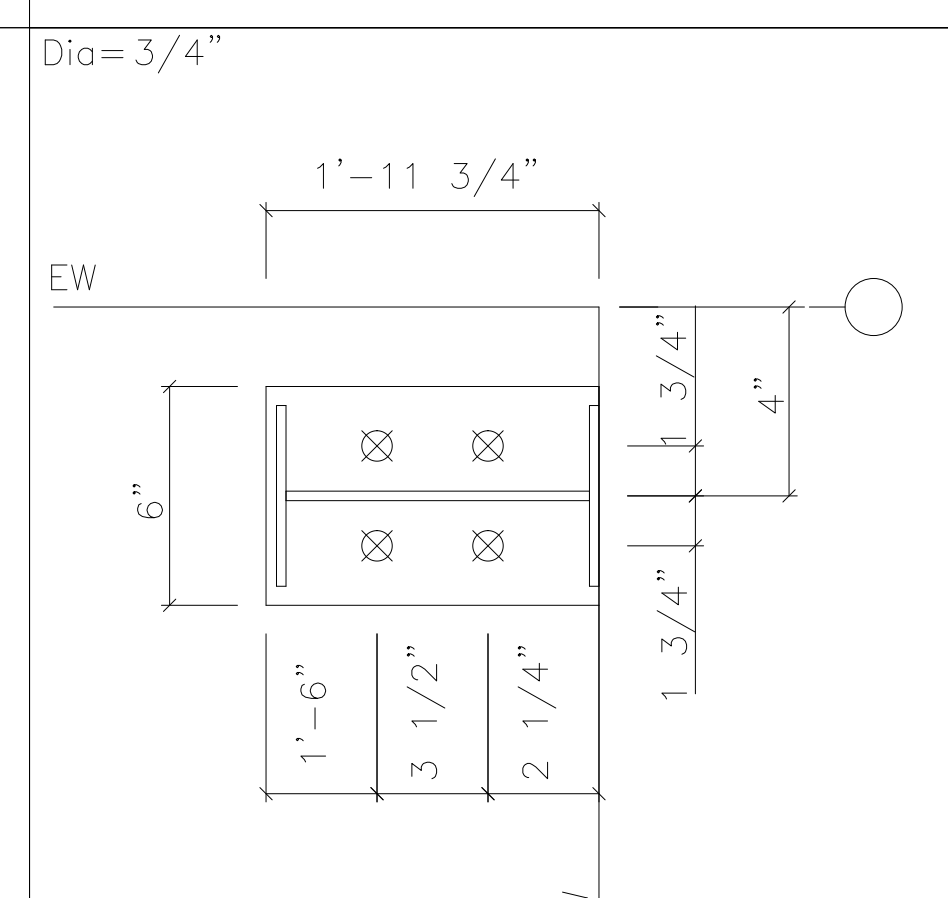
DETAIL G



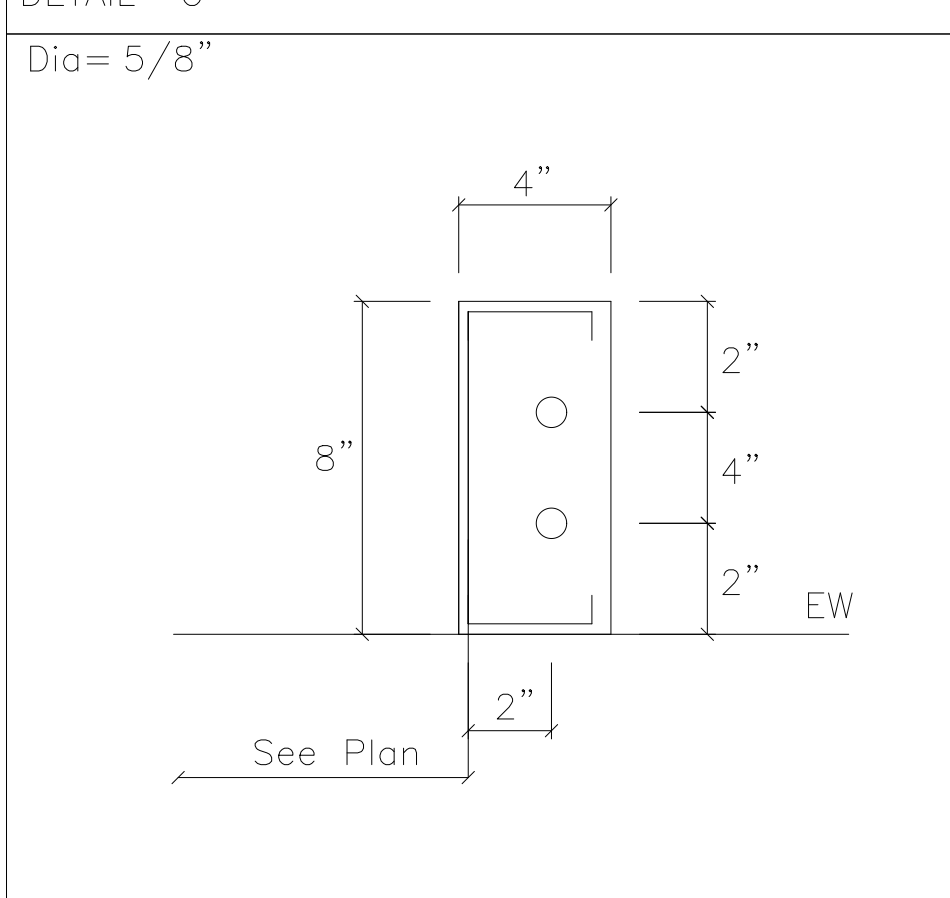
DETAIL M



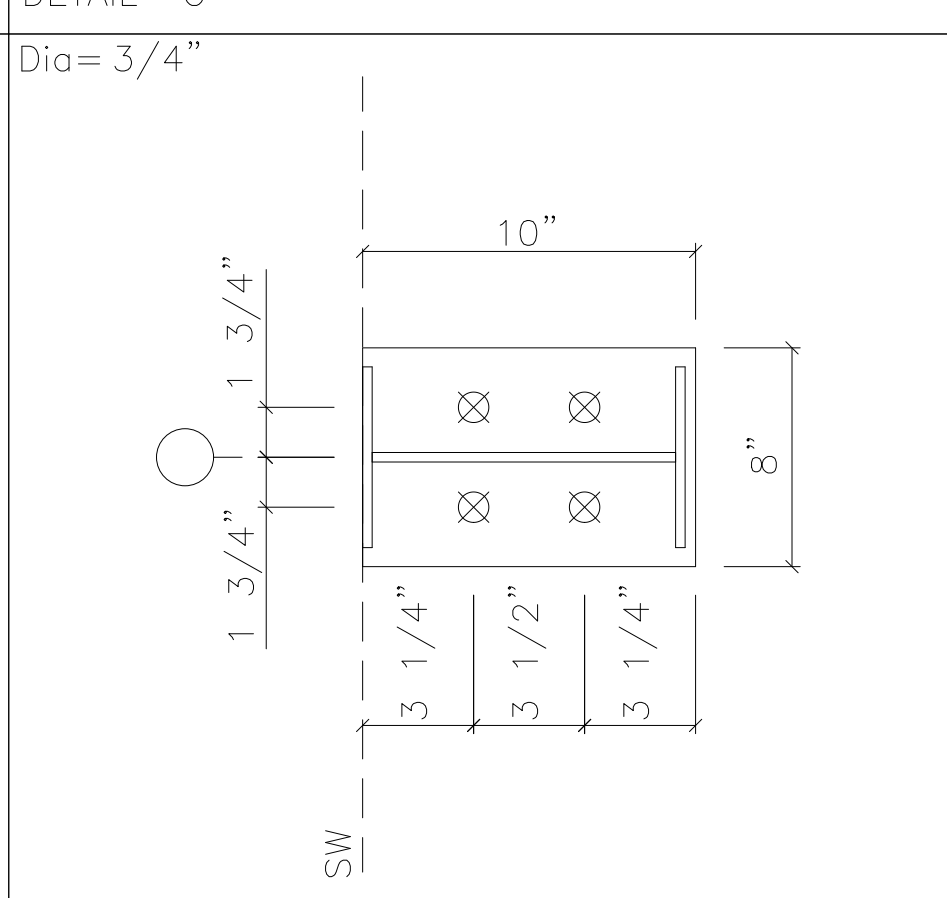
DETAIL R



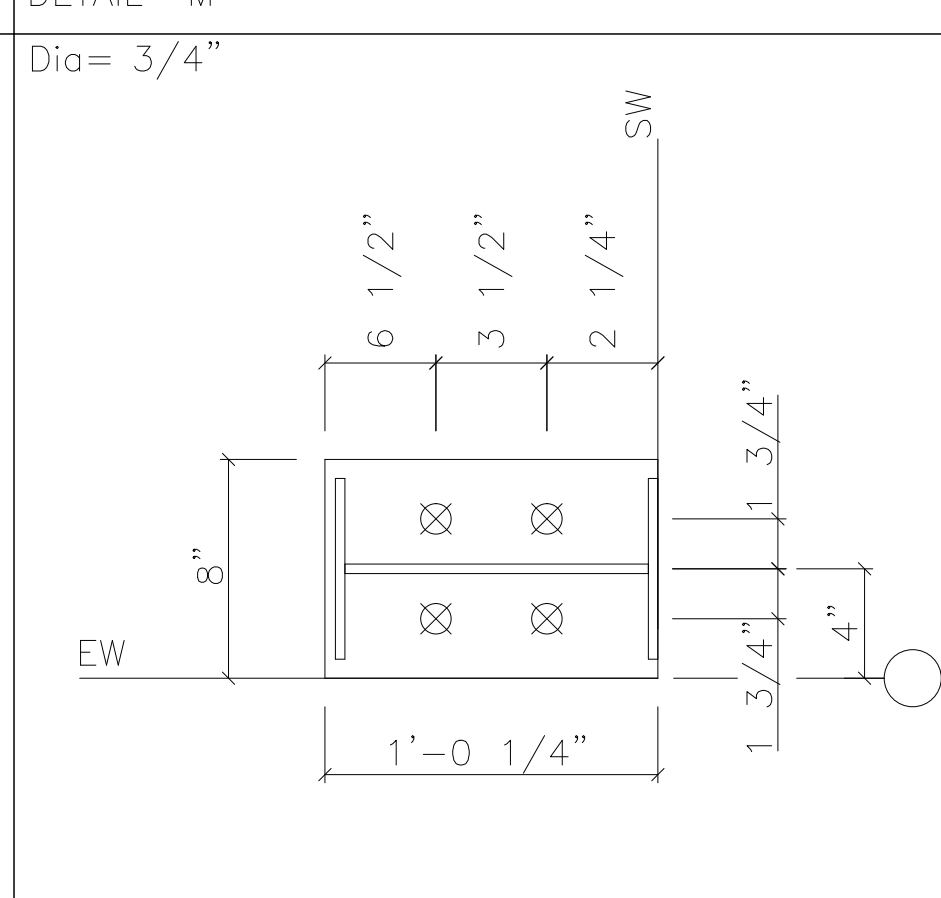
DETAIL R1



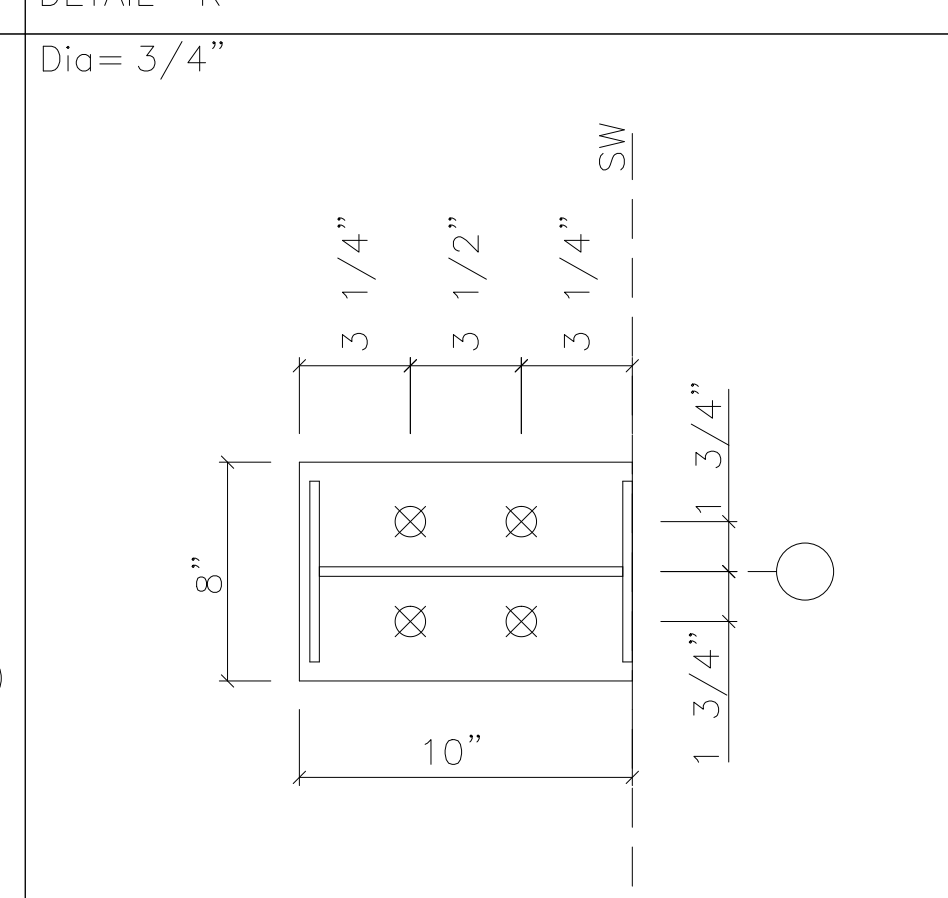
DETAIL D



DETAIL J



DETAIL N



DETAIL S

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION: ANCHOR ROD DETAILS
 CUSTOMER: HOPKINS CONSTRUCTION & MAINTENANCE
 LOCATION: Central, LA (East Baton Rouge Parish)
 Detailer: MVU
 Checker: JJ
 Designer: HA
 Job No: 251599
 Sheet: F2
 Issue: A



FRAME LINES: D C A



NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 35.9
 - Length (ft) = 50.5
 - Eave Height (ft) = 13.5/ 16.5
 - Roof Slope (rise/12) = 1.00
 - Roof Dead Load (psf) = 2.2
 - Wall Dead Load
 - Left Endwall (psf) = 40.0
 - Right Endwall (psf) = 2.2
 - Front Sidewall (psf) = 2.2
 - Back Sidewall (psf) = 40.0
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 14.9
 - Collateral Load (psf) = 6.0
 - Wind Speed (mph) = 136.0
 - Wind Code = IBC 21
 - Exposure = B
 - Closure = Enclosed
 - Internal Wind Coeff = -0.18, +0.18
 - Risk Category = IV - Post
 - Importance - Wind = 1.00
 - Importance - Seismic = 1.50
 - Seismic Design Category = C
 - Seismic Coeff (Sms) = 0.14
- Loading conditions are:
 - Dead+Collateral+Live
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Long1L
 - 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Left
 - 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
 - 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - Dead+0.6Wind_Right2+0.6Wind_Suction

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
D	1	1	2.3	8.2	2	-2.6	-7.0	4	0.750	6.000	10.25	0.375	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
C	1	1	3.8	13.6	2	-3.0	-8.2	4	0.750	6.000	10.25	0.375	0.0
					3	-0.7	-9.4						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
A	1	1	2.3	8.2	2	-2.6	-7.0	4	0.750	6.000	10.25	0.375	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
D	1	0.4	1.4	0.7	2.3	1.3	4.5	-4.6	-13.2	-1.0	-7.8	-4.2	-9.2
C	1	0.5	2.0	1.1	3.9	2.2	7.7	-5.5	-15.6	-0.7	-10.4	-5.3	-8.8
A	1	0.4	1.4	0.7	2.3	1.3	4.5	-4.6	-13.2	-1.0	-7.8	-4.2	-9.2

Frame Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right		Seismic_Long	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
D	1	-0.6	-3.8	-1.5	-10.9	-0.5	-7.4	0.0	0.0	0.0	0.0	0.0	-0.7
C	1	-0.5	-3.5	-1.7	-17.7	0.0	-11.9	0.0	0.0	0.0	0.0	0.0	-0.7
A	1	-0.6	-3.8	-1.5	-10.9	-0.5	-7.4	0.0	0.0	0.0	0.0	0.0	-0.7

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press		Wind Suct		Seis Long	
			Horz	Vert	Horz	Vert	Horz	Vert
A	2	0.2	-2.3	2.5	0.0			

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
A	2	5	1.5	0.1	6	-1.4	0.1	4	0.750	6.000	8.000	0.250	0.0
		7	1.5	0.2									

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k)				Panel_Shear (lb/ft)		Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	D							(h)	
F_SW	3							(e)	
R_EW	A							(h)	
B_SW	1	D,C	1.4	1.4	0.7	0.7		(b)	

(b) Wind bent in bay, base above finish floor
(e) Bracing loads must be applied to supporting building
(h) Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8."

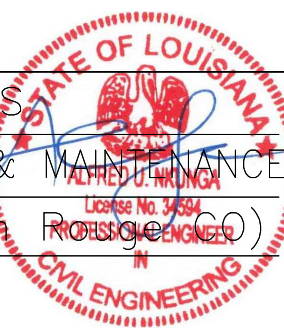
BLDG-A REACTIONS

PLEASE VERIFY REACTIONS

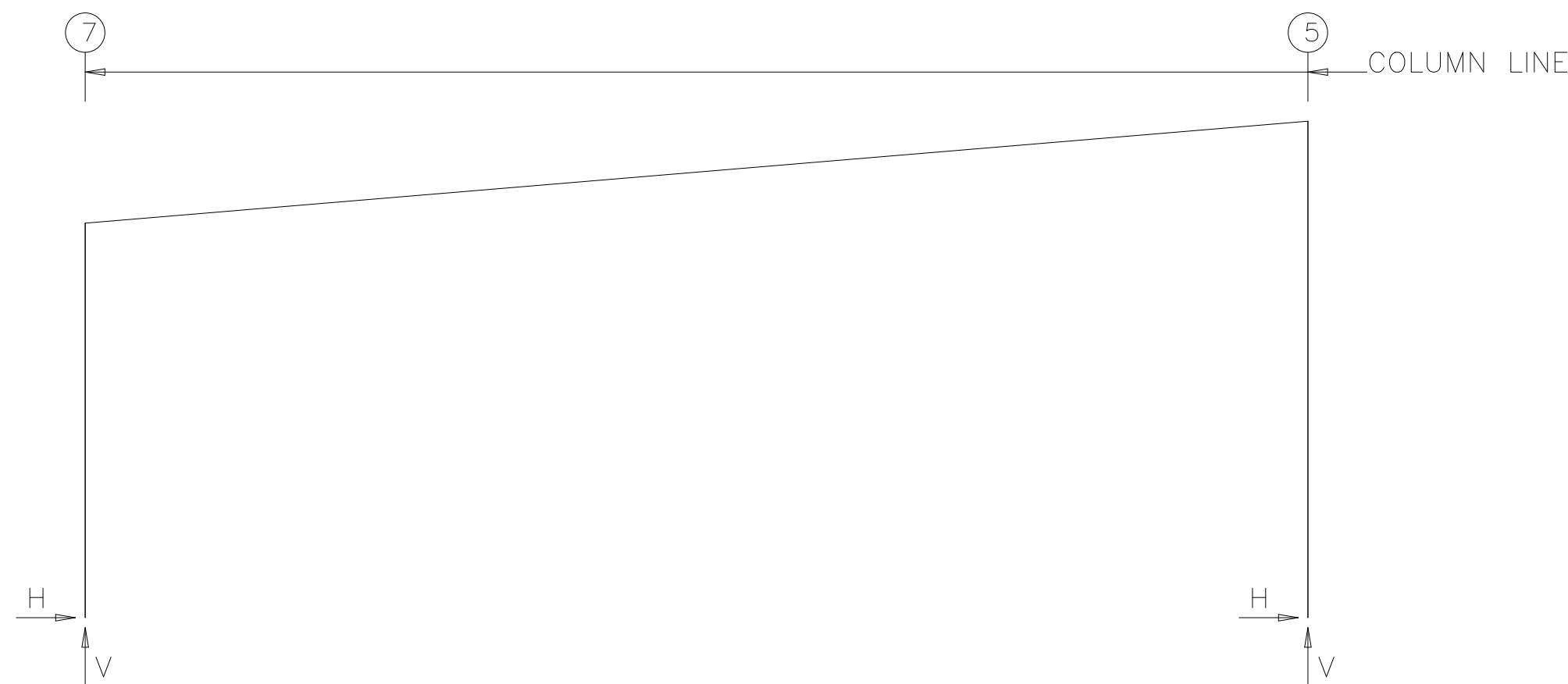
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	ANCHOR ROD REACTIONS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	MVU	Checker	JJ
Designer	11/25/2025 HA		
Job No.	251599	Sheet	F3
Issue	A		



FRAME LINES: B



NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 51.4
 - Length (ft) = 36.2
 - Eave Height (ft) = 16.6/ 20.9
 - Roof Slope (rise/12) = 1.00
 - Roof Dead Load (psf) = 2.2
 - Wall Dead Load
 - Left Endwall (psf) = 2.2
 - Right Endwall (psf) = 40.0
 - Front Sidewall (psf) = 2.2
 - Back Sidewall (psf) = 2.2
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 12.0
 - Collateral Load (psf) = 6.0
 - Wind Speed (mph) = 136.0
 - Wind Code = IBC 21
 - Exposure = B
 - Closure = Enclosed
 - Internal Wind Coeff = -0.18, +0.18
 - Risk Category = IV - Post
 - Importance - Wind = 1.00
 - Importance - Seismic = 1.50
 - Seismic Design Category = C
 - Seismic Coeff (Sms) = 0.14

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
B	7	1	1.5	6.2	2	-0.6	-3.8	4	0.750	8.000	10.25	0.375	0.0
B	5	2	1.4	-4.3	1	-1.5	6.4	4	0.750	8.000	10.25	0.375	0.0
		1	-1.5	6.4	2	1.4	-4.3						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Wind_Left1---		---Wind_Right1---		---Wind_Left2---	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
B	7	0.3	1.3	0.4	1.4	0.9	3.5	-1.2	-7.6	-0.7	-4.3	-0.9	-5.4
B	5	-0.3	1.4	-0.4	1.4	-0.9	3.6	2.6	-8.7	1.5	-4.9	1.9	-6.2
Frame Line	Column Line	---Wind_Right2---		---Wind_Long1---		---Wind_Long2---		---Seismic_Left---		Seismic_Right			
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
B	7	-0.3	-2.1	-0.4	-5.5	0.0	-3.5	-0.2	-0.1	0.2	0.1		
B	5	0.7	-2.4	0.9	-5.9	0.2	-3.6	-0.1	0.1	0.1	-0.1		

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind		Seis	
			Press Horz	Suct Horz	Long Horz	Long Horz
B	6	0.2	-4.3	4.7	0.0	
D	6	0.2	-4.3	4.7	0.4	

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
B	6	4	2.8	0.1	5	-2.6	0.1	4	0.750	6.000	8.000	0.250	0.0
		6	2.8	0.2									
D	6	4	2.8	0.1	5	-2.6	0.1	4	0.750	6.000	8.000	0.250	0.0
		6	2.8	0.2									

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8.

BLDG-C REACTIONS

PLEASE VERIFY REACTIONS

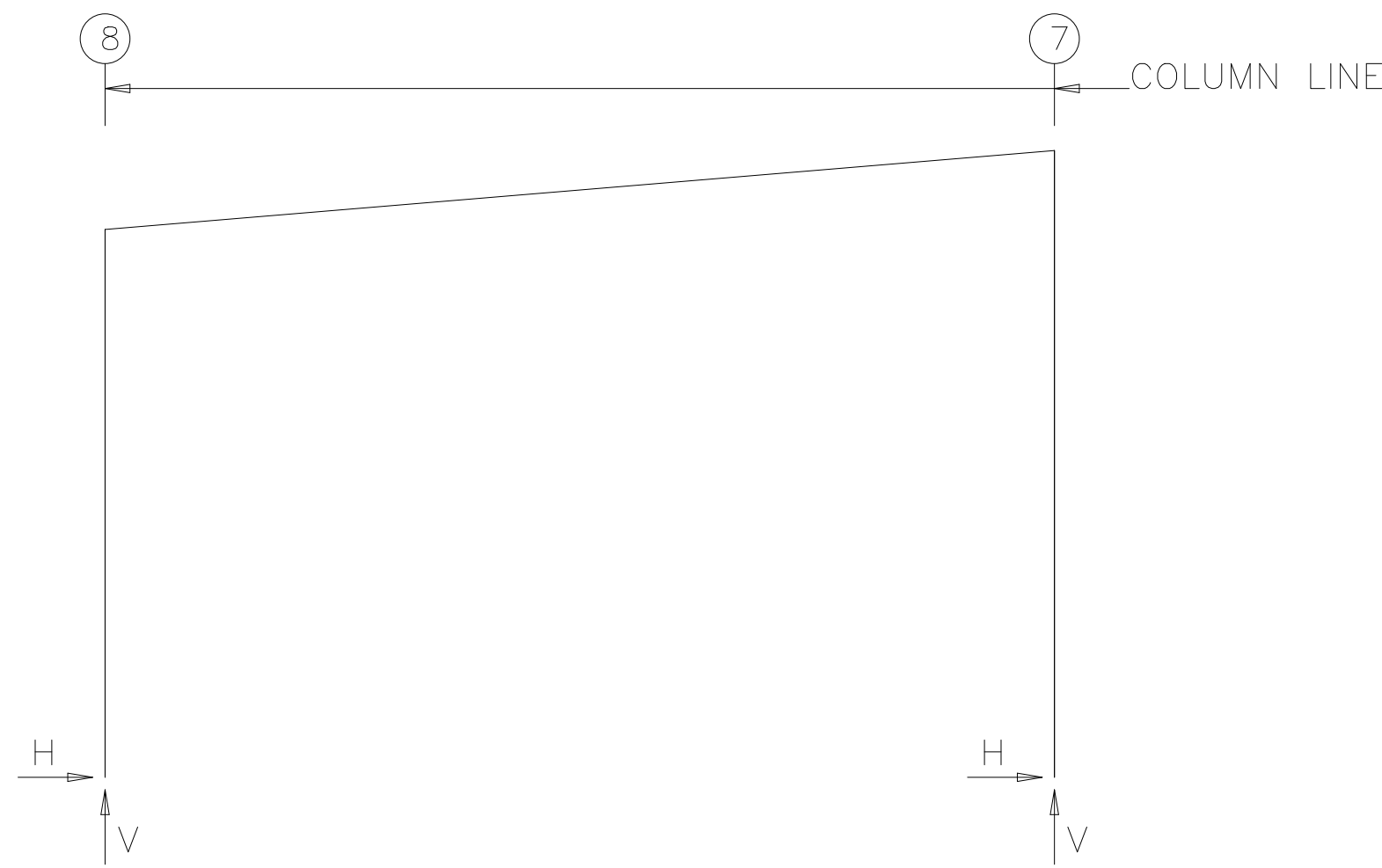
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	ANCHOR ROD REACTION		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	MVU	Checker	JJ
Designer	11/25/2025 HA		
Job No.	251599	Sheet	F4
Issue	A		



FRAME LINES: A



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)							Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick		
A	8	2	2.5	0.2	8	-2.8	-3.4	4	0.750	8.000	12.25	0.375	0.0	
		1	0.7	5.6	6	-2.3	-4.6							
A	7	9	2.6	-2.2	3	-1.5	2.4	4	0.750	6.000	12.25	0.375	0.0	
		1	-0.7	5.5	7	2.2	-3.4							

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	-----Dead-----		-----Collateral-----		-----Live-----		-----Wind_Left1-----		-----Wind_Right1-----		-----Wind_Left2-----	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
A	8	0.1	0.9	0.2	1.2	0.5	3.5	-4.0	-8.7	3.9	-1.3	-4.7	-6.6
A	7	-0.1	0.9	-0.2	1.2	-0.5	3.4	-2.0	-5.1	3.7	-6.5	-1.3	-3.2

Frame Line	Column Line	--Wind_Right2--		--Wind_Long1--		--Wind_Long2--		--Seismic_Left		Seismic_Right		--Seismic_Long	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
A	8	3.2	0.7	1.2	-4.7	1.2	-3.1	-0.3	-0.3	0.3	0.3	0.0	0.0
A	7	4.4	-4.6	-1.0	-4.8	-1.3	-2.9	-0.2	0.3	0.2	-0.3	0.0	0.0

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR ROD LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8".

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 25.1
 - Length (ft) = 50.5
 - Eave Height (ft) = 14.5/ 16.6
 - Roof Slope (rise/12) = 1.00
 - Roof Dead Load (psf) = 2.2
 - Wall Dead Load
 - Left Endwall (psf) = 2.2
 - Right Endwall (psf) = 40.0
 - Front Sidewall (psf) = 2.2
 - Back Sidewall (psf) = 40.0
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 17.0
 - Collateral Load (psf) = 6.0
 - Wind Speed (mph) = 136.0
 - Wind Code = IBC 21
 - Exposure = B
 - Closure = Enclosed
 - Internal Wind Coeff = -0.18, +0.18
 - Risk Category = IV - Post
 - Importance - Wind = 1.00
 - Importance - Seismic = 1.50
 - Seismic Design Category = C
 - Seismic Coeff (Sms) = 0.14
- Loading conditions are:
 - Dead+Collateral+Live
 - Dead+0.6Wind_Right1
 - Dead+Collateral+0.75Live+0.45Wind_Left1
 - Dead+Collateral+0.75Live+0.45Wind_Right1
 - Dead+Collateral+0.75Live+0.45Wind_Long2L
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Right1
 - 0.6Dead+0.6Wind_Left2
 - 0.6Dead+0.6Wind_Right2
 - 0.6Dead+0.6Wind_Long1L

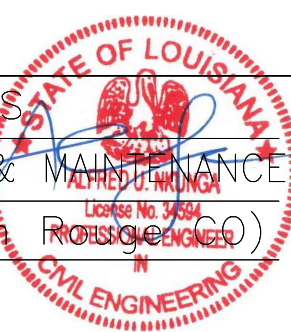
PLEASE VERIFY REACTIONS

BLDG-D REACTIONS

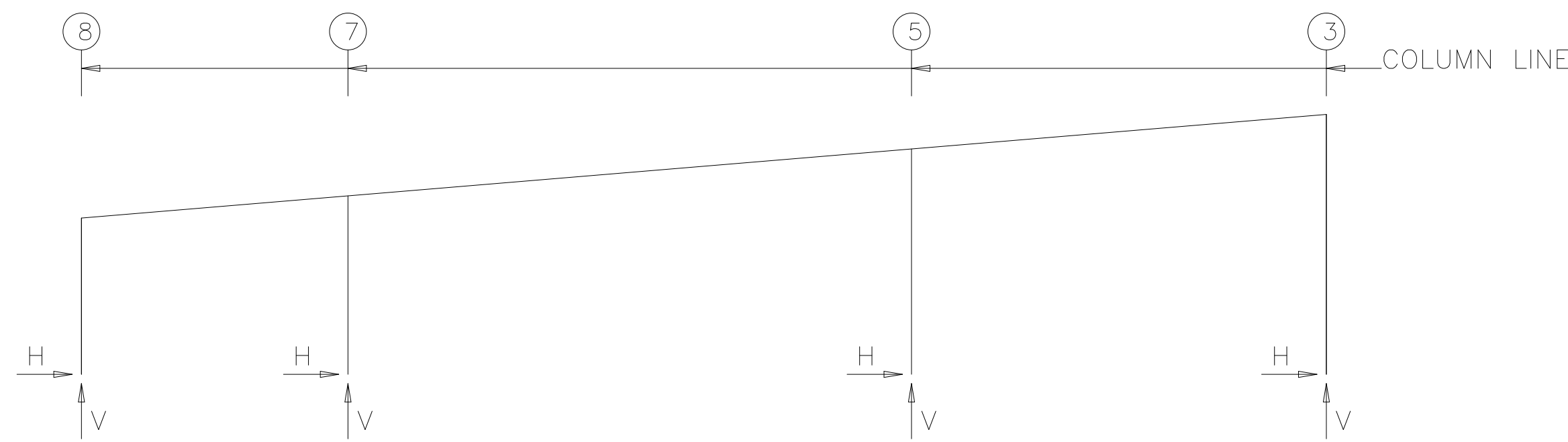
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	ANCHOR ROD REACTION		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	MVU	Checker	JJ
Designer	11/25/2025 HA		
Job No.	251599	Sheet	F5
Issue	A		



FRAME LINES: C D



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
C	8	2	4.2	4.7	7	-4.2	-2.0	4	0.750	8.000	18.25	0.750	0.0
		13	1.6	7.8	8	1.6	-5.5						
C	3	3	0.9	12.7	9	-1.3	-16.8	4	0.750	8.000	18.25	0.750	0.0
		1	0.6	20.5	8	-0.9	-19.6						
C	7	3	1.7	19.8	5	-1.1	-16.9	4	0.750	6.000	10.00	0.750	0.0
		11	1.1	30.5	8	-0.6	-20.1						
C	5	6	0.4	-10.8	11	-0.6	25.5	4	0.750	6.000	10.00	0.750	0.0
		12	-0.2	33.6	8	0.2	-23.5						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
D	8	2	1.8	3.1	7	-1.5	-1.2	4	0.750	8.000	12.25	0.500	0.0
		16	0.7	4.7	8	0.6	-4.1						
D	3	10	0.6	9.4	9	-0.7	-12.5	4	0.750	8.000	12.25	0.500	0.0
		4	-0.3	11.2	8	-0.5	-13.8						
D	7	10	1.7	13.8	5	-1.1	-11.4	4	0.750	6.000	10.00	0.500	0.0
		14	1.0	17.0									
D	5	6	0.4	-6.3	14	-0.6	11.9	4	0.750	6.000	10.00	0.500	0.0
		15	-0.3	15.6	5	0.1	-12.8						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
C	8	0.2	1.1	0.5	1.5	1.0	2.9	-4.7	-6.8	5.9	-0.2	-7.2	-4.4
C	3	0.1	3.4	0.2	5.7	0.4	11.4	-1.4	-18.0	0.0	-20.1	-0.6	-8.8
C	7	0.2	4.4	0.4	8.1	0.8	16.2	-2.1	-32.4	0.4	-24.4	-1.6	-18.3
C	5	0.0	5.1	0.0	9.4	-0.1	18.7	0.0	-38.4	0.7	-23.1	-0.2	-23.0
C	8	4.7	2.4	2.4	-10.2	0.8	-9.9	-1.6	-1.3	1.6	1.3	0.0	-0.7
C	3	0.9	-10.9	-1.6	-36.0	-2.3	-31.4	-0.5	0.4	0.5	-0.3	0.0	-2.3
C	7	1.0	-10.6	-1.3	-37.8	-1.4	-23.9	-0.5	1.2	0.5	-1.2	0.0	-0.7
C	5	0.6	-7.6	0.4	-44.3	0.0	-30.4	-0.2	-0.3	0.2	0.3	0.0	-1.1
C	8	-0.4	1.5	-0.3	-2.4	0.8	5.3	-0.8	-3.1				
C	3	0.4	-1.5	-0.3	5.6	-0.9	7.3	0.5	-1.6				
C	7	0.5	18.0	0.8	11.9	-0.2	3.7	0.8	13.1				
C	5	-0.5	11.0	-0.1	19.1	0.3	7.5	-0.5	11.3				
D	8	0.1	0.7	0.3	1.7	0.4	1.4	-2.0	-3.9	2.4	-0.8	-2.6	-2.6
D	3	0.0	2.2	0.1	3.0	0.2	5.9	-0.7	-12.8	-0.4	-14.7	-0.2	-7.9
D	7	0.2	2.5	0.3	6.6	0.7	7.2	-2.1	-21.4	0.0	-13.4	-1.5	-15.3
D	5	0.0	2.9	-0.1	4.2	-0.1	8.3	0.3	-24.3	0.7	-13.4	0.1	-17.5
D	8	2.0	0.4	0.9	-7.6	0.3	-7.4	-1.8	-1.6	1.8	1.6	0.0	-0.7
D	3	0.0	-9.8	-0.9	-25.1	-1.2	-23.1	-0.5	0.5	0.5	-0.4	0.0	-2.3
D	7	0.6	-7.3	-1.2	-18.7	-1.3	-12.6	-1.2	1.4	1.2	-1.4	0.0	-0.7
D	5	0.6	-6.5	0.3	-23.4	0.0	-17.2	-0.5	-0.2	0.5	0.2	0.0	-1.1
D	8	-0.1	0.8	-0.3	-1.0	0.3	2.2	-0.4	-1.2				
D	3	0.2	-0.6	-0.2	2.6	-0.4	3.2	0.2	-0.6				
D	7	0.5	7.9	0.6	5.3	-0.2	1.8	0.7	5.7				
D	5	-0.5	4.8	-0.2	8.5	0.3	3.5	-0.5	4.9				

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 115.4
 - Length (ft) = 50.5
 - Eave Height (ft) = 14.5/ 24.1
 - Roof Slope (rise/12) = 1.00
 - Roof Dead Load (psf) = 2.2
 - Wall Dead Load (psf) = 2.2
 - Left Endwall (psf) = 2.2
 - Right Endwall (psf) = 40.0
 - Front Sidewall (psf) = 2.2
 - Back Sidewall (psf) = 2.2
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 12.0
 - Collateral Load (psf) = 6.0
 - Wind Speed (mph) = 136.0
 - Wind Code = IBC 21
 - Exposure = B
 - Closure = Enclosed
 - Internal Wind Coeff = -0.18, +0.18
 - Risk Category = IV - Post
 - Importance - Wind = 1.00
 - Importance - Seismic = 1.50
 - Seismic Design Category = C
 - Seismic Coeff (Sms) = 0.14

- Loading conditions are:
 - 1 Dead+Collateral+Live
 - 2 Dead+Collateral+0.75Live+0.45Wind_Right1
 - 3 Dead+Collateral+0.75Live+0.45Wind_Right2
 - 4 Dead+Collateral+0.75Live+0.45Wind_Long2R
 - 5 0.6Dead+0.6Wind_Left1
 - 6 0.6Dead+0.6Wind_Right1
 - 7 0.6Dead+0.6Wind_Left2
 - 8 0.6Dead+0.6Wind_Long1L
 - 9 0.6Dead+0.6Wind_Long2L
 - 10 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Right
 - 11 Dead+Collateral+F1PAT_LL_1
 - 12 Dead+Collateral+F1PAT_LL_2
 - 13 Dead+Collateral+F1PAT_LL_3
 - 14 Dead+Collateral+F2PAT_LL_1
 - 15 Dead+Collateral+F2PAT_LL_2
 - 16 Dead+Collateral+F2PAT_LL_3

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTORS RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8.

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k)				Panel Shear (lb/ft)		Note
		Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
F_SW	3	7.0	13.3	1.2	2.3			(b)
R_SW	D							(h)
B_SW	8	4.4	4.8	0.6	0.7			(b)
INT	7	3.0	3.7	0.5	0.7			(d)
	5	4.2	6.8	0.7	1.1			(d)

(b) Wind bent in bay, base above finish floor
 (d) X-Bracing above wind bent, base above finish floor
 (h) Rigid frame at endwall

Reactions for seismic represent shear force, Eh
 Reaction values shown are unfactored

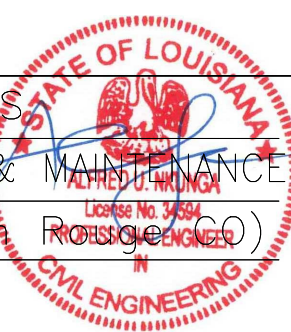
BLDG-B,C&D REACTIONS

PLEASE VERIFY REACTIONS

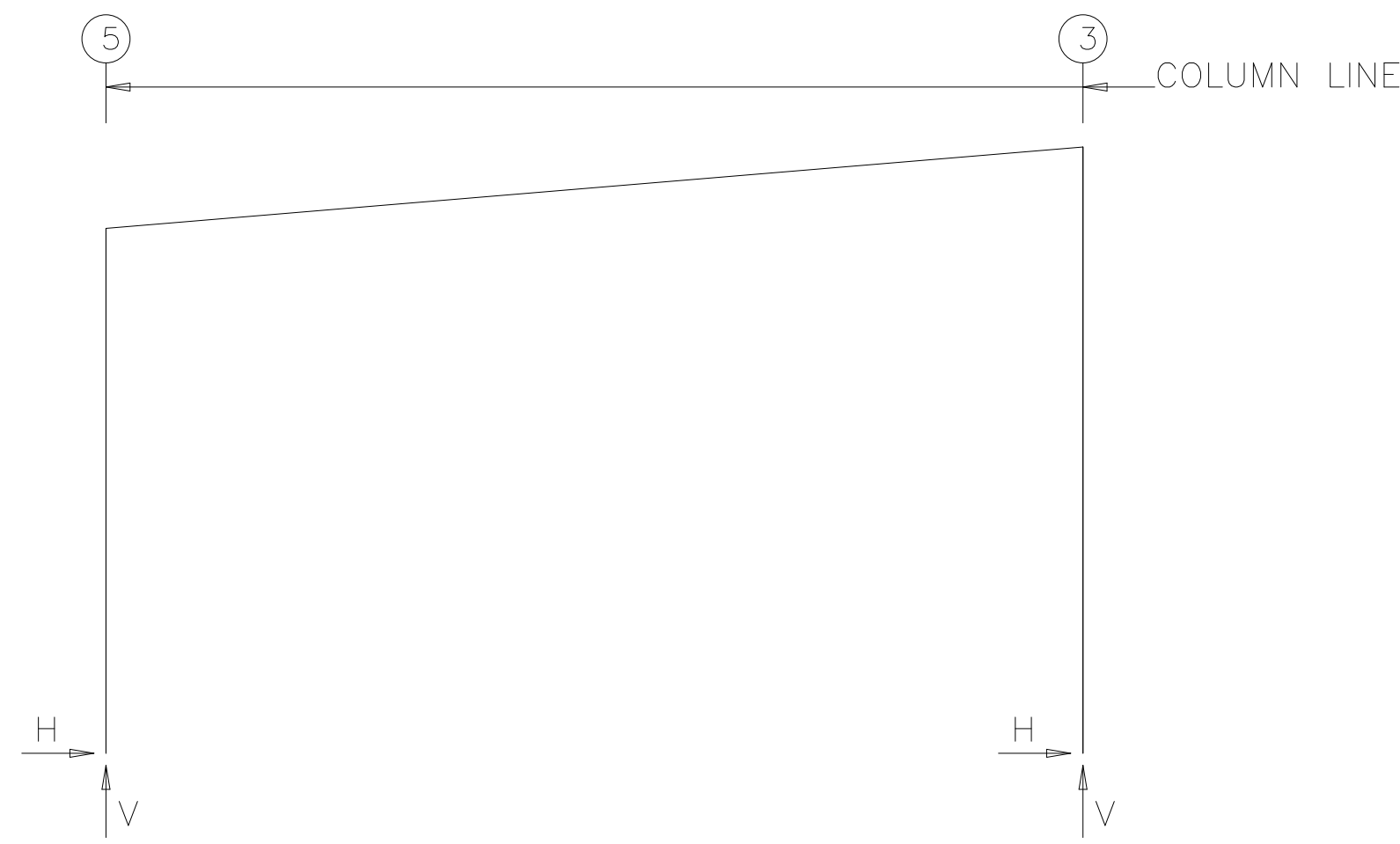
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	ANCHOR ROD REACTIONS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	MVU	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	F6
Issue	A		



FRAME LINES: A C D



NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 38.9
 - Length (ft) = 50.5
 - Eave Height (ft) = 20.9/ 24.1
 - Roof Slope (rise/12) = 1.00
 - Roof Dead Load (psf) = 2.2
 - Wall Dead Load
 - Left Endwall (psf) = 2.2
 - Right Endwall (psf) = 40.0
 - Front Sidewall (psf) = 2.2
 - Back Sidewall (psf) = 2.2
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 13.3
 - Collateral Load (psf) = 6.0
 - Wind Speed (mph) = 136.0
 - Wind Code = IBC 21
 - Exposure = B
 - Closure = Enclosed
 - Internal Wind Coeff = -0.18, +0.18
 - Risk Category = IV - Post
 - Importance - Wind = 1.00
 - Importance - Seismic = 1.50
 - Seismic Design Category = C
 - Seismic Coeff (Sms) = 0.14

- Loading conditions are:
 - 1 Dead+Collateral+Live
 - 2 Dead+0.6Wind_Left2
 - 3 Dead+0.6Wind_Long2R
 - 4 Dead+Collateral+0.75Live+0.45Wind_Left2
 - 5 Dead+Collateral+0.75Live+0.45Wind_Right2
 - 6 0.6Dead+0.6Wind_Left1
 - 7 0.6Dead+0.6Wind_Right1
 - 8 0.6Dead+0.6Wind_Right2
 - 9 0.6Dead+0.6Wind_Long1L
 - 10 1.01Dead+1.01Collateral+0.75Live+0.53Seismic_Left
 - 11 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
 - 12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - 13 Dead+0.6Wind_Right2+0.6Wind_Suction
 - 14 0.6Dead+0.6Wind_Pressure
 - 15 Dead+0.6Wind_Suction

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
A	4	5	2.6	6.1	6	-2.9	-5.6	4	0.750	6.000	23.75	0.375	0.0
A	2	8	1.3	-6.0	3	-0.9	-3.5	4	0.750	6.000	23.75	0.375	0.0
		1	-0.1	13.3	7	0.6	-8.5						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
B	4	7	8.4	-2.5	4	-3.9	13.0	4	0.750	10.00	30.75	0.625	0.0
B	2	7	-2.6	26.0	6	0.8	-16.0						
		1	-5.6	-16.7	10	-5.7	28.1	4	0.750	10.00	30.75	0.625	0.0
		1	-5.6	31.0	9	4.2	-22.5						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
C	4	7	3.1	-2.4	2	-2.5	-6.1	4	0.750	6.000	23.81	0.500	0.0
C	2	7	-0.1	11.0	6	-1.3	-9.2						
		1	1.5	-9.6	10	-1.8	12.5	4	0.750	6.000	23.75	0.500	0.0
		1	-1.2	13.6	9	1.1	-12.4						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
A	4	0.2	1.5	0.3	1.6	0.8	3.7	-5.0	-10.8	3.3	-3.1	-4.9	-7.1
A	2	-0.1	2.4	0.0	3.2	0.0	7.7	-1.1	-13.0	1.1	-16.6	0.0	-8.8
B	4	-0.3	4.3	-1.2	9.5	-1.2	12.2	1.6	-31.0	14.2	-8.5	-3.4	-22.3
B	2	-0.7	5.1	-2.5	10.7	-2.4	15.2	5.6	-27.6	9.8	-32.9	3.1	-13.9
C	4	0.0	2.2	-0.1	2.9	0.0	6.0	-2.1	-17.5	5.1	-6.1	-4.2	-13.7
C	2	-0.2	2.6	-0.3	3.6	-0.7	7.4	1.3	-15.8	2.7	-18.6	0.4	-9.1

Frame Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right		Seismic_Long	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
A	4	3.4	0.6	1.0	-6.6	1.2	-4.5	-0.6	-0.6	0.5	0.6	0.0	0.0
A	2	2.2	-12.5	-0.7	-12.5	-1.4	-9.8	-0.6	0.6	0.6	-0.5	0.0	0.0
B	4	9.2	0.2	12.1	-27.2	12.7	-22.5	-1.3	-1.5	1.3	1.4	0.0	-1.7
B	2	7.4	-19.2	7.7	-42.6	6.2	-36.4	-1.3	1.5	1.3	-1.4	0.0	-1.7
C	4	3.0	-2.3	4.0	17.5	4.2	-15.3	-1.3	-1.4	1.3	1.3	0.0	-1.7
C	2	1.7	-12.0	2.1	-23.2	1.4	-20.5	-1.3	1.4	1.3	-1.3	0.0	-1.7

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press		Wind Suct		Seis Long	
			Horz	Vert	Horz	Vert	Horz	Vert
A	3	0.3	-4.2	4.6	0.0			
C	3	0.3	-4.2	4.6	0.4			

SOLDIER COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collateral		Live		Wind_Press		Wind_Suct		Wind Long1		Seis Left Vert
			Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	
B1	4	1.2	-0.1	1.6	-0.2	4.0	3.2	-9.2	-2.8	-5.1	-2.3	0.0	

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
A	3	11	2.8	0.2	12	-2.5	0.2	4	0.750	6.000	8.000	0.250	0.0
		13	2.8	0.3									
C	3	11	2.8	0.2	12	-2.5	0.2	4	0.750	6.000	8.000	0.250	0.0
		13	2.8	0.3									

SOLDIER COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
B1	4	14	1.9	-4.8	15	-1.7	-1.9	4	0.750	6.000	8.000	0.375	0.0
		1	-0.3	6.7	14	1.9	-4.8						

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Reactions(k)				Panel_Shear (lb/ft)		Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L-EW	A							(h)	
F-SW	2	B,C	3.9	7.4	0.9	1.7		(b)	
R-EW	C							(h)	
B-SW	4	B,C	4.9	7.9	1.0	1.7		(b)	

(b) Wind bent in bay, base above finish floor
(h) Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

GENERAL NOTES

- ANCHOR RODS ARE NOT DESIGNED TO STABILIZE THE COLUMNS DURING ERECTION. TEMPORARY BRACING AS NEEDED FOR SAFETY AND STABILITY IS THE ERECTOR'S RESPONSIBILITY.
- FOUNDATION DESIGN AND ANCHOR RODS LENGTHS ARE NOT THE RESPONSIBILITY OF PINNACLE STRUCTURES, INC.
- THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE ANCHOR ROD SUMMARY TABLE REPORTS THE ROD DIAMETERS.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.
- ANCHOR RODS SHALL BE ACCURATELY SET TO A TOLERANCE OF 1/8".

BLDG-B REACTIONS

PLEASE VERIFY REACTIONS

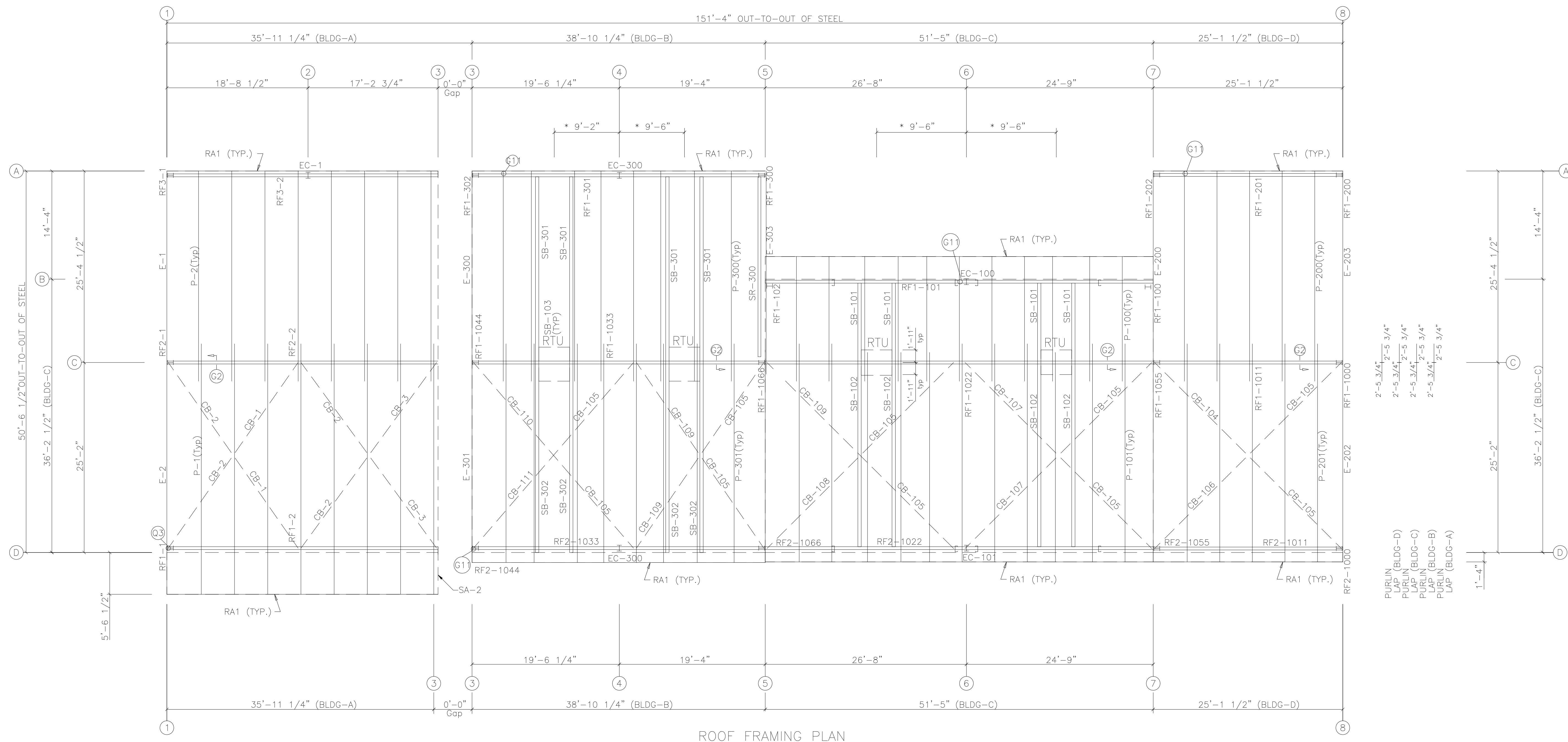
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	ANCHOR ROD PLAN		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge)		
Detailer	MVU	Checker	JJ
Designer	11/25/2025 HA		
Job No.	251599	Sheet	F7
Issue	A		



NOTE:
 Alternate Arrows ▽-△
 Up And Down From Bay
 To Bay For Purlins To Lap.



ROOF FRAMING PLAN

MEMBER TABLE	
ROOF FRAMING PLAN	
MARK	PART
BLDG-A	
P-1	10X35Z14
P-2	10X35Z14
F-1	10ES14
F-2	10ES14
CB-1	1/2"ROD
CB-2	1/2"ROD
CB-3	1/2"ROD
BLDG-C	
P-100	10X35Z14
P-101	10X35Z14
SB-101	W10542
SB-102	W10542
SB-103	W10542
BLDG-D	
P-200	10X35Z14
P-201	10X35Z14
F-200	10ES14
F-202	10ES14
F-203	10ES14
BLDG-B	
SR-300	W12642
CB-301	W10542
CB-302	W10542
SB-103	W10542
P-300	10X35Z14
P-301	10X35Z14
F-300	10ES14
F-301	10ES14
F-303	10ES14
BRACINGS	
CB-104	1/2"ROD
CB-105	1/2"ROD
CB-106	1/2"ROD
CB-107	1/2"ROD
CB-108	1/2"ROD
CB-109	1/2"ROD
CB-110	1/2"ROD
CB-111	1/2"ROD

2'-5 3/4" 2'-5 3/4"
 2'-5 3/4" 2'-5 3/4"
 2'-5 3/4" 2'-5 3/4"
 2'-5 3/4" 2'-5 3/4"

COLLATERAL LOAD NOTE:
 Roof purlin has been designed for the collateral load listed on the cover. The total applied loads due to ceiling panels, ducts, sprinkler distribution lines, electrical equipment, conduit, fireproofing, other piping or mechanical loads cannot exceed this maximum uniform load. Pinnacle Structures, Inc. is not responsible for lateral or longitudinal bracing of suspended members subject to lateral seismic or wind loading.

Loads supported directly from the purlins must have connections through the web of the purlin.

Loads supported between purlins must be supported such that the loads are applied to the webs of the purlins.

**** PLEASE VERIFY ALL DIMENSIONS ****

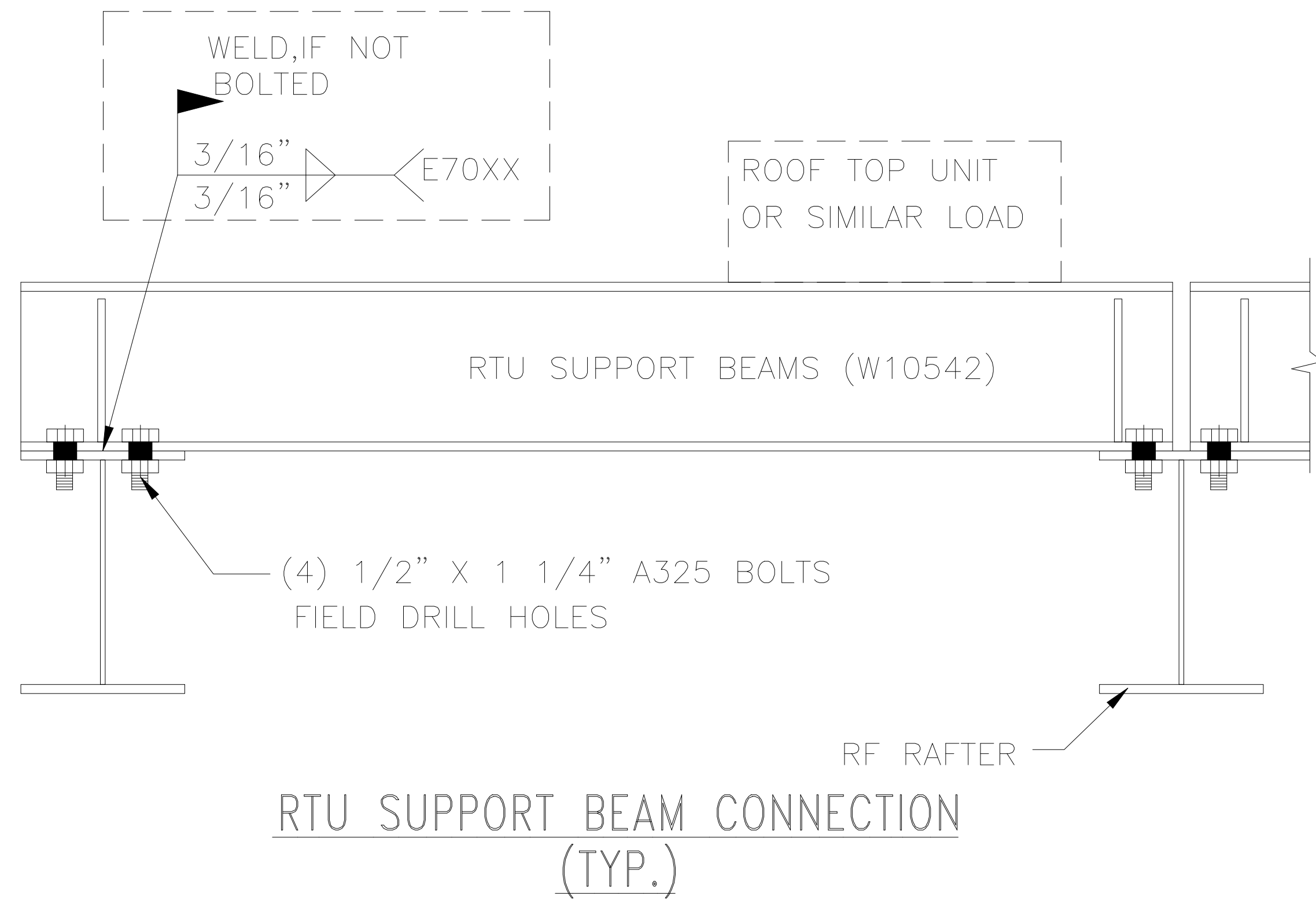
**** CUSTOMER TO CONFIRM ALL RTU OPENING LOCATIONS ****

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

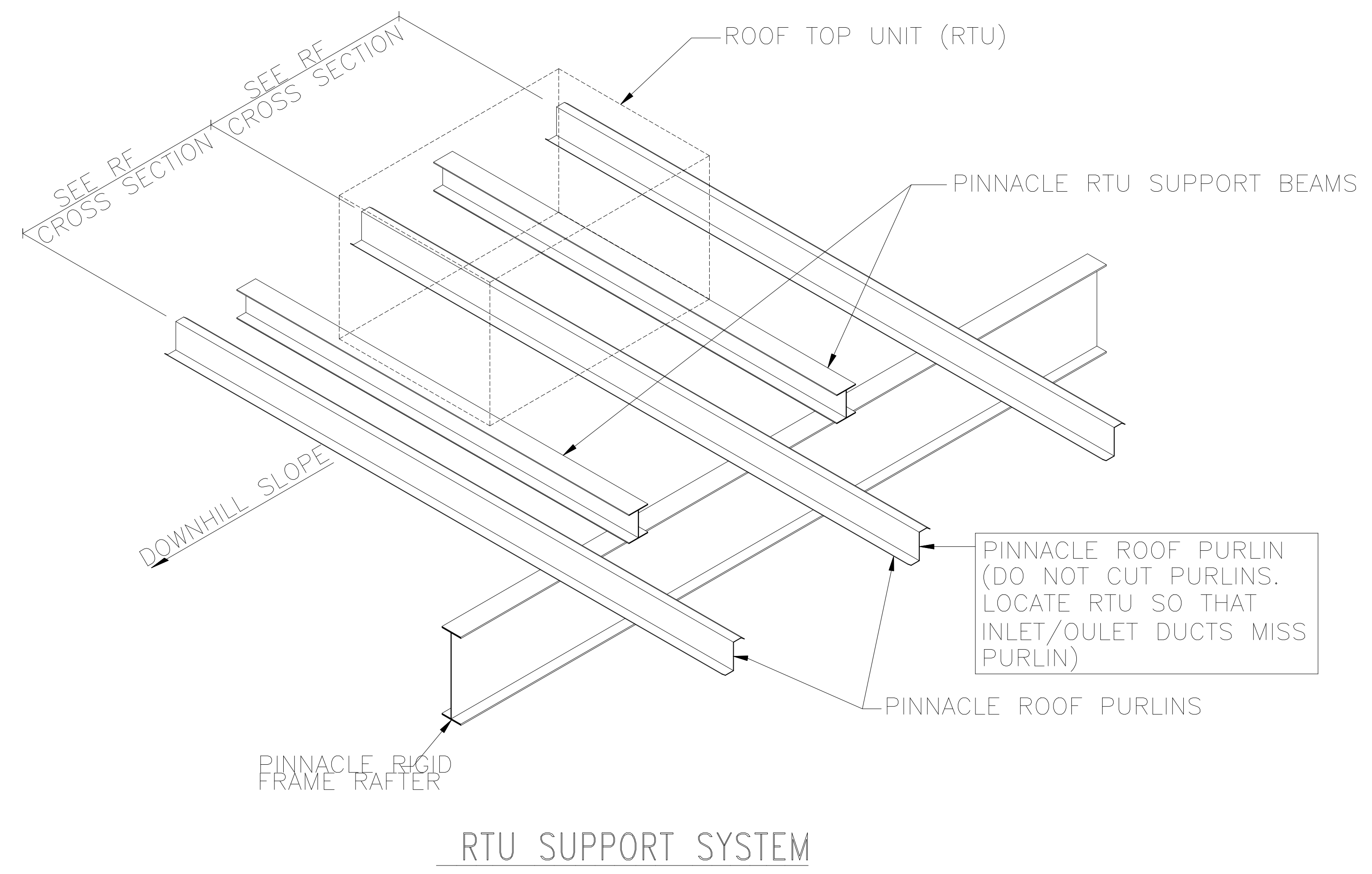
PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF FRAMING PLAN				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	SAN	Checker	JJ	Designer	HA
Job No.	251599	Sheet	E01	Issue	A





**** PLEASE VERIFY PURLIN SPACING IS ADEQUATE FOR RTU INSTALLATION WITHOUT CUTTING PURLINS. ****



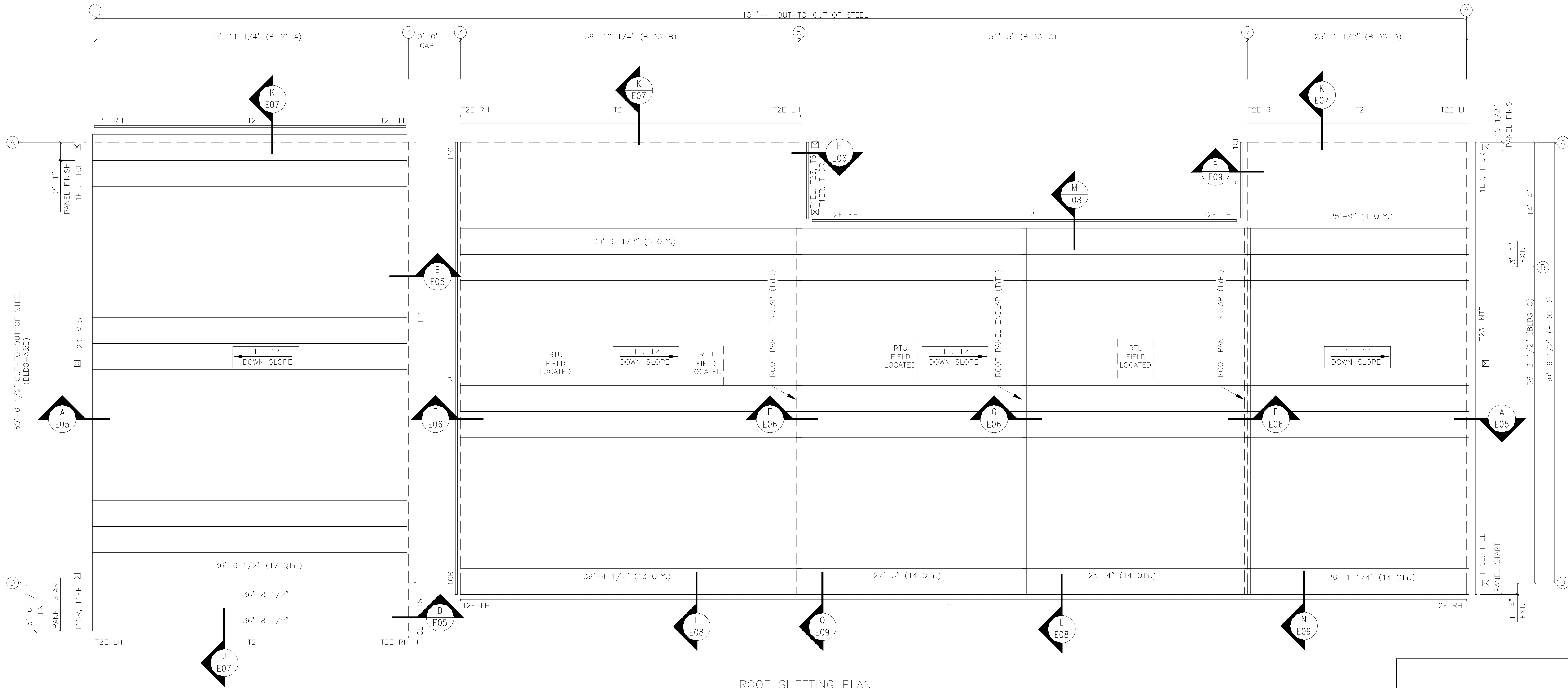
ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF FRAMING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge)		
Detailer	SAN	Checker	JJ
Designer	11/25/2025 HA		
Job No.	251599	Sheet	E02
Issue	A		



☒ - INDICATES DOWNSPOUT LOCATIONS



ROOF SHEETING PLAN
 PANELS: 24 Gg. PBR - NEED COLOR
 NOTE: FIELD CUT/LAP ROOF PANELS AS REQUIRED

**** CUSTOMER TO VERIFY ALL THE DIMENSIONS ****

**** SEE ROOF FAMING PLAN FOR RTU LOCATION ****

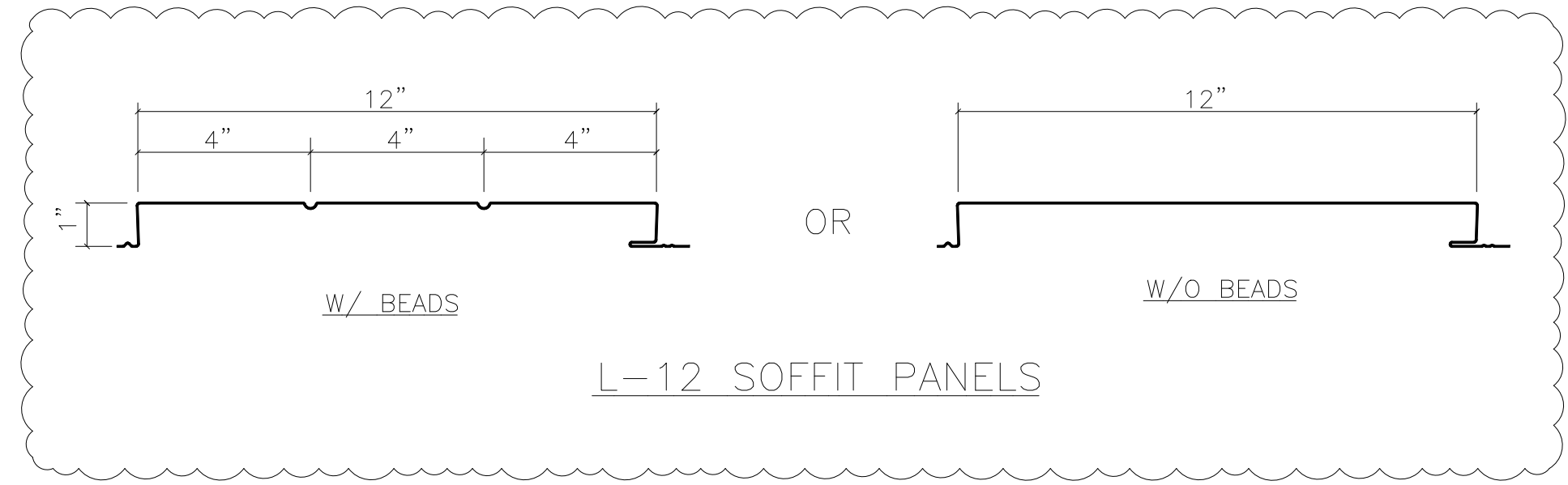
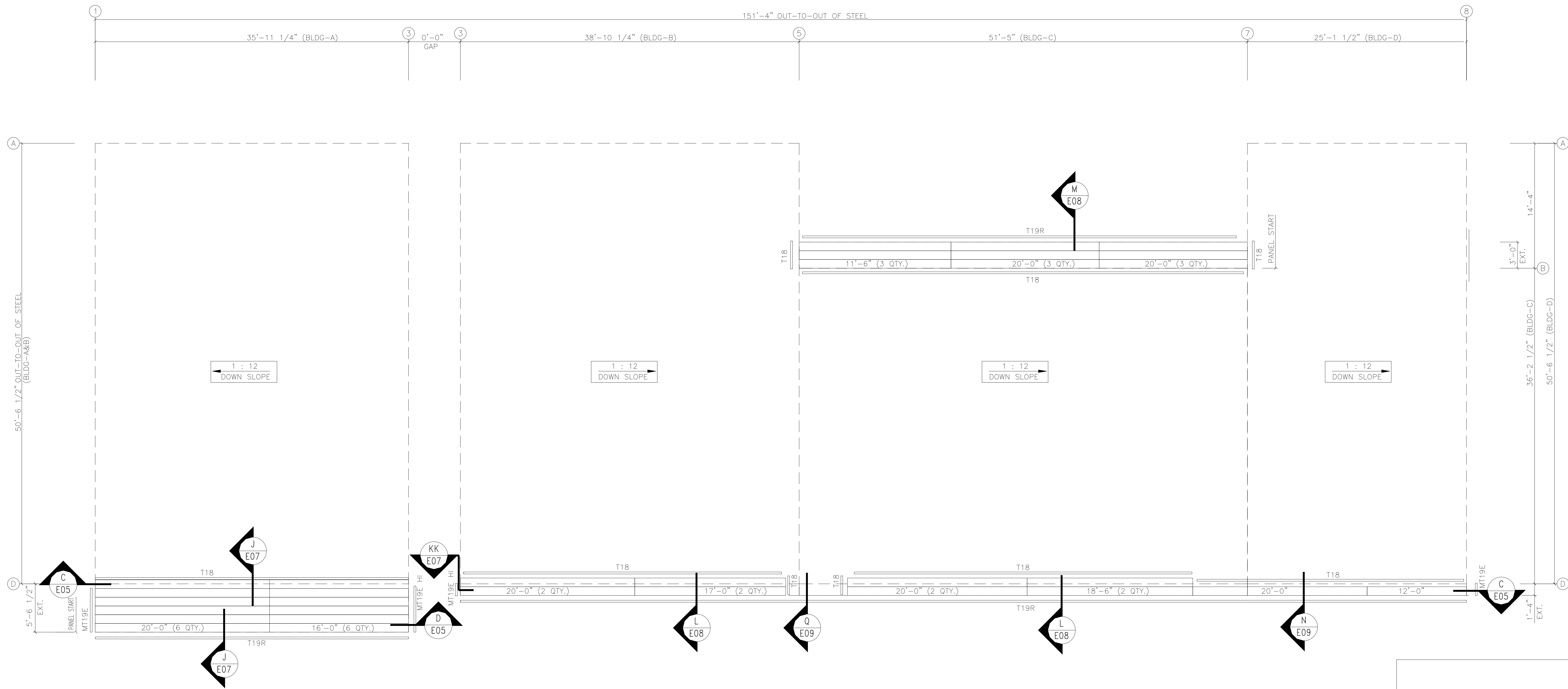
ROOF CURB NOTE:
 It is the responsibility of the customer to verify with the P.E.M.B. manufacturer that any roof curbs installed on this roof meet the requirements for the roof warranty when applicable.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF SHEETING PLAN		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E03
Issue	A		





ROOF SOFFIT SHEETING PLAN
 PANELS: 24 Ga. L-12 - NEED COLOR
 NOTE: FIELD CUT SOFFIT PANELS AS REQUIRED
 NOTE: USE PANEL DROP AS A SPACER IF REQUIRED

ROOF CURB NOTE:
 It is the responsibility of the customer to verify with the P.E.M.B. manufacturer that any roof curbs installed on this roof meet the requirements for the roof warranty when applicable.

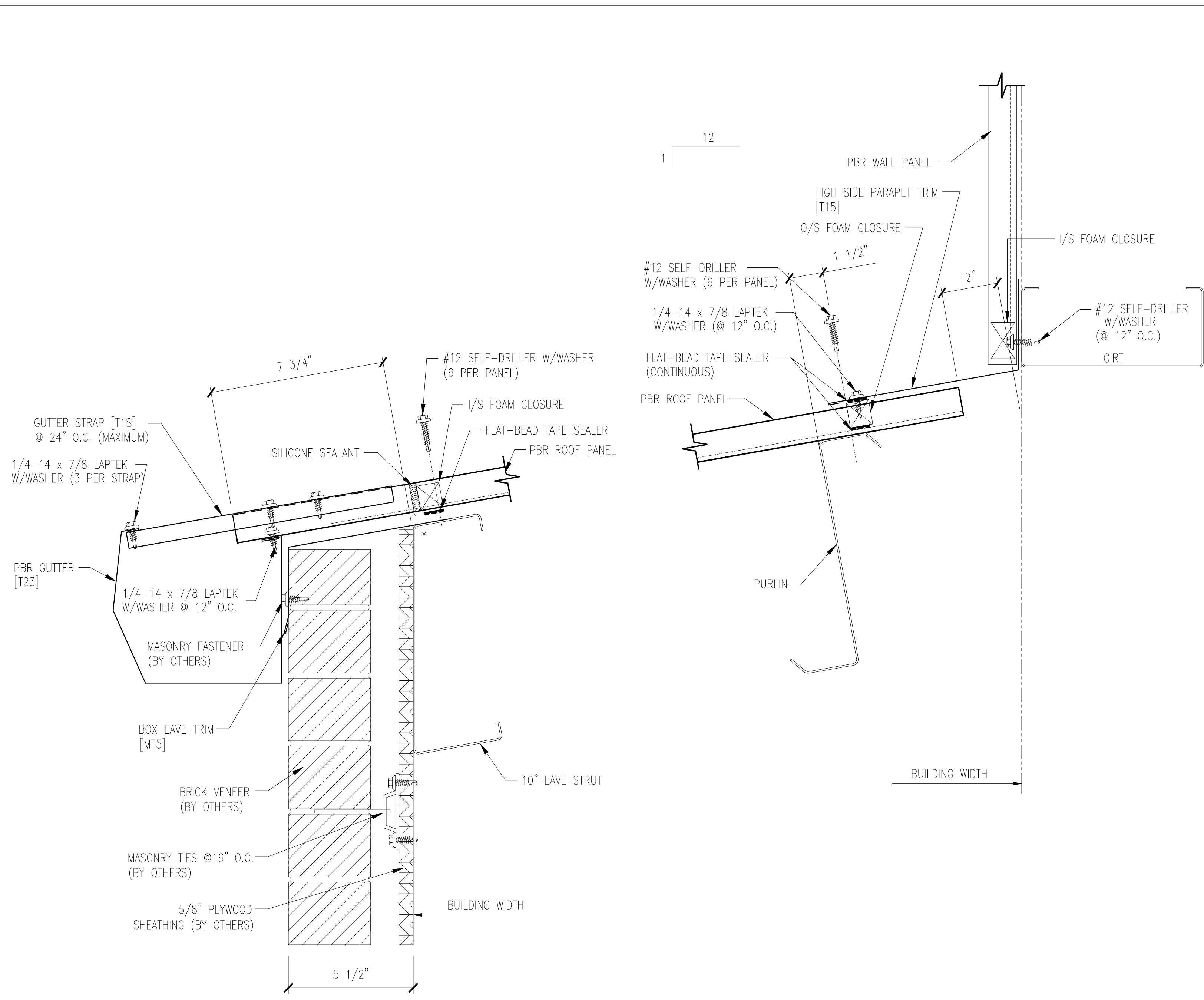
**** CUSTOMER TO VERIFY CLOUDED INFORMATION ****

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

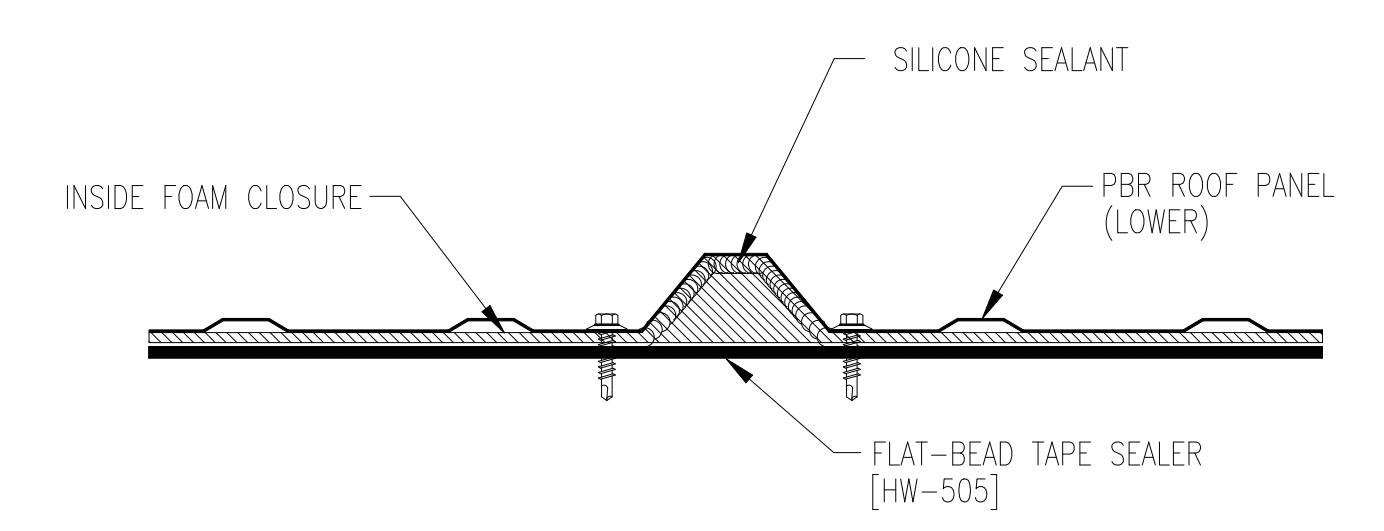
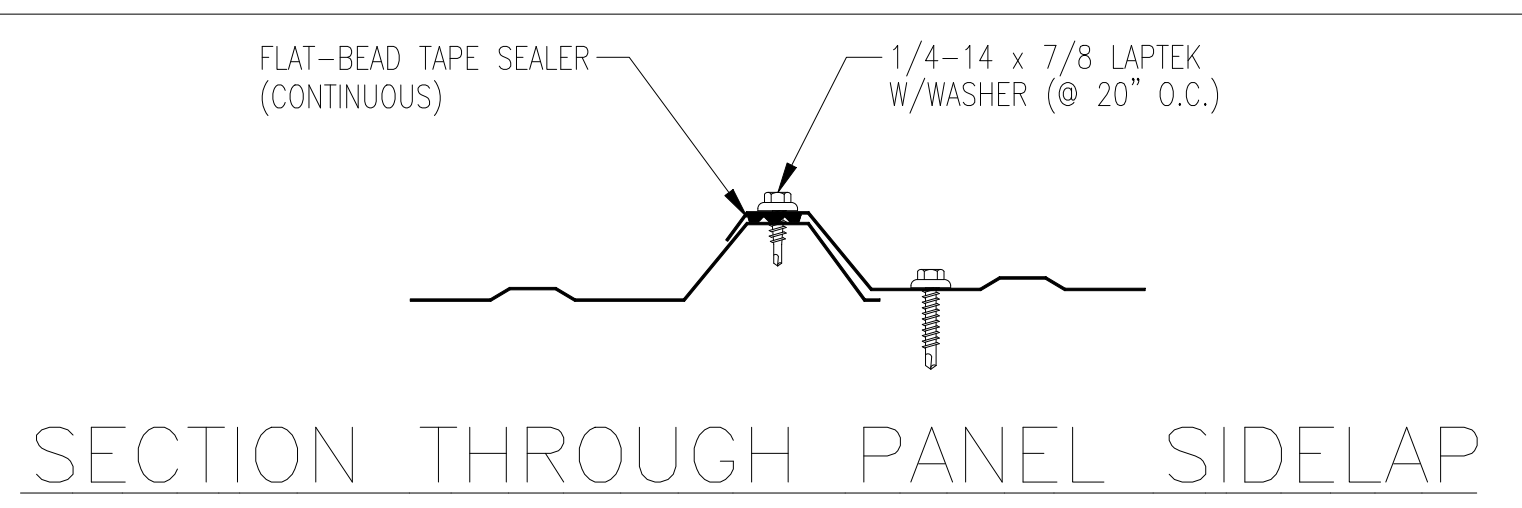
DESCRIPTION:	ROOF SOFFIT SHEETING PLAN		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Job No.	251599	Sheet	E04
Designer	HA	Issue	A



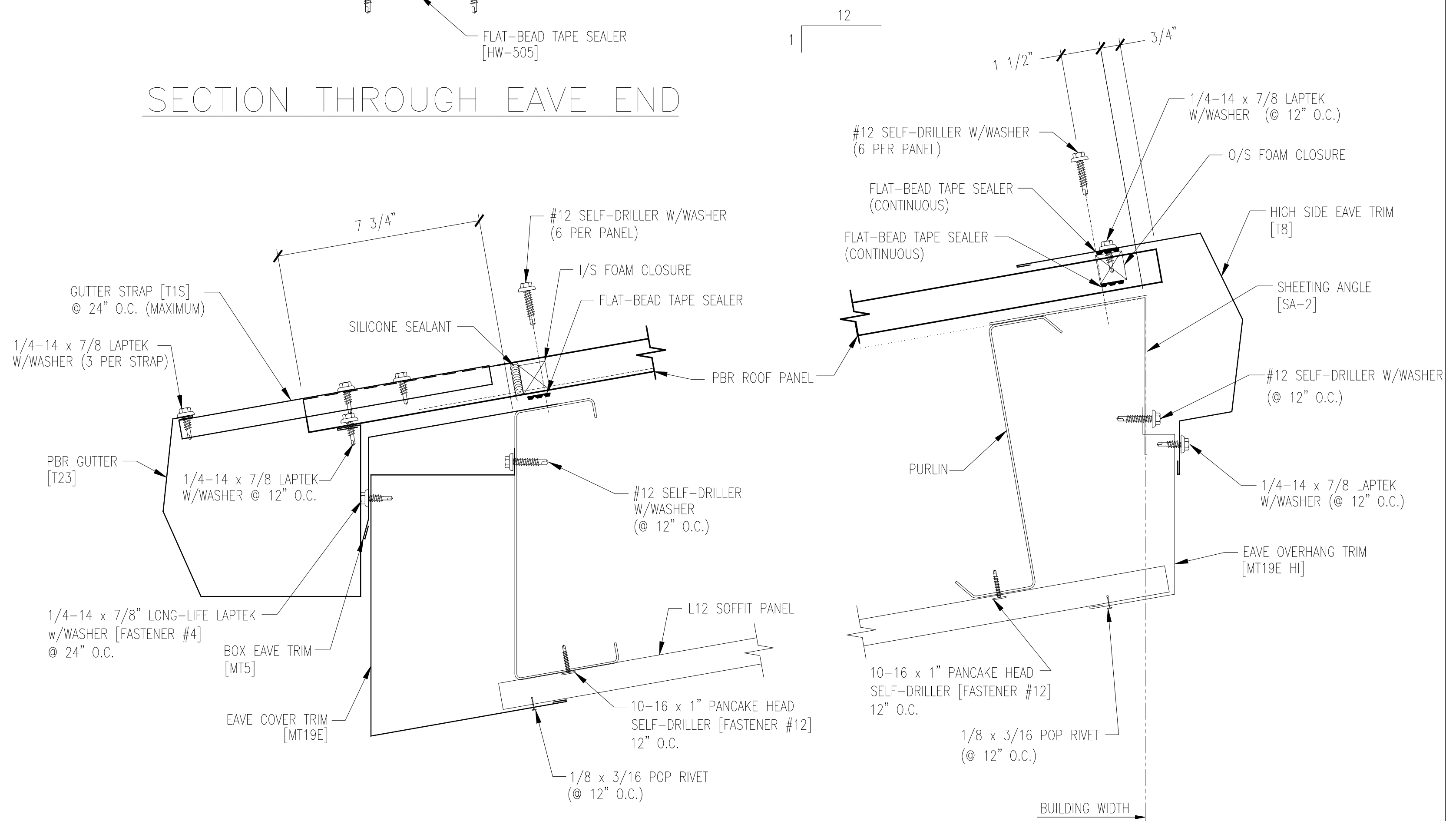


A SECTION THROUGH EAVE STRUT
E05 STANDARD GUTTER
 * INDICATES INSULATION BY OTHERS

B SECTION THROUGH TIE-IN
E05



SECTION THROUGH EAVE END



C SECTION THROUGH EAVE OVERHANG w/ GUTTER
E05

D SECTION THROUGH HIGH-SIDE EAVE
E05

**** CUSTOMER NEED TO VERIFY ALL DETAILS ****

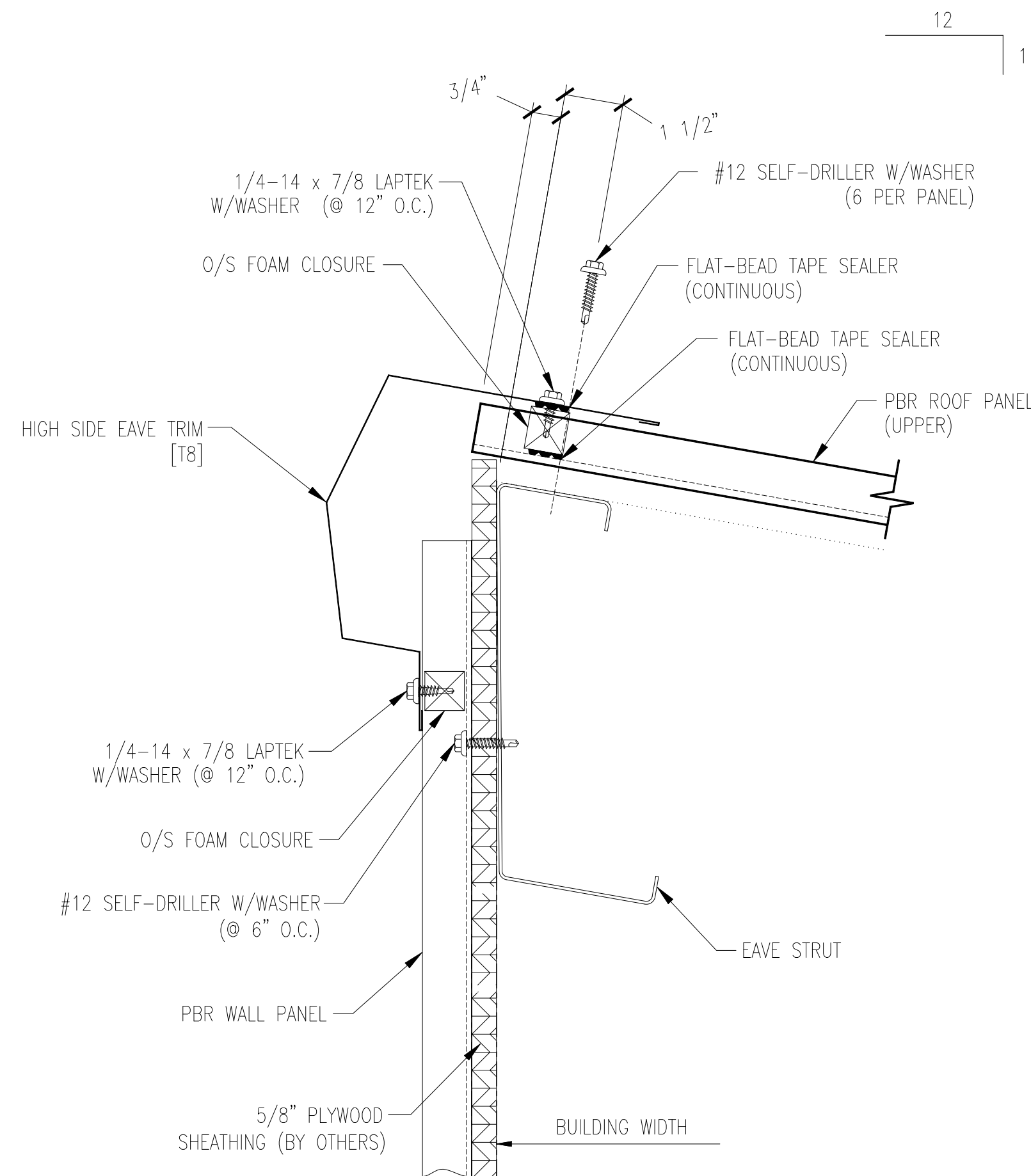
NOTE:
 STANDARD PINNACLE TRIM IS SHOWN.
 IF A DIFFERENT PROFILE IS DESIRED, THEN
 P.S.I. MUST BE SUPPLIED WITH A SKETCH AND
 DIMENSIONS.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

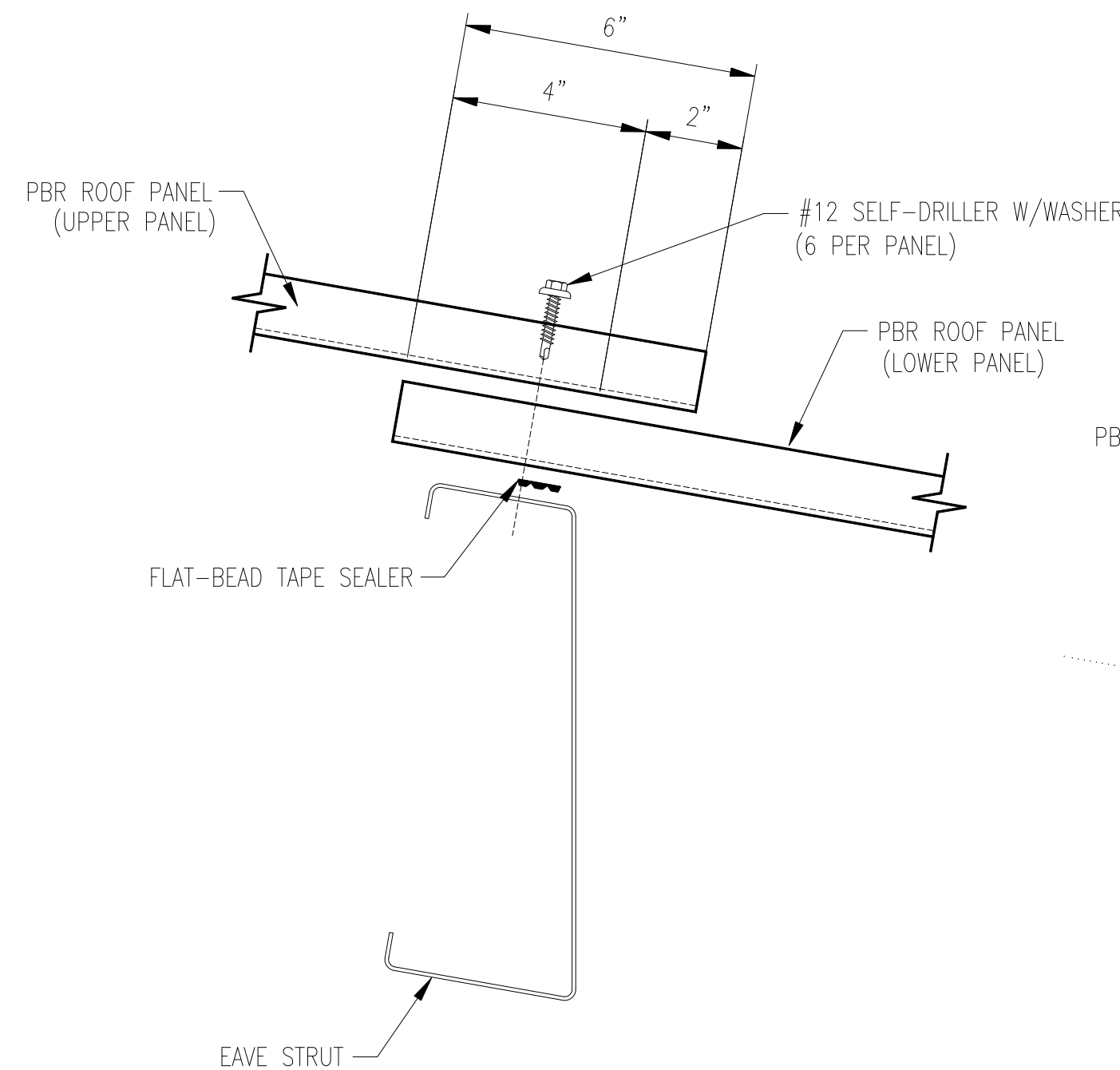


DESCRIPTION:	ROOF SHEETING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E05
Issue	A		

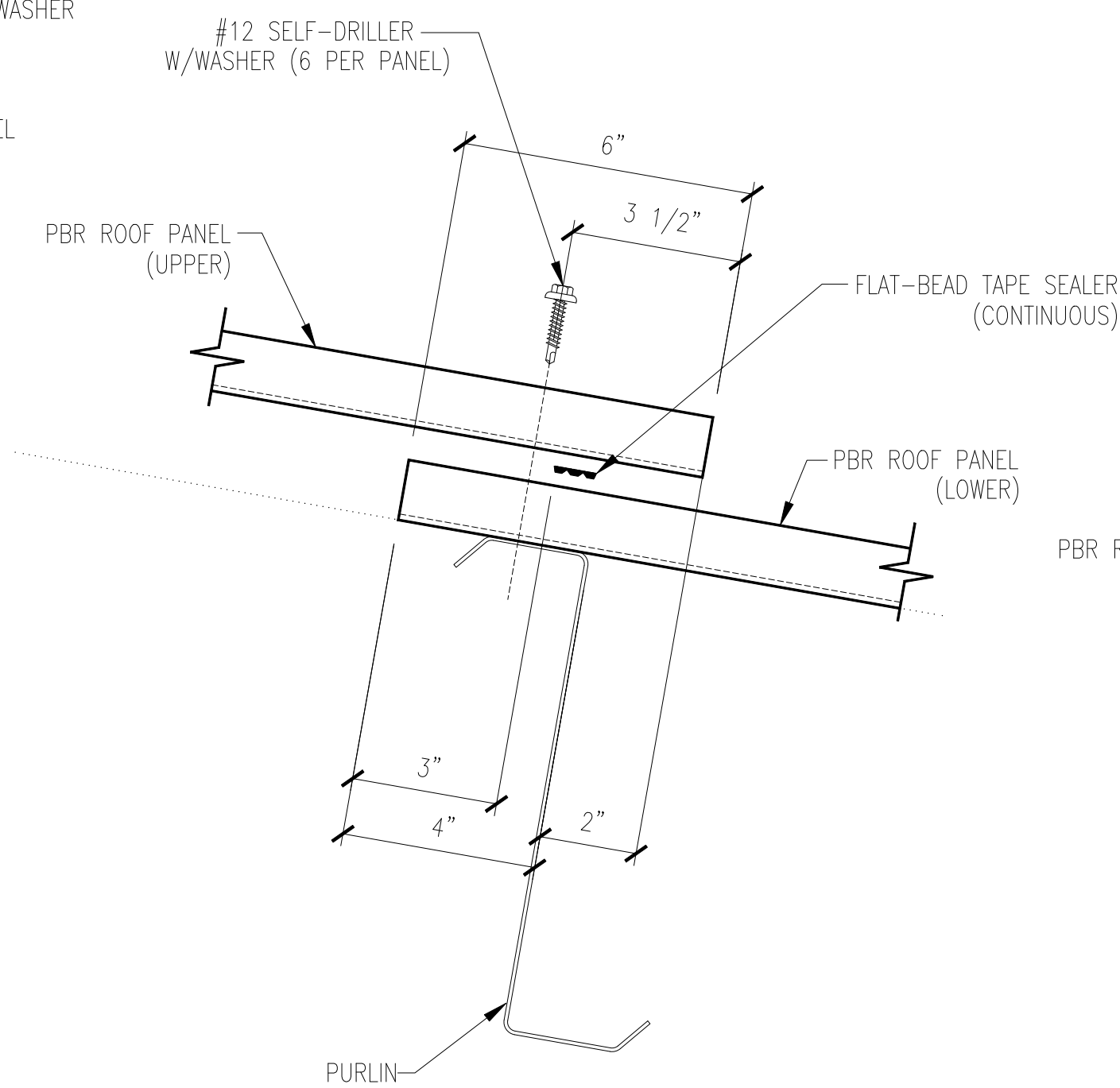




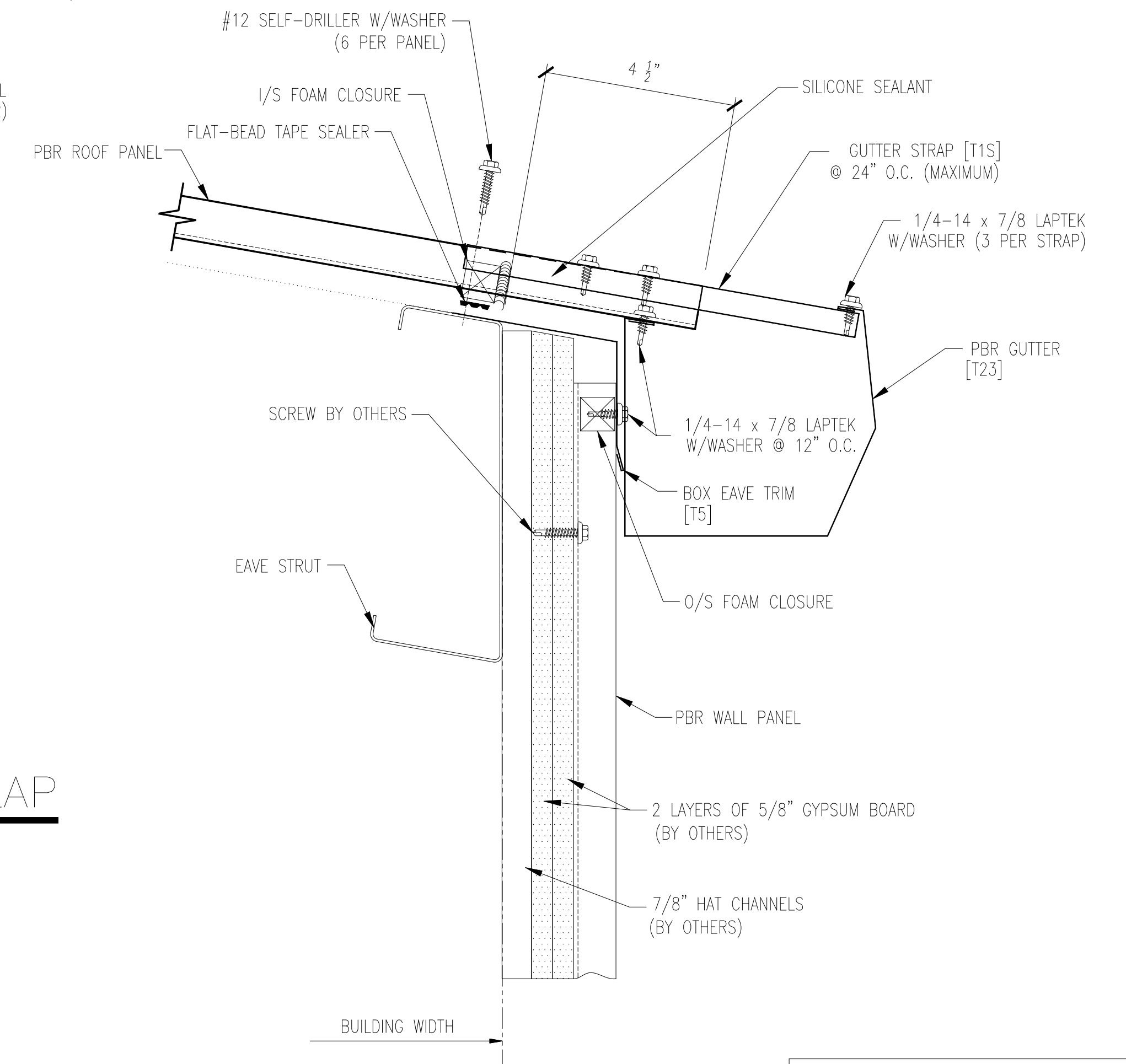
E SECTION THROUGH
E06 HIGH-SIDE EAVE STRUT



F SECTION THROUGH EAVE STRUT
E06 WITH CONTINUOUS ROOF PANEL



G SECTION THROUGH ENDLAP
E06



H SECTION THROUGH
E06 EAVE STRUT w/ GUTTER

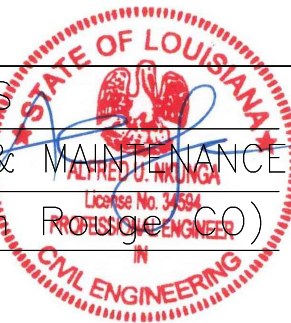
**** CUSTOMER NEED TO VERIFY ALL DETAILS ****

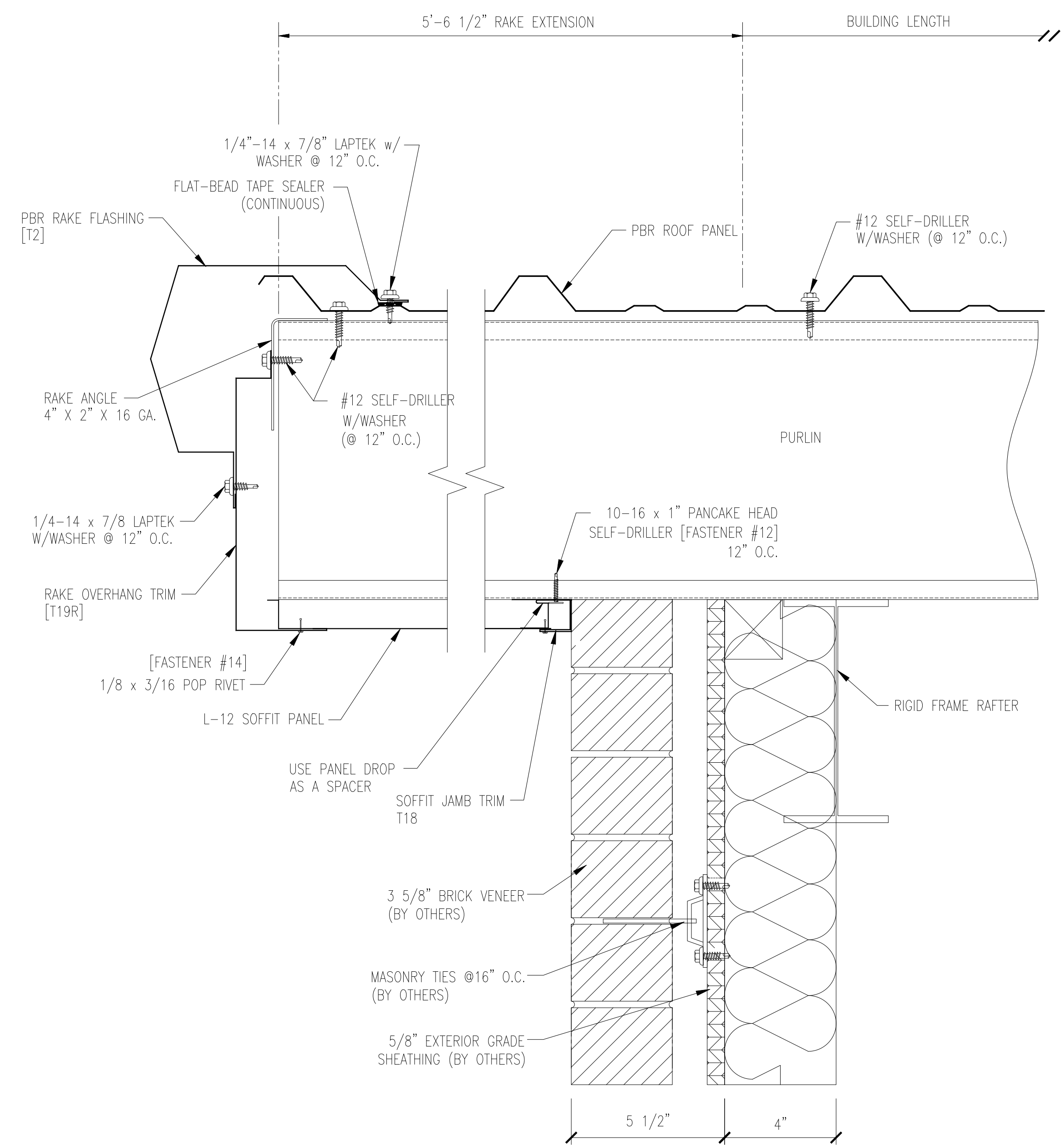
NOTE:
STANDARD PINNACLE TRIM IS SHOWN.
IF A DIFFERENT PROFILE IS DESIRED, THEN
P.S.I. MUST BE SUPPLIED WITH A SKETCH AND
DIMENSIONS.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

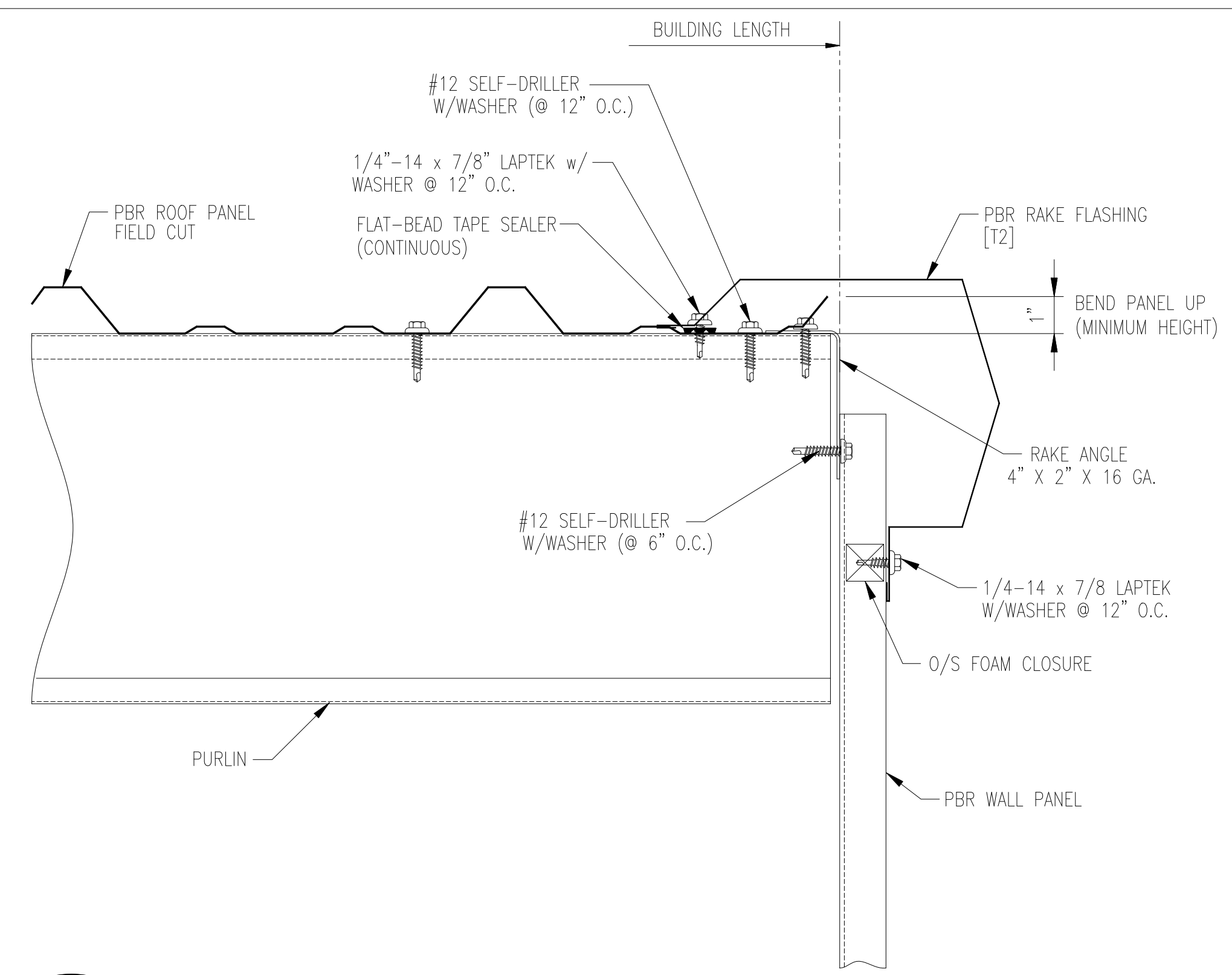
PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ROOF SHEETING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E06
Issue	A		

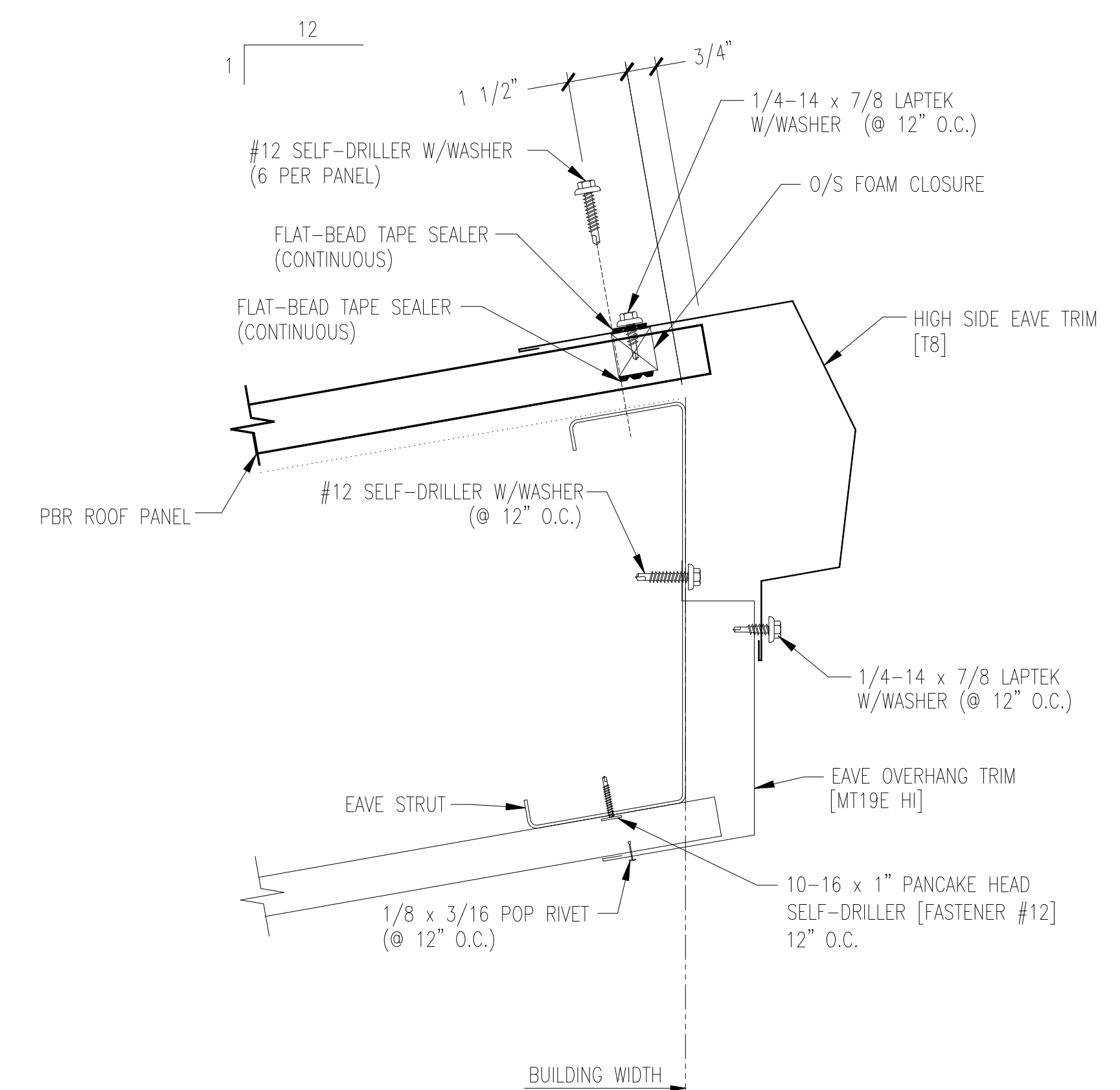




J SECTION THROUGH RAKE
E07 RAKE START ON MODULE



K SECTION THROUGH RAKE FINISH
E07 ROOF PANEL FINISHES OFF MODULE



KK SECTION THROUGH
E07 HIGH-SIDE EAVE

**** CUSTOMER NEED TO VERIFY ALL DETAILS ****

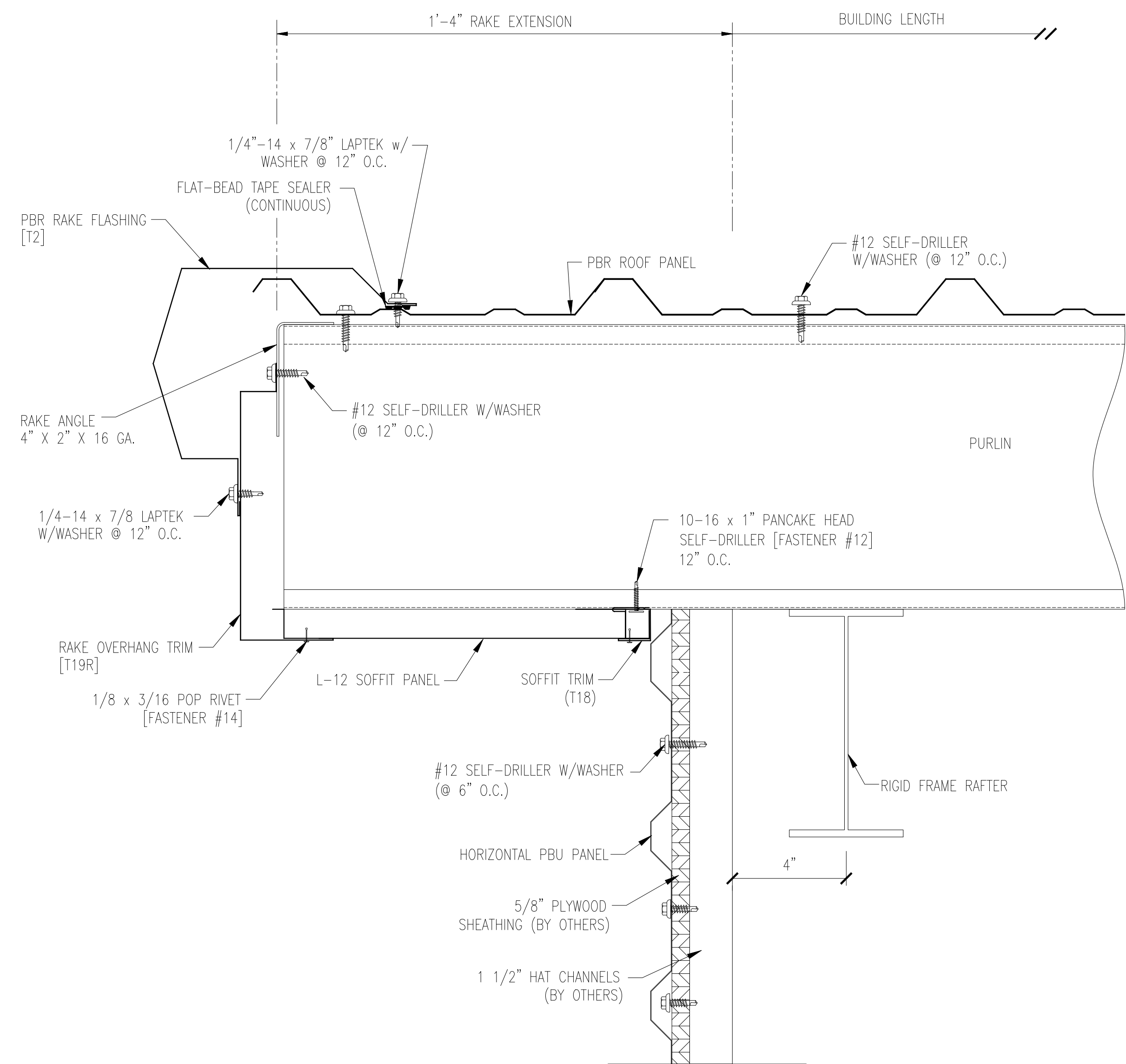
NOTE:
STANDARD PINNACLE TRIM IS SHOWN.
IF A DIFFERENT PROFILE IS DESIRED, THEN
P.S.I. MUST BE SUPPLIED WITH A SKETCH AND
DIMENSIONS.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

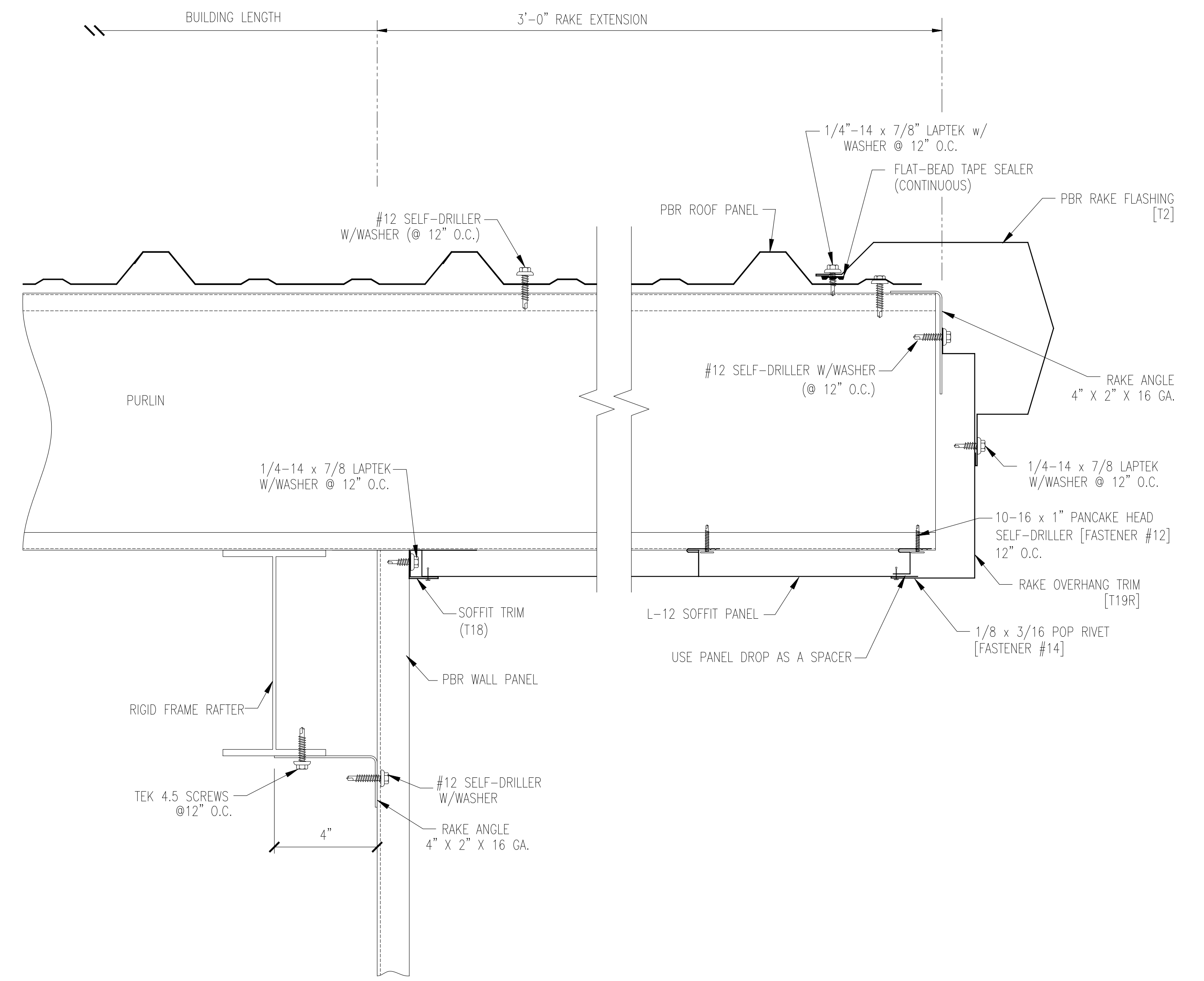


DESCRIPTION:	ROOF SHEETING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E07
Issue	A		





L SECTION THROUGH RAKE
E08 RAKE START ON MODULE



M SECTION THROUGH RAKE
E08 RAKE FINISH OFF MODULE

**** CUSTOMER NEED TO VERIFY ALL DETAILS ****

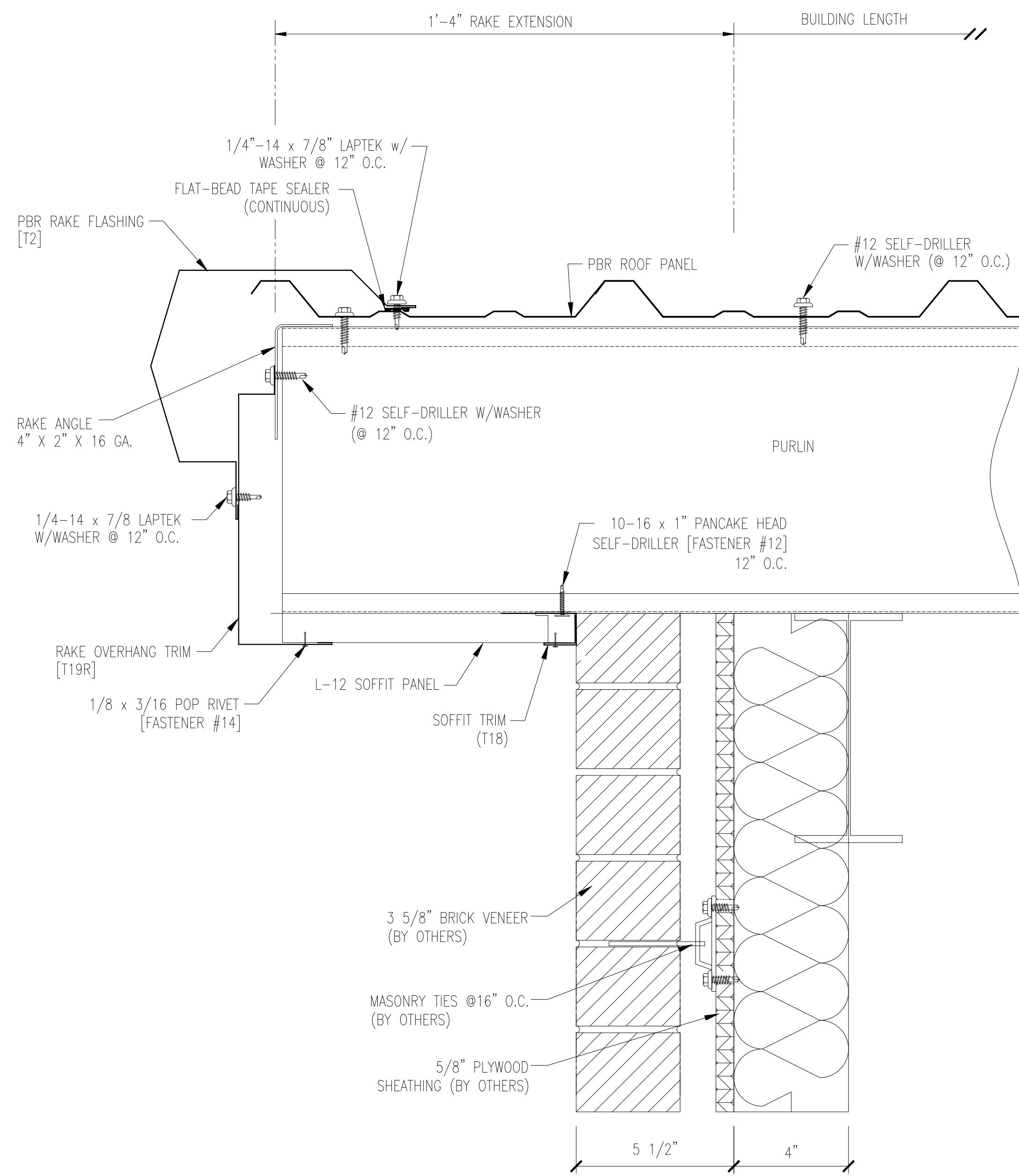
NOTE:
STANDARD PINNACLE TRIM IS SHOWN.
IF A DIFFERENT PROFILE IS DESIRED, THEN
P.S.I. MUST BE SUPPLIED WITH A SKETCH AND
DIMENSIONS.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

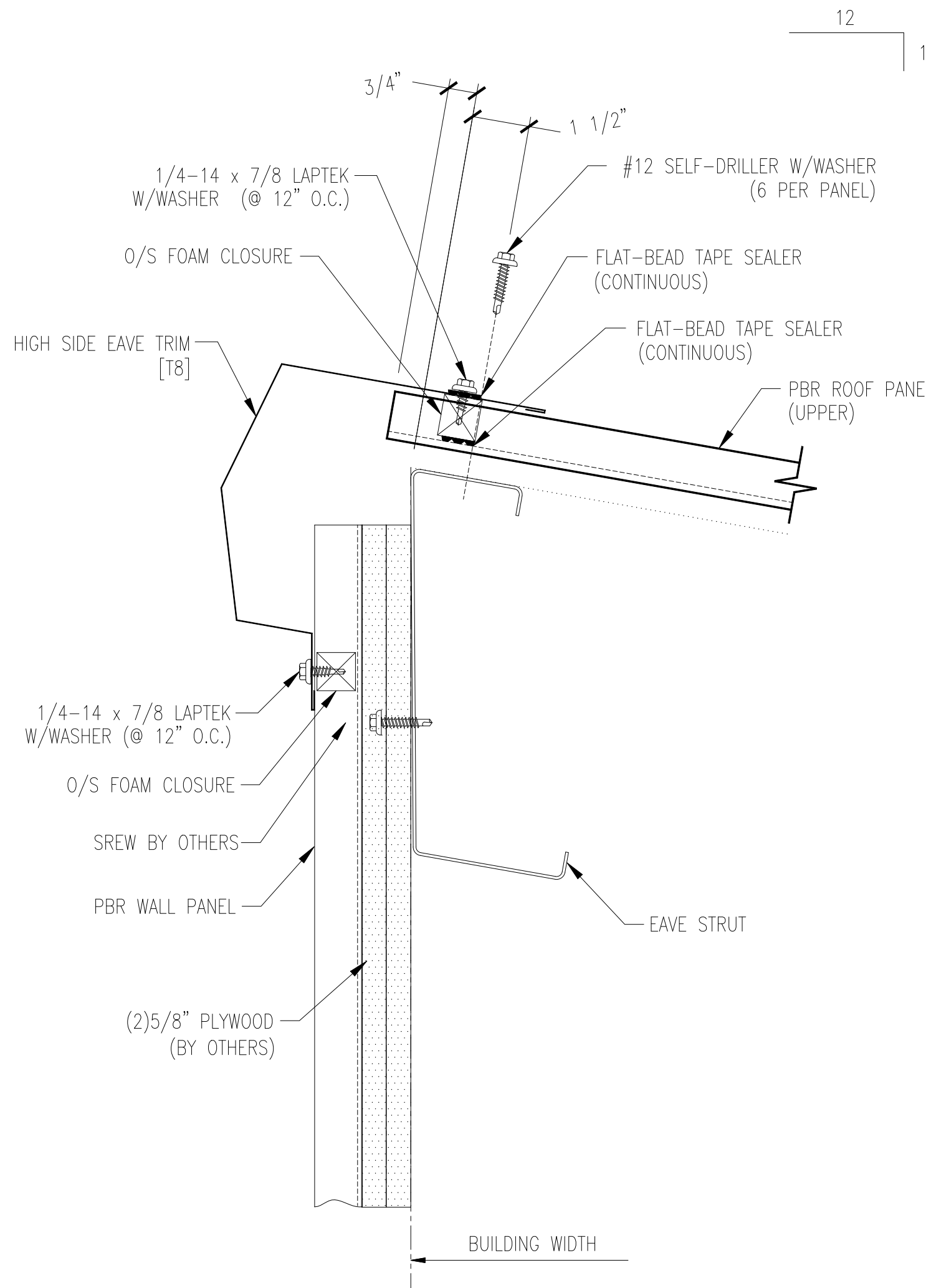


DESCRIPTION:	ROOF SHEETING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E08
Issue	A		

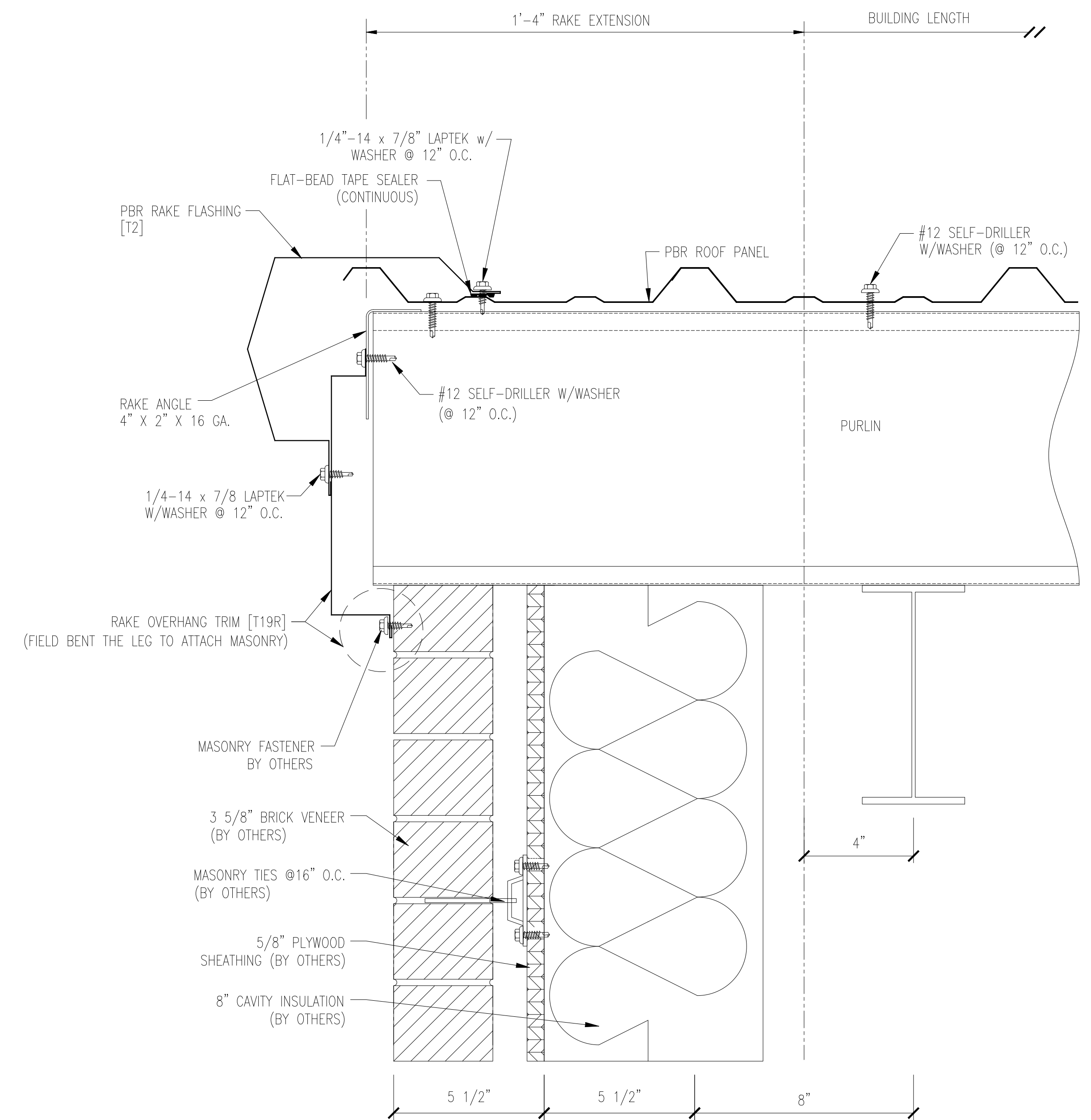




N SECTION THROUGH RAKE
E09 RAKE START ON MODULE



P SECTION THROUGH
E09 HIGH-SIDE EAVE STRUT



Q SECTION THROUGH RAKE
E09 RAKE START ON MODULE

**** CUSTOMER NEED TO VERIFY ALL DETAILS ****

NOTE:
STANDARD PINNACLE TRIM IS SHOWN.
IF A DIFFERENT PROFILE IS DESIRED, THEN
P.S.I. MUST BE SUPPLIED WITH A SKETCH AND
DIMENSIONS.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

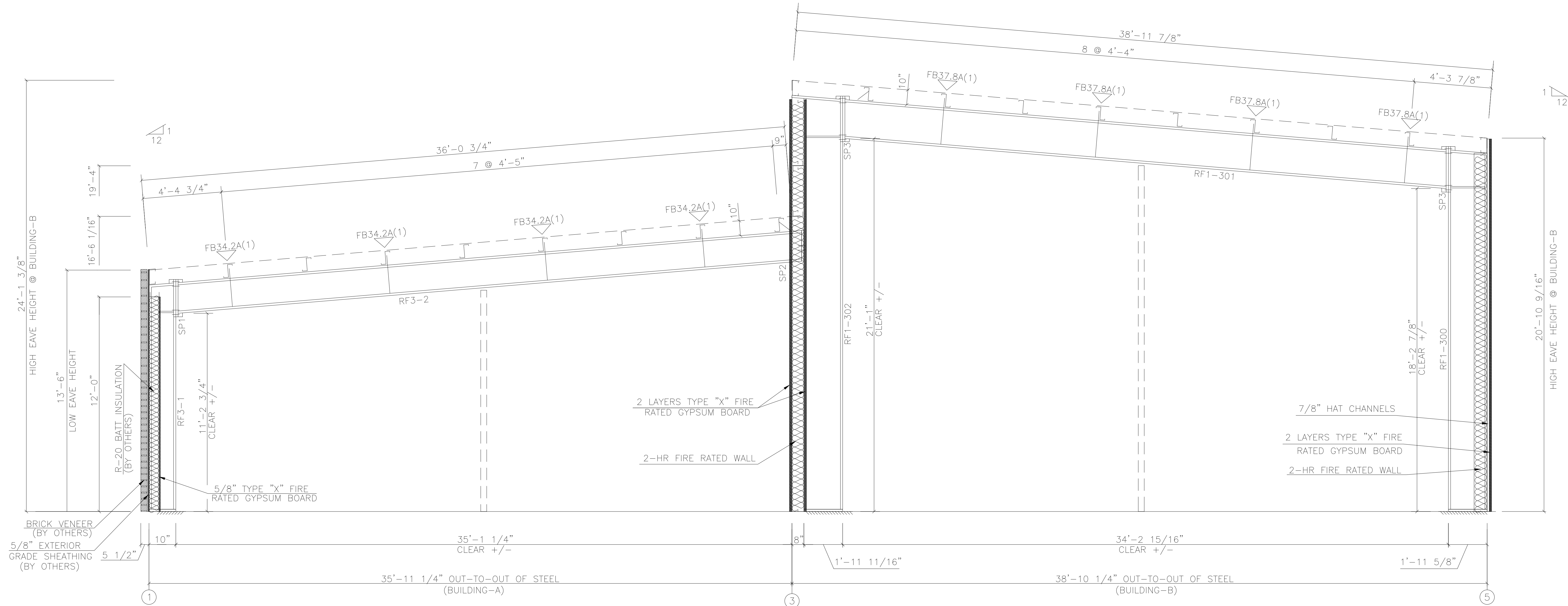
DESCRIPTION:	ROOF SHEETING DETAILS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E09
Issue	A		



SPLICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP1	4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11 7/8"
SP2	4	0	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-6 1/8"
SP3	4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	2'-5 3/8"

▽ FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A - L2X2X1/8

Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk	x Length	W x Thk	x Length
RF3-1	9.5	9.5	0.188	152.6	6 x 1/4"	x 151.8	6 x 1/4"	x 131.9
RF3-2	17.5	17.5	0.164	240.0	6 x 1/4"	x 9.8	6 x 1/4"	x 240.5
	17.5	17.5		187.8	6 x 1/4"	x 185.8		
RF1-300	23.0	23.0	0.164	12.0	6 x 5/16"	x 239.8	6 x 5/16"	x 215.6
RF1-301	23.0	23.0	0.164	229.8	6 x 5/16"	x 23.4	6 x 1/4"	x 240.5
	23.0	23.0		240.0	6 x 1/4"	x 171.0		
RF1-302	23.0	23.0	0.164	173.4	6 x 1/4"	x 31.5	6 x 5/16"	x 12.0
	23.0	23.0		240.0	6 x 5/16"	x 89.6		
				38.0	6 x 5/16"	x 170.2	6 x 5/16"	x 237.8



RIGID FRAME ELEVATION: FRAME LINE A
 (NON-EXPANDABLE END FRAME)

**** PLEASE VERIFY ALL DIMENSIONS AND CLEARANCES ****

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

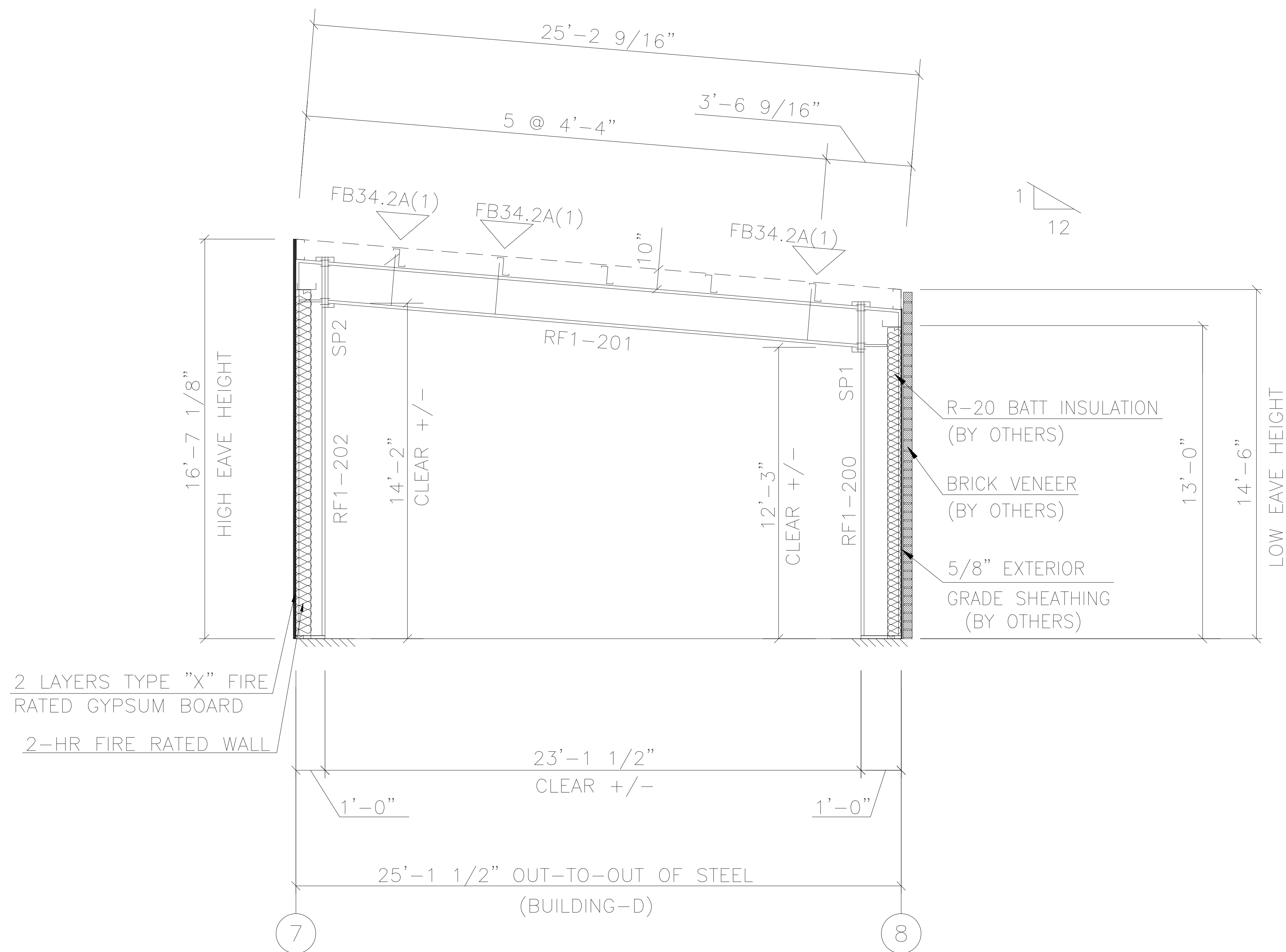
DESCRIPTION:	RIGID FRAME ELEVATION				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	MVU	Checker	JJ	Designer	HA
Job No.	251599	Sheet	E10	Issue	A



SPLICE PLATE & BOLT TABLE										
Mark	Qty		Int	Type	Dia	Length	Width	Thick	Length	
	Top	Bot								
SP1	4	4	0	A325	3/4"	1 3/4"	8"	3/8"	1'-11 7/8"	
SP2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11 7/8"	

MEMBER TABLE										
Mark	Web Depth		Web Plate		Outside Flange			Inside Flange		
	Start	End	Thick	Length	W	Thk	Length	W	Thk	Length
RF1-200	11.5	11.5	0.164	164.8	8	1/4"	163.8	8	1/4"	144.1
					8	1/4"	11.8			
RF1-201	17.5	17.5	0.164	240.0	6	1/4"	240.5	6	1/4"	240.5
	17.5	17.5	0.164	33.4	6	1/4"	31.4	6	1/4"	31.4
RF1-202	11.5	11.5	0.164	188.4	6	1/4"	11.8	6	1/4"	166.7
					6	1/4"	188.4			

▽ FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A - L2X2X1/8



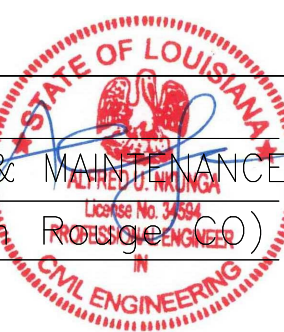
RIGID FRAME ELEVATION: FRAME LINE A
 (NON-EXPANDABLE END FRAME)

** PLEASE VERIFY ALL DIMENSIONS AND CLEARANCES **

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

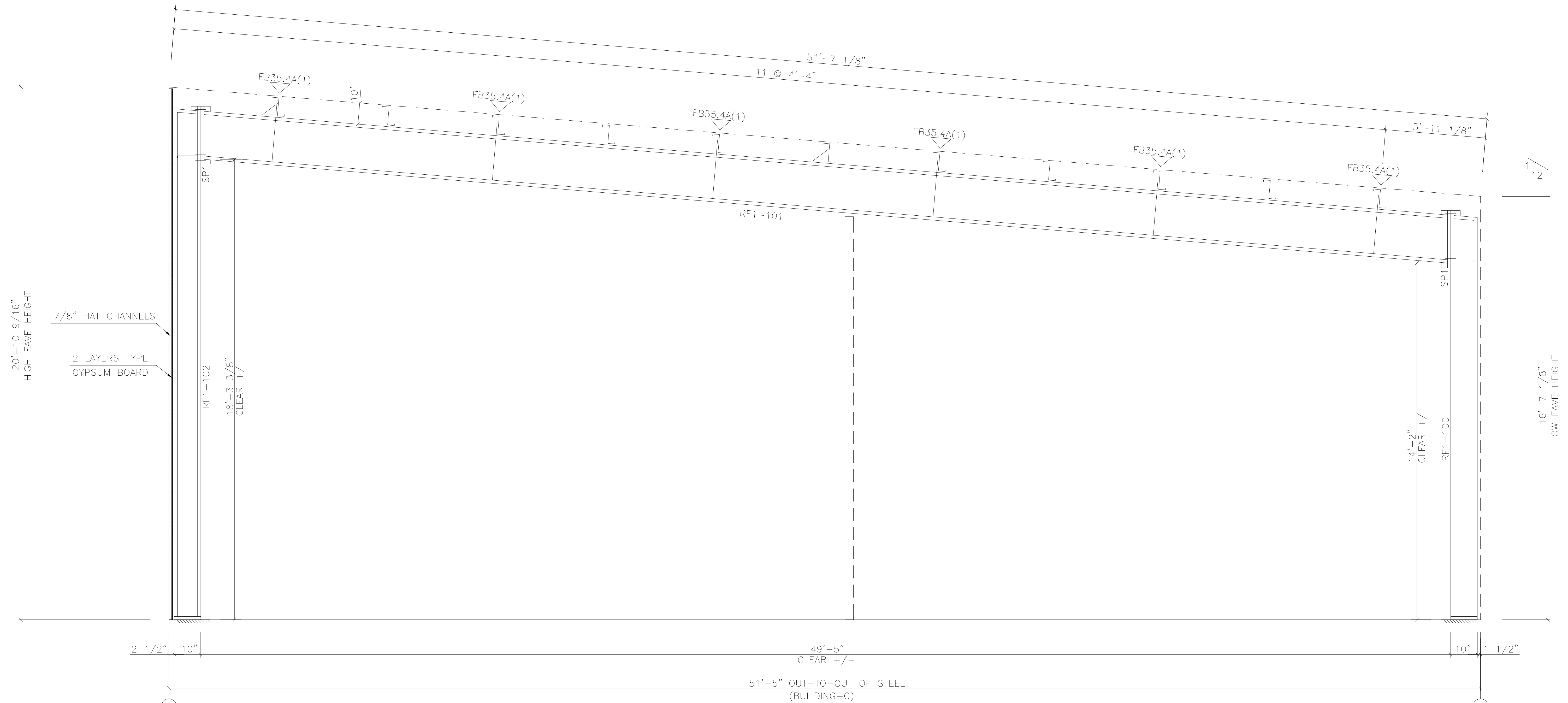
DESCRIPTION:	RIGID FRAME ELEVATION				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	MVU	Checker	JJ	Designer	11/25/2025 HA
Job No.	251599	Sheet	E11	Issue	A



SPLICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP1	4			0	A325	3/4"	1 3/4"	8"	3/8"	2'-1 7/8"

Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start/End	Thick	Length	Thick	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF1-100	9.5/ 9.5	0.164	189.4		8 x 1/4" x 188.6	8 x 1/4" x 166.7		
RF1-101	19.5/19.5	0.164	240.0		6 x 1/4" x 240.5	6 x 1/4" x 240.5		
	19.5/19.5	0.164	115.7		6 x 1/4" x 113.0	6 x 1/4" x 113.0		
RF1-102	9.5/ 9.5	0.164	239.7		8 x 1/4" x 9.8	8 x 1/4" x 216.1		
					8 x 1/4" x 239.7			

▽ FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A - L2X2X1/8



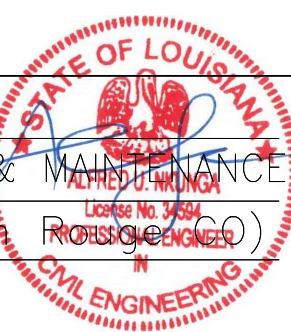
RIGID FRAME ELEVATION: FRAME LINE B
 (NON-EXPANDABLE END FRAME)

** PLEASE VERIFY ALL DIMENSIONS AND CLEARANCES **

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

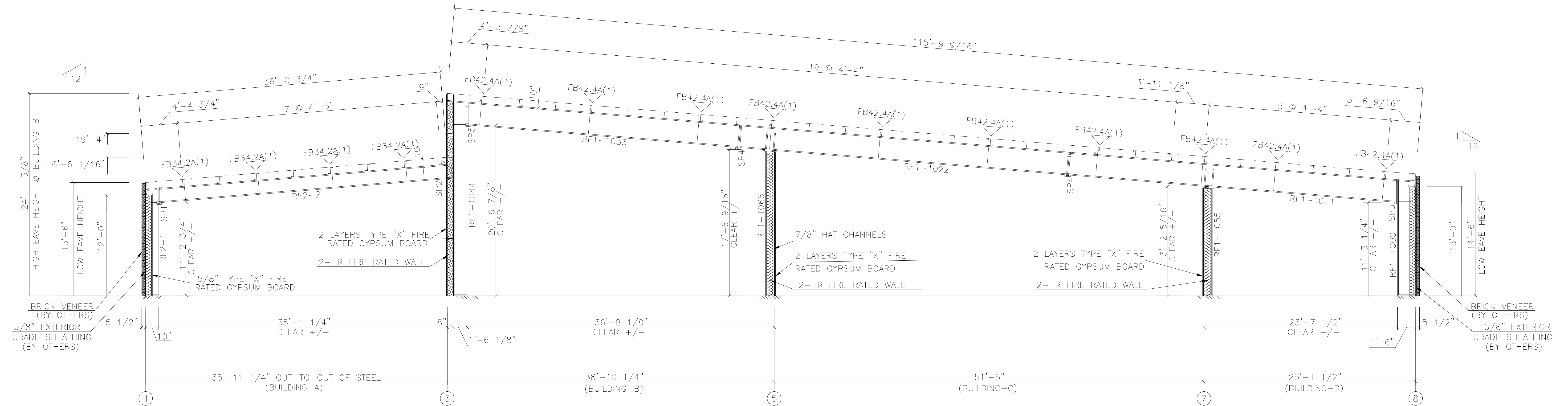
DESCRIPTION:	RIGID FRAME ELEVATION		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	MVU	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E12
Issue	A		



SPLICE PLATE & BOLT TABLE										CAP PLATE BOLTS					
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length	Mark	Qty	Type	Dia	Length
SP1	4	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11 7/8"	RF1-1055	8	A325	5/8"	1 3/4"
SP2	4	0	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-6 1/8"	RF1-1066	8	A325	5/8"	1 3/4"
SP3	4	4	0	0	A325	3/4"	2"	6"	1/2"	2'-9 1/8"					
SP4	4	4	0	0	A325	3/4"	2"	6"	1/2"	2'-8 7/8"					
SP5	4	4	0	0	A325	3/4"	2"	6"	1/2"	2'-9 1/4"					

MEMBER TABLE										
Mark	Web Depth		Web Plate		Outside Flange			Inside Flange		
	Start/End	Thick	Length	Thick	Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF2-1	9.5/ 9.5	0.188	152.6			6 x 1/4" x 151.8	6 x 1/4" x 131.9			
RF2-2	17.5/17.5	0.164	240.0			6 x 1/4" x 9.8	6 x 1/4" x 240.5	6 x 1/4" x 240.5		
	17.5/17.5	0.164	185.8			6 x 1/4" x 183.8	6 x 1/4" x 183.8	6 x 1/4" x 183.8		
RF1-1000	17.5/17.5	0.188	164.8			8 x 3/8" x 17.9	8 x 1/4" x 132.0	8 x 1/4" x 132.0		
RF1-1011	29.5/29.5	0.164	240.0			6 x 1/4" x 240.5	6 x 1/4" x 240.5	6 x 1/4" x 240.5		
	29.5/29.5	0.164	234.5			6 x 1/4" x 231.5	6 x 1/4" x 231.5	6 x 1/4" x 231.5		
RF1-1022	29.5/29.5	0.164	240.0			6 x 1/4" x 240.5	6 x 1/4" x 240.5	6 x 1/4" x 240.5		
	29.5/29.5	0.164	230.7			6 x 1/4" x 230.2	6 x 1/4" x 230.2	6 x 1/4" x 230.2		
RF1-1033	29.5/29.5	0.164	240.0			6 x 1/4" x 240.5	6 x 1/4" x 240.5	6 x 1/4" x 240.5		
	29.5/29.5	0.164	153.5			6 x 1/4" x 153.0	6 x 1/4" x 153.0	6 x 1/4" x 153.0		
RF1-1044	17.5/17.5	0.188	240.0			8 x 3/8" x 25.9	8 x 1/4" x 12.0	8 x 1/4" x 12.0		
	17.5/17.5	0.188	37.5			8 x 1/4" x 89.6	8 x 1/4" x 89.6	8 x 1/4" x 89.6		
RF1-1055	W10843					8 x 1/4" x 169.8	8 x 1/4" x 169.8	8 x 1/4" x 169.8		
RF1-1066	W10843									

▽ FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A - L2X2X1/8



CRIGID FRAME ELEVATION: FRAME LINE C

**** PLEASE VERIFY ALL DIMENSIONS AND CLEARANCES ****

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

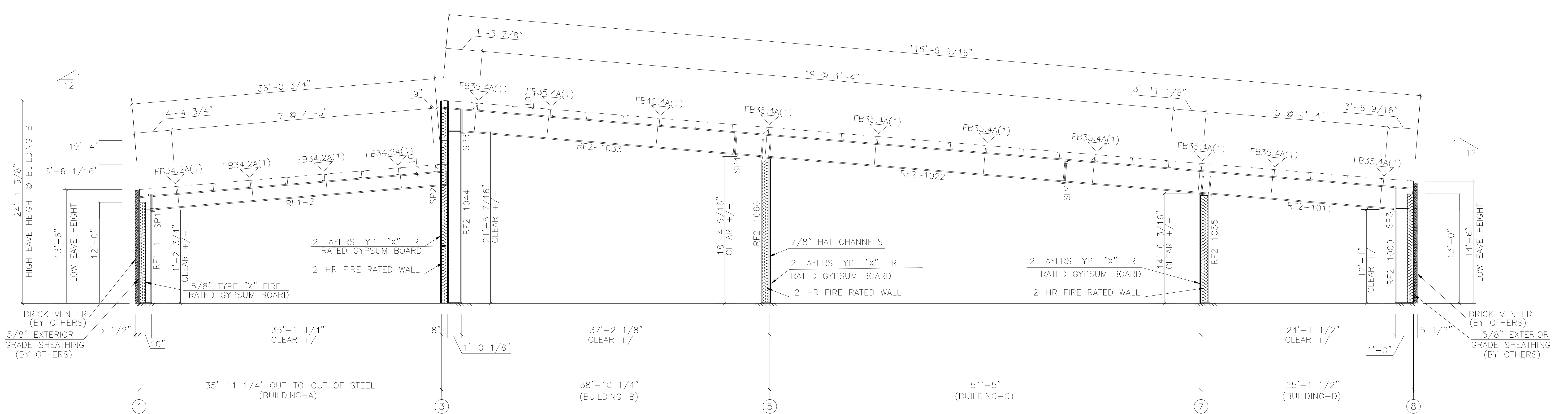
DESCRIPTION:	RIGID FRAME ELEVATION
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE
LOCATION:	Central, LA (East Baton Rouge Parish)
Detailer:	MVU
Checker:	JJ
Designer:	HA
Job No.:	251599
Sheet:	E13
Issue:	A



SPLICE PLATE & BOLT TABLE										CAP PLATE BOLTS				
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length	Width	Thick	Length	Mark	Qty	Type	Dia	Length
SP1	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11 7/8"	RF2-1055	8	A325	5/8"	2"
SP2	4	0	0	A325	3/4"	1 3/4"	6"	3/8"	1'-6 1/8"	RF2-1066	8	A325	5/8"	1 3/4"
SP3	4	4	0	A325	3/4"	2"	6"	1/2"	1'-11 1/8"					
SP4	4	4	0	A325	3/4"	2"	6"	1/2"	1'-10 7/8"					

Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start/End	Thick	Length	Thick	W x Thk x Length	W x Thk x Length	W x Thk x Length	
RF1-1	9.5/9.5	0.188	152.6		6 x 1/4" x 151.8	6 x 1/4" x 131.9		
RF1-2	17.5/17.5	0.164	240.0		6 x 1/4" x 240.5	6 x 1/4" x 240.5		
RF2-1000	11.5/11.5	0.164	164.5		6 x 1/4" x 183.8	6 x 1/4" x 183.8		
RF2-1011	19.5/19.5	0.164	240.0		8 x 3/8" x 163.5	8 x 1/4" x 141.8		
RF2-1011	19.5/19.5	0.164	239.7		6 x 1/4" x 240.5	6 x 1/4" x 240.5		
RF2-1022	19.5/19.5	0.164	240.0		6 x 1/4" x 240.5	6 x 1/4" x 192.7		
RF2-1022	19.5/19.5	0.164	230.7		6 x 1/4" x 230.2	6 x 1/4" x 240.5		
RF2-1033	19.5/19.5	0.164	240.0		6 x 1/4" x 240.5	6 x 1/4" x 240.5		
RF2-1033	19.5/19.5	0.164	159.6		6 x 1/4" x 159.1	6 x 1/4" x 157.4		
RF2-1044	11.5/11.5	0.164	240.0		8 x 3/8" x 19.9	8 x 1/4" x 13.6		
RF2-1044	11.5/11.5	0.164	37.8		8 x 1/4" x 89.6	8 x 1/4" x 240.5		
RF2-1055	W10843				8 x 1/4" x 170.1			
RF2-1066	W10843							

FLANGE BRACES: (1) One Side; (2) Two Sides
 FBxxA(1): xx=length(in)
 A = L2X2X1/8



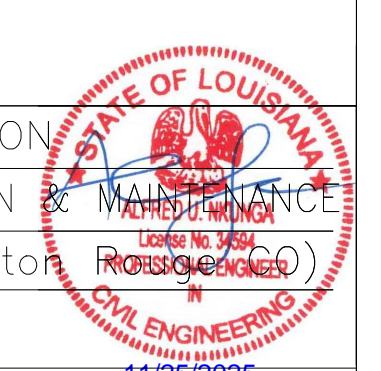
CRIGID FRAME ELEVATION: FRAME LINE D
 (NON-EXPANDABLE END FRAME)

**** PLEASE VERIFY ALL DIMENSIONS AND CLEARANCES ****

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

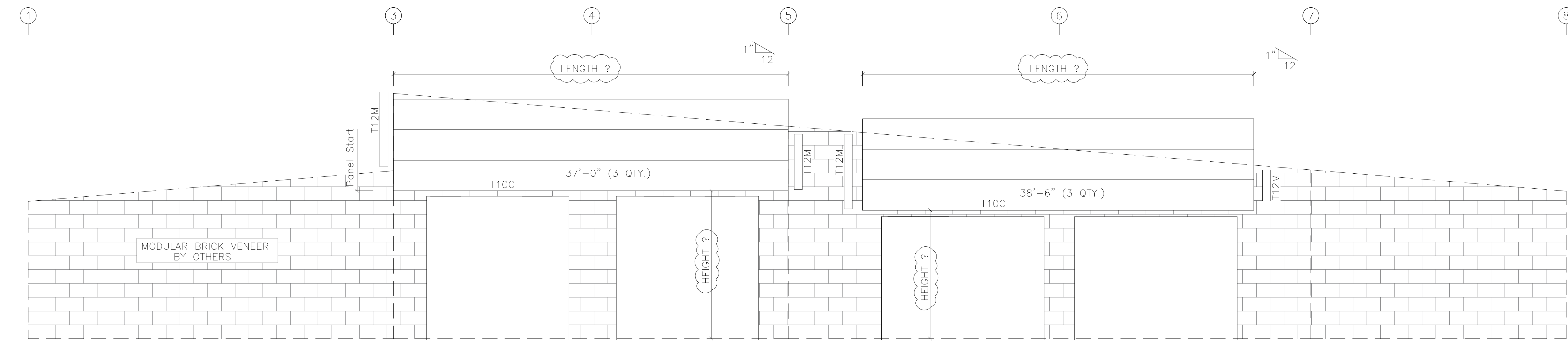
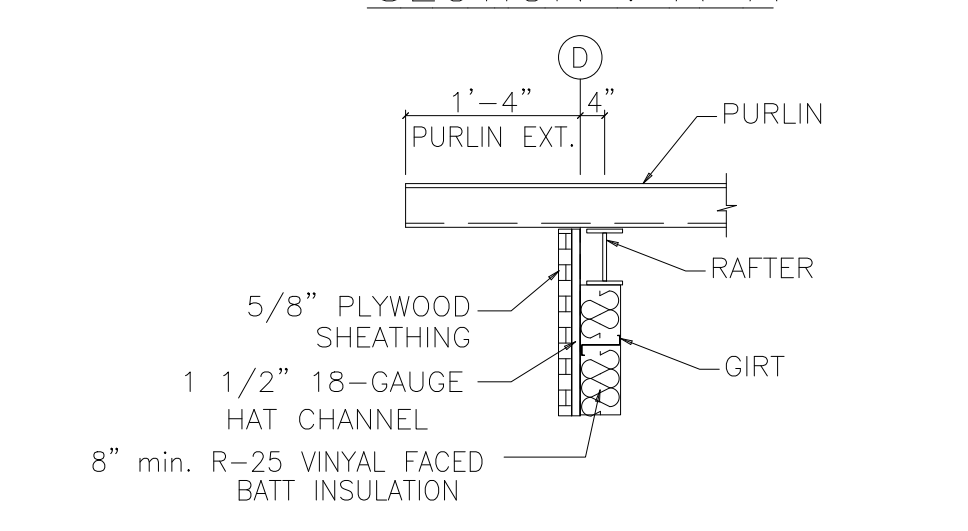
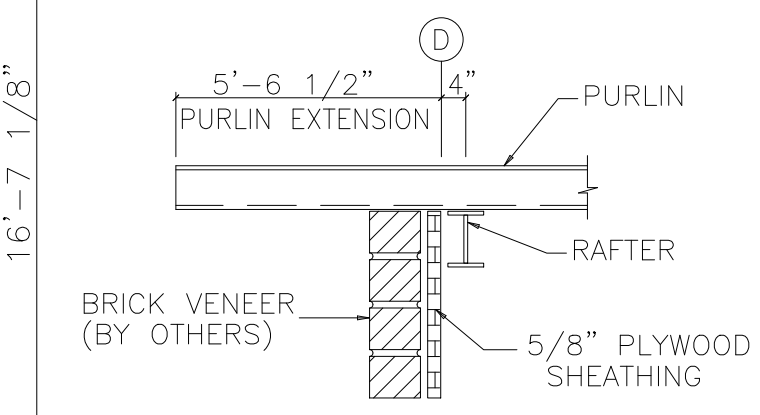
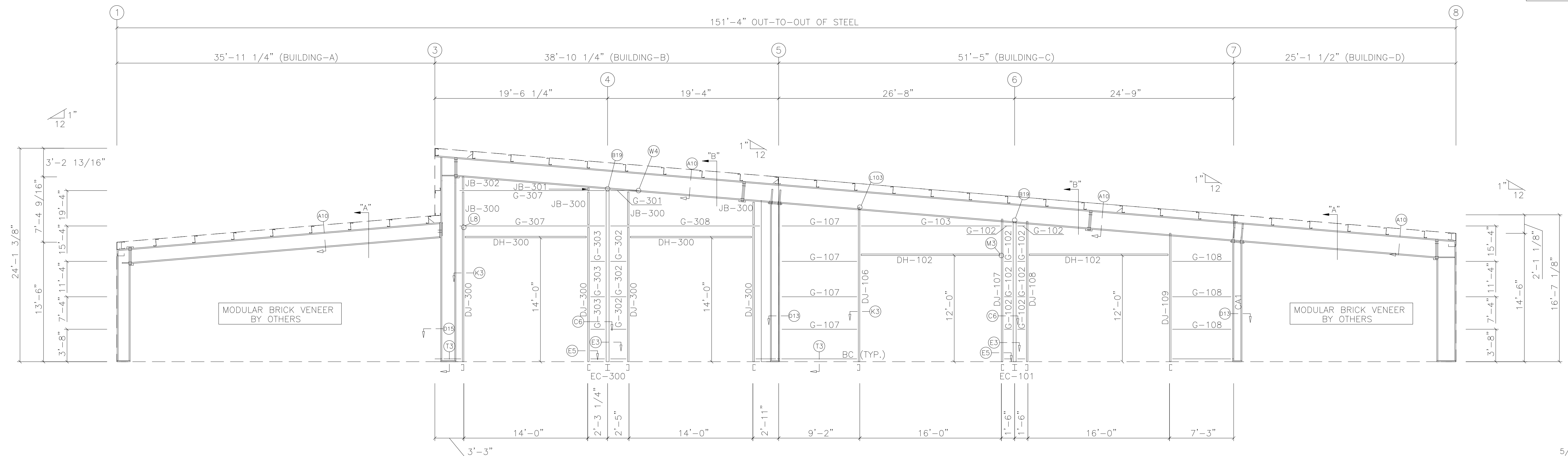
PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	RIGID FRAME ELEVATION		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer:	MVU	Checker:	JJ
Designer:	HA	Issue:	A
Job No.:	251599	Sheet:	E14



BOLT TABLE				
FRAME LINE D				
LOCATION	QUAN	TYPE	DIA	LENGTH
251599C				
Columns/Raf	2	A325	5/8"	1 1/2"
Jamb	2	A325	5/8"	1 1/2"
251599B				
Columns/Raf	2	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE D	
MARK	PART
251599C	
EC-101	W08542
DJ-106	8X25C12
DJ-107	8X25C14
DJ-108	8X35C16
DJ-109	8X25C14
DH-102	8X25C16
G-102	8X25Z16
G-103	8X25Z16
G-107	8X25Z16
G-108	8X25Z16
251599B	
EC-300	W08542
DJ-300	8X25C16
DH-300	8X25C16
G-301	8X25Z16
G-302	8X25Z16
G-303	8X25Z16
G-307	8X35Z16
G-308	8X35Z14
JB-300	8X25C16
JB-301	8X25C16
JB-302	8X25C16



**** PLEASE VERIFY ALL BUILDING DIMENSIONS & FRAMED OPENING LOCATIONS ****

**** CUSTOMER TO VERIFY THE CLOUDED DIMENSIONS & ****

GENERAL NOTES:
 1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.
 4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

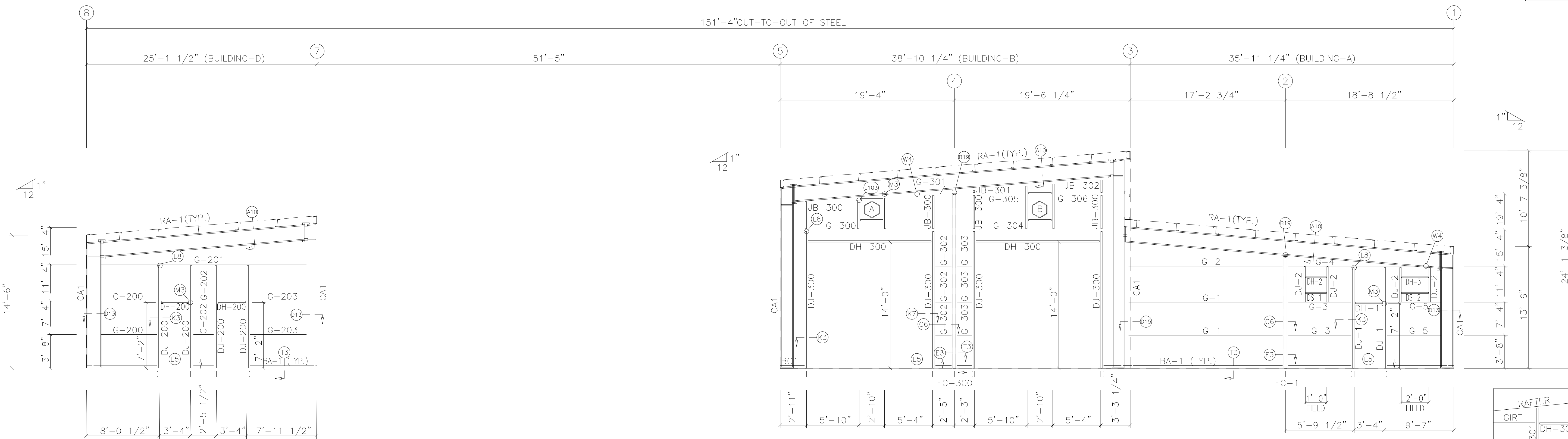
PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ENDWALL FRAMING & SHEETING
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE
LOCATION:	Central, LA (East Baton Rouge Parish)
Detailer	SAN
Checker	JJ
Designer	HA
Job No.	251599
Sheet	E15
Issue	A

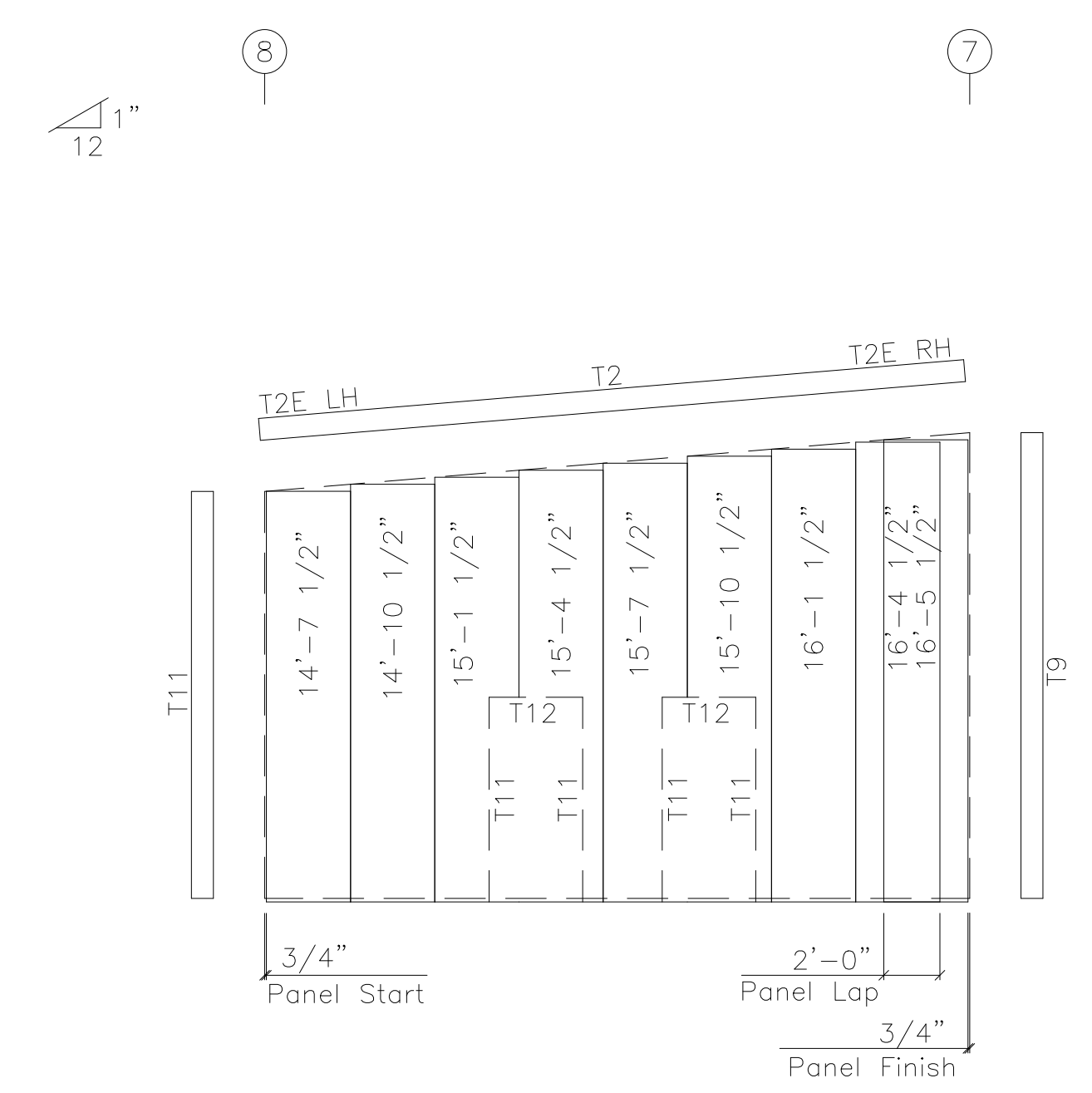
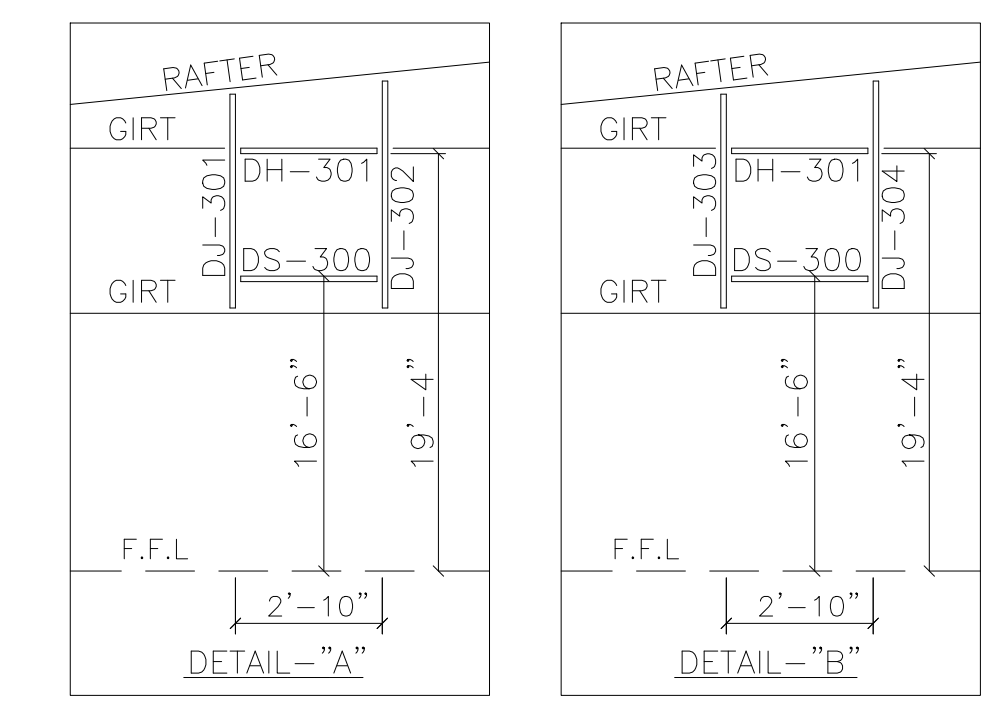


BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
251599A				
Columns/Raf	2	A325	5/8"	1 1/2"
251599B				
Columns/Raf	2	A325	5/8"	1 1/2"
Jamb	2	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE A	
MARK	PART
251599A	
EC-1	W08542
DJ-1	8X25C16
DS-1	8X25C16
DS-2	8X25C16
DH-1	8X25C16
DH-2	8X25C16
DH-3	8X25C16
G-1	8X25Z14
G-2	8X25Z14
G-3	8X25Z16
G-4	8X25Z12
G-5	8X25Z16
251599B	
EC-300	W08542
DJ-300	8X25C16
DJ-301	8X25C16
DJ-302	8X25C16
DJ-303	8X25C16
DJ-304	8X25C16
DH-300	8X25C16
DH-301	8X25C16
DS-300	8X25C16
G-300	8X35Z12
G-301	8X25Z16
G-302	8X25Z16
G-303	8X25Z16
G-304	8X35Z12
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G-306	8X25Z16
JB-300	8X25C16
JB-301	8X25C16
JB-302	8X25C16

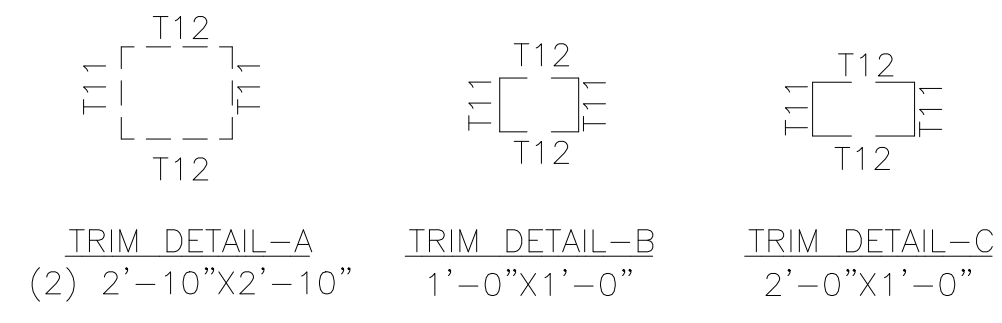


ENDWALL FRAMING: FRAME LINE A



ENDWALL SHEETING & TRIM: FRAME LINE A

PANELS: 24 Ga. PBR - NEED COLOR
NOTE: FIELD CUT WALL PANELS AS REQUIRED



**** PLEASE VERIFY ALL BUILDING DIMENSIONS & FRAMED OPENING LOCATIONS ****

GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

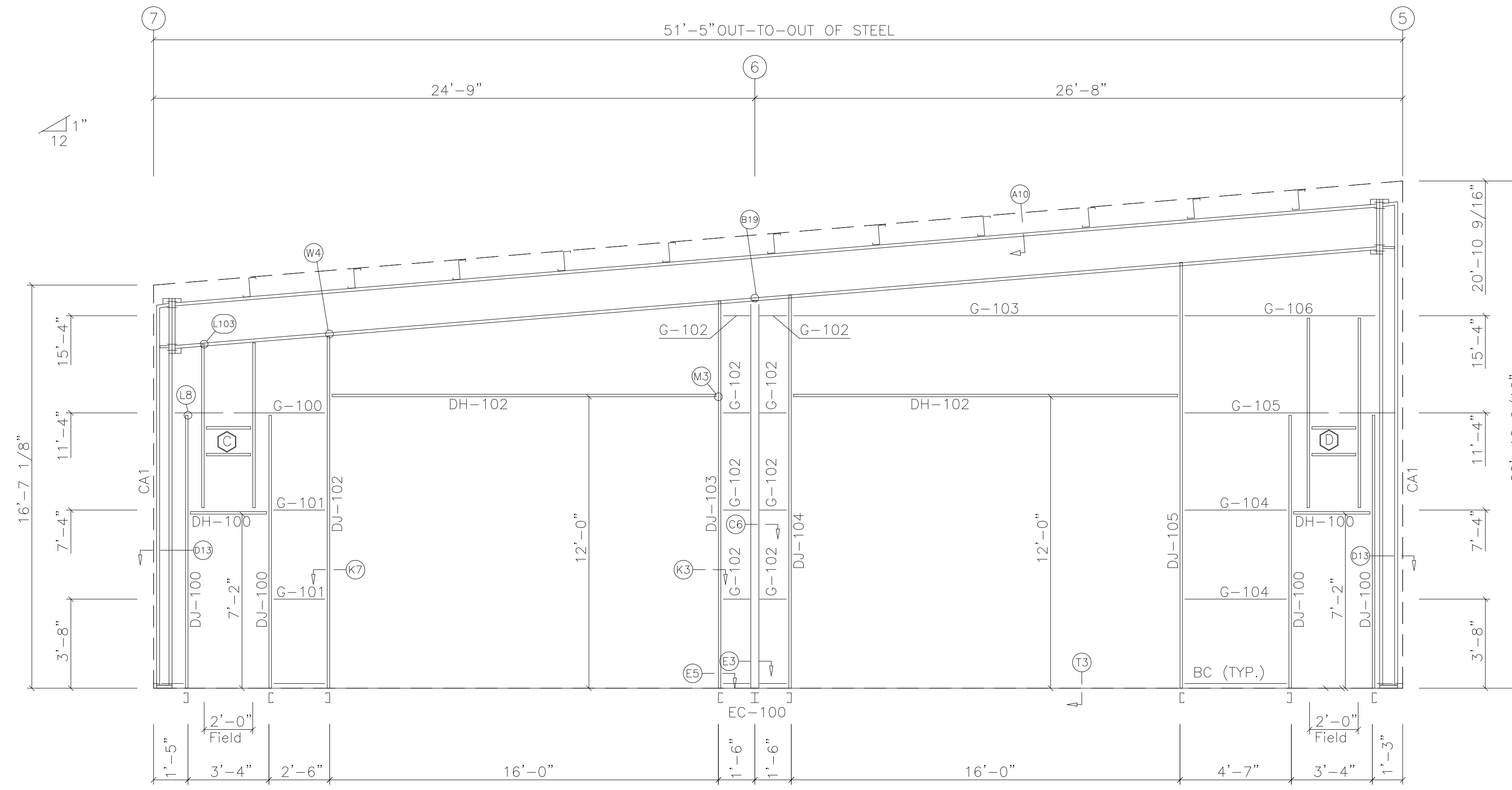
PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	ENDWALL FRAMING & SHEETING				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	SAN	Checker	JJ	Designer	HA
Job No.	251599	Sheet	E16	Issue	A

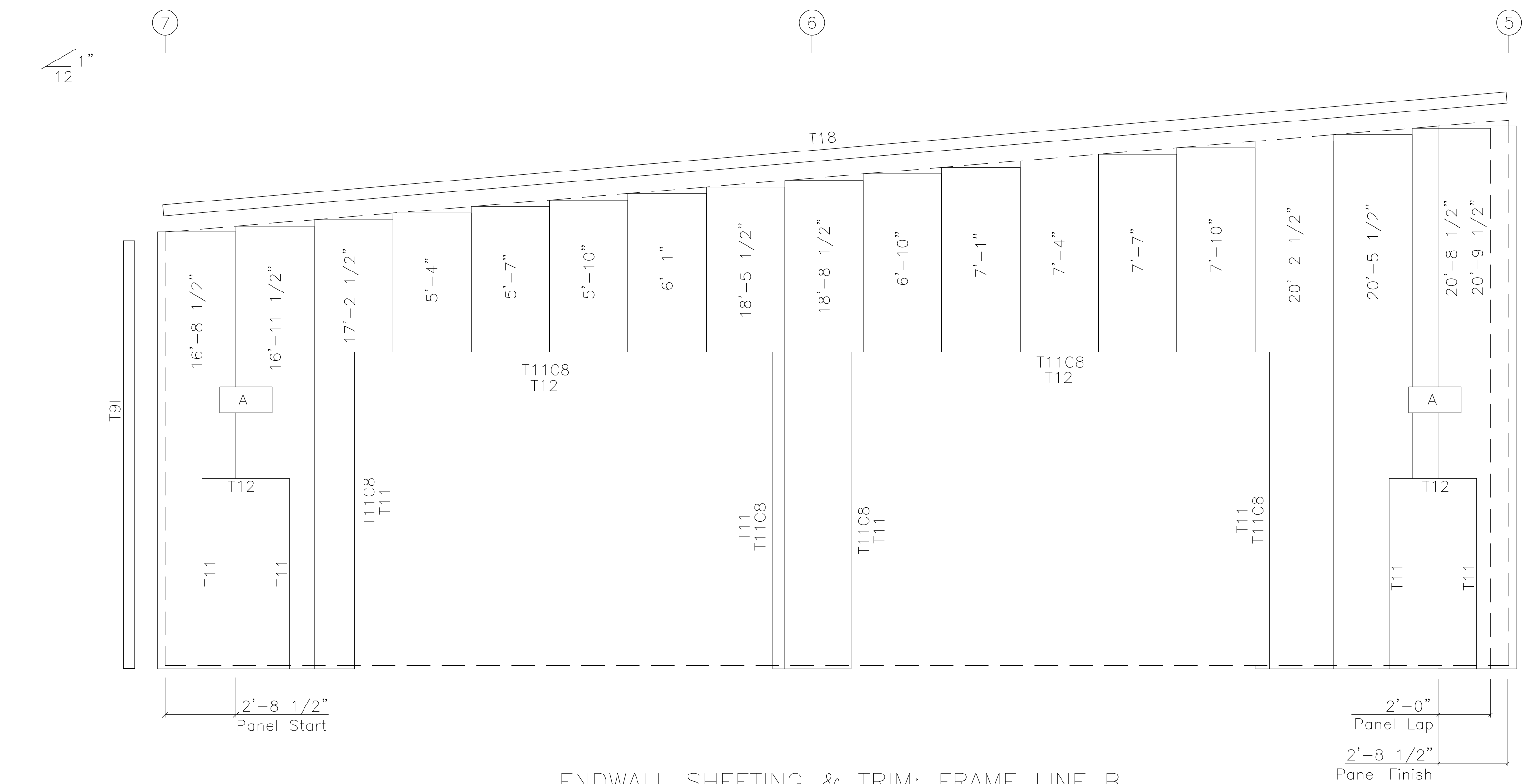
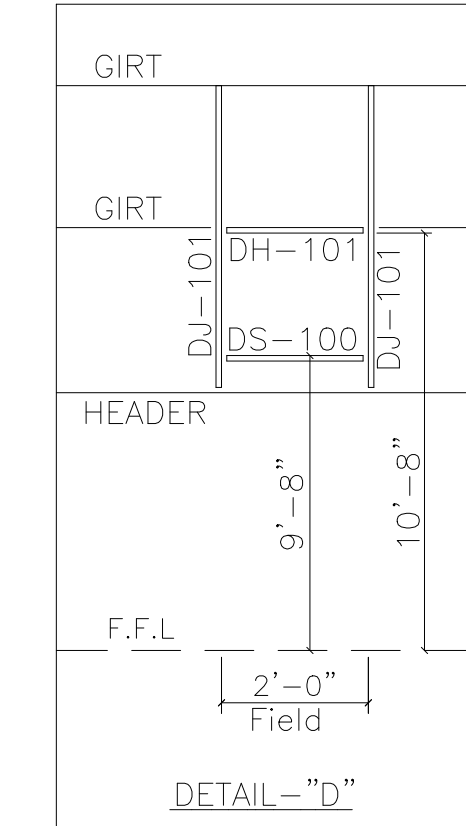
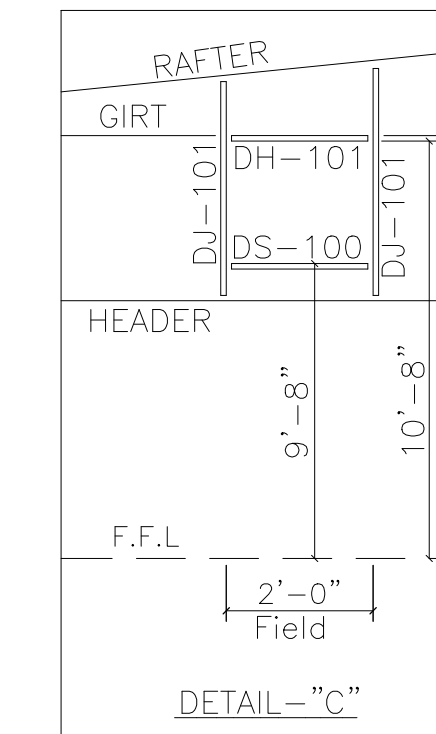


BOLT TABLE				
FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	5/8"	1 1/2"
Jamb	2	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE B	
MARK	PART
EC-100	W08542
DJ-100	8X25C16
DJ-101	8X25C16
DJ-102	8X25C16
DJ-103	8X35C16
DJ-104	8X25C14
DJ-105	8X25C12
DH-100	8X25C16
DH-101	8X25C16
DH-102	8X25C16
DS-100	8X25C16
G-100	8X25Z16
G-101	8X25Z16
G-102	8X25Z16
G-103	8X25Z16
G-104	8X25Z16
G-105	8X25Z16
G-106	8X25Z16



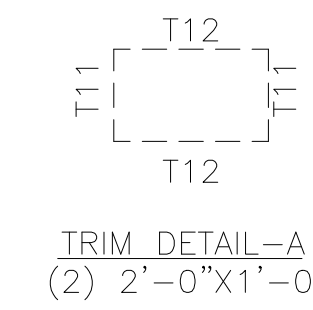
ENDWALL FRAMING: FRAME LINE B



ENDWALL SHEETING & TRIM: FRAME LINE B

PANELS: 24 Ga. PBR - NEED COLOR

NOTE: FIELD CUT WALL PANELS AS REQUIRED



**** PLEASE VERIFY ALL BUILDING DIMENSIONS & FRAMED OPENING LOCATIONS ****

GENERAL NOTES:

1. Pinnacle standard trim lap is 3 inches max.
2. Pinnacle pre-cuts wall panels at factory located openings as required.
3. Slot girts in field for cable passage at flush walls as required.
4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

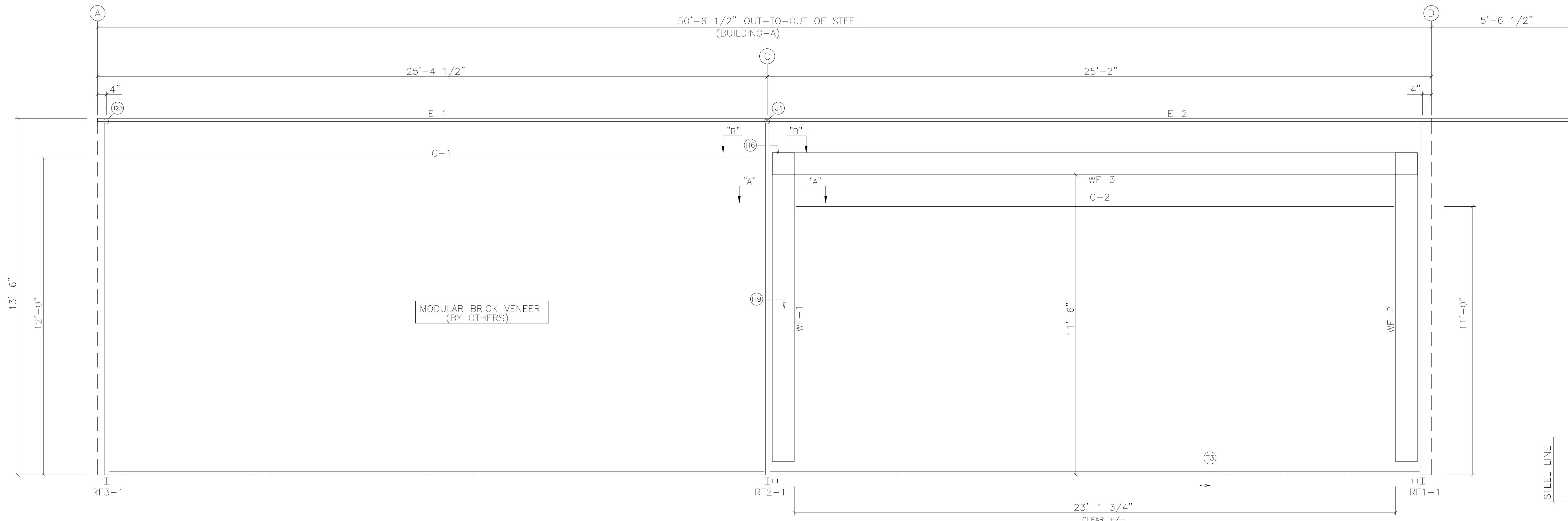


DESCRIPTION:	ENDWALL FRAMING & SHEETING				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	SAN	Checker	JJ	Designer	HA
Job No.	251599	Sheet	E17	Issue	A

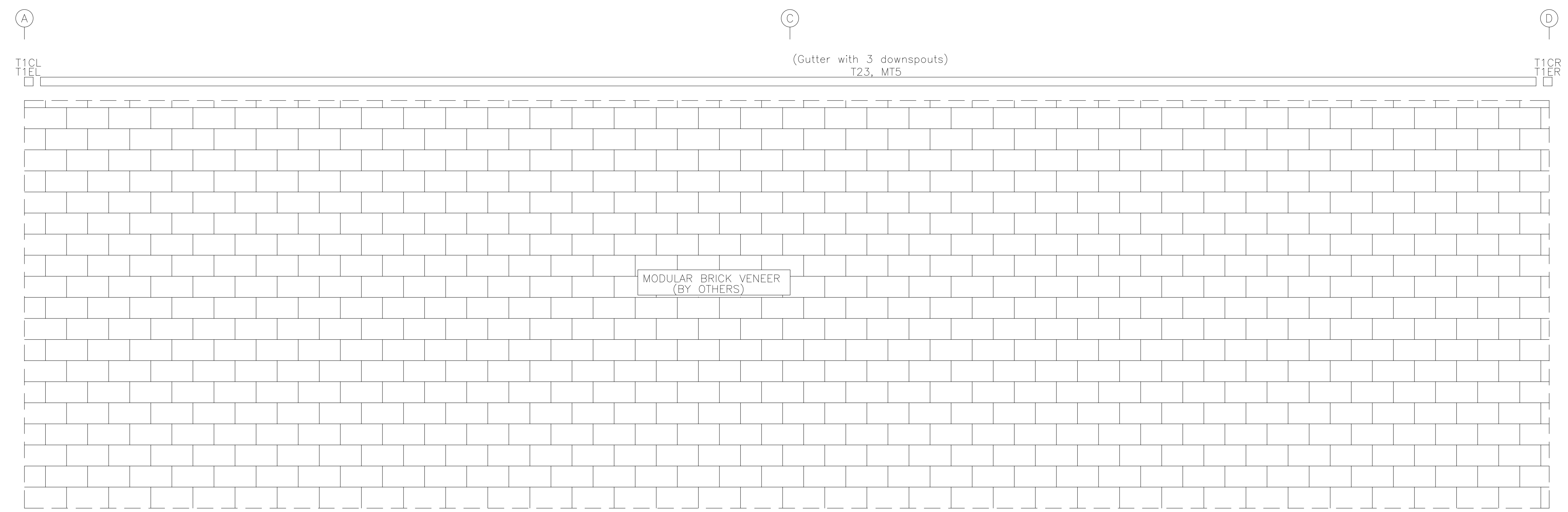
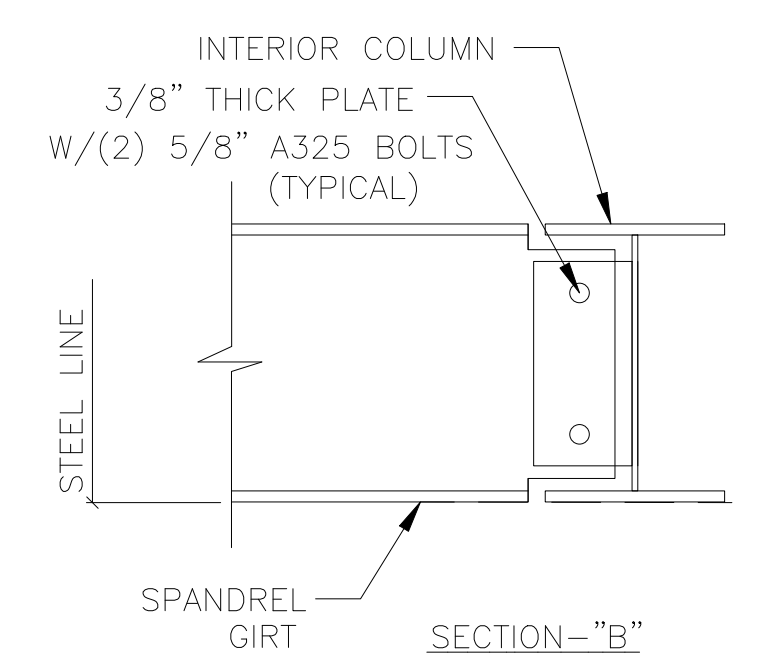
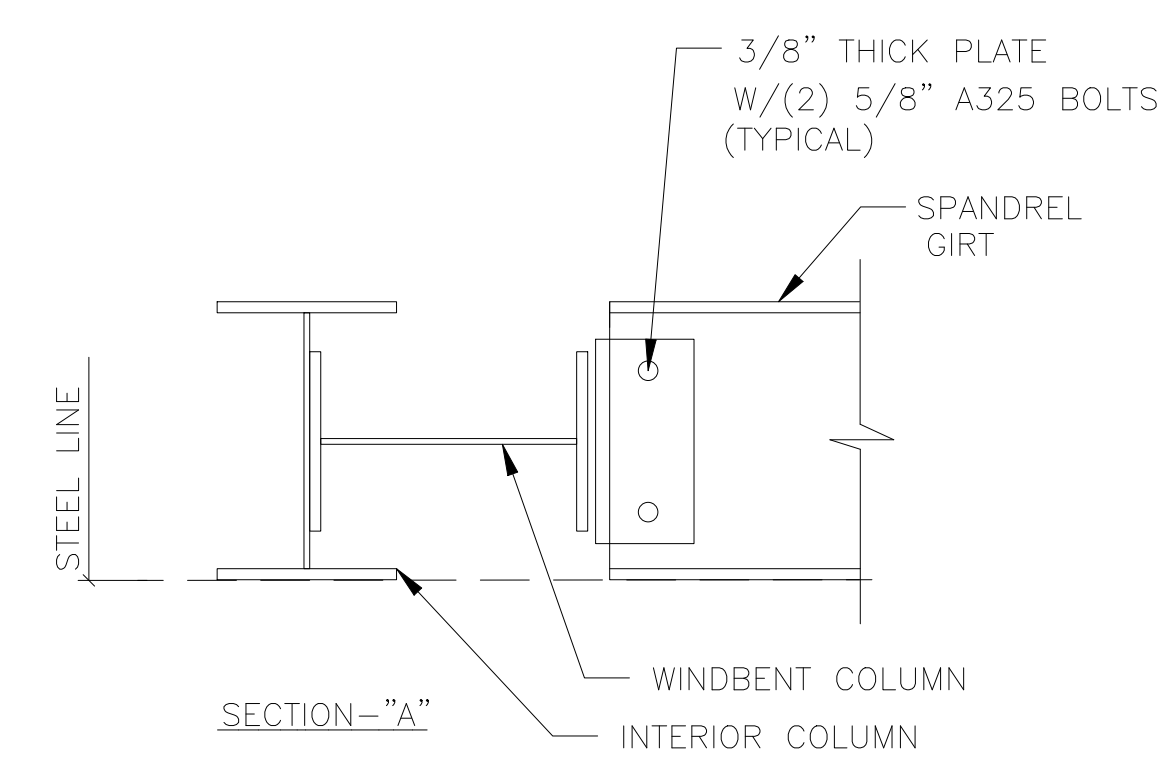


BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-3	8	A325	3/4"	1 3/4"
WF-2 - WF-3	8	A325	3/4"	1 3/4"
WF-1 - RF2-1	8	A325	5/8"	1 1/2"
WF-2 - RF1-1	8	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE 1	
MARK	PART
WF-1	W10642
WF-2	W10642
WF-3	W10642
E-1	10ES14
E-2	10ES14
G-1	C10x15.3
G-2	C10x15.3



SIDEWALL FRAMING: FRAME LINE 1



SIDEWALL TRIM: FRAME LINE 1

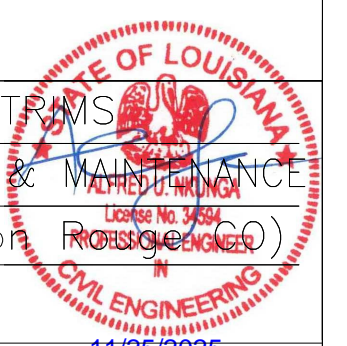
**** PLEASE VERIFY ALL BUILDING DIMENSIONS & CLEAR HEIGHTS ****

- GENERAL NOTES:
1. Pinnacle standard trim lap is 3 inches max.
 2. Pinnacle pre-cuts wall panels at factory located openings as required.
 3. Slot girts in field for cable passage at flush walls as required.
 4. PSI is NOT responsible for attachment of material by others.

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

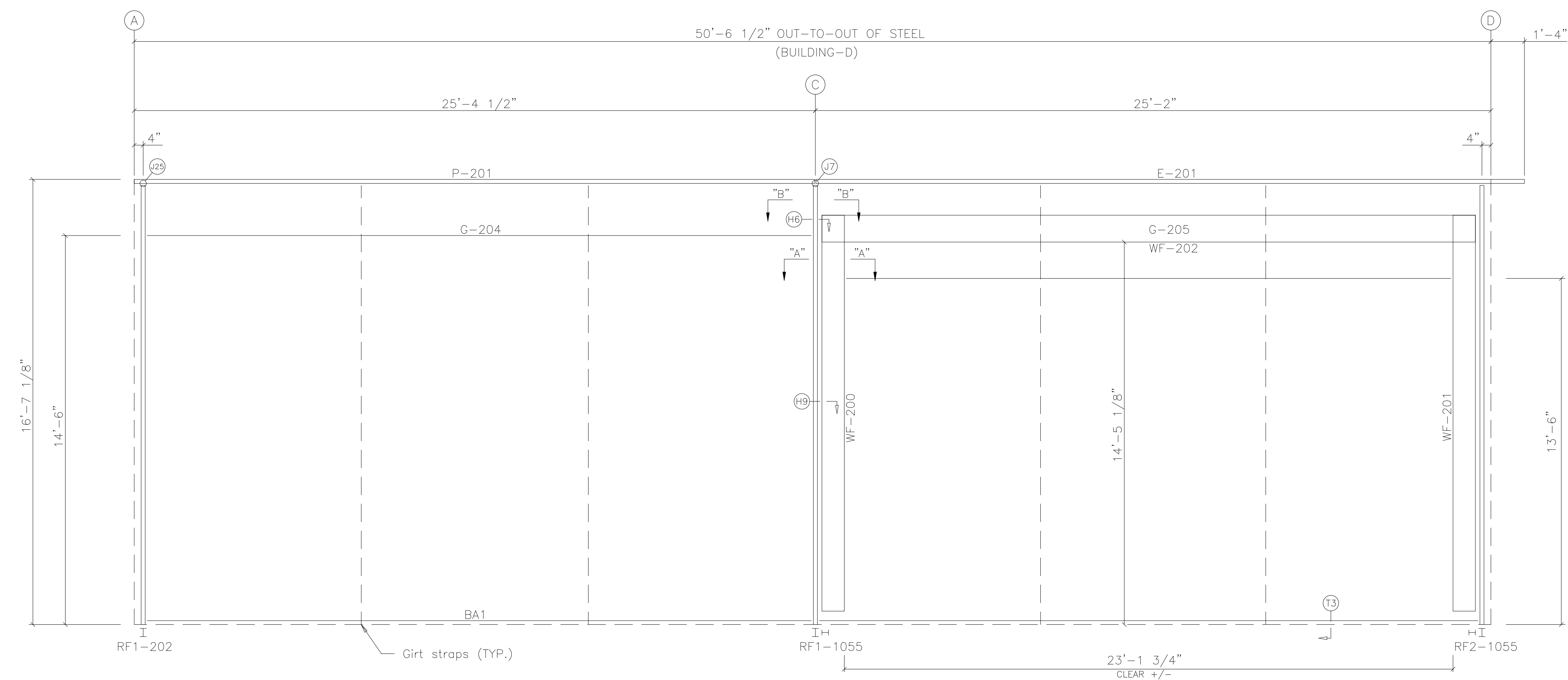
PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION: SIDEWALL FRAMING & TRIM
 CUSTOMER: HOPKINS CONSTRUCTION & MAINTENANCE
 LOCATION: Central, LA (East Baton Rouge Parish)
 Designer: HA
 Issue: A
 Detailer: SAN
 Checker: JJ
 Job No: 251599
 Sheet: E18

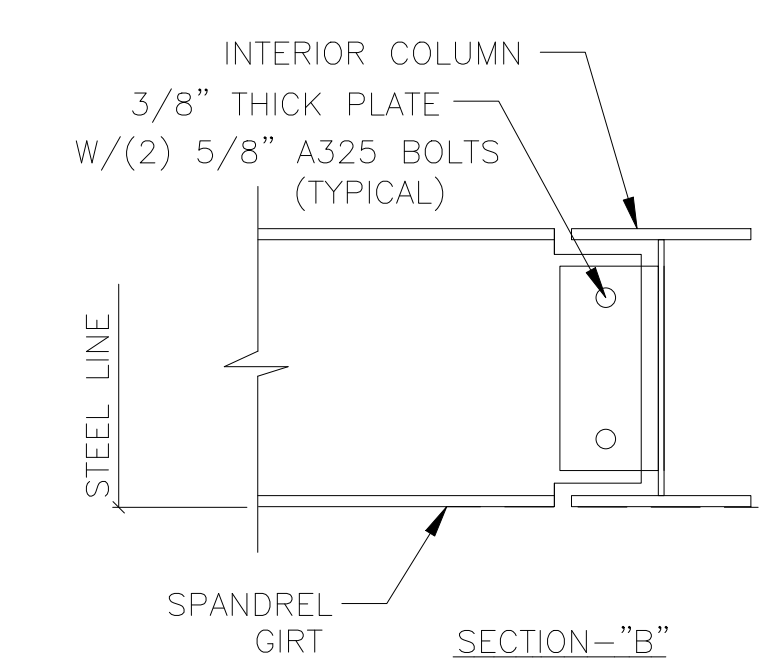
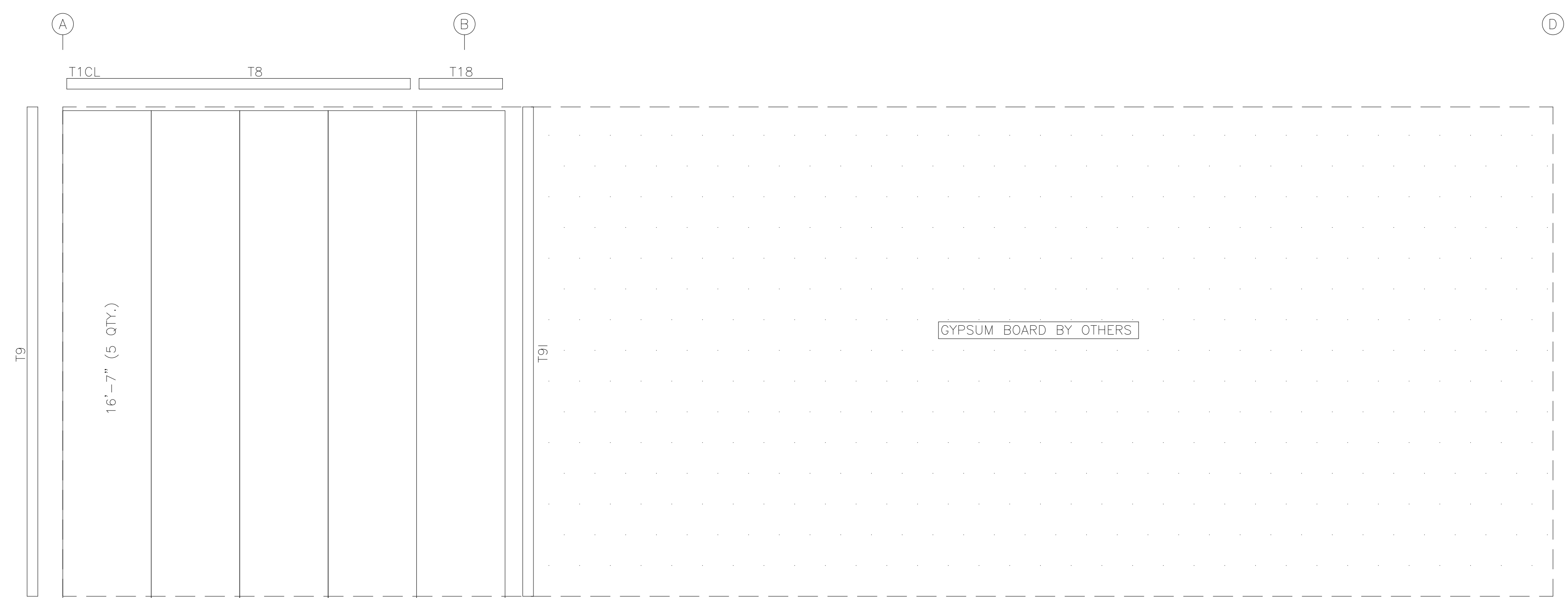
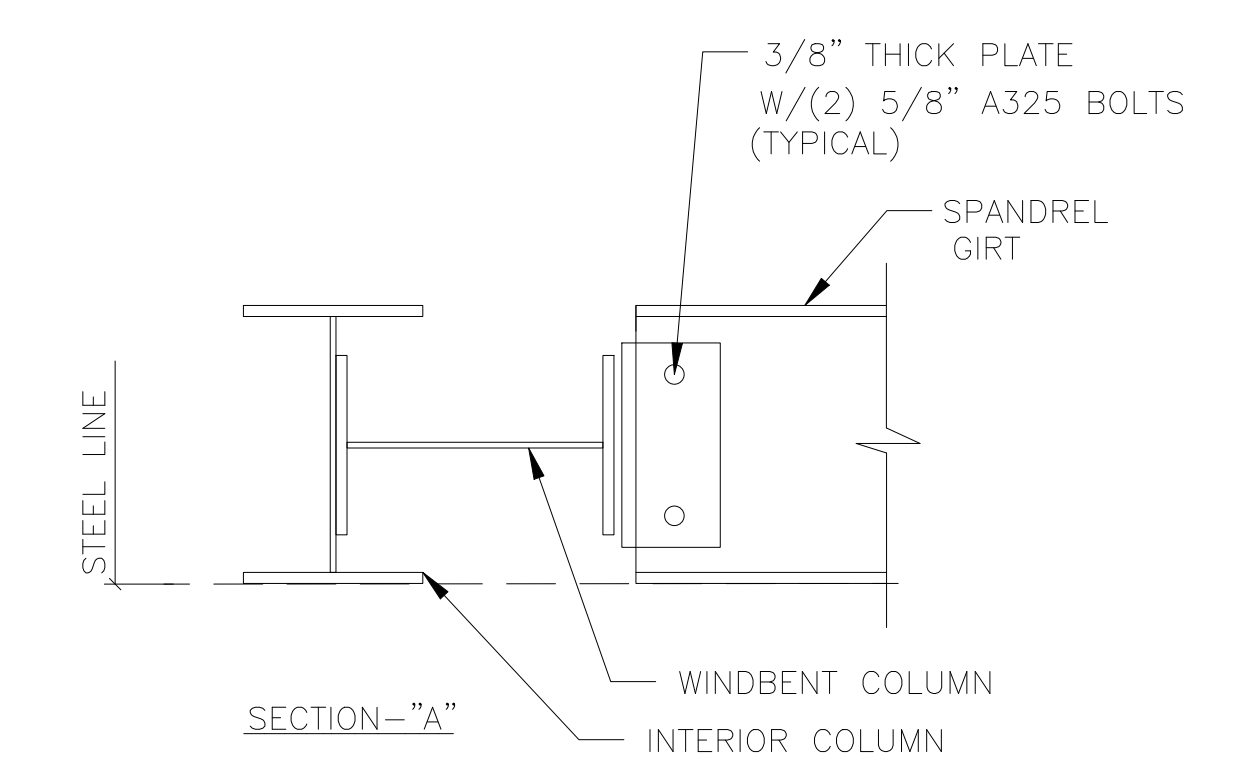


BOLT TABLE				
FRAME LINE 7				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-200 - WF-202	8	A325	3/4"	1 3/4"
WF-201 - WF-202	8	A325	3/4"	1 3/4"
WF-200 - RF1-1055	8	A325	5/8"	1 1/2"
WF-201 - RF2-1055	8	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE 7	
MARK	PART
WF-200	W10843
WF-201	W10843
WF-202	W12842
P-201	10X35Z14
E-201	10ES14
G-204	C10x15.3
G-205	C10x15.3



SIDEWALL FRAMING: FRAME LINE 7



SIDEWALL SHEETING & TRIM: FRAME LINE 7

PANELS: 24 Ga. PBR - NEED COLOR
NOTE: FIELD CUT WALL PANELS AS REQUIRED

ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

Pinnacle
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	SIDEWALL FRAMING & SHEETING				
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE				
LOCATION:	Central, LA (East Baton Rouge Parish)				
Detailer	SAN	Checker	JJ	Designer	HA
Job No.	251599	Sheet	E19	Issue	A

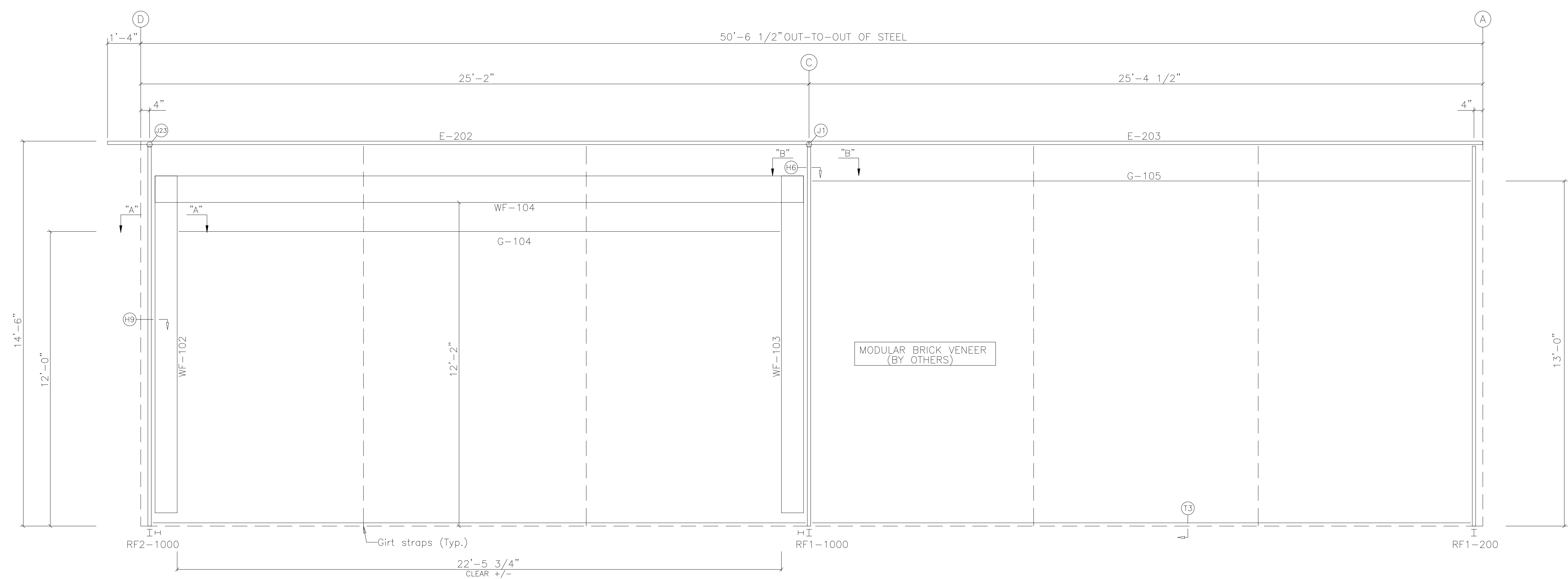
**** PLEASE VERIFY ALL BUILDING DIMENSIONS & CLEAR HEIGHTS ****

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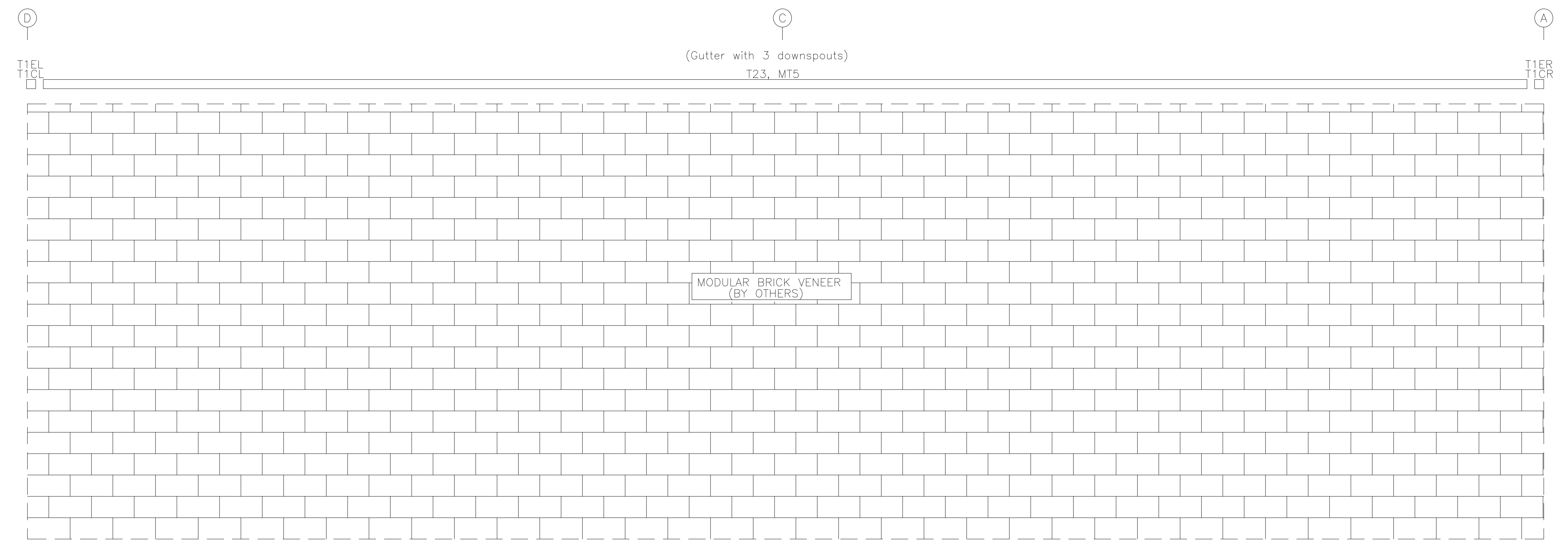


BOLT TABLE				
FRAME LINE 8				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-102-WF-104	8	A325	3/4"	1 3/4"
WF-103-WF-104	8	A325	3/4"	1 3/4"
WF-102-RF2-1000	8	A325	5/8"	1 1/2"
WF-103-RF1-1000	8	A325	5/8"	1 1/2"

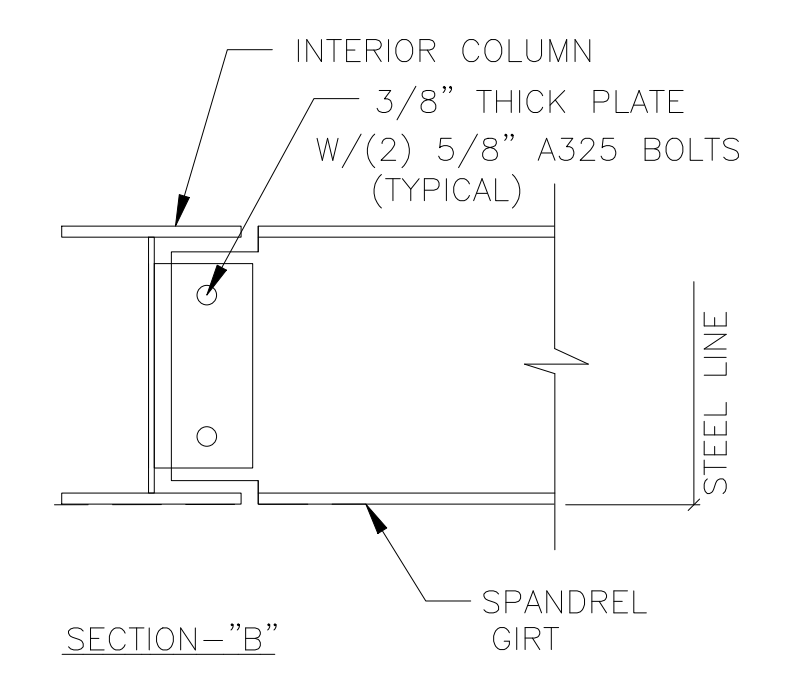
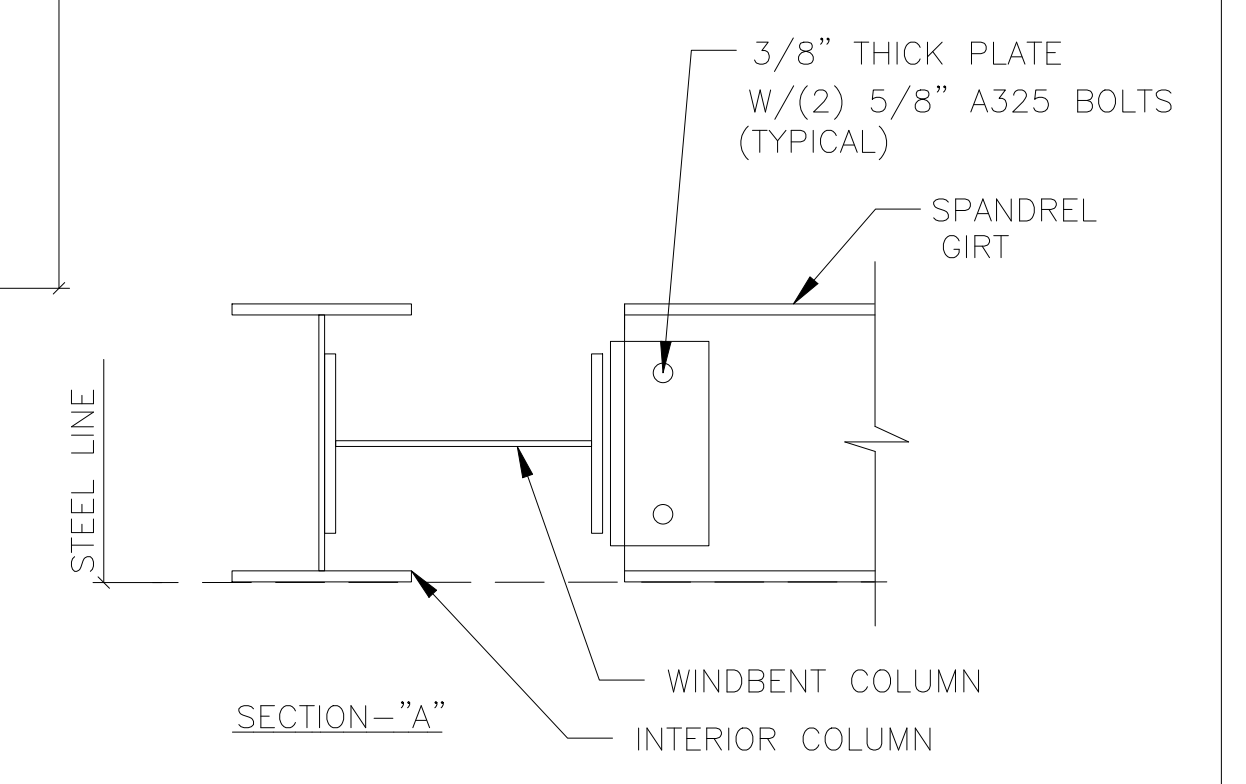
MEMBER TABLE	
FRAME LINE 8	
MARK	PART
WF-102	W14843
WF-103	W14843
WF-104	W14842
E-202	10ES14
E-203	10ES14
G-104	C10x15.3
G-105	C10x15.3



SIDEWALL FRAMING: FRAME LINE 8



SIDEWALL TRIM: FRAME LINE 8



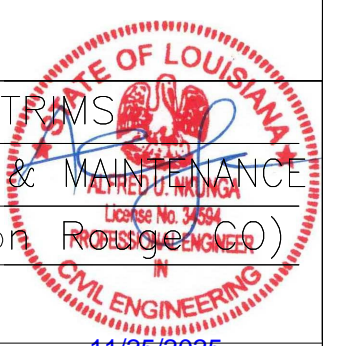
PLEASE VERIFY ALL BUILDING DIMENSIONS & CLEAR HEIGHTS

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ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

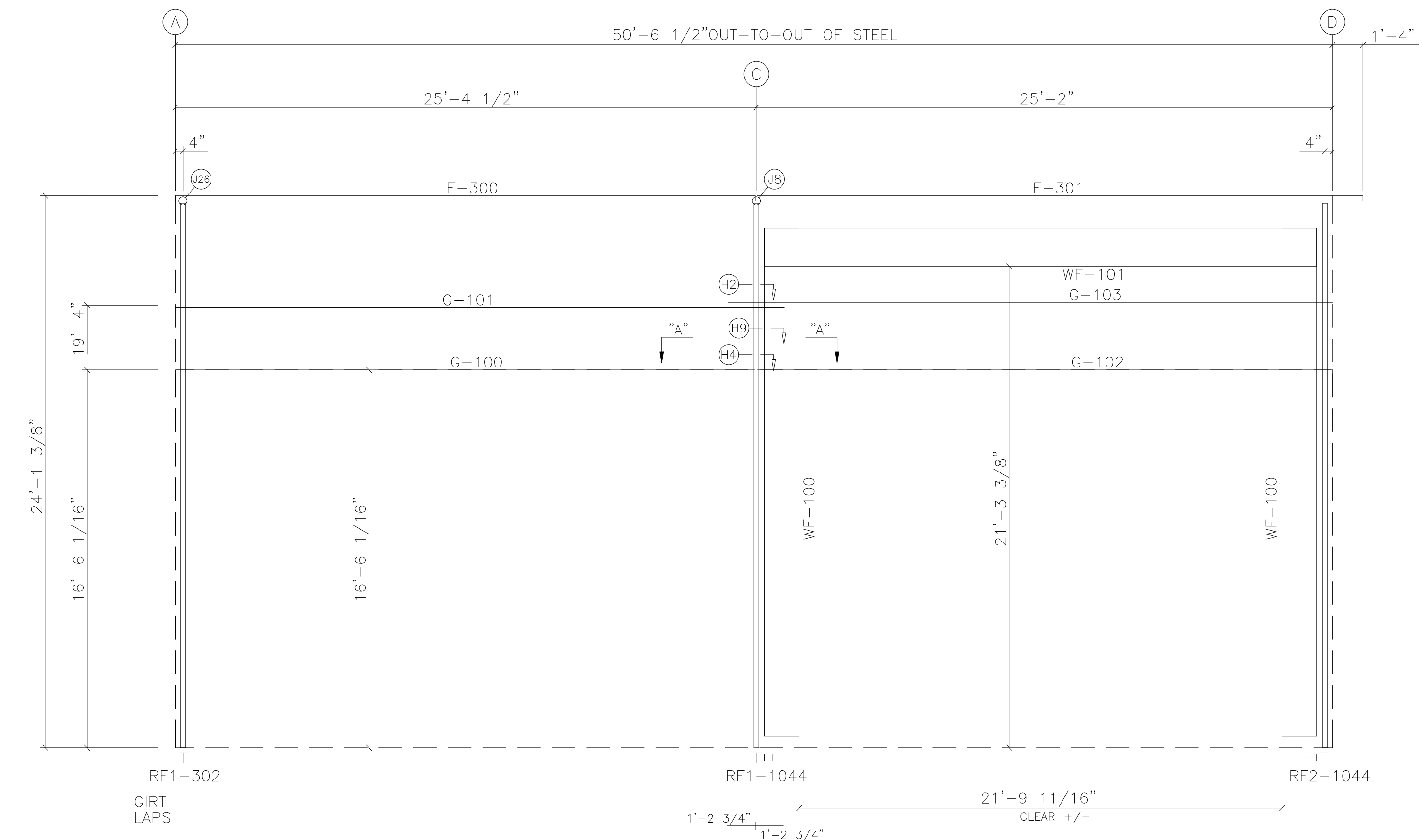
PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION: SIDEWALL FRAMING & TRIM
 CUSTOMER: HOPKINS CONSTRUCTION & MAINTENANCE
 LOCATION: Central, LA (East Baton Rouge Parish)
 Detailer: SAN, Checker: JJ, Designer: HA
 Job No.: 251599, Sheet: E20, Issue: A

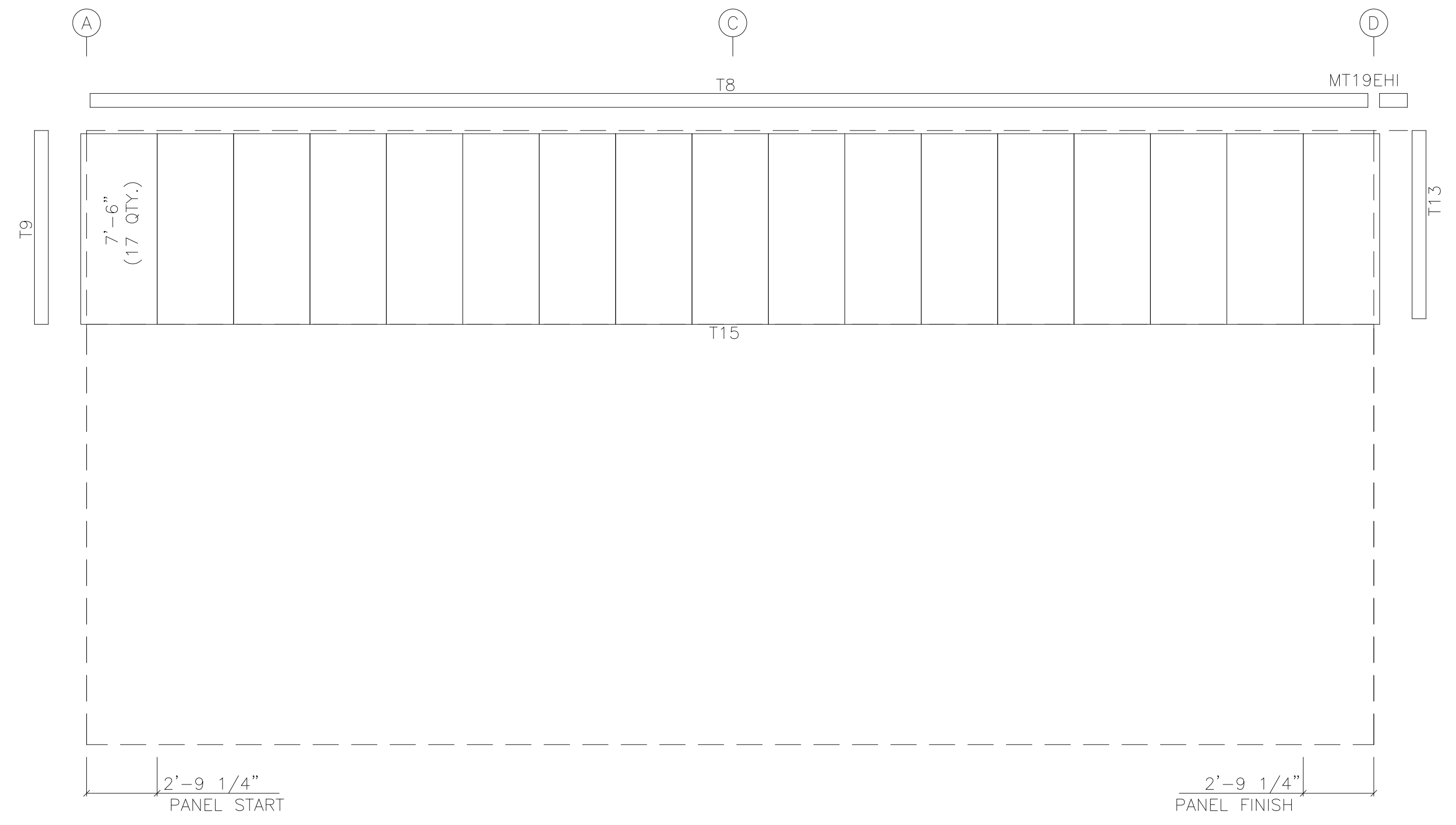
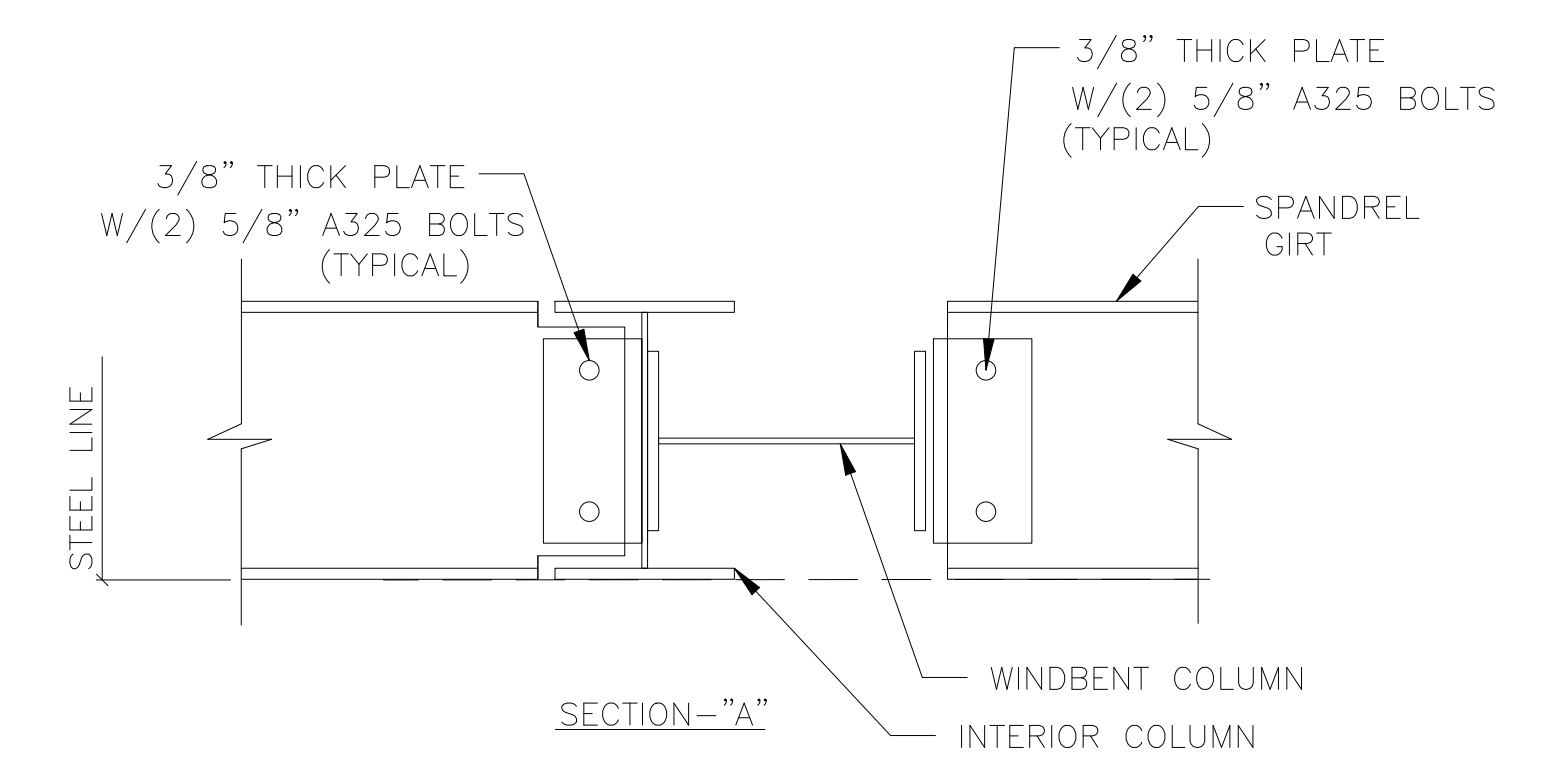


BOLT TABLE				
FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-100 - WF-101	8	A325	3/4"	2"
WF-100 - RF1-1044	12	A325	5/8"	1 1/2"
WF-100 - RF2-1044	12	A325	5/8"	1 1/2"

MEMBER TABLE	
FRAME LINE 3	
MARK	PART
WF-100	W18853
WF-101	W20842
E-300	10ES14
E-301	10ES14
G-100	C8x11.5
G-101	8X25Z16
G-102	C8x11.5
G-103	8X25Z16



SIDEWALL FRAMING: FRAME LINE 3



SIDEWALL SHEETING & TRIM: FRAME LINE 3

PANELS: 24 Ga. PBR - NEED COLOR
NOTE: FIELD CUT WALL PANELS AS REQUIRED

**** PLEASE VERIFY ALL BUILDING DIMENSIONS & FRAMED OPENING LOCATIONS ****

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ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

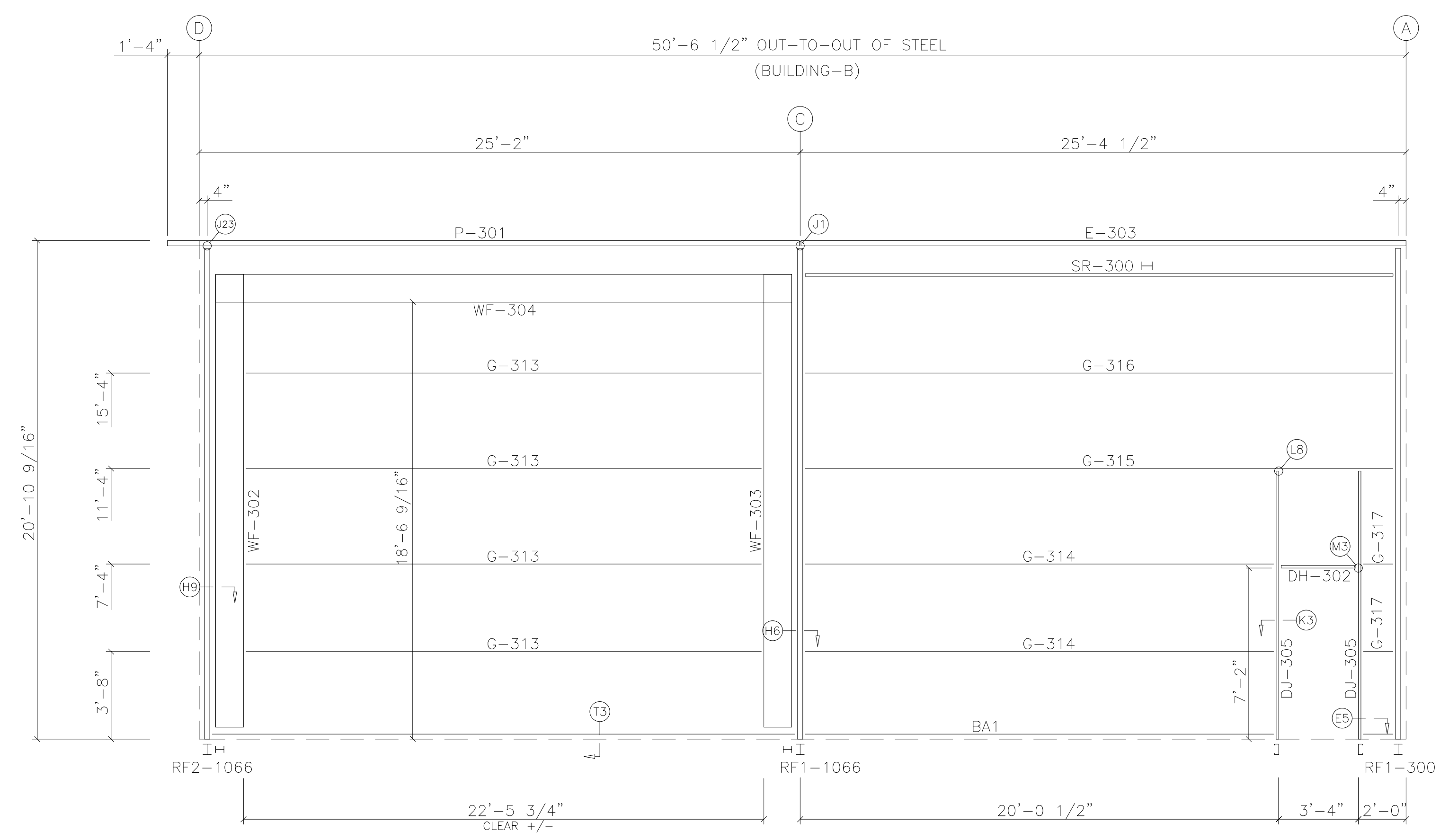
PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	SIDEWALL FRAMING & SHEETING		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Job No.	251599	Sheet	E21
Designer	HA	Issue	A
			11/25/2025

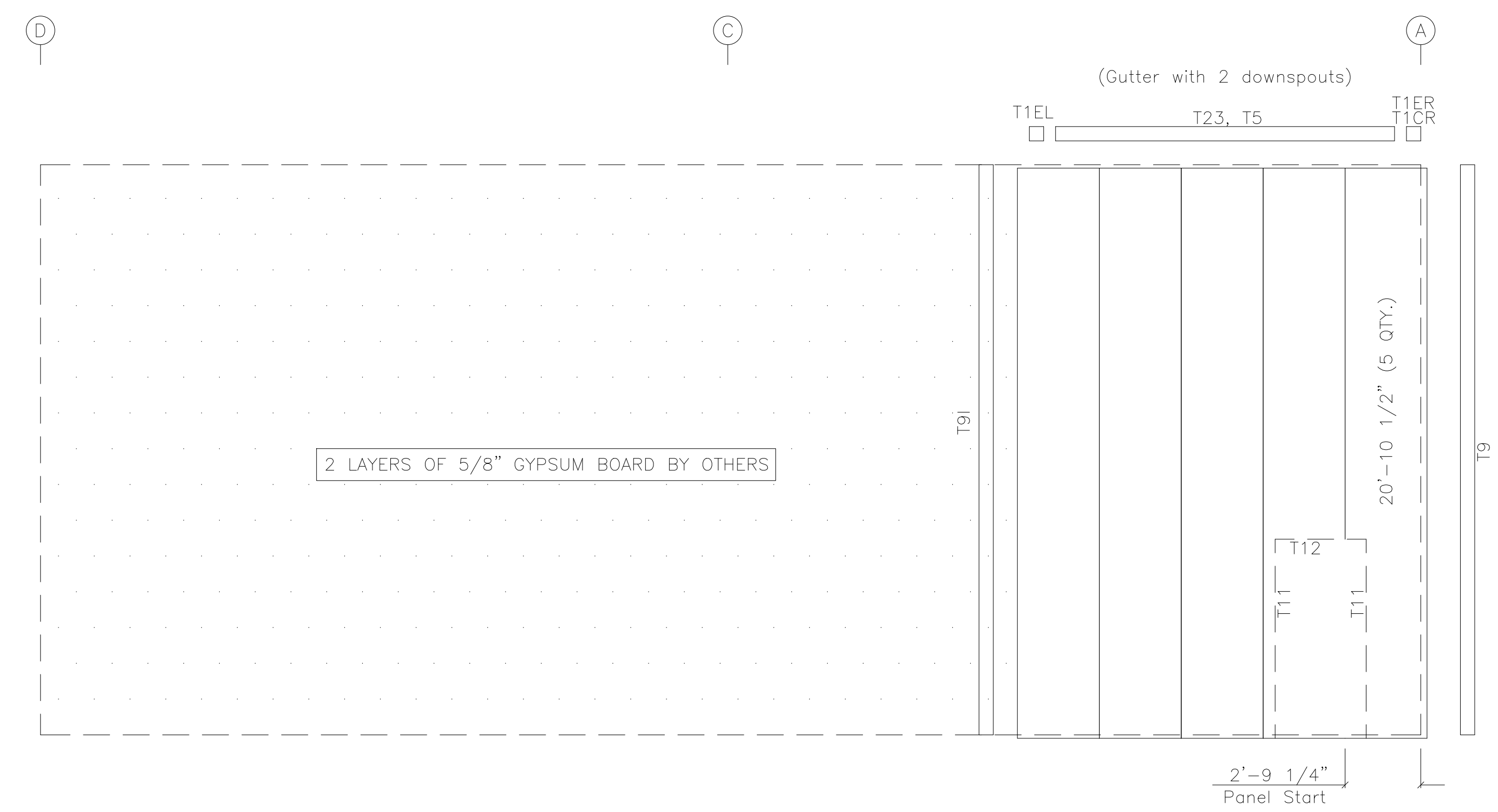


BOLT TABLE				
FRAME LINE 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-302 - WF-304	8	A325	3/4"	1 3/4"
WF-303 - WF-304	8	A325	3/4"	1 3/4"
WF-302 - RF2-1066	10	A325	5/8"	1 1/2"
WF-303 - RF1-1066	10	A325	5/8"	1 1/2"
SR-300	2	A325	5/8"	1 1/2"
RF1-300	4	A307	1/2"	1 1/4"

MEMBER TABLE	
FRAME LINE 5	
MARK	PART
WF-302	W14843
WF-303	W14843
WF-304	W14842
SR-300	W12642
DJ-305	10X25C16
DH-302	10X25C16
P-301	10X35Z14
E-303	10ES14
G-313	10X35Z12
G-314	10X35Z14
G-315	10X70D14
G-316	10X35Z12
G-317	10X25Z16



SIDEWALL FRAMING: FRAME LINE 5



SIDEWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 24 Ga. PBR - NEED COLOR
 NOTE: FIELD CUT WALL PANELS AS REQUIRED

**** PLEASE VERIFY ALL BUILDING DIMENSIONS & FRAMED OPENING LOCATIONS ****

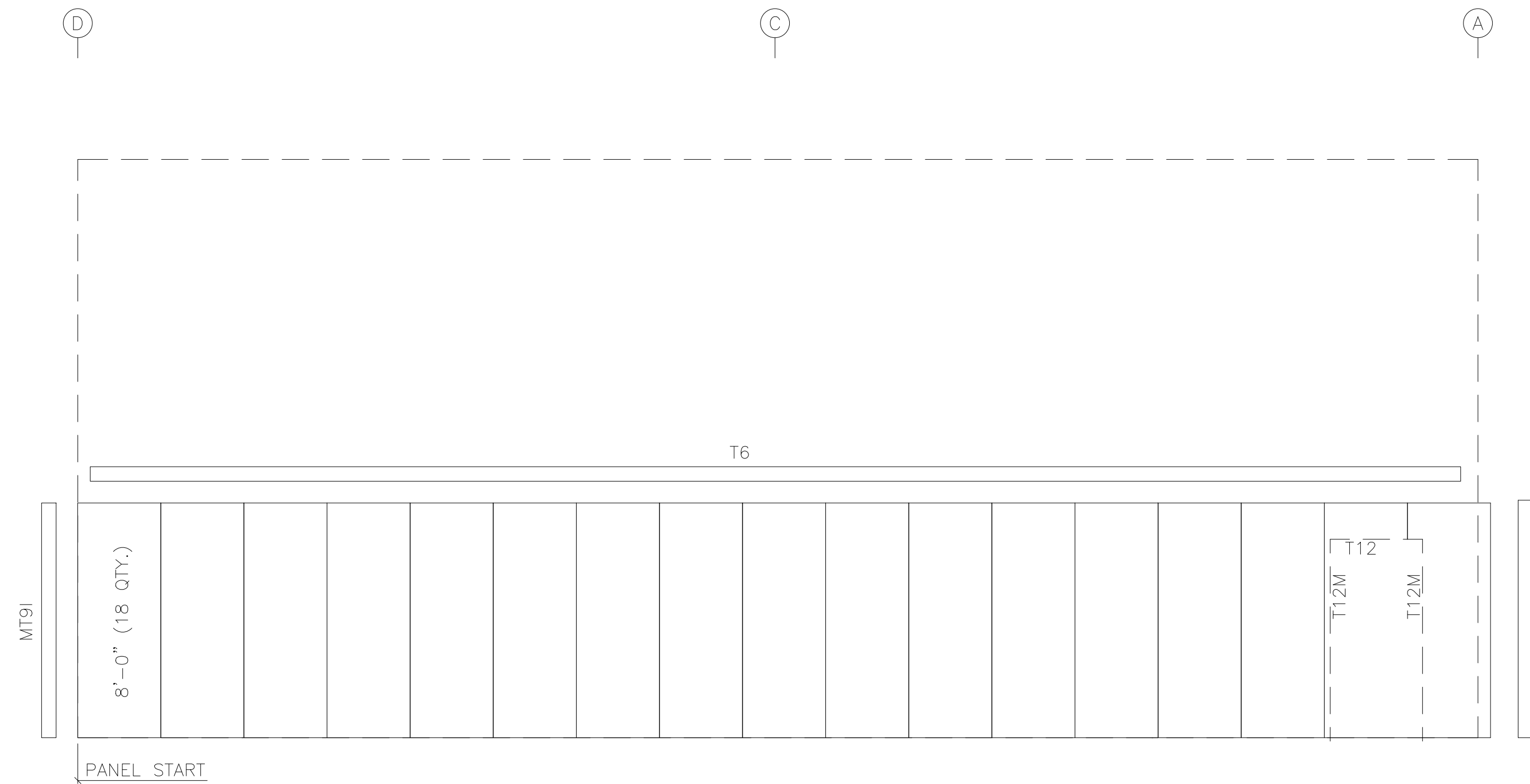
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ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
 STRUCTURES, INC.
 PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	SIDEWALL FRAMING & SHEETING		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E22
Issue	A		

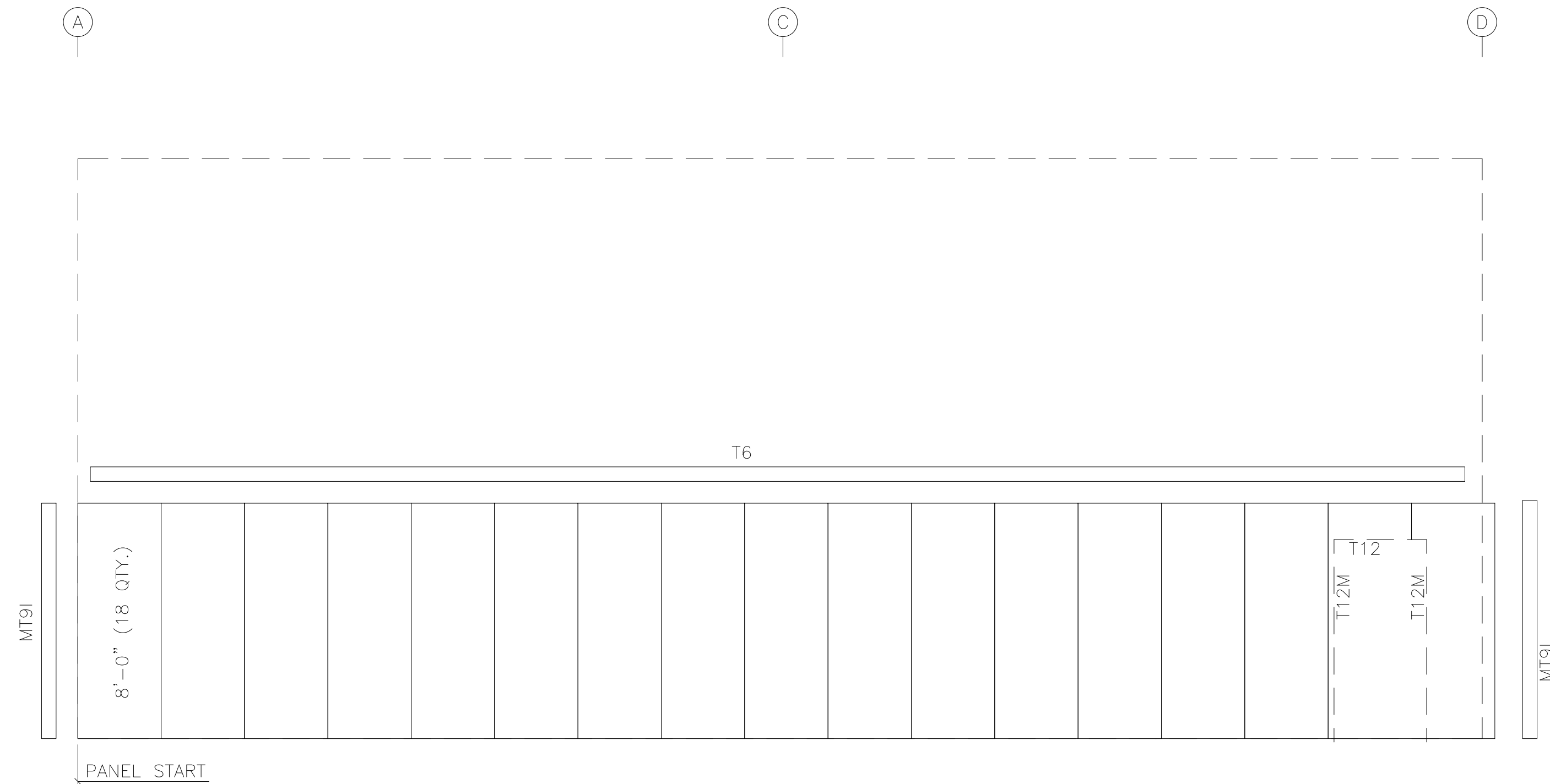




PARTITION SHEETING & TRIM: FRAME LINE 3

PANELS: 24 Ga. PBU - NEED COLOR

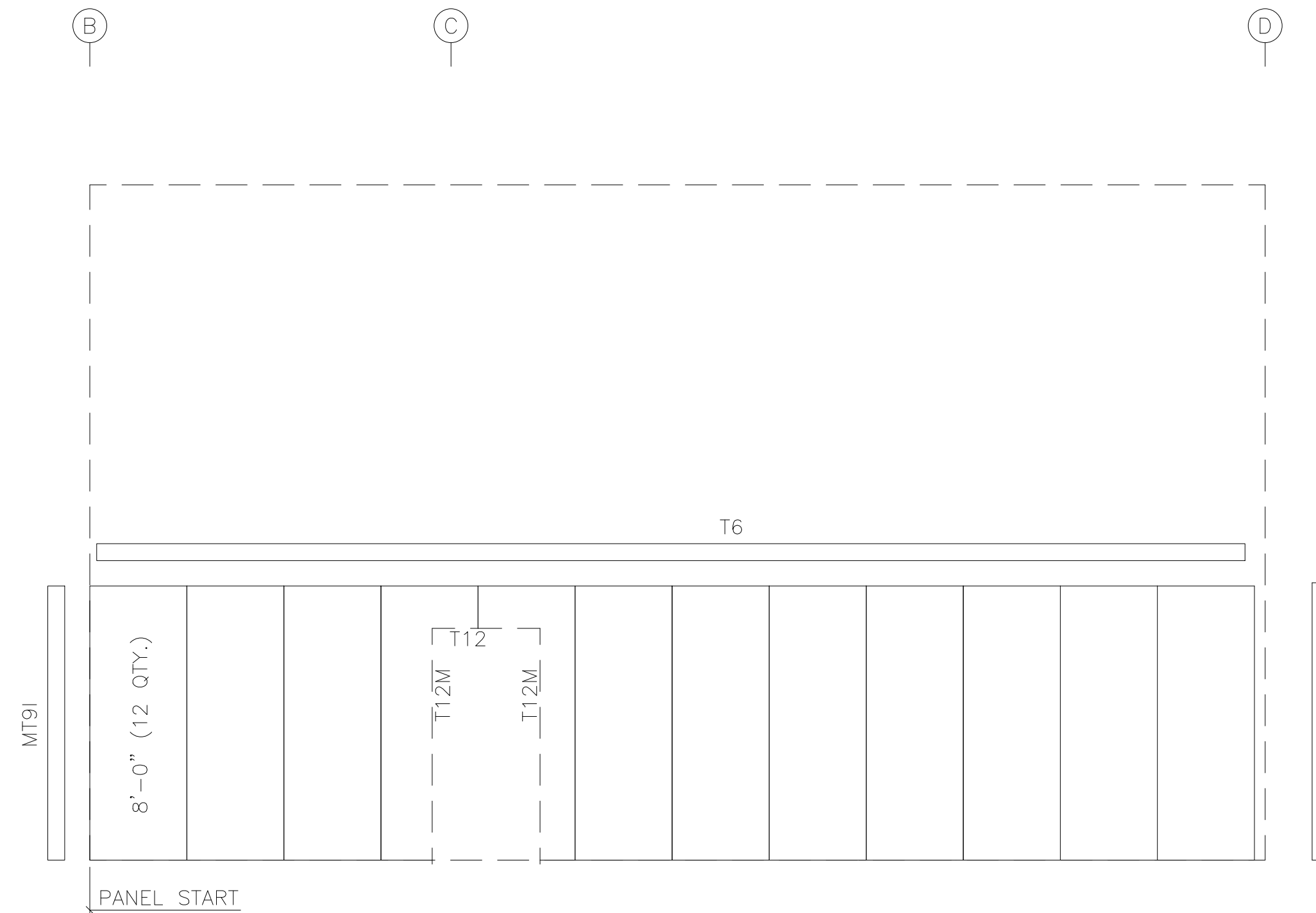
NOTE: FIELD CUT PARTITION PANELS AS REQUIRED



PARTITION SHEETING & TRIM: FRAME LINE 5

PANELS: 24 Ga. PBU - NEED COLOR

NOTE: FIELD CUT PARTITION PANELS AS REQUIRED



PARTITION SHEETING & TRIM: FRAME LINE 7

PANELS: 24 Ga. PBU - NEED COLOR

NOTE: FIELD CUT PARTITION PANELS AS REQUIRED

**** PLEASE VERIFY PARTITION ELEVATIONS ****

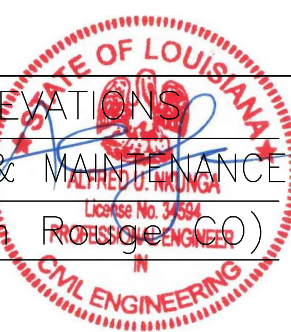
GENERAL NOTES:

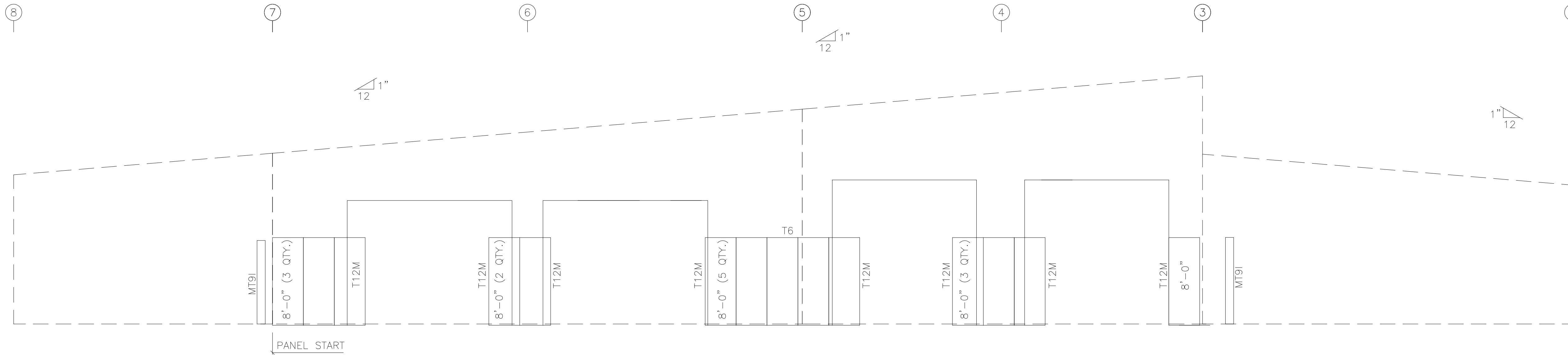
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ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	



DESCRIPTION:	PARTITION SHEETING ELEVATIONS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E23
Issue	A		



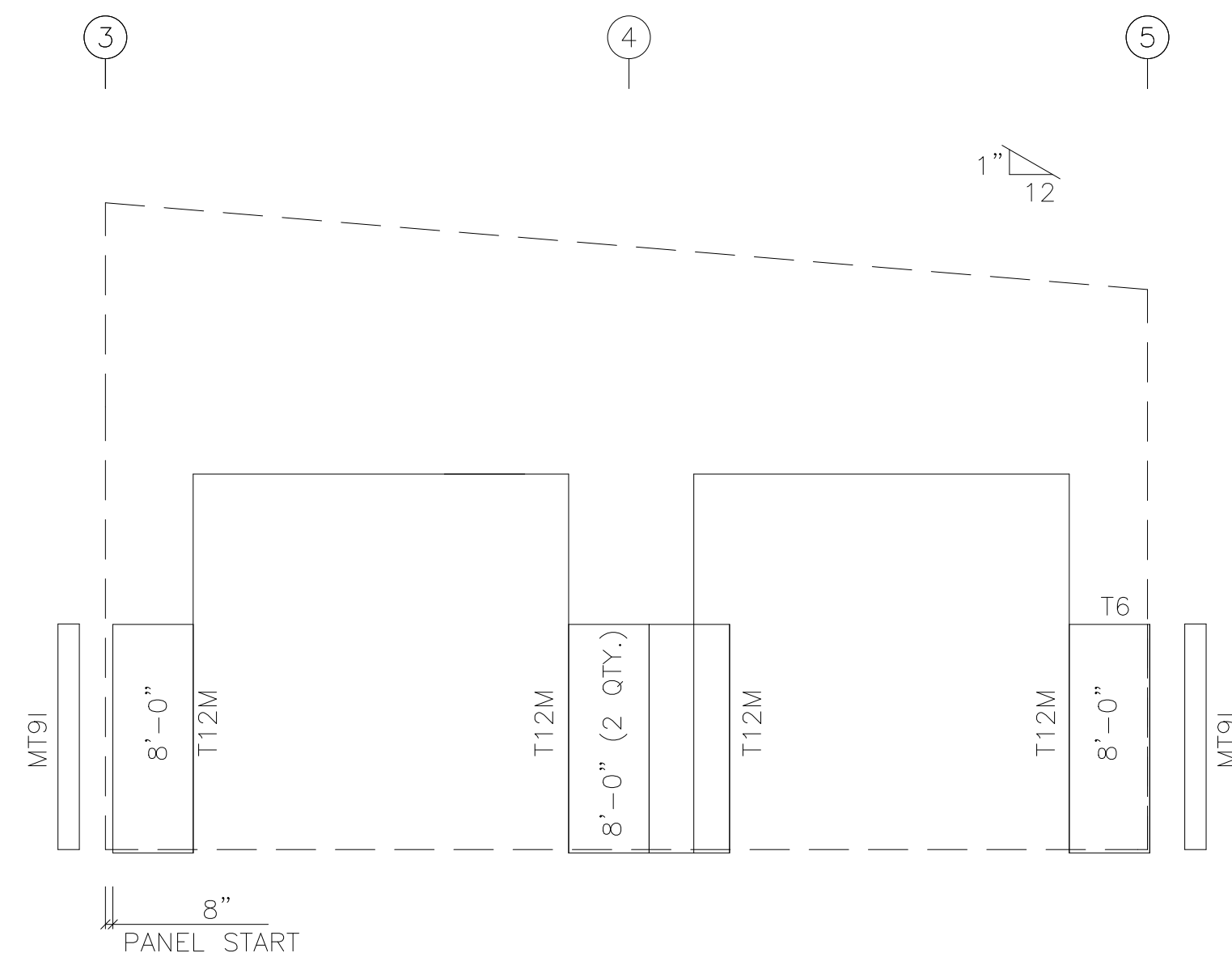


ENDWALL LINER SHEETING & TRIM: FRAME LINE D

PANELS: 24 Ga. PBU - NEED COLOR

NOTE: FIELD CUT WALL LINER PANELS AS REQUIRED

(NOTE: VIEW FROM THE INSIDE OF THE BUILDING)

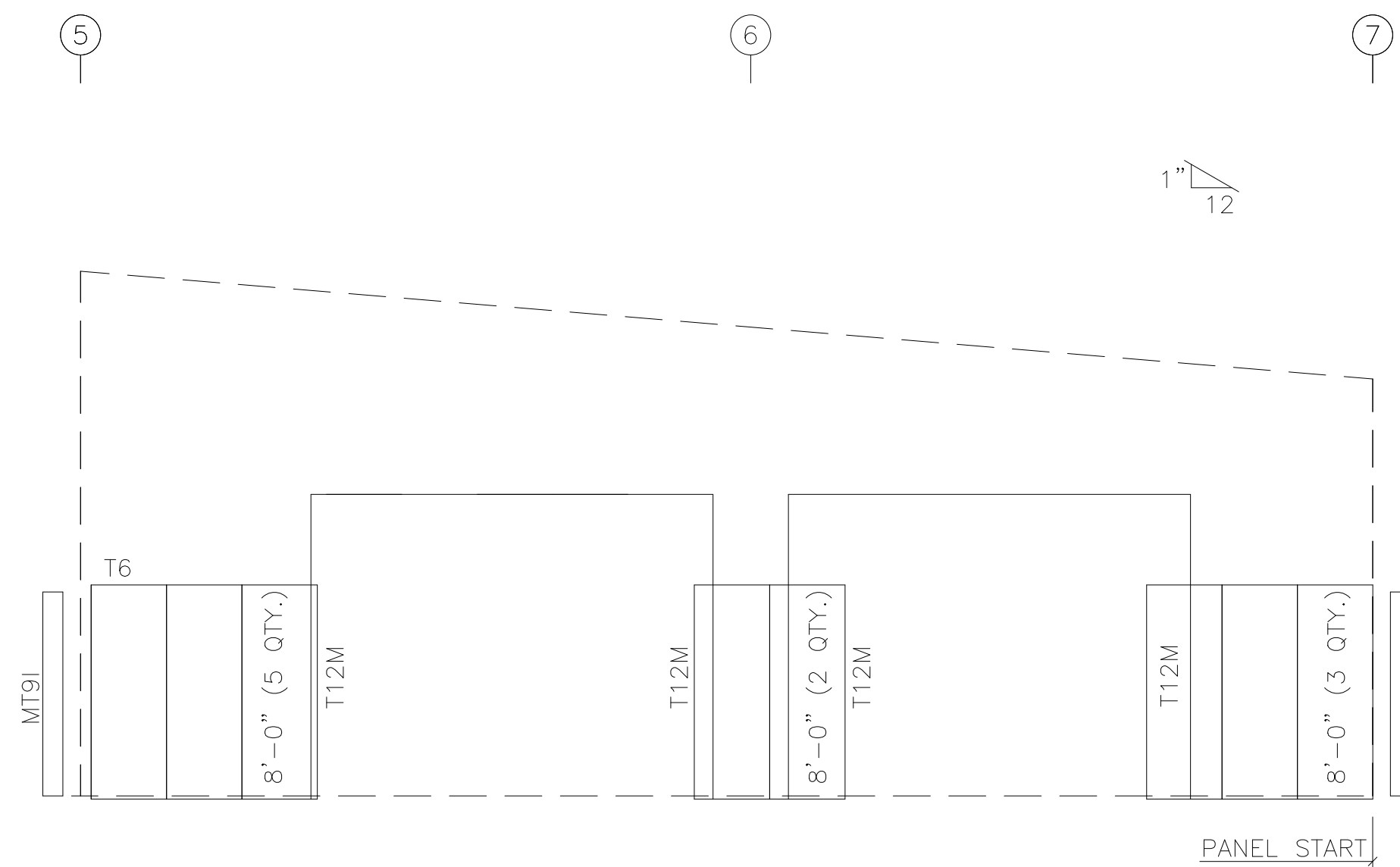


ENDWALL LINER SHEETING & TRIM: FRAME LINE A

PANELS: 24 Ga. PBU - NEED COLOR

NOTE: FIELD CUT WALL LINER PANELS AS REQUIRED

(NOTE: VIEW FROM THE INSIDE OF THE BUILDING)



ENDWALL LINER SHEETING & TRIM: FRAME LINE B

PANELS: 24 Ga. PBU - NEED COLOR

NOTE: FIELD CUT WALL LINER PANELS AS REQUIRED

(NOTE: VIEW FROM THE INSIDE OF THE BUILDING)

**** PLEASE VERIFY LINER ELEVATIONS ****

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ISSUE	DESCRIPTION	DATE	MARK
A	APPROVAL	11/24/25	

PINNACLE
STRUCTURES, INC.
PO BOX 1268 - CABOT, AR 72023 (501) 941-3929

DESCRIPTION:	LINER SHEETING ELEVATIONS		
CUSTOMER:	HOPKINS CONSTRUCTION & MAINTENANCE		
LOCATION:	Central, LA (East Baton Rouge Parish)		
Detailer	SAN	Checker	JJ
Designer	HA		
Job No.	251599	Sheet	E24
Issue	A		

